

DESIGN CRITERIA:

- THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE 2018 KENTUCKY BUILDING CODE AND THE 2015 INTERNATIONAL BUILDING CODE (IBC)
- DESIGN LOADS
 - ROOF:
 - DEAD LOAD:
 - OFFICE: 25 PSF
 - PBMS COLLATERAL DL:
 - DR-2 STEEP ROOF: 1.0 PSF
 - MAINTENANCE: 7 PSF
 - LIVE LOAD = 20 PSF MINIMUM
 - FLOOR:
 - DEAD LOAD:
 - MEZZANINES: 70 PSF
 - LIVE LOAD:
 - MEZZANINES: 125 PSF

- RISK CATEGORY: III
 - GROUND SNOW LOAD, P_g : 15 PSF
 - EXPOSURE FACTOR, C_e : 1.0
 - IMPORTANCE FACTOR, I_p : 1.1
 - THERMAL FACTOR, C_t : 1.0
 - SLOPE FACTOR, C_s : 1.0
 - FLAT ROOF SNOW LOAD, P_f : 11.6 PSF
 - MINIMUM ROOF SNOW LOAD, P_m : 16.5 PSF
 - DRIFT SURCHARGE LOAD: REF S0.2
- WIND LOADS:
 - ULTIMATE DESIGN WIND SPEED, V_{ult} (3-SEC GUST): 120 MPH
 - NOMINAL DESIGN WIND SPEED, V_{nom} : 93 MPH
 - EXPOSURE CATEGORY: C
 - ENCLOSURE CATEGORY: ENCLOSED
 - INTERNAL PRESSURE COEFFICIENT, GC_p : ± 0.18
 - DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING NOT SPECIFICALLY DESIGNED WITHIN THE CONSTRUCTION DOCUMENTS: REF S0.2
 - ROOF EFFECTIVE AREA = 50 FT²
 - ZONE 1: +16.0, -26.5
 - ZONE 2: +16.0, -43.3
 - ZONE 3: +16.0, -49.3
 - WALL EFFECTIVE AREA = 50 FT²
 - ZONE 4: +29.9, -32.7
 - ZONE 5: +29.9, -37.7

- WIND LOADS HARDENED AREA:
 - DESIGN WIND SPEED, V_{ult} (3-SEC GUST): 250 MPH
 - EXPOSURE CATEGORY: C
 - ENCLOSURE CATEGORY: PARTIALLY ENCLOSED
 - INTERNAL PRESSURE COEFFICIENT, GC_p : ± 0.55
 - IMPACT LOAD (15 LB 2x4 LUMBER @): 100 MPH AT VERTICAL SURFACES
 - 67 MPH AT HORIZONTAL SURFACES
- DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING NOT SPECIFICALLY DESIGNED WITHIN THE CONSTRUCTION DOCUMENTS:
 - ROOF EFFECTIVE AREA = 50 FT²
 - ZONE 1: +111.6, -169.3
 - ZONE 2: +111.6, -233.1
 - ZONE 3: +111.6, -233.1
 - WALL EFFECTIVE AREA = 50 FT²
 - ZONE 4: +175.0, -187.2
 - ZONE 5: +175.0, -208.9

- EARTHQUAKE LOADS:
 - SEISMIC IMPORTANCE FACTOR, I_s : 1.25
 - MAPPED SPECTRAL ACCELERATION FOR SHORT PERIOD, S_s : 0.191
 - MAPPED SPECTRAL ACCELERATION FOR 1 SEC PERIOD, S_1 : 0.105
 - SITE CLASS: C
 - DESIGN SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIOD, S_{ss} : 0.203
 - DESIGN SPECTRAL RESPONSE ACCELERATION FOR 1 SEC PERIOD, S_{1s} : 0.166
 - SEISMIC DESIGN CATEGORY: B
 - OFFICE BUILDING:
 - BASIC SEISMIC-FORCE RESISTING SYSTEM:
 - STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
 - RESPONSE MODIFICATION COEFFICIENT, R : 3
 - DEFLECTION AMPLIFICATION FACTOR, C_d : 3
 - SEISMIC RESPONSE COEFFICIENT, C_s : 0.064
 - DESIGN BASE SHEAR, V : 31.21 KIPS
 - WAREHOUSE MEZZANINES:
 - BASIC SEISMIC-FORCE RESISTING SYSTEM:
 - STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
 - RESPONSE MODIFICATION COEFFICIENT, R : 3
 - DEFLECTION AMPLIFICATION FACTOR, C_d : 3
 - SEISMIC RESPONSE COEFFICIENT, C_s : 0.064
 - DESIGN BASE SHEAR, V : 8.80 KIPS
 - MAINTENANCE MEZZANINE:
 - BASIC SEISMIC-FORCE RESISTING SYSTEM:
 - ORDINARY REINFORCED MASONRY SHEAR WALLS
 - RESPONSE MODIFICATION COEFFICIENT, R : 2
 - DEFLECTION AMPLIFICATION FACTOR, C_d : 1.34
 - SEISMIC RESPONSE COEFFICIENT, C_s : 0.064
 - DESIGN BASE SHEAR, V : 8.96 KIPS
 - WAREHOUSE, MAINTENANCE AND COVERED STORAGE:
 - BASIC SEISMIC-FORCE RESISTING SYSTEM:
 - DETERMINED BY MANUFACTURER
 - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
 - EXISTING BUILDINGS NOT EVALUATED FOR CURRENT SEISMIC CRITERIA.

STEEL DECK:

- GENERAL
 - METAL DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.
 - SEE PLANS FOR DECK TYPES AND GAUGES.
 - LAP DECK 4" MINIMUM AT SPLICES CENTERED ON SUPPORT.
- FABRICATE DECK UNITS IN LENGTHS TO SPAN THREE OR MORE SUPPORT SPACINGS.
- ROOF DECK:
 - 1 1/2" DEEP (WIDE RIB) DECK AND ROLLED OF STEEL SHEETS CONFORMING TO ASTM A653. NO METAL ROOF DECK SHALL HAVE SECTION PROPERTIES PER FOOT OF WIDTH LESS THAN THE FOLLOWING:
 - 20 GAUGE: $I_x=0.197$ IN⁴, $I_y=0.217$ IN⁴, $S_x=0.224$ IN³, $S_y=0.229$ IN³, $f_y=50$ KSI
 - DO NOT SUSPEND POINT LOADS FROM DECK INCLUDING HANGERS FOR: CEILINGS, PIPES, DUCTS, EQUIPMENT, ETC. CONTRACTOR INSTALLING SUCH POINT LOADS SHALL PROVIDE SUB-FRAMING TO TRANSFER LOAD TO STRUCTURE SUPPORTING DECK.
- CONTRACTOR SHALL COORDINATE SIZE AND LOCATION OF ROOF OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

- COMPOSITE METAL DECK
 - UNLESS NOTED OTHERWISE, TEMPORARY SHORING OF COMPOSITE DECK IS NOT REQUIRED FOR ANTICIPATED CONSTRUCTION LOAD (WET CONCRETE WEIGHT PLUS 20 PSF CONSTRUCTION LOAD). SEE SPECIFICATIONS FOR DEFLECTION LIMITATION. ALLOWABLE CONSTRUCTION LOADS SHALL BE SHOWN ON THE ERECTION DRAWINGS.
 - CONTRACTOR SHALL FURNISH THE ADDITIONAL CONCRETE DUE TO WET CONCRETE DEFLECTION OF THE COMPOSITE DECK AND BEAMS.
 - DECK SHOP DRAWINGS SHALL INDICATE THE TYPE, SIZE, SHEAR VALUE, AND LAYOUT OF SHEAR CONNECTORS (S.C.) REQUIRED FOR COMPOSITE BEAMS. SEE SPECIFICATIONS AND COMPOSITE BEAM NOTES FOR S.C. REQUIREMENTS.
 - THE DECK ACTING COMPOSITELY WITH THE SLAB SHALL BE CAPABLE OF SUPPORTING BOTH THE FINAL DESIGN LIVE LOAD AND SUPERIMPOSED DEAD LOAD SPECIFIED ON THE DRAWINGS. NO METAL FLOOR DECK USED IN THE BUILDING SHALL HAVE SECTION PROPERTIES PER FOOT OF WIDTH LESS THAN THE FOLLOWING:
 - COMPOSITE STEEL DECK MUST BE:
 - 20 GAUGE: $I_x=0.409$ IN⁴, $I_y=0.407$ IN⁴, $S_x=0.326$ IN³, $S_y=0.337$ IN³, $f_y=50$ KSI
 - CONTRACTOR SHALL COORDINATE SIZE AND LOCATION OF FLOOR OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

- WELD STUDS THROUGH METAL DECK.
- ELECTRICAL CONDUIT SHALL BE ROUTED BELOW METAL DECK NO CONDUIT, PIPING, JUNCTION BOXES, OR OTHER ITEMS SHALL BE ALLOWED TO BE EMBEDDED IN CONCRETE SLAB WITHOUT PRIOR WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
- DECK SUPPLIER TO PROVIDE DECK CLOSURE WHERE REQUIRED FOR CONCRETE POUR.
- THE CONTRACTOR SHALL PLACE CONCRETE FOR SLABS SO THAT THE FINISH SURFACE IS SCREENED LEVEL WITH AN ELEVATION WITHIN 1/4" OF THE TOP OF SLAB ELEVATION SHOWN ON THE PLANS.
- SLABS SHALL HAVE A TROWELED FINISH AND AS A MINIMUM BE TRUE PLANES WITHIN 1/2" IN 10 FEET. AS DETERMINED BY A 10 FOOT STRAIGHTEDGE PLACED ANYWHERE ON THE SLAB IN ANY DIRECTION.
- CONSIDERATION SHALL BE GIVEN TO SEQUENCING OF CONCRETE PLACEMENT SO AS TO CONTROL FINISH ELEVATIONS WITHIN THE SPECIFIED LIMITS. NO MORE THAN 1/2" OF ADDITIONAL CONCRETE THICKNESS SHALL BE POURED TO MAINTAIN FLOOR FLATNESS OR LEVELNESS. IF SUCH A SITUATION OCCURS THE GC SHALL STOP POUR, LET DECK HARDEN AND FINISH WITH A TOPPING SLAB.

FOUNDATIONS:

- GENERAL
 - FOUNDATION DESIGN IS BASED UPON GEOTECHNICAL ENGINEERING REPORT PREPARED BY:
 - STV INCORPORATED
 - DATED APRIL 15, 2028
 - REPORT # 225-370
- ALL BEARING MATERIAL SHALL BE INSPECTED BY A QUALIFIED TECHNICIAN PRIOR TO CONCRETE PLACEMENT. A QUALIFIED TECHNICIAN SHALL BE THE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
- SHALLOW FOUNDATIONS
 - FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE UNDER FULL SERVICE DEAD AND LIVE LOADS OF:
 - ISOLATED FOOTINGS: 1,500 PSF
 - STRIP FOOTINGS: 1,500 PSF
 - SOIL MODULUS OF SUBGRADE REACTION, K_s : 90 PCF
 - PASSIVE EQUIVALENT FLUID PRESSURE: 100 PCF/FT
 - COEFFICIENT OF FRICTION: 0.45
 - BACKFILL
 - DO NOT BACKFILL BELOW GRADE WALLS UNTIL ADEQUATE TEMPORARY BRACING IS INSTALLED.
 - BACKFILL UNDER FOUNDATION WITH CONCRETE OR AS APPROVED BY SOILS ENGINEER.
 - BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 2'-0" OR FROST DEPTH BELOW FINAL GRADE.
 - FROST DEPTH: 2'-0"
 - FOOTINGS MAY BE EARTH FORMED.
 - DO NOT POUR CONCRETE FOOTINGS ON GROUND THAT IS FROZEN.
 - FOUNDATIONS SHALL BE PROTECTED AGAINST FREEZING DURING PLACEMENT FOR 5 DAYS AFTER.
- RETAINING WALLS
 - ACTIVE DESIGN PRESSURE IS EQUAL TO AN EQUIVALENT FLUID PRESSURE OF 64 PCF
 - AT-NET DESIGN PRESSURE IS EQUAL TO AN EQUIVALENT FLUID PRESSURE OF 84 PCF
 - PASSIVE DESIGN PRESSURE IS EQUAL TO AN EQUIVALENT FLUID PRESSURE OF 119 PCF
 - SURCHARGE LOADS ARE 250 PSF AT THE SURFACE.
 - COEFFICIENT OF SLIDING FRICTION:
 - CONCRETE/SOIL: 0.3
 - FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE.
 - ZONE 4: +175.0, -187.2
 - WHERE FOUNDATION WALLS ARE TO HAVE EARTH PLACED ON EACH SIDE, PLACE FILL SIMULTANEOUSLY SO AS TO MAINTAIN A COMMON ELEVATION ON EACH SIDE OF THE WALL.
 - DO NOT BACKFILL UNTIL CONCRETE HAS REACHED ITS 28 DAY STRENGTH.

- ACTIVE DESIGN PRESSURE IS EQUAL TO AN EQUIVALENT FLUID PRESSURE OF 64 PCF
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 - ZONE 4: +175.0, -187.2
- WHERE FOUNDATION WALLS ARE TO HAVE EARTH PLACED ON EACH SIDE, PLACE FILL SIMULTANEOUSLY SO AS TO MAINTAIN A COMMON ELEVATION ON EACH SIDE OF THE WALL.
- DO NOT BACKFILL UNTIL CONCRETE HAS REACHED ITS 28 DAY STRENGTH.

- CUT & FILL
 - THE SITE IS ANTICIPATED TO HAVE CUTS AND FILL OF UP TO 5'-6". THE FINAL GRADING PLANS SHOULD BE REVIEWED AND APPROVED BY THE GEOTECHNICAL ENGINEER IN ORDER TO EVALUATE THE POTENTIAL NEED FOR AND EXTENT OF EXPANSIVE SOIL REMEDIATION, STABILIZING SOFTER SOILS NEAR EXISTING GROUND SURFACE, TREATMENT OF EXPANSIVE SOILS DURING GRADING, AND ANY NEED TO ALLOW TIME FOR SETTLEMENT IN DEEP FILL AREAS.

COLD-FORMED STEEL ROOF TRUSSES:

- TRUSS FABRICATOR TO DESIGN AND FURNISH CONNECTIONS FOR ALL TRUSS TO TRUSS CONNECTIONS AND TRUSS TO BEARING CONNECTIONS. PROVIDE WELDED CONNECTIONS FOR TRUSS TO BEARING.
- TRUSS MANUFACTURER SHALL DESIGN TRUSSES, DRAG TRUSSES, OUTRIGGERS, STRUCTURAL FACIA MEMBERS, ETC. AS DESCRIBED IN THE GENERAL NOTES AND FOR ANY ADDITIONAL LOADS NOTED ON THE PLAN.
- TRUSS MFR TO PROVIDE FULLY DIMENSIONED TRUSS PLACEMENT PLAN SHOWING TRUSS SPACING AND OFFSETS. THE PLACEMENT PLAN WILL BE UTILIZED TO SET TRUSSES IN THE FIELD. THE PLACEMENT PLAN MUST ALSO BE COMMUNICATED TO THE STEEL FABRICATOR RESPONSIBLE FOR SUPPLYING EMBED PLATES.
- TRUSSES SHALL BE STAGGERED AT INTERIOR BEARINGS AND HAVE FULL BEARING OVER THE EMBED PLATE IN THE CMU WALL. SOME TRUSSES MAY BUTT END TO END AND ARE TO HAVE THEIR END VERTICALS CONTINUOUSLY CONNECTED TOGETHER. SEE THE ROOF PLAN.
- TRUSS MANUFACTURER SHALL COORDINATE THIS PLAN WITH ARCHITECTURAL PLAN FOR VAULTS, OUTRIGGERS, HIGH-HEIGHT CMU WALLS, ETC.
- TRUSS LAYOUT IS FOR SCHEMATIC PURPOSES ONLY. TRUSS MANUFACTURER TO COORDINATE FINAL LAYOUT WITH ARCHITECT AND STRUCTURAL ENGINEER. TRUSS MFR. TO SUBMIT PRELIMINARY FRAMING PLAN & PROPOSED CONNECTION DETAILS (PRIOR TO SHOPS) FOR APPROVAL AND COORDINATION.
- TRUSS MANUFACTURER SHALL SUBMIT A PERMANENT TRUSS BRACING PLAN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT LOCATIONS.
- TRUSS MANUFACTURER SHALL SUBMIT ENGINEERED TRUSS DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT LOCATION.
- TRUSSES SHALL BE DESIGNED PER THE APPLICABLE AISI SP14 AND ALL OTHER WORK SHALL CONFORM TO AISI S202.

- GENERAL
 - TABLES ARE BASED ON ACI 318. LAP LENGTHS ARE BASED ON CONCRETE CLEAR COVER CONTROLLING. IF $c < c_{min}$ SPACING BETWEEN BARS IS LESS THAN 1.0 x PLUS TWICE THE CONCRETE COVER THEN TABLES ARE NO LONGER VALID AND LAP LENGTHS SHOULD BE DETERMINED PER ACI 318 SECTION 25.5.2 FOR OTHER CASES.
 - SPLICE PLAIN WELDED WIRE FABRIC BY LAPPING ONE FULL MESH SPACE PLUS 2".
- TENSION LAP SPICE TABLE
 - ALL SPLICES TO BE CLASS "B" TENSION SPICE UNLESS OTHERWISE NOTED.
 - VALUES LISTED ARE FOR ALL BARS EXCEPT TOP BARS. TOP BARS ARE DEFINED AS ANY BAR WITHIN 18" OF THE CONCRETE CAST BELOW THE BAR. FOR TOP BARS, MULTIPLY LENGTHS IN TABLE BY 1.3.
 - FOR LIGHT WEIGHT CONCRETE, MULTIPLY LENGTHS IN TABLE BY 1.3.
 - FOR EPOXY COATED REINFORCEMENT, MULTIPLY LENGTHS IN TABLE BY 1.5.
- COMPRESSION LAP SPICE TABLE
 - COMPRESSION DOWEL EMBEDMENT: 22 BAR DIAMETERS.

CONCRETE:

- CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH AND DENSITY, IN ACCORDANCE WITH THE FOLLOWING:
 - FILL SLABS & STAIR PAN FILL
 - STRENGTH, f_c : 3000 PSI
 - DENSITY, ρ_c : 145 PCF
 - WATER/CEMENT RATIO: 0.50
 - SLABS ON METAL DECK AND SLAB ON GRADE:
 - STRENGTH, f_c : 4000 PSI
 - DENSITY, ρ_c : 1500 PCF
 - WATER/CEMENT RATIO: 0.48
 - FOOTINGS AND FOUNDATIONS:
 - STRENGTH, f_c : 5000 PSI
 - DENSITY, ρ_c : 145 PCF
 - WATER/CEMENT RATIO: 0.45
 - ALL OTHER CONCRETE UNITS:
 - STRENGTH, f_c : 4000 PSI
 - DENSITY, ρ_c : 145 PCF
 - WATER/CEMENT RATIO: 0.45
- SLAB ON GRADE TO HAVE 3/4" MAXIMUM AGGREGATE SIZE FOR SLABS UP TO 6" AND 1 1/2" MAXIMUM AGGREGATE SIZE FOR SLABS THICKER THAN 6" PER MANUFACTURER SPECIFICATION TO BE APPROVED BY ENGINEER.
- REINFORCING SHALL CONFORM TO ASTM A615, GR. 60, INCLUDING TIES AND STIRRUPS.
- WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185.
- ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED, IN ACCORDANCE WITH LATEST ACI DETAILING MANUAL (S16-60)
- ALL REINFORCING SHALL BE SUPPORTED IN FORMS, SPACED WITH NECESSARY BACKFILL UNDER AND SHARED WITH REINFORCING. REINFORCING WITH LATEST C.R.S.I. "MANUAL OF STANDARD PRACTICE" DO NOT STICK DOWELS INTO WET CONCRETE. TIE IN PLACE PRIOR TO START OF CONCRETE PLACEMENT.
- MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:
 - UNFORMED SURFACE IN CONTACT WITH THE GROUND: 3"
 - FORMED SURFACES EXPOSED TO EARTH OR WEATHER:
 - #5 BAR OR SMALLER: 1 1/2"
 - #6 BAR OR LARGER: 2"
 - FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:
 - WALLS, SLABS: 3/4"
 - BEAMS, GIRDERS AND COLUMNS (TO TIES OR STIRRUPS): 1 1/2"
- ALL CONSTRUCTION JOINTS SHOWN ON DRAWINGS SHALL BE INCORPORATED INTO THE STRUCTURE, UNLESS THEIR ELIMINATION IS APPROVED BY THE ENGINEER. ADDITIONAL CONSTRUCTION JOINTS, REQUIRED TO FACILITATE CONSTRUCTION, SHALL BE LOCATED AT POINTS OF MINIMUM SHEAR AND SHALL BE DETAILED ON SHOP DRAWINGS. REINFORCEMENT SHALL PASS CONTINUOUSLY THROUGH THE JOINT. ALL CONSTRUCTION JOINTS SHALL BE ROUGHENED TO A DEPTH OF 1/4" FOR VERTICAL CONSTRUCTION JOINTS IN BEAMS, SLABS, WALLS, ETC. IT IS ACCEPTABLE TO USE "STAYFORM".
- ALL BUTTING CONCRETE MEMBERS SHALL BE DOWELED TOGETHER, UNLESS POURED MONOLITHICALLY. DOWELS SHALL BE EQUAL IN SIZE AND SPACING TO THE REINFORCING IN THE ADJACENT MEMBER.
- PROVIDE CONTINUOUS REINFORCEMENT AROUND CORNERS AND AT INTERSECTIONS AND THROUGH CONSTRUCTION JOINTS, CONTROL JOINTS, CONTRACTION JOINTS, AND JOINTS BETWEEN ALL BUTTING MEMBERS.
- UNLESS OTHERWISE SHOWN IN THE ARCHITECTURAL DRAWINGS, PROVIDE 3/4" CHAMFERS AT ALL EDGES THAT ARE EXPOSED TO VIEW IN THE FINISHED STRUCTURE.
- SEE ARCHITECTURAL DRAWINGS FOR DOOR AND WINDOW OPENINGS, DRIP SLOTS, REGLETS, MASONRY ANCHORS, PRECAST BEARING BRIDGES, BRICK LEDGE ELEVATIONS AND FOR MISCELLANEOUS EMBEDDED PLATES, BOLTS, ANCHORS, ANGLES, ETC.
- REFER TO ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHES. WHERE FINISH IS NOT SPECIFIED, CONFORM TO REQUIREMENTS OF ACI 301.

- MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS SHALL BE REFERRED TO FOR DRAINS, SLEEVES, OUTLET BOXES, CONDUIT, ANCHORS, ETC. THE VARIOUS TRADES ARE RESPONSIBLE FOR PLACING THEIR ITEMS. ALL CONDUITS TO HAVE TOP OF CONDUIT 8" BELOW BOTTOM OF SLAB ON GRADE.
- LAP SPICES SHALL BE IN ACCORDANCE WITH THE TENSION LAP SPICE TABLE BELOW, UNLESS NOTED OTHERWISE. WHERE CLASSES ARE NOT CALLED OUT ON DRAWINGS, USE CLASS "B" SPLICES.
- REFER TO TYPICAL DETAILS FOR HOUSEKEEPING PADS AND INERTIA BASES AT MECHANICAL EQUIPMENT. REFER TO MECHANICAL/ELECTRICAL/PLUMBING DRAWINGS FOR LOCATIONS.
- BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC., BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 3" CONCRETE.
- MASONRY DOWELS: PROVIDE, PLACE, AND SPACE TO MATCH MASONRY WALL VERTICAL REINFORCING.
- PROVIDE STANDARD HOOKS ON BARS TERMINATING AT A CONCRETE FACE UNLESS NOTED OTHERWISE (E.G. EDGES OF OPENINGS, SLAB EDGES, EXPANSION JOINTS, ENDS OF BEAMS, AND AT TOP, BOTTOM AND ENDS OF WALLS, ETC.).
- PROVIDE (2) #45 BARS (MIN.) @ EACH SIDE OF OPENING. EXTEND 2'-0" BEYOND OPENINGS.
- SEE MISC. NOTES FOR EPOXY / ADHESIVE ANCHORS.
- ALL CONCRETE CONCRETES, PROTRUDING PILASTERS, NON-PROTRUDING PILASTERS AND PIERS SHALL HAVE (3) #3 TIES @ 3" O.C. FROM TOP OF CONCRETE.

TENSION LAP SPICE LAP SPICE LENGTH (INCHES)												
$f_c = 4000$ PSI GRADE 60 REBAR $f_y = 60$ KSI												
CLEAR COVER	3/4"	1 1/2"	2"	3"								
BAR SIZE	A	B	A	B	A	B	A	B				
#3	12	16	12	16	12	16	12	16				
#4	15	20	12	16	12	16	12	16				
#5	21	28	15	20	15	20	15	20				
#6	29	38	18	24	18	24	18	24				
#7	46	60	29	38	25	33	25	33				
#8	57	75	36	47	29	38	29	38				
#9	69	90	44	58	36	47	33	43				
#10	83	108	54	71	44	58	37	49				
#11	98	128	65	85	53	69	41	54				

TENSION LAP SPICE LAP SPICE LENGTH (INCHES)												
$f_c = 4500$ PSI GRADE 60 REBAR $f_y = 60$ KSI												
CLEAR COVER	3/4"	1 1/2"	2"	3"								
BAR SIZE	A	B	A	B	A	B	A	B				
#3	12	16	12	16	12	16	12	16				
#4	14	19	12	16	12	16	12	16				
#5	20	28	14	19	14	19	14	19				
#6	27	36	17	23	17	23	17	23				
#7	44	58	27	36	24	32	24	32				
#8	54	71	34	45	27	36	27	36				
#9	65	85	42	55	34	45	31	41				
#10	79	103	51	67	42	55	35	46				
#11	92	120	61	80	50	65	38	50				

COMPRESSION LAP SPICE LAP SPICE LENGTH (INCHES)												
$f_c = 3000$ PSI OR GREATER												
BAR SIZE	30 db											
#3	12											
#4	15											
#5	19											
#6	23											
#7	26											
#8	30											
#9	34											
#10	38											
#11	42											

- GENERAL
 - TABLES ARE BASED ON ACI 318. LAP LENGTHS ARE BASED ON CONCRETE CLEAR COVER CONTROLLING. IF $c < c_{min}$ SPACING BETWEEN BARS IS LESS THAN 1.0 x PLUS TWICE THE CONCRETE COVER THEN TABLES ARE NO LONGER VALID AND LAP LENGTHS SHOULD BE DETERMINED PER ACI 318 SECTION 25.5.2 FOR OTHER CASES.
 - SPLICE PLAIN WELDED WIRE FABRIC BY LAPPING ONE FULL MESH SPACE PLUS 2".
- TENSION LAP SPICE TABLE
 - ALL SPLICES TO BE CLASS "B" TENSION SPICE UNLESS OTHERWISE NOTED.
 - VALUES LISTED ARE FOR ALL BARS EXCEPT TOP BARS. TOP BARS ARE DEFINED AS ANY BAR WITHIN 18" OF THE CONCRETE CAST BELOW THE BAR. FOR TOP BARS, MULTIPLY LENGTHS IN TABLE BY 1.3.
 - FOR LIGHT WEIGHT CONCRETE, MULTIPLY LENGTHS IN TABLE BY 1.3.
 - FOR EPOXY COATED REINFORCEMENT, MULTIPLY LENGTHS IN TABLE BY 1.5.
- COMPRESSION LAP SPICE TABLE
 - COMPRESSION DOWEL EMBEDMENT: 22 BAR DIAMETERS.

MASONRY:

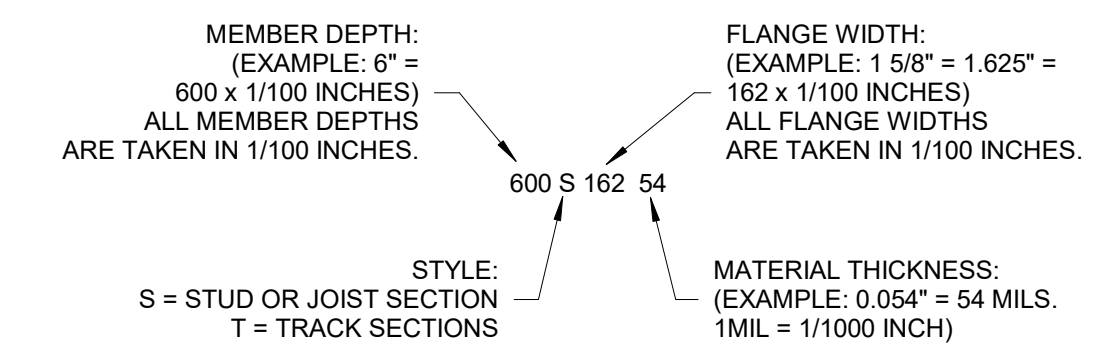
- NOTES APPLY TO MASONRY SHOWN ON STRUCTURAL DRAWINGS AND SHALL BE THE MINIMUM REQUIREMENTS FOR MASONRY SHOWN ON THE ARCHITECTURAL DRAWINGS.
- CONCRETE MASONRY:
 - COMPRESSIVE STRENGTH OF MASONRY, PRISM STRENGTH $f_m = 2000$ PSI (MIN)
 - COMPRESSIVE STRENGTH OF CMU = 2800 PSI
 - CONCRETE MASONRY UNITS: ASTM C90, GRADE N, TYPE 1-2 CELL UNITS
 - NORMAL WEIGHT UNITS
 - ASTM C90, BE AT NOT LESS THAN 28 DAYS = 125% OF f_m , 2500 PSI (MIN)
 - MORTAR: ASTM C270 TYPE - S, HYDRATED LIME REQUIRED
- REINFORCING: BARS SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED ON DRAWINGS. WIRES SHALL CONFORM TO ASTM A82.
- DOW

STRUCTURAL STEEL:

- STEEL SHALL CONFORM TO THE FOLLOWING GRADES:
 - W (U.N.O.): A992 GRADE 50 ($f_y = 50$ KSI)
 - ANGLES, BASE PLATES, CONN. PLATES (U.N.O.): A36 ($f_y = 36$ KSI)
 - STRUCTURAL PIPE: A53 ($f_y = 35$ KSI)
 - STRUCTURAL TUBE: A500 GRADE C ($f_y = 50$ KSI)
 - E HEADED STUDS: AWS D1.1, TYPE B
 - THREADED ROD: A36
 - ANCHOR ROD: F1554, GR36 U.N.O.
 - BOLTS: A325 / F1852
 - ELECTRODES: MATCHING STRENGTH, 70 KSI
- ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE APPLICABLE AISC 303 CODE OF STANDARD PRACTICE, EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS.
- CONNECTIONS MAY BE BOLTED OR WELDED. THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF CONNECTIONS NOT COMPLETELY DETAILED ON THE DRAWINGS. GENERALLY, CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE SCHEMATIC AND ARE ONLY INTENDED TO SHOW THE RELATIONSHIP OF MEMBERS CONNECTED. ANY CONNECTION THAT IS NOT SHOWN OR IS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY AN ENGINEER, REGISTERED IN THE STATE OF THE PROJECT LOCATION, RETAINED BY THE FABRICATOR, COMPLETELY DETAILED MEANS THE FOLLOWING INFORMATION IS SHOWN ON THE DETAIL:
 - ALL PLATE DIMENSIONS AND GRADES.
 - ALL WELD SIZES, LENGTHS, PITCHES, AND RETURNS.
 - ALL HOLE SIZES AND SPACINGS.
 - NUMBER AND TYPES OF BOLTS: WHERE BOLTS ARE SHOWN BUT NO NUMBER IS GIVEN, THE CONNECTION HAS NOT BEEN COMPLETELY DETAILED.
 - WHERE PARTIAL INFORMATION IS GIVEN, IT SHALL BE THE MINIMUM REQUIREMENT FOR THE CONNECTION.
- DESIGN CALCULATIONS FOR TYPICAL BEAM CONNECTIONS AND ALL PRIMARY BRACING AND HANGER CONNECTIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
- CONNECTION DESIGN FORCES:
 - BEAMS:
 - 55% OF TOTAL ALLOWABLE UNIFORM LOAD CAPACITY FROM THE APPLICABLE AISC STEEL CONSTRUCTION TABLES FOR ALLOWABLE LOADS ON BEAMS, W_{UL} .
 - REACTIONS SHOWN ON DRAWINGS ARE ALLOWABLE LOADS.
 - 10 KIPS MINIMUM.
 - MOMENT CONNECTIONS INDICATED ON THE DRAWINGS THUS: DESIGN FOR MOMENT SHOWN OR, IF NOT SHOWN, DEVELOP MOMENT CAPACITY OF MEMBER WITH $\phi_b = 0.66$.
 - MAINTAIN TENSION CAPACITY OF COLUMNS, DIAGONALS AND MEMBERS SUBJECT TO TENSION AT BOLT HOLES, NOTCHES, OR COPES.
 - CONNECTION FORCE NOTATION:
 - P = AXIAL FORCE IN KIPS: (+) TENSION, (-) COMPRESSION
 - V OR V_s = SHEAR IN KIPS
 - M = MOMENT IN KIP-FT
 - T = TORSION IN KIP-FT
 - LOADS SHOWN INCLUDE COMPENSATION FOR CODE PERMITTED STRESS INCREASES AND LOAD REDUCTIONS FOR CONNECTION DESIGN.
- THE MINIMUM PLATE THICKNESS SHALL BE 3/8".
- COMPOSITE BEAMS
 - NUMBER OF SHEAR CONNECTORS (S.C.) SHOWN ON THE DRAWINGS ARE BASED UPON THE ASSUMED 3/4" DIA SHEAR CAPACITIES THAT FOLLOW:
 - 11.5K EACH FOR NORMAL WEIGHT CONCRETE (145 PCF)
 - 9.5K EACH FOR LIGHT WEIGHT CONCRETE (115 PCF)
 - S.C. CAPACITY VARIES WITH THE TYPE AND LENGTH OF THE S.C. AND WITH THE PROPERTIES OF THE COMPOSITE DECK. THE S.C. SUPPLIER SHALL DETERMINE THE TYPE AND LENGTH OF SHEAR CONNECTOR CONSISTENT WITH THE COMPOSITE DECK PROPERTIES AND IN ACCORDANCE WITH ALL AISC REQUIREMENTS, INCLUDING MAXIMUM ALLOWABLE SPACING. THE NUMBER OF SHEAR CONNECTORS SHALL BE ADJUSTED AS REQUIRED.
 - S.C. TYPE, LENGTH, SHEAR VALUE, AND DETAILED LAYOUT SHALL BE SUBMITTED WITH THE COMPOSITE METAL DECK SHOP DRAWINGS. SEE COMPOSITE METAL DECK NOTES.
 - MAXIMUM HEIGHT OF S.C. SHALL NOT EXCEED THE SLAB DEPTH - 1".
 - MINIMUM HEIGHT OF S.C. SHALL BE COMPOSITE DECK DEPTH + 1"
 - SPACING OF S.C. WITHIN A GIVEN LENGTH SHALL BE AS UNIFORM AS POSSIBLE.
 - UNLESS NOTED OTHERWISE NON-COMPOSITE AND COMPOSITE BEAMS DO NOT REQUIRE TEMPORARY SHORING.
 - CONTRACTOR SHALL FURNISH THE ADDITIONAL CONCRETE DUE TO WET CONCRETE DEFLECTION OF THE COMPOSITE BEAMS.
 - NO SHOP PAINT ON S.C. OR ON THE TOP SURFACE OF THE BEAMS THAT RECEIVE FIELD WELDED S.C.
- BOLTED CONNECTIONS:
 - MINIMUM BOLT DIAMETER = 3/4"
 - SLIP CRITICAL CONNECTIONS OF A325SC OR A490SC BOLTS SHALL BE USED FOR ALL BOLTED CONNECTIONS OF BRACING MEMBERS, MOMENT CONNECTIONS, CANTILEVERS, AND AS SHOWN ON THE DRAWINGS. OVERSIZED AND LONG-SLOTTED HOLES ARE ALLOWED FOR FRICTION CONNECTIONS.
 - ALL OTHER BOLTED CONNECTIONS SHALL BE BEARING TYPE USING A325N OR A490N BOLTS. OVERSIZED HOLES AND LONG-SLOTTED HOLES ARE NOT ALLOWED UNLESS SHOWN ON THE DRAWINGS.
 - A307 BOLTS MAY BE USED WHERE INDICATED ON THE DRAWINGS.
 - PROTRUDING BOLT HEADS, SHAFTS OR NUTS SHALL NOT EXTEND INTO NOR PROHIBIT THE APPLICATION OF ARCHITECTURAL FINISHES AND THEY SHALL NOT EXTEND INTO NOR PROHIBIT THE PLACEMENT OF STEEL DECKING TO THE CORRECT LINE AND ELEVATION.
 - THE FABRICATOR IS RESPONSIBLE FOR VERIFYING THE TENSION CAPACITY OF AXIALLY LOADED MEMBERS AFTER A SECTION IS REDUCED FOR BOLT HEADS. MEMBER SIZE MAY BE INCREASED OR CONNECTION PLATES ADDED AS REQUIRED.
 - SHOP DRAWINGS SHALL INDICATE THE TYPE OF BOLT USED IN EACH CONNECTION AND THE ALLOWABLE VALUES USED FOR THE VARIOUS BOLT TYPES.
- WELDED CONNECTIONS:
 - WELDS ARE CONTINUOUS UNLESS NOTED OTHERWISE.
 - ALL FILLET WELDS: AISC MINIMUM BUT NOT LESS THAN 1/8" UNLESS NOTED OTHERWISE.
 - ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE" (AWS D1.1) PUBLISHED BY THE AMERICAN WELDING SOCIETY. ELECTRODES FOR WELDING SHALL COMPLY WITH THE REQUIREMENTS OF TABLE 3.1 OF (AWS D1.1).
 - ALL GROOVE WELDS SHALL BE COMPLETE PENETRATION UNLESS NOTED OTHERWISE.
- SPACING OF STEEL MEMBERS, UNLESS SHOWN ON THE DRAWINGS, IS PROHIBITED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT.
- NO CHANGE IN SIZE OR POSITION OF THE STRUCTURAL ELEMENTS SHALL BE MADE OF HOLES, SLOTS, CUTS, ETC., AND ARE NOT PERMITTED THROUGH ANY MEMBER UNLESS THEY ARE DETAILED ON THE APPROVED SHOP DRAWINGS.
- NO FINAL BOLTING OR WELDING SHALL BE MADE UNTIL AS MUCH OF THE STRUCTURE HAS BEEN PROPERLY ALIGNED AND WILL THEREBY BE STIFFENED.
- UNLESS NOTED OTHERWISE, BEAMS SHALL BEAR 8" MINIMUM ON CONCRETE OR MASONRY. ANCHOR BEAMS TO MASONRY WITH A GOVERNMENT-TYPE ANCHOR.
- FABRICATE ALL BEAMS WITH THE MILL CAMBER UP.
- SHEAR STUDS: CONFORM TO AWS D1.1, SHOP WELD EXCEPT WHERE APPLIED THROUGH METAL DECK.
- SEE MISC. NOTES FOR EPOXY / ADHESIVE ANCHORS.

COLD-FORMED STEEL:

- ALL SIZING BASED ON STEEL STUD MANUFACTURERS ASSOCIATION (ICC ESR-3064P) PRODUCT TECHNICAL INFORMATION.
- MATERIALS SHALL CONFORM TO THE FOLLOWING:
 - GALVANIZED MATERIAL:
 - ALL GALVANIZED STUDS AND JOISTS 12, 14 AND 16 GAUGE (97, 68 AND 54 MIL) SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF ASTM A563 SS, GRADE 50, CLASS 1 OR 3 WITH A MINIMUM YIELD OF 50,000 PSI
 - ALL GALVANIZED 18 AND 20 GAUGE (43 AND 33 MIL) STUDS AND JOISTS: ALL GALVANIZED TRACK, BRIDGING, END CLOSURES AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF ASTM A563 SS, GRADE 33 WITH A MINIMUM YIELD OF 33,000 PSI
 - ALL GALVANIZED STUDS, JOISTS, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A GALVANIZED COATING MEETING THE REQUIREMENTS OF ASTM A525.
 - PROPERTIES:
 - THE PHYSICAL AND STRUCTURAL PROPERTIES LISTED BY THE STEEL STUD MANUFACTURER ASSOCIATION AND AISI DESIGN MANUAL SHALL BE CONSIDERED THE MINIMUM PERMITTED FOR ALL FRAMING MEMBERS. SPECIFICALLY, THE FOLLOWING MINIMUM PROPERTIES, CALCULATED IN ACCORDANCE WITH THE LATEST AISI SPECIFICATION SHALL BE PROVIDED:
 - f_y (N) S_x (IN³) A (IN²) R_x (IN) I_x (IN⁴) R_y (IN) I_y (IN⁴) R_z (IN) I_z (IN⁴) R_t (IN) I_t (IN⁴) R_z (IN) I_z (IN⁴) R_t (IN) I_t (IN⁴)
 - SUBSTITUTIONS:
 - ANY SUBSTITUTIONS MUST BE APPROVED IN WRITING PRIOR TO DELIVERY, BY THE ARCHITECT AND/OR ENGINEER OF RECORD.
- INSTALLATION OF STUDS SHALL BE AS PER THE LATEST:
 - ASTM C1007 "INSTALLATION OF LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS AND ACCESSORIES"
 - ASTM C885 "SPECIFICATION FOR LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS, RUNNERS (TRACK) AND BRACING OR BRIDGING FOR SCREW APPLICATION OF GYPSUM BOARD AND METAL PLASTER BASES"
 - ASTM C754 "SPECIFICATION FOR INSTALLATION OF STEEL FRAMING MEMBERS TO RECEIVE SCREW ATTACHED GYPSUM BOARD"
- ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS, OR AS REQUIRED FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.
- ALL TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK SHALL BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, OR THEY SHALL BE BUTT-WELDED OR SPLICED TOGETHER.
- ALL STUD BRIDGING SHALL BE ATTACHED IN A MANNER TO PREVENT STUD ROTATION. BRIDGING ROWS SHALL BE SPACED ACCORDING TO DIETRICH INDUSTRY RECOMMENDATION.
- TEMPORARY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED.
- STUD ENDS MUST BE SQUARELY SEATED AGAINST THE TRACK WEB. BOTH STUD FLANGES MUST BE ATTACHED TO TRACK MEMBERS AT TOP AND BOTTOM.
- STUD BRIDGING SHALL BE PROVIDED BY 1-1/2" COLD ROLLED U-CANNEL. THE U-CANNEL MUST BE ATTACHED TO EACH STUD BY WELDING OR ATTACHING WITH CLIP ANGLES AND SCREWS. HORIZONTAL STRAPPING AND SOLID BRIDGING WITH TRACK MEMBERS CAN ALSO BE USED FOR BRIDGING. BRIDGING SHALL BE SPACED AT 4'-0" O.C. MAX.
- THE FOLLOWING MINIMUM COLD FORMED STEEL ATTACHMENTS SHALL BE PROVIDED U.N.O.:
 - TRACK TO:
 - STUD: (1) - #10 TEK SCREW EACH FLANGE, EACH STUD
 - STRUCTURAL STEEL: (1) - 1/4" DIA. POWER DRIVEN FASTENER @ 2'-0" O.C.
 - CONCRETE: (1) - 1/4" DIA. POWER DRIVEN FASTENER @ 16" O.C.
 - METAL DECK: (1) - #10 TEK SCREW @ 1'-4" O.C.
 - MASONRY: (1) - 1/4" DIA. POWER DRIVEN FASTENER @ 2'-8" O.C.
 - STUD TO:
 - STRUCTURAL STEEL: (1) - L2x2x - 14 GA. CLIP ANGLE CONNECTION W/ (3) - #10 TEK SCREWS INTO METAL STUD AND (3) - 1/4" DIA. POWER DRIVEN FASTENERS INTO STRUCTURAL STEEL OR (3) 1/4" TEK SCREWS INTO STEEL

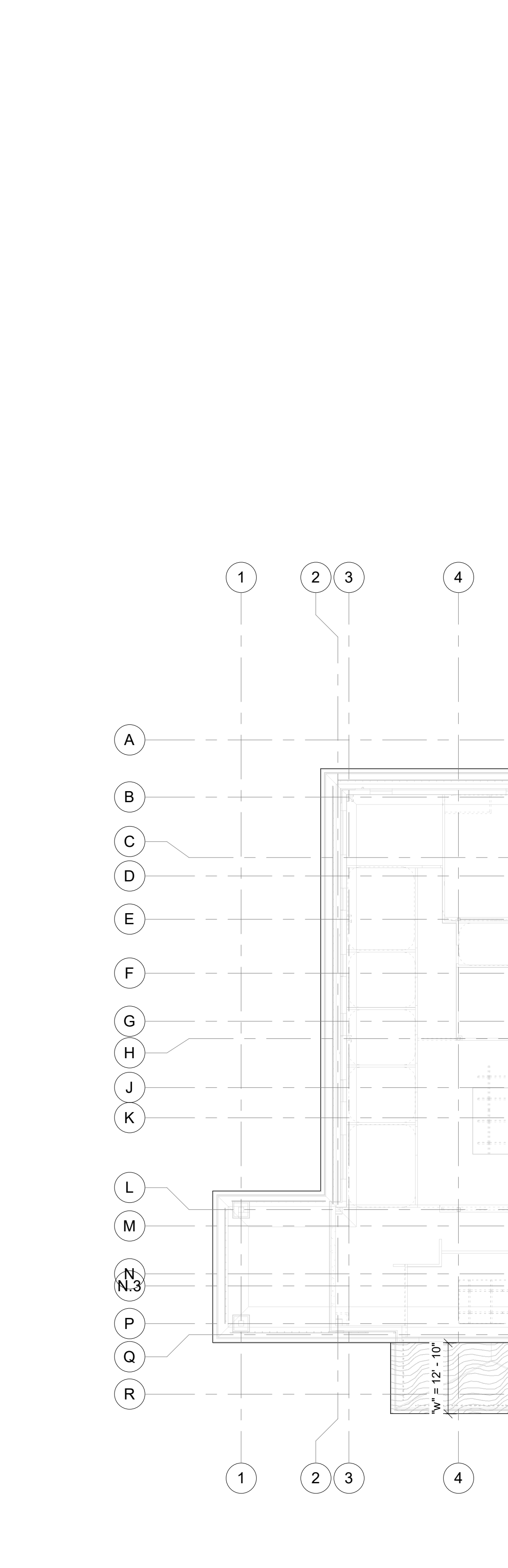


PRE-ENGINEERED METAL BUILDING:

- THE PRE-ENGINEERED METAL BUILDING STRUCTURE, INCLUDING THE SUPPORT FRAME, COLUMNS, COLUMN CONNECTION TO FRAME, ROOF PURLINS, WALL GIRTS, BRACING, ROOF PANELS AND WALL PANELS TO BE DESIGNED BY THE PRE-ENGINEERED METAL BUILDING SUPPLIER. THE CONTRACT DOCUMENTS SHOW GENERAL LAYOUT AND OVERALL DESIGN INTENT AND NOT SHOW ALL ELEMENTS NECESSARY TO PROVIDE A COMPLETE STRUCTURAL SYSTEM. THE DESIGN AND FINAL LAYOUT OF THE PRE-ENGINEERED METAL BUILDING SYSTEM SHALL BE THE RESPONSIBILITY OF THE PRE-ENGINEERED METAL BUILDING MANUFACTURER.
- COLLATERAL, SUPERIMPOSED GRAVITY AND WIND LOADS. SEE GENERAL NOTES & SPECIFIC ROOF PLANS FOR LOAD REQUIREMENTS.
- THE PRE-ENGINEERED METAL BUILDING SUPPLIER SHALL SUBMIT CALCULATED COLUMN LOADS, ANCHOR BOLT LOCATIONS AND SIZES TO ENGINEER PRIOR TO FOOTING CONSTRUCTION. SUBMITTAL TO SHOW GRAPHICALLY MAXIMUM COLUMN REACTIONS FROM ALL LOAD CONDITIONS AND COMBINATIONS.
- MAXIMUM ALLOWABLE DISPLACEMENTS:
 - LATERAL DISPLACEMENT: H/200
 - IN + GAVE HEIGHT, BASED ON THE LOW EAVE)
 - VERTICAL DISPLACEMENT OF MAIN FRAME RAFTERS: L/180, TOTAL LOAD.
 - VERTICAL DISPLACEMENT OF ROOF SUB-FRAMING: L/180 TOTAL LOAD.
 - NO REDUCTIONS ARE ALLOWED. FULL DESIGN LOADS SHALL BE USED IN CALCULATING LATERAL & VERTICAL LOADS.
- WHEN CALCULATING NET UPLIFT USE ONLY STRUCTURE SELF WEIGHT. DO NOT INCLUDE SNOW LOADS OR MECHANICAL EQUIPMENT LOADS.
- SIGNIFICANT DESIGN DEVIATIONS WILL BE CONSIDERED FOR APPROVAL ONLY IF FOUNDATION STRUCTURAL CALCULATIONS ARE SUBMITTED WITH THE PROPOSED REVISIONS. CALCULATIONS ARE TO BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF THE PROJECT LOCATION. SSC ENGINEERING INC. RESERVES THE RIGHT TO DETERMINE IF A DEVIATION IS CONSIDERED TO BE "SIGNIFICANT".
- THE BUILDING FRAMES SHALL BE MOMENT RESISTING UNLESS NOTED OTHERWISE IN THE PRE-ENGINEERED METAL BUILDING DESIGN DRAWINGS.
- ALL FLASHING, PANELS, ATTACHMENTS, INSULATIONS, ETC. ARE TO BE PROVIDED BY THE METAL BUILDING MANUFACTURER UTILIZING STANDARD PRODUCTS.
- ROOF AND WALL OPENINGS SHALL CONSIST OF APPROPRIATE CURBS AND FLASHING DETAILS TO PROVIDE FOR A WEATHER TIGHT CONDITION.
- REFER TO THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR COLUMN/RAFTER DEPTH RESTRICTIONS AND DESIGN REQUIREMENTS.
- METAL BUILDING VERTICAL BRACING SHALL CONSIST OF PORTAL FRAMES AND X-BRACING AS INDICATED AT LOCATIONS SHOWN ON PLANS. METAL BUILDING MANUFACTURER SHALL COORDINATE LOCATION OF ALL BRACES TO MINIMIZE INTERFERENCE WITH ARCHITECTURAL FEATURES, MECHANICAL EQUIPMENT AND DUCTS. ROD OR CABLE BRACES MAY NOT BE SUBSTITUTED WHERE PORTAL FRAMES ARE SHOWN.
- METAL BUILDING FRAMING LAYOUT AND MEMBERS SHOWN ARE SUGGESTED ONLY. MANUFACTURER IS RESPONSIBLE FOR COORDINATING REQUIREMENTS WITH THE ARCHITECT AND OWNER AND PROVIDING A COMPLETE STRUCTURAL FRAMING SYSTEM DESIGNED BY THE MANUFACTURER. METAL BUILDING MANUFACTURER SHALL COORDINATE ALL DIMENSIONS, ELEVATIONS, BRACING AND SIZES AND SHAPES OF MEMBERS WITH ARCHITECT AND OWNER PRIOR TO FABRICATION AND CONSTRUCTION. ALL MEMBERS CONNECTIONS AND DECKING NOT SPECIFICALLY SIZED ON DRAWINGS SHALL BE DESIGNED AND SUPPLIED BY THE METAL BUILDING MANUFACTURER.
- METAL BUILDING MANUFACTURER SHALL SUBMIT CALCULATIONS AND SHOW DRAWINGS, STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT FOR WHICH WORK IS BEING PERFORMED FOR REVIEW PRIOR TO FABRICATION.
- FOUNDATIONS AND PIERS HAVE NOT BEEN COORDINATED WITH FINAL FOR CONSTRUCTION PRE-ENGINEERED METAL BUILDING REACTIONS AND DRAWINGS. FOOTINGS AND PIER SIZES AND/OR LOCATIONS MAY CHANGE ONCE FINAL FOR CONSTRUCTION REACTIONS AND DRAWINGS ARE PROVIDED TO SSC FOR OUR USE.

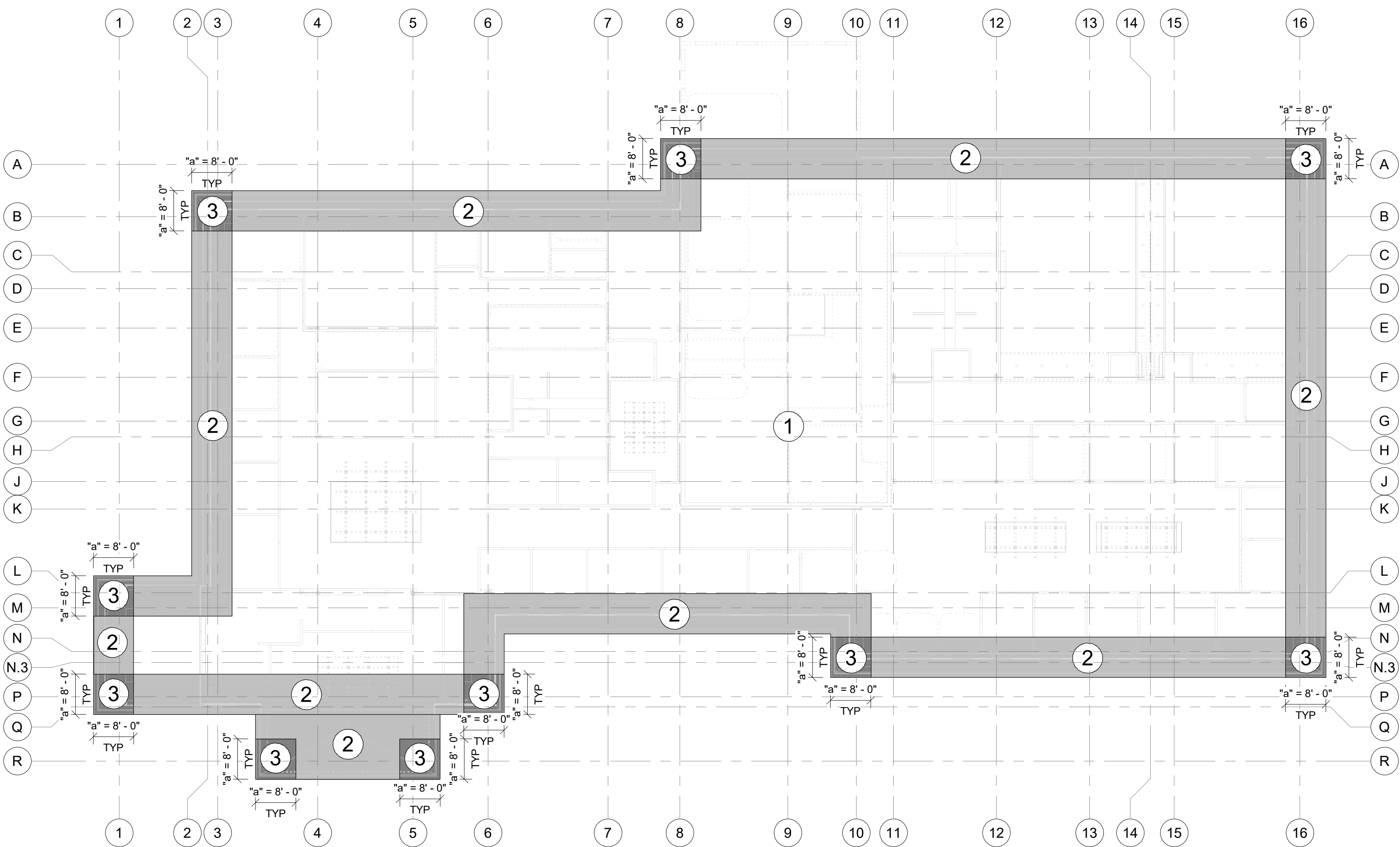
MISCELLANEOUS:

- STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL, PLUMBING, ELECTRICAL, FP AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND WORK.
- NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
- NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
- OPENINGS 1'-4" AND LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
- THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
- UNLESS OTHERWISE NOTED, FIRE PROOFING METHODS AND MATERIALS FOR STRUCTURAL MEMBERS ARE NOT SHOWN ON STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR FIRE RATING REQUIREMENTS, FIRE PROOFING METHODS AND MATERIALS.
- DO NOT SCALE THESE DRAWINGS, USE DIMENSIONS.
- CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD. EXPANSION JOINTS SHOWN ON THE DRAWINGS HAVE BEEN DESIGNED TO ACCOMMODATE ANTICIPATED THERMAL MOVEMENT AFTER THE BUILDING IS COMPLETE.
- THE CONTRACTOR SHALL INFORM THE ARCHITECT IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY FOR SUCH DEVIATION BY THE ARCHITECT'S APPROVAL OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS HE HAS SPECIFICALLY INFORMED THE ARCHITECT OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE ARCHITECT HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
- ALL THINGS WHICH, IN THE OPINION OF THE CONTRACTOR, APPEAR TO BE DEFICIENCIES, OMISSIONS, CONTRADICTIONS, OR AMBIGUITIES, IN THE PLANS AND SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT. PLANS AND/OR SPECIFICATIONS WILL BE CORRECTED, OR WRITTEN INTERPRETATION OF THE ALLEGED DEFICIENCY, OMISSION, CONTRADICTION OR AMBIGUITY WILL BE MADE BY THE ARCHITECT BEFORE THE EFFECTED WORK PROCEEDS.
- CHECK ALL DIMENSIONS AGAINST REQUIREMENTS OF OTHER CONTRACT DOCUMENTS. FIELD VERIFY DIMENSIONS RELATING TO EXISTING CONDITIONS PRIOR TO ORDERING MATERIALS AND FABRICATION.
- WHERE DIMENSIONS OR WEIGHTS OF EQUIPMENT OR SYSTEMS ARE VARIABLE FROM MANUFACTURER TO MANUFACTURER, VERIFY DIMENSIONS AND WEIGHTS SHOWN ON DRAWINGS WITH SELECTED MANUFACTURER PRIOR TO ORDERING MATERIALS. NOTIFY STRUCTURAL ENGINEER OF DISCREPANCIES, DO NOT PLACE EQUIPMENT WHEN SHIPPING OR OPERATING WEIGHT EXCEEDS WEIGHT INDICATED ON STRUCTURAL DRAWINGS.
- NO MODIFICATION, ALTERATION OR REPAIR SHALL BE MADE WITHOUT PRIOR REVIEW BY STRUCTURAL ENGINEER. SUBMIT DETAILS AND CALCULATIONS PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN STATE WHERE PROJECT IS LOCATED AND EMPLOYED BY CONTRACTORS.
- EPOXY / ADHESIVE ANCHORS SHALL BE INSTALLED WITHIN THE TEMPERATURE REQUIREMENTS PROVIDED BY THE EPOXY / ADHESIVE MANUFACTURER. THE GENERAL CONTRACTOR SHALL NOTIFY THE ARCHITECT / ENGINEER IF TEMPERATURES ARE NOT WITHIN THE PROPER RANGE.



WIND LOADING PLAN NOTES:

- COMPONENTS & CLADDING WIND PRESSURES (PSF) BASED ON ULTIMATE WIND SPEEDS PER ASCE 7:
 - FOR CLADDING ATTACHMENT (10 SQ FEET EFFECTIVE AREA)
 - TYPICAL
 - ZONE 1: +19.2, -30.5
 - ZONE 2: +19.2, -33.1
 - ZONE 3: +19.2, -33.1
 - ZONE 4: +33.3, -36.2
 - ZONE 5: +33.3, -44.7
 - FOR JOISTS (250 SQ FEET EFFECTIVE AREA)
 - ZONE 1: +16.0, -27.7
 - ZONE 2: +16.0, -39.0
 - ZONE 3: +16.0, -39.0
 - NET WIND UPLIFT (PSF) BASED ON ALLOWABLE WIND SPEEDS (0.60 + 0.60W)
 - FOR JOISTS
 - ZONE 1: -4.7
 - ZONE 2: -11.4
 - ZONE 3: -11.4
- BUILDING WIND PRESSURES LISTED IN THE SCHEDULE ABOVE SHALL BE USED FOR DESIGN OF COMPONENTS AND CLADDING ELEMENTS NOT EXPLICITLY DESIGNED WITHIN THESE DOCUMENTS AND NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL.
- THE WIND LOADS NOTED ABOVE ARE BASED ON ZONES WHICH LOCATE SPECIFIC AREAS ON THE ROOF AND WALL SURFACES. THESE ZONES ARE LOCATED AS NOTED BELOW:
 - ZONE 1: TYPICAL ROOF SURFACE NOT LOCATED WITHIN ZONE 2 OR 3
 - ZONE 2: EDGES OF ALL ROOF SURFACES LOCATED WITH A DISTANCE "1" FROM ALL ROOF PEAKS AND HIPS
 - ZONE 3: CORNERS OF ALL ROOF SURFACES LOCATED WITHIN A DISTANCE "1" FROM EACH EDGE OF ROOF
 - ZONE 4: TYPICAL WALL SURFACE AREA NOT LOCATED WITHIN ZONE 5 (EXCLUDES PARAPETS)
 - ZONE 5: CORNERS OF ALL WALL SURFACES LOCATED WITHIN A DISTANCE "1" FROM ALL CORNERS (EXCLUDES PARAPETS)



SNOW LOADING PLAN NOTES:

- UNIFORM ROOF DESIGN SNOW LOAD, $P_m = 16.5$ PSF
- UNIFORM FLAT ROOF SNOW LOAD, $P_f = 11.5$ PSF
- THE SNOW DRIFT LOADING INDICATED ON PLAN IS IN ADDITION TO THE UNIFORM FLAT ROOF SNOW LOAD INDICATED ABOVE.
- "w" IS THE WIDTH OF SNOW DRIFT

TABLE 1705.6		
REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS		
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	---	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	---	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	---	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	---
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	---	X

TABLE 1705.3				
REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION				
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	---	X	ACI 318: Ch. 20, 25.2, 25.3 & 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND c. INSPECT ALL OTHER WELDS.	---	X X X	AWS D1.4 ACI 318: 26.6.4	---
3. INSPECT ANCHORS CAST IN CONCRETE.	---	X	ACI 318: 17.8.2	---
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	X	X X	ACI 318: 17.8.2.4 ACI 318: 17.8.2	---
5. VERIFYING USE OF REQUIRED DESIGN MIX.	---	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TEST, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	---	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	---	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	---	X	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECTION OF PRESTRESSED CONCRETE: a. APPLICATION OF PRESTRESSED FORCES; AND b. GROUTING OF BONDED PRESTRESSING TENDONS	X X	---	ACI 318: 26.10	---
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	---	X	ACI 318: 26.9	---
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	---	X	ACI 318: 26.11.2	---
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	---	X	ACI 318: 26.11.1.2(b)	---

TMS 602-16 TABLE 3 (LEVEL 2)	
MINIMUM VERIFICATION REQUIREMENTS OF MASONRY	
TYPE	REFERENCED STANDARD
1. PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS ARE REQUIRED.	TMS 602: 1.5
2. PRIOR TO CONSTRUCTION, VERIFICATION OF f_m AND f_{mcc} , EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE IS REQUIRED.	TMS 602: 1.4 B
3. DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE IS REQUIRED.	TMS 602: 1.5 & 1.6.3
4. DURING CONSTRUCTION, VERIFICATION OF f_m AND f_{mcc} , FOR EVERY 5,000 SQ. FT. IS NOT REQUIRED.	TMS 602: 1.4 B
5. DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT IS NOT REQUIRED.	TMS 602: 1.4 B

TMS 602-16 TABLE 4 (LEVEL 2)				
MINIMUM SPECIAL INSPECTION REQUIREMENTS OF MASONRY CONSTRUCTION				
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	
			TMS 402	TMS 602
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: a. PROPORTIONS OF SITE-PREPARED MORTAR b. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES c. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES d. PRESTRESSING TECHNIQUE e. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY f. SAMPLE PANEL CONSTRUCTION	---	X X X X ---		2.1, 2.6 A & 2.6 C 2.4 B & 2.4 H 3.4 & 3.6 A 3.6 B 2.1 C.1 1.6 D
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: a. GROUT SPACE b. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES c. PLACEMENT OF REINFORCEMENT, CONNECTORS AND ANCHOR BOLTS d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	---	X X X X	10.8 & 10.9 6.1, 6.3.1, 6.3.6 & 6.3.7	3.2 D & 3.2 F 2.4 & 3.6 3.2 E & 3.4 2.6B & 2.4 G.1.b
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION: a. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS b. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION c. SIZE AND LOCATION OF STRUCTURAL MEMBERS d. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION e. WELDING OF REINFORCEMENT f. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F (4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90°F (32.2°C)) g. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE h. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE i. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	---	X X X X X X X X X	1.2.1(e), 6.2.1 & 6.3.1 6.1.6.1.2	1.5 3.3 B 3.3 F 1.8 C & 1.8 D 3.6 B 3.5 & 3.6 C 3.3 B.9 & 3.3 F.1.b
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS	---	X		1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3 & 1.4 B.4

AISC 360-16 TABLE N5.4-1				
INSPECTION TASKS PRIOR TO WELDING				
TYPE	QUALITY CONTROL		QUALITY ASSURANCE	
	OBSERVE	PERFORM	OBSERVE	PERFORM
1. WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	---	X	X	---
2. WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	---	X	---	X
3. MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	---	X	---	X
4. MATERIAL IDENTIFICATION (TYPE/GRADE)	X	---	X	---
5. WELDER IDENTIFICATION SYSTEM	X	---	X	---
6. FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) a. JOINT PREPARATIONS b. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) c. CLEANLINESS (CONDITION OF STEEL SURFACES) d. TACKING (TACK WELD QUALITY AND LOCATION) e. BACKING TYPE AND FIT (IF APPLICABLE)	X	---	X	---
7. FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y- AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY) a. JOINT PREPARATIONS b. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) c. CLEANLINESS (CONDITION OF STEEL SURFACES) d. TACKING (TACK WELD QUALITY AND LOCATION)	---	X	X	---
8. CONFIGURATION AND FINISH OF ACCESS HOLES	X	---	X	---
9. FIT-UP OF FILLET WELDS a. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) b. CLEANLINESS (CONDITION OF STEEL SURFACES) c. TACKING (TACK WELD QUALITY AND LOCATION)	X	---	X	---
10. CHECK WELDING EQUIPMENT	X	---	---	---

AISC 360-16 TABLE N5.4-2				
INSPECTION TASKS DURING WELDING				
TYPE	QUALITY CONTROL		QUALITY ASSURANCE	
	OBSERVE	PERFORM	OBSERVE	PERFORM
1. CONTROL AND HANDLING OF WELDING CONSUMABLES a. PACKAGING b. EXPOSURE CONTROL	X	---	X	---
2. NO WELDING OVER CRACKED TACK WELDS	X	---	X	---
3. ENVIRONMENTAL CONDITIONS a. WIND SPEED WITHIN LIMITS b. PRECIPITATION AND TEMPERATURE	X	---	X	---
4. WPS FOLLOWED a. SETTING ON WELDING EQUIPMENT b. TRAVEL SPEED c. SELECTED WELDING MATERIALS d. SHIELDING GAS TYPE/FLOW RATE e. PREHEAT APPLIED f. INTERPASS TEMPERATURE MAINTAINED (MIN/MAX) g. PROPER POSITION (F, V, H, OH)	X	---	X	---
5. WELDING TECHNIQUES a. INTERPASS AND FINAL CLEANING b. EACH PASS WITHIN PROFILE LIMITATIONS c. EACH PASS MEETS QUALITY REQUIREMENTS	X	---	X	---
6. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	---	X	---	X

AISC 360-16 TABLE N5.4-3				
INSPECTION TASKS AFTER WELDING				
TYPE	QUALITY CONTROL		QUALITY ASSURANCE	
	OBSERVE	PERFORM	OBSERVE	PERFORM
1. WELDS CLEANED	X	---	X	---
2. SIZE, LENGTH AND LOCATION OF WELDS	---	X	---	X
3. WELDS MEET VISUAL ACCEPTANCE CRITERIA a. CRACK PROHIBITION b. WELD/BASE-METAL FUSION c. CRATER CROSS SECTION d. WELD PROFILES e. WELD SIZE f. UNDERCUT g. POROSITY	---	X	---	X
4. ARC STRIKES	---	X	---	X
5. k-AREA	---	X	---	X
6. WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES	---	X	---	X
7. BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	---	X	---	X
8. REPAIR ACTIVITIES	---	X	---	X
9. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	---	X	---	X
10. NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	X	---	X	---

AISC 360-16 TABLE N5.6-1				
INSPECTION TASKS PRIOR TO BOLTING				
TYPE	QUALITY CONTROL		QUALITY ASSURANCE	
	OBSERVE	PERFORM	OBSERVE	PERFORM
1. MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	X	---	---	X
2. FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	X	---	X	---
3. CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	X	---	X	---
4. CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	X	---	X	---
5. CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	X	---	X	---
6. PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	---	X	X	---
7. PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	X	---	X	---

AISC 360-16 TABLE N5.6-2				
INSPECTION TASKS DURING BOLTING				
TYPE	QUALITY CONTROL		QUALITY ASSURANCE	
	OBSERVE	PERFORM	OBSERVE	PERFORM
1. FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED	X	---	X	---
2. JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	X	---	X	---
3. FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	X	---	X	---
4. FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	X	---	X	---

AISC 360-16 TABLE N5.6-3				
INSPECTION TASKS AFTER BOLTING				
TYPE	QUALITY CONTROL		QUALITY ASSURANCE	
	OBSERVE	PERFORM	OBSERVE	PERFORM
1. DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	---	X	---	X

SPECIAL INSPECTION TABLE NOTES:

- THE CONSTRUCTION INSPECTIONS LISTED ARE IN ADDITION TO THE CALLED INSPECTIONS REQUIRED BY IBC, AS AMENDED. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY A CITY / COUNTY INSPECTOR. SPECIALLY INSPECTED WORK WHICH IS INSTALLED OR COVERED WITHOUT THE APPROVAL OF THE CITY / COUNTY INSPECTOR IS SUBJECT TO REMOVAL OR EXPOSURE.
- CONTINUOUS INSPECTION IS ALWAYS REQUIRED DURING THE PERFORMANCE OF THE WORK UNLESS OTHERWISE SPECIFIED.
- THE SPECIAL INSPECTORS MUST BE CERTIFIED BY THE CITY / COUNTY TO PERFORM THE TYPES OF INSPECTION SPECIFIED.
- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INFORM THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. WORK PERFORMED WITHOUT REQUIRED SPECIAL INSPECTION IS SUBJECT TO REMOVAL.
- SUBMIT WRITTEN REPORTS WITHIN TWO DAYS OF TESTING TO ENGINEER OF RECORD. ALL REPORTS MUST IDENTIFY DISCREPANCIES. IT IS THE RESPONSIBILITY OF SPECIAL INSPECTOR TO FURNISH REPORTS AND SHOW ALL DISCREPANCIES HAVE BEEN CORRECTED PRIOR TO OCCUPANCY. FAILURE TO FOLLOW IBC CHAPTER 17 REQUIREMENTS IS A VIOLATION OF THE CODE FOR WHICH THE BUILDING HAD BEEN DESIGNED.

SPECIAL INSPECTIONS:

- GENERAL
 - ALL SECTIONS & TABLES LISTED BELOW ARE REFERENCING THE 2015 INTERNATIONAL BUILDING CODE.
- SOILS & FOUNDATIONS
 - SOILS
 - SPECIAL INSPECTIONS & TESTS OF EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION & TABLE 1705.6
 - THE APPROVED GEOTECHNICAL REPORT & THE CONSTRUCTION DOCUMENTS SHALL BE USED TO DETERMINE COMPLIANCE.
 - DURING FILL PLACEMENT, THE SPECIAL INSPECTOR SHALL VERIFY THAT PROPER MATERIALS & PROCEDURES ARE USED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT.
- CONCRETE CONSTRUCTION
 - SPECIAL INSPECTIONS & TESTS OF CONCRETE CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 1705.3 & TABLE 1705.3
 - SPECIAL INSPECTIONS OF WELDING & QUALIFICATIONS OF SPECIAL INSPECTORS FOR WELDING REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF AWS D1.4 FOR SPECIAL INSPECTION & SPECIAL INSPECTOR QUALIFICATION.
- MASONRY CONSTRUCTION
 - SPECIAL INSPECTIONS & TESTS OF MASONRY CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE QUALITY ASSURANCE PROGRAM REQUIREMENTS OF THE APPLICABLE TMS 402 & TMS 602.
- STEEL CONSTRUCTION
 - SPECIAL INSPECTIONS & NONDESTRUCTIVE TESTS OF STEEL CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 1705.2
 - SPECIAL INSPECTIONS & NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES & PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF THE APPLICABLE AISC 360.
 - SPECIAL INSPECTIONS & QUALIFICATIONS OF WELDING SPECIAL INSPECTORS FOR COLD-FORMED STEEL FLOOR & ROOF DECK SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF SDI QA/QC.
- SPECIAL INSPECTIONS FOR WIND RESISTANCE
 - SPECIAL INSPECTIONS FOR WIND RESISTANCE ARE REQUIRED FOR BUILDINGS AND STRUCTURES CONSTRUCTED IN THE FOLLOWING AREAS:
 - IN WIND EXPOSURE CATEGORY B, WHERE V_{min} AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1 IS 120 MPH OR GREATER.
- SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY A CITY/COUNTY INSPECTOR.
- SPECIAL INSPECTED WORK WHICH IS INSTALLED OR COVERED WITHOUT APPROVAL OF LOCAL AND SPECIAL INSPECTORS IS SUBJECT TO REMOVAL OR EXPOSURE.
- SPECIAL INSPECTORS MUST BE CERTIFIED BY THE CITY/COUNTY TO PERFORM THE TYPES OF INSPECTIONS SPECIFIED.
- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INFORM THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. ANY WORK PERFORMED WITHOUT REQUIRED SPECIAL INSPECTION IS SUBJECT TO REMOVAL.
- SUBMIT WRITTEN REPORTS WITHIN TWO DAYS OF TESTING TO ENGINEER OF RECORD.

M+H

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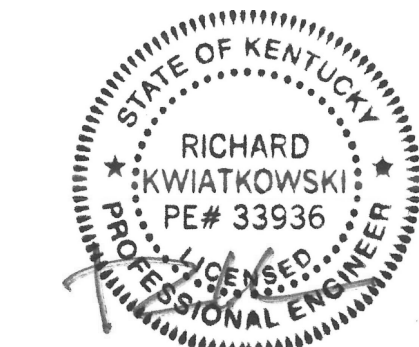
TAYLOR COUNTY RECC

685 WATER TOWER BYPASS

CAMPBELLVILLE, KY 42718



RICHARD E. KWIAKOWSKI
Professional Engineer
KY LIC# PE-33936
EXP: 06/30/2027



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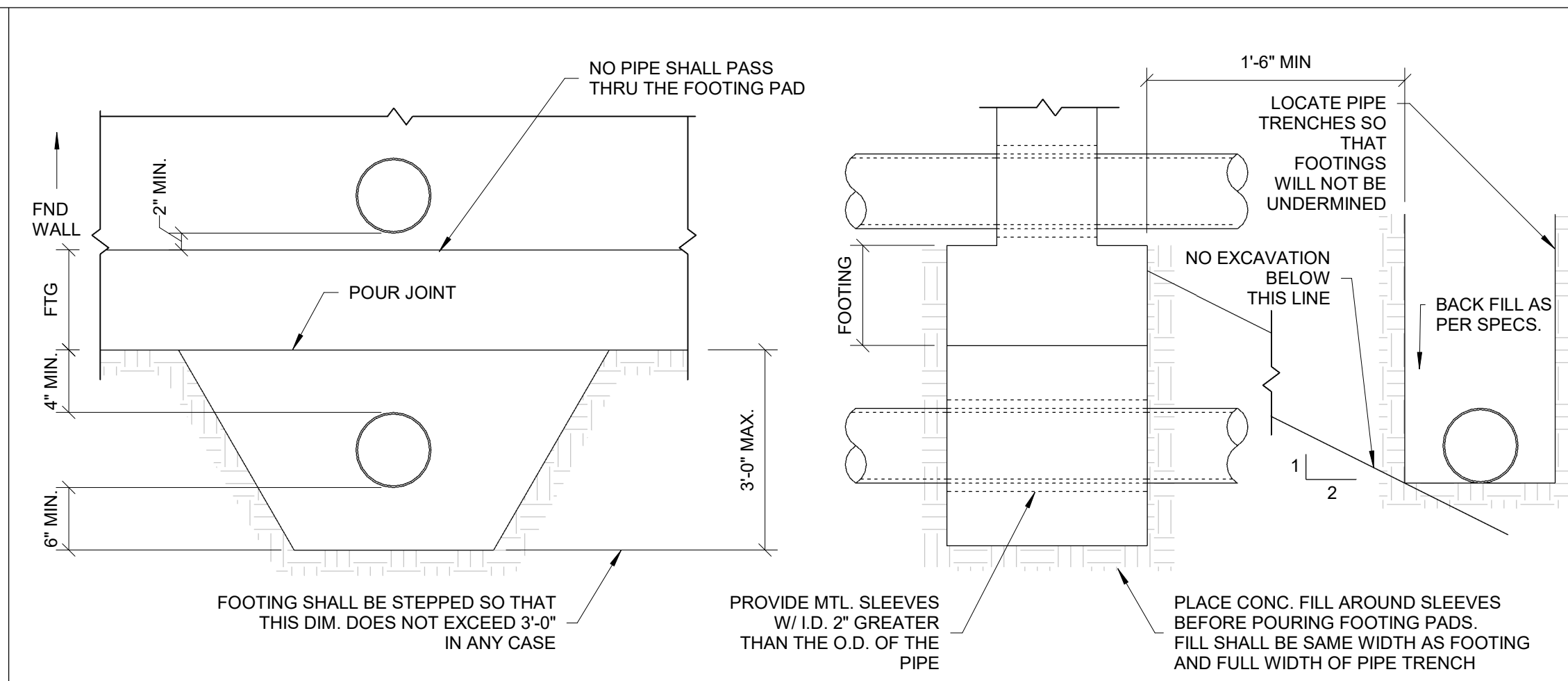
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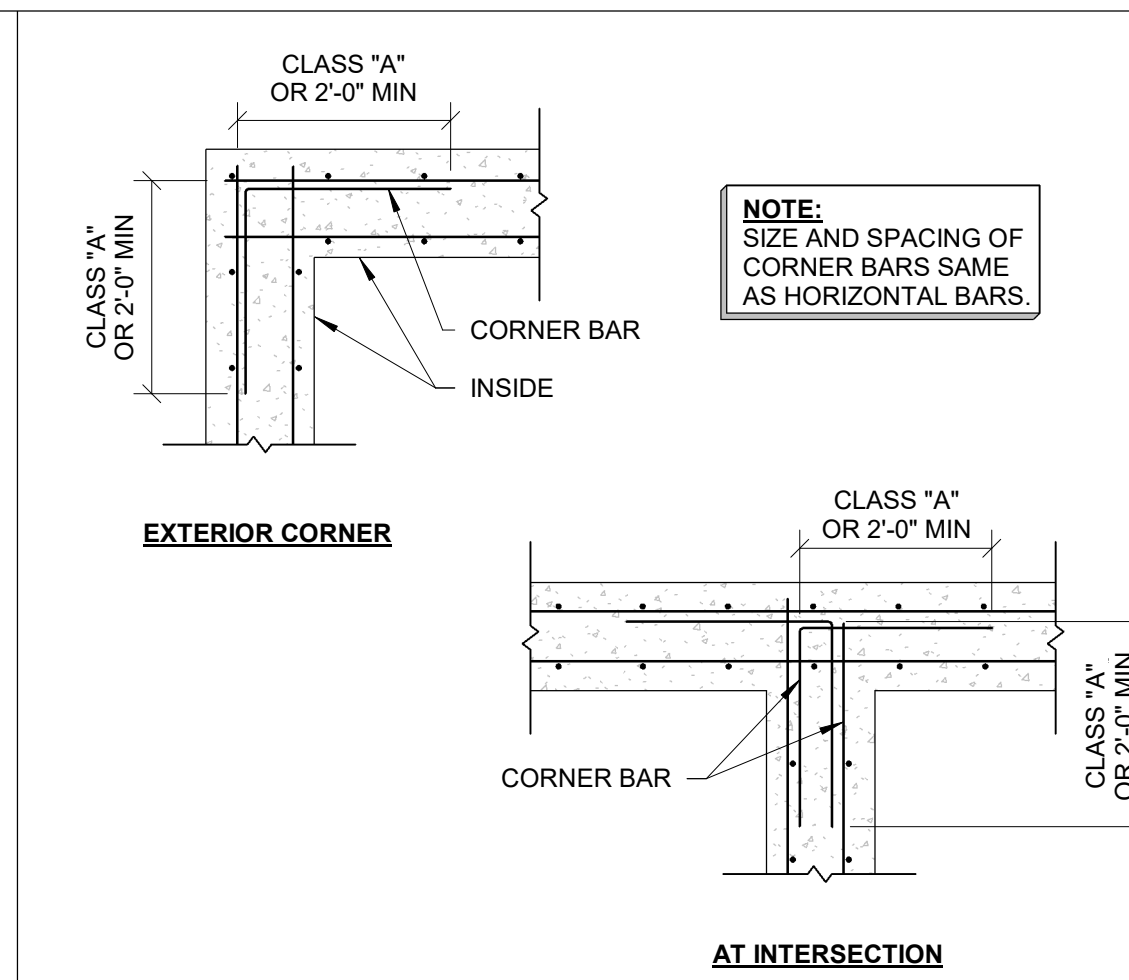
STRUCTURAL SPECIAL INSPECTIONS

S0.3

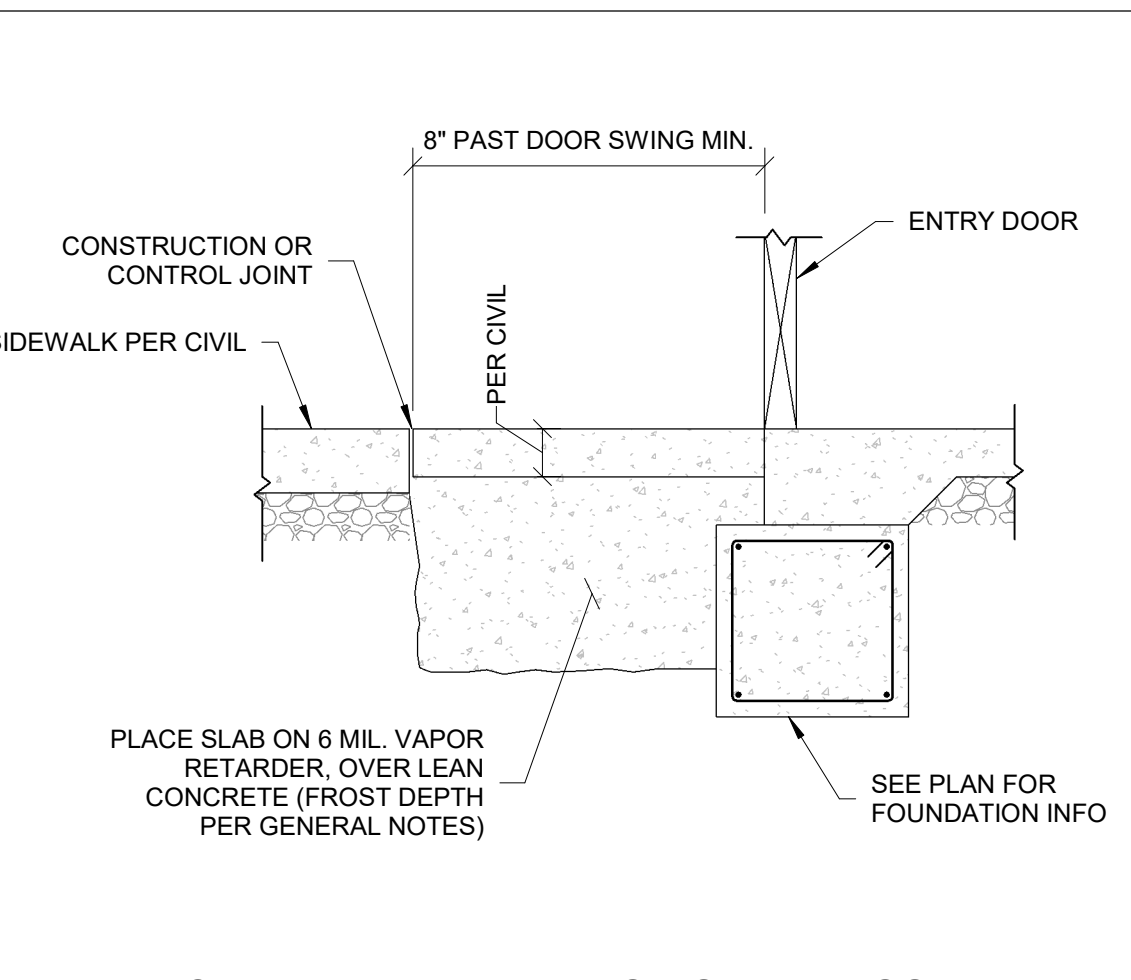
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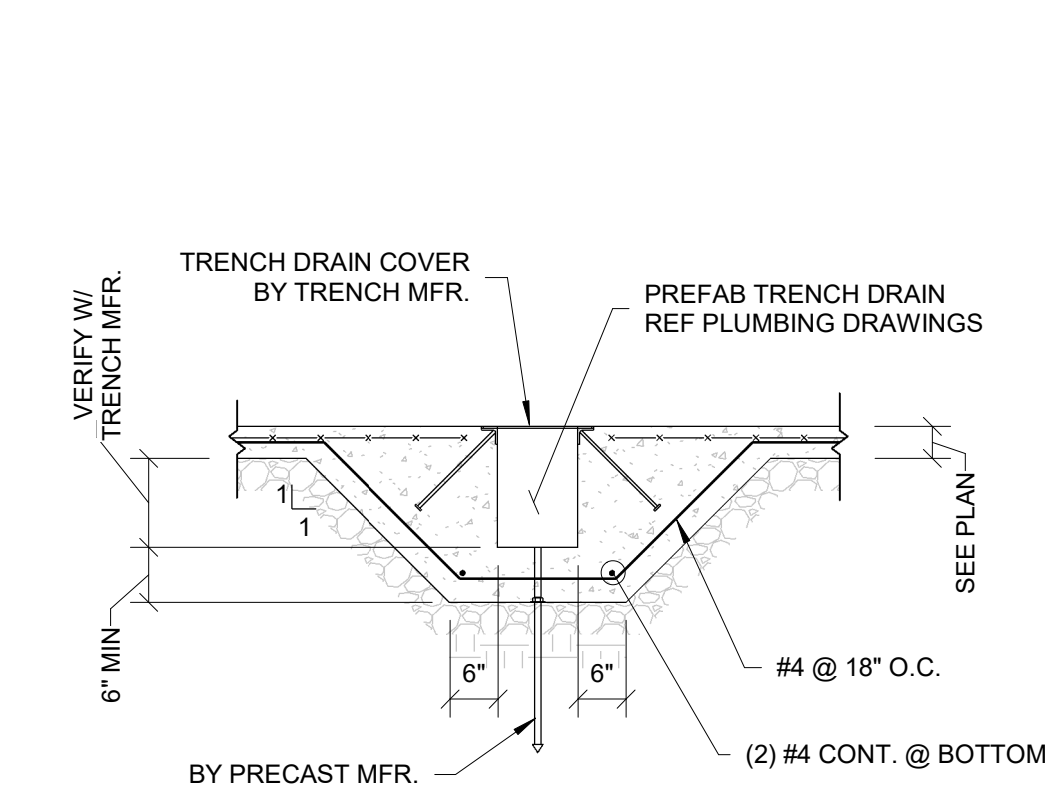
20 TYPICAL PIPE TRENCH FOOTING DETAIL
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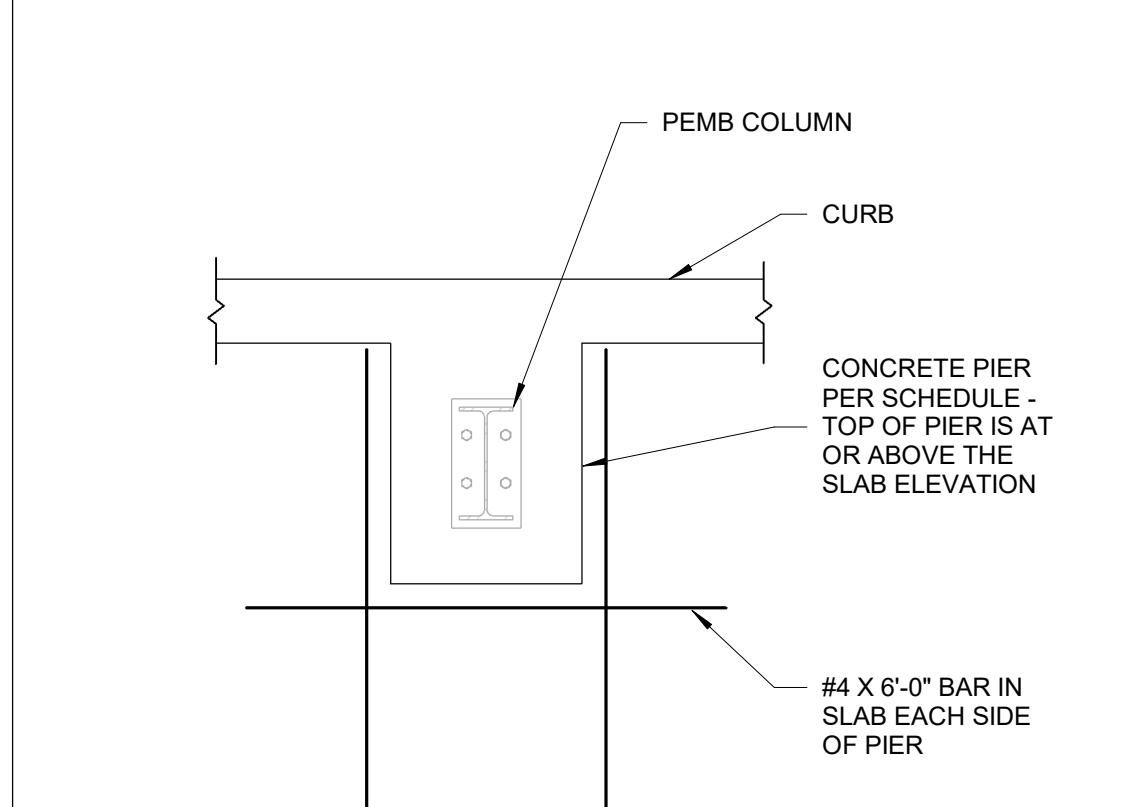
10 TYPICAL CONCRETE CORNER BAR DETAILS
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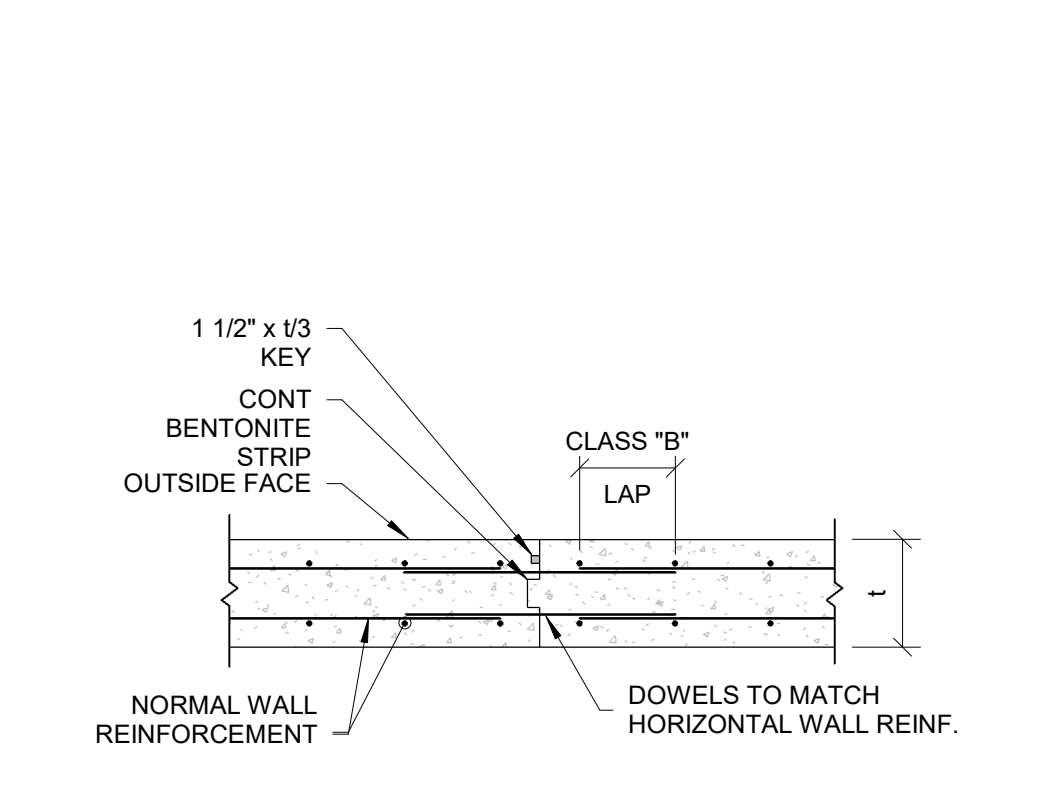
5 TYPICAL ENTRY DETAIL FOR SLAB FROST HEAVE PREVENTION
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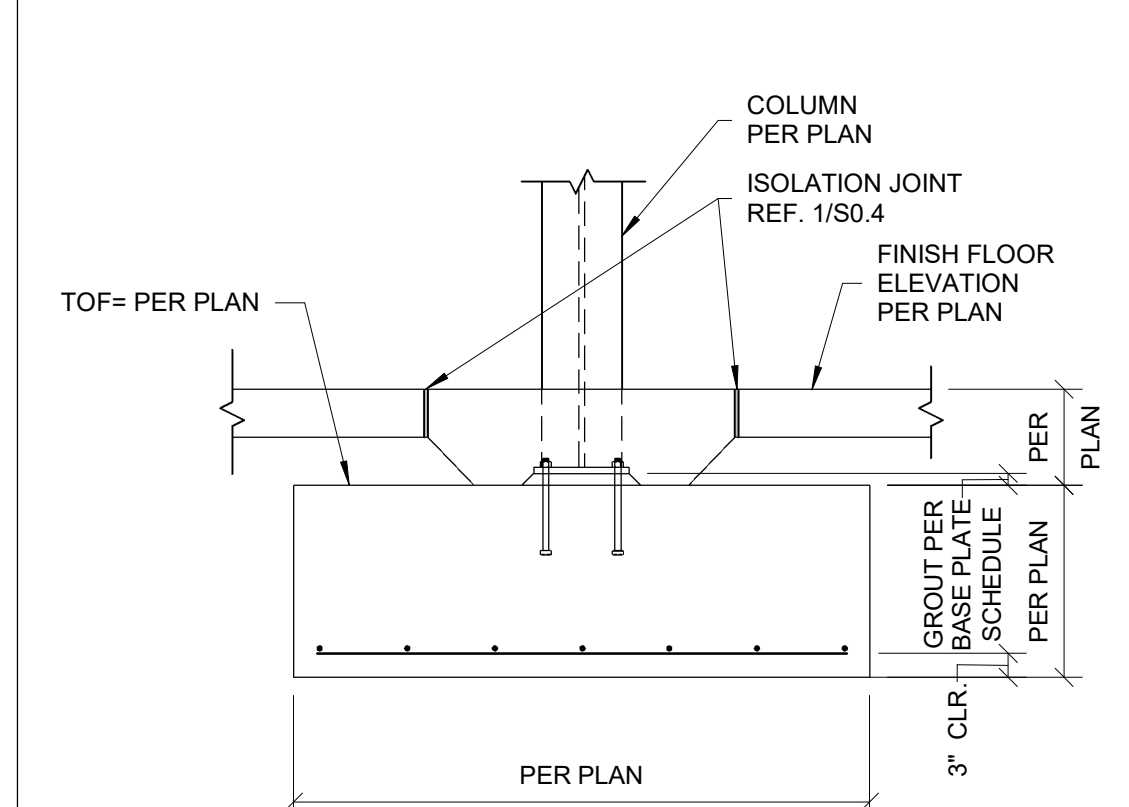
24 PREFAB TRENCH DRAIN
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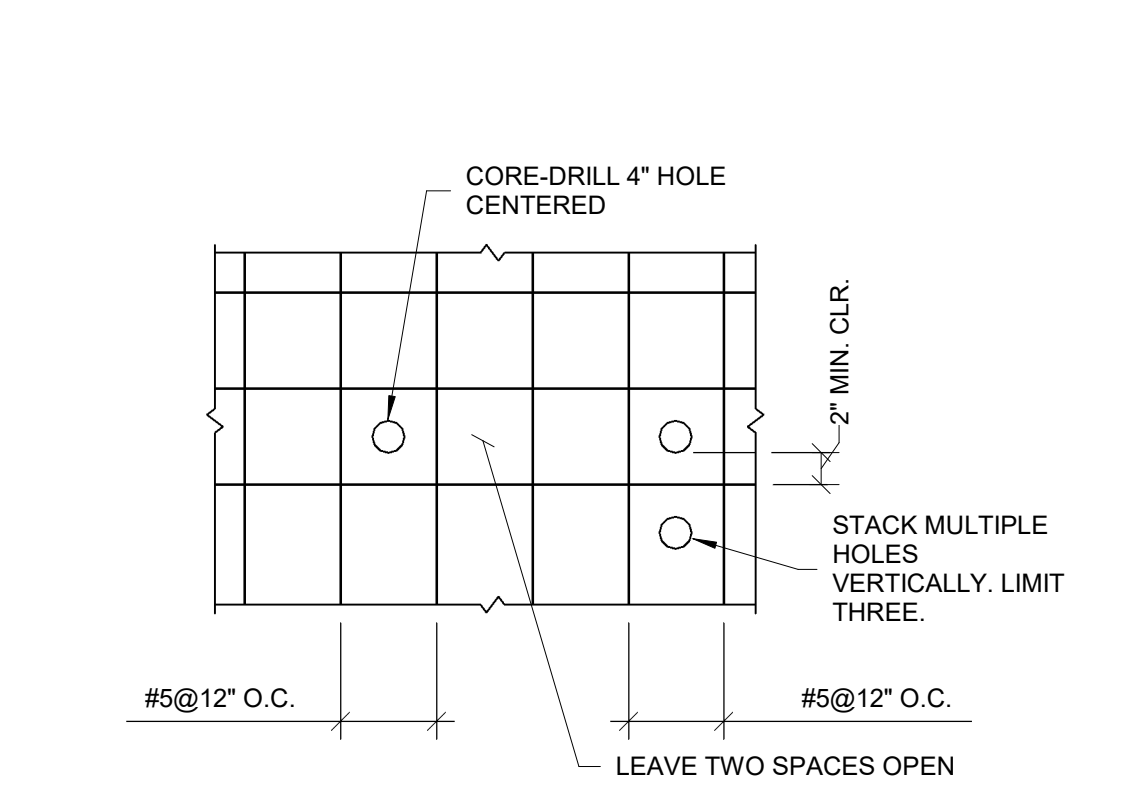
19 TYPICAL SLAB REINF AROUND PIERS
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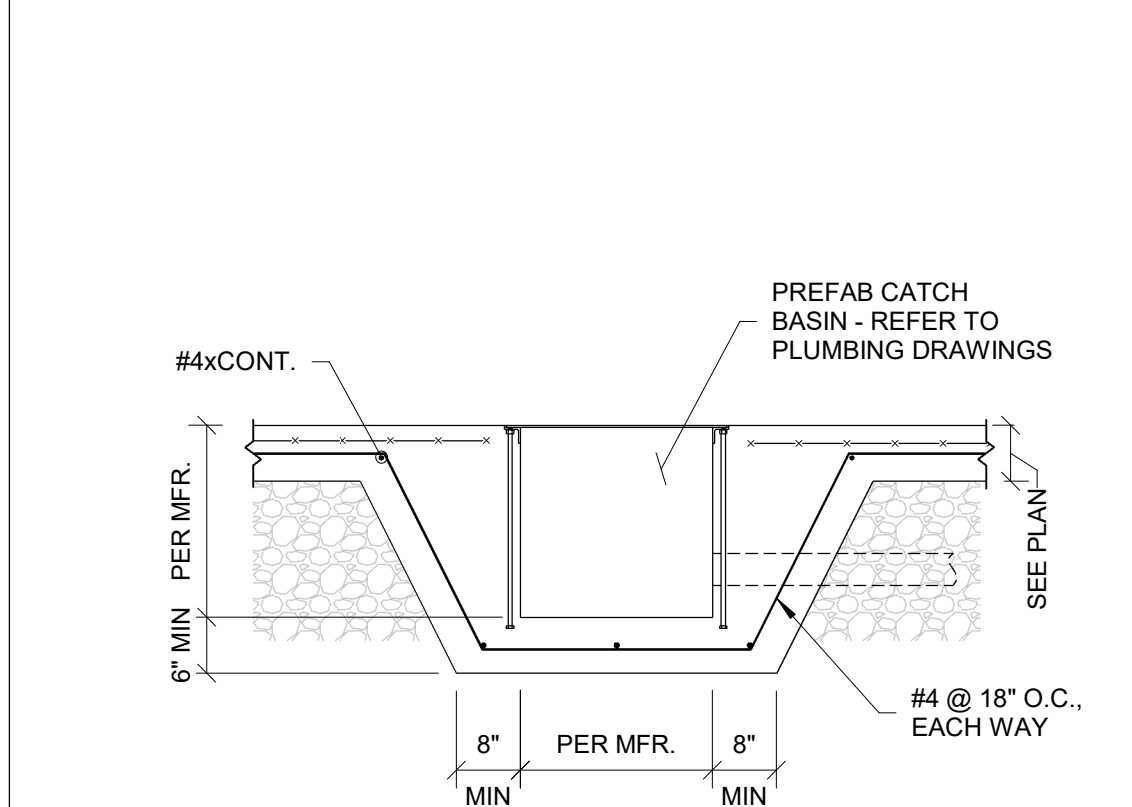
14 TYPICAL VERTICAL C.J. IN CONCRETE WALL
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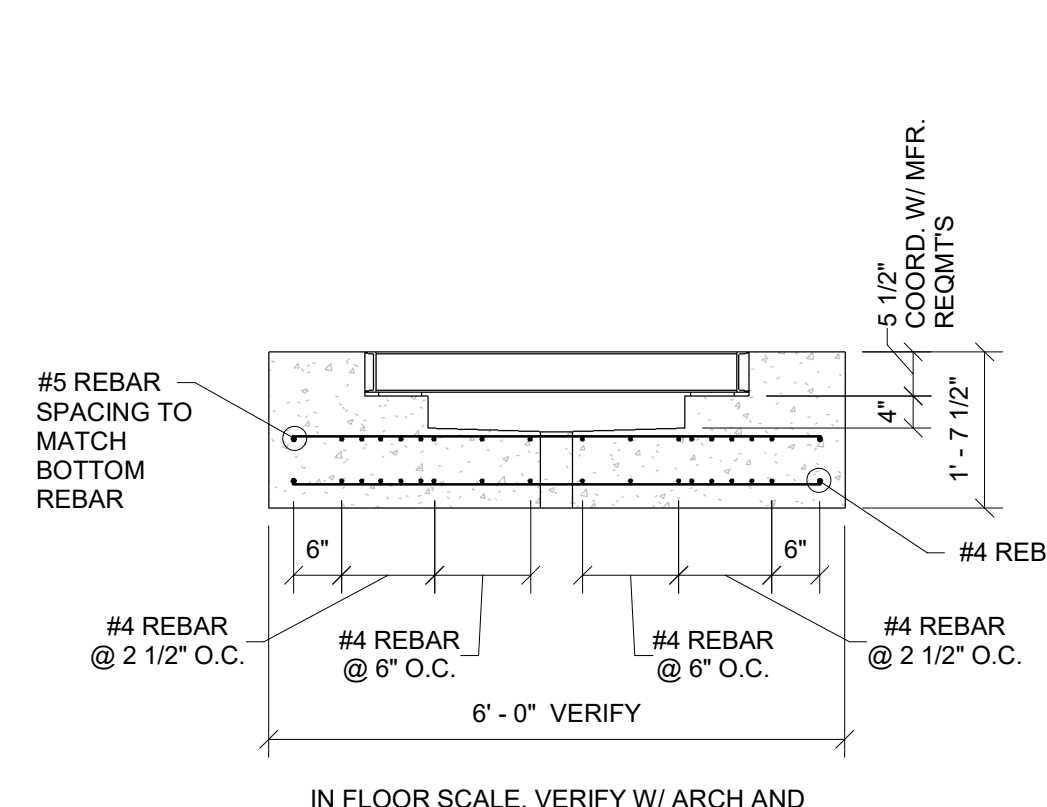
9 TYPICAL DETAIL OF INTERIOR COLUMN AT SPREAD FOOTING
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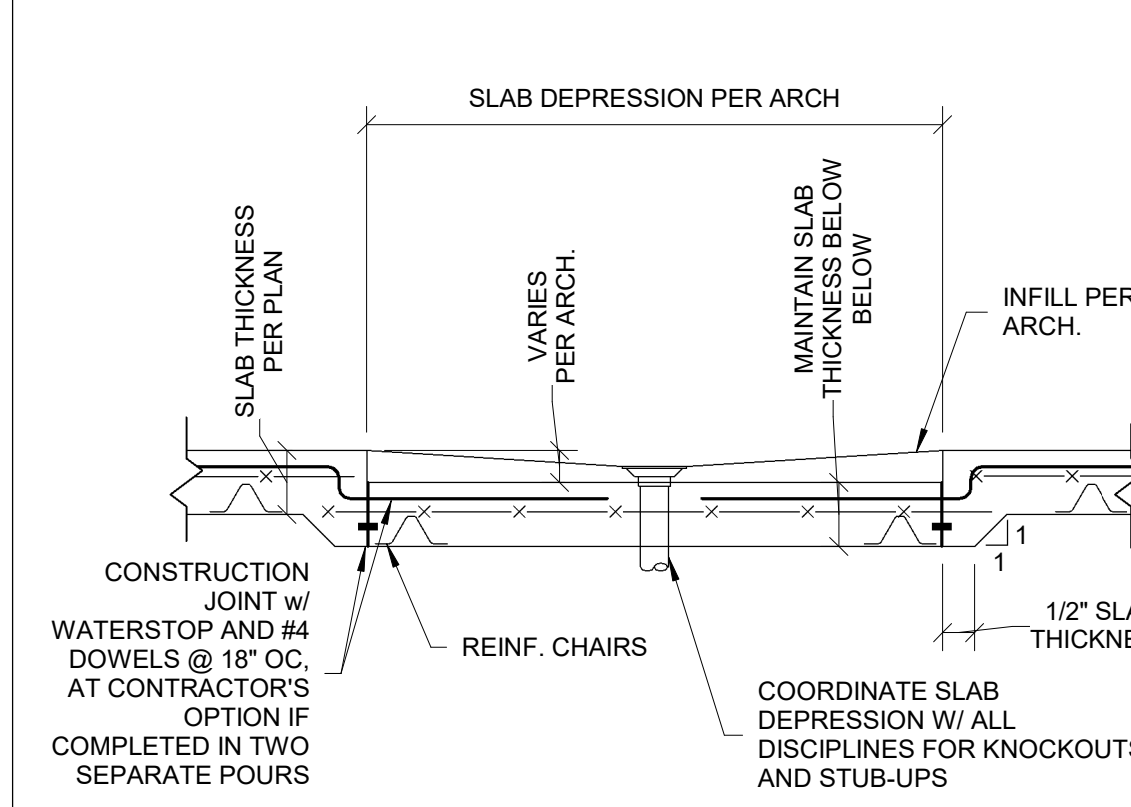
4 TYPICAL CONDUIT SLEEVE THRU CONC WALL
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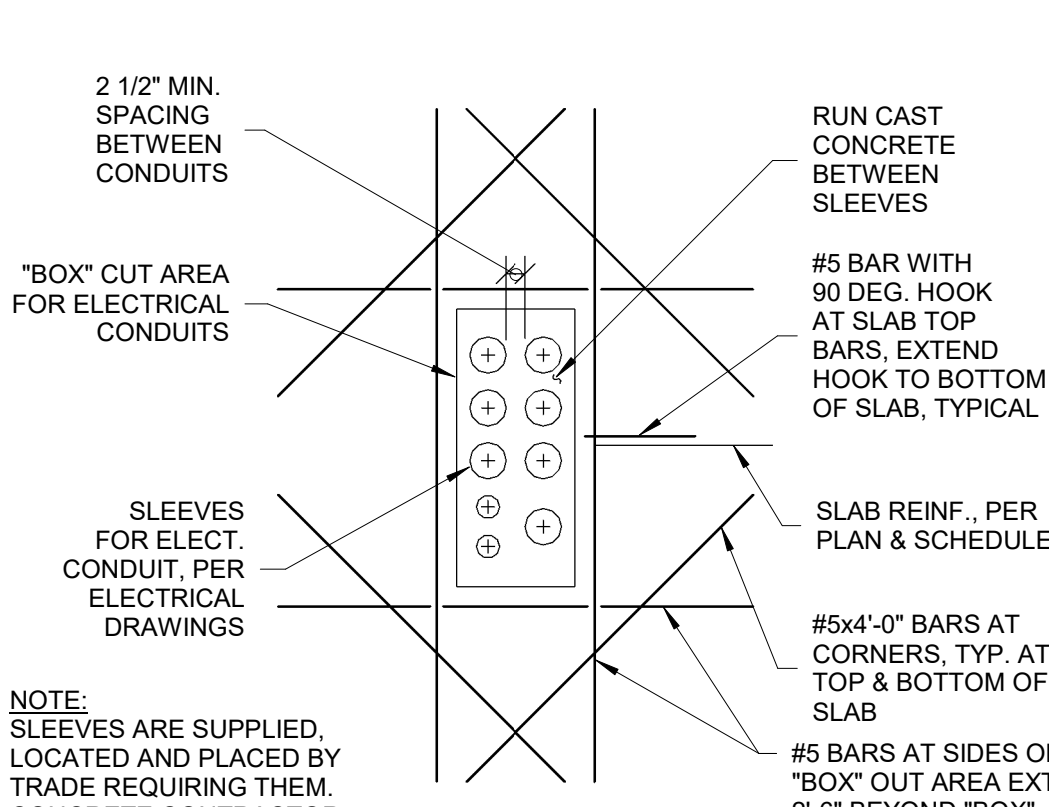
28 TYPICAL PREFAB CATCH BASIN
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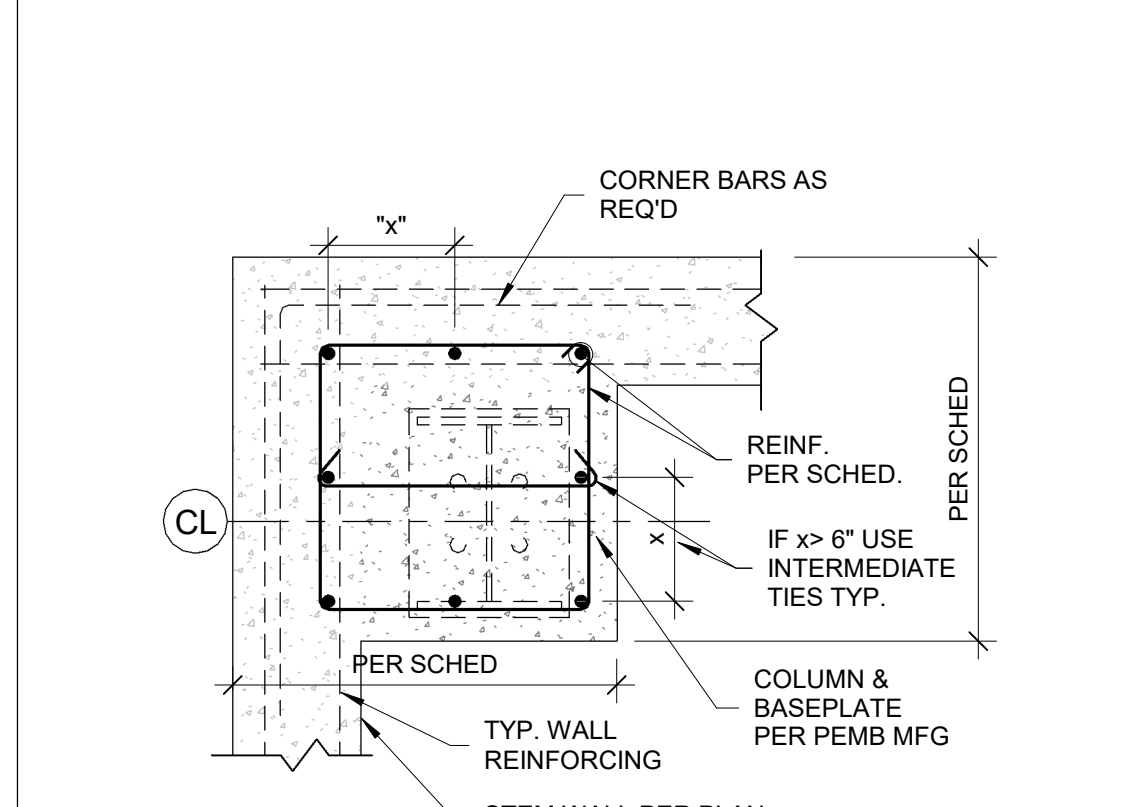
23 SECTION @ INFLOOR SCALE
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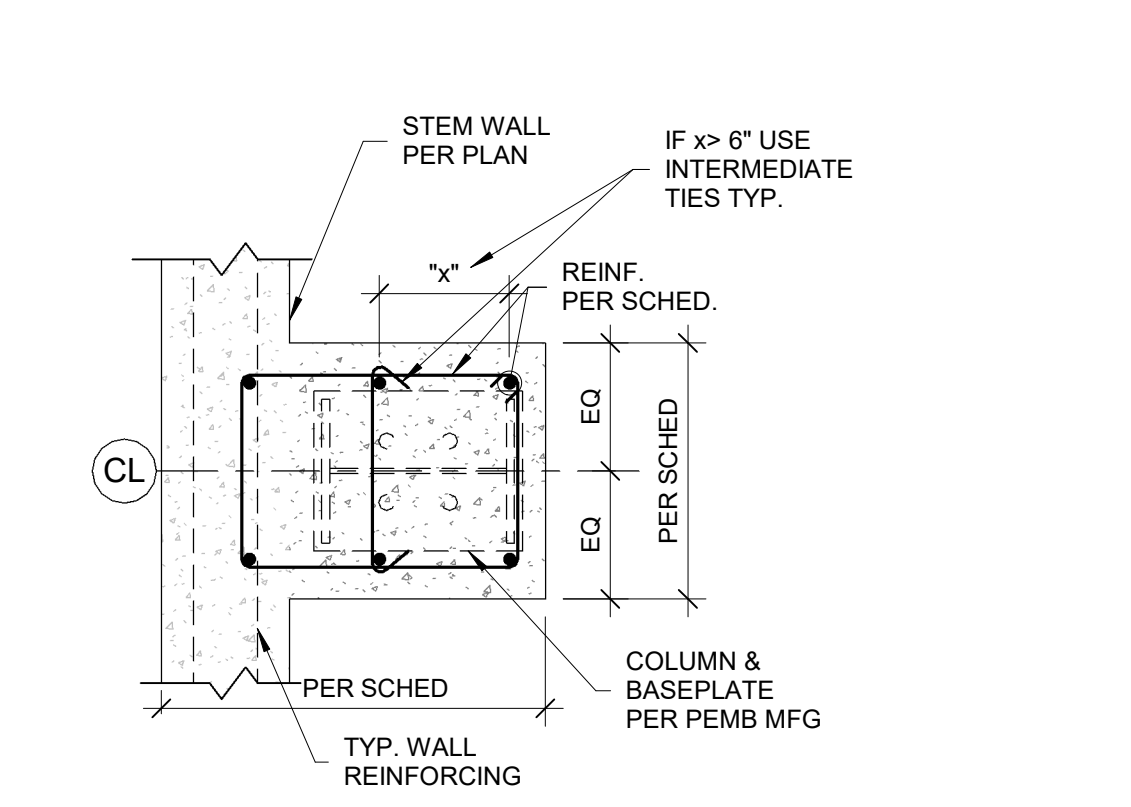
18 TYPICAL SLAB DEPRESSION DETAIL
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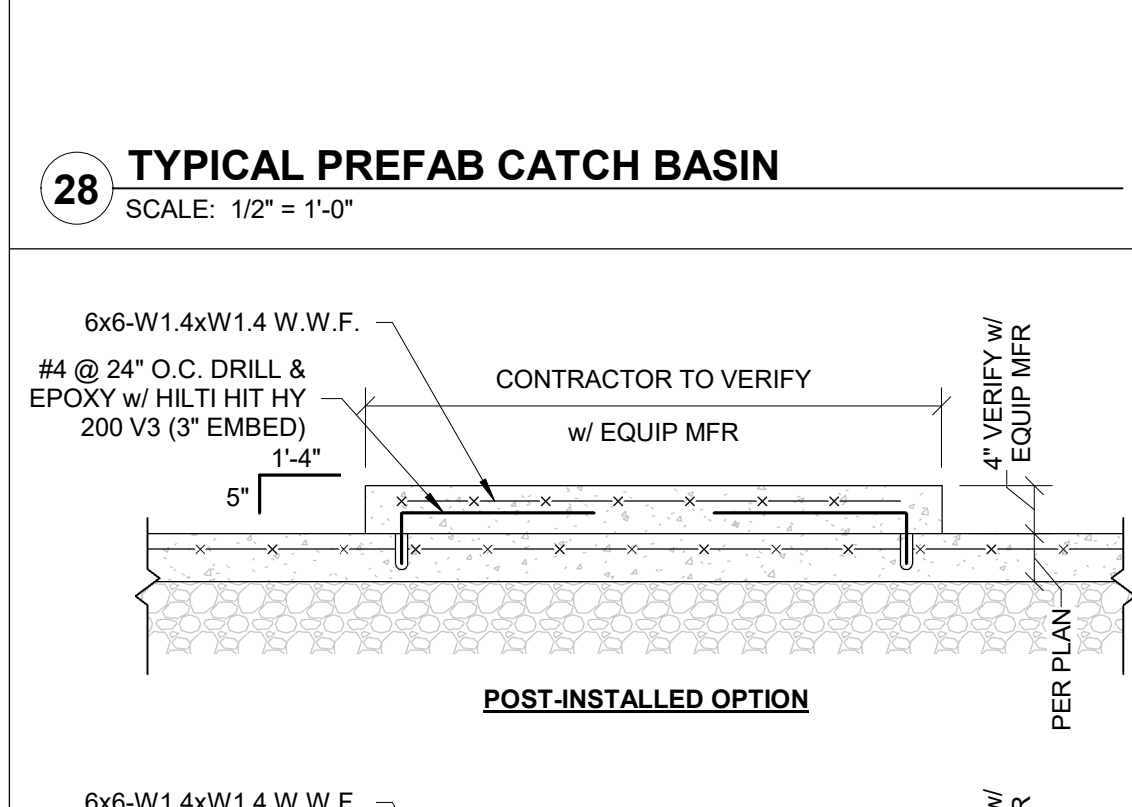
13 TYPICAL SLAB REINFORCEMENT @ SLEEVED OPENINGS
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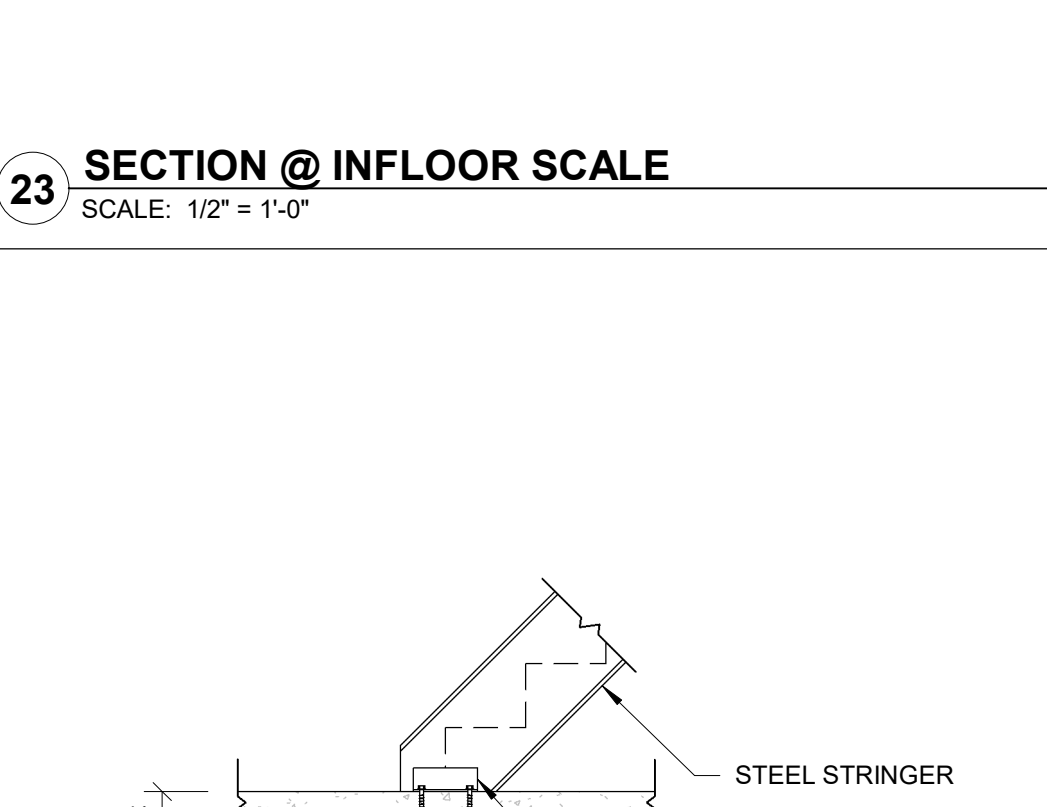
8 TYPICAL PEMB PIER AT STEM WALL
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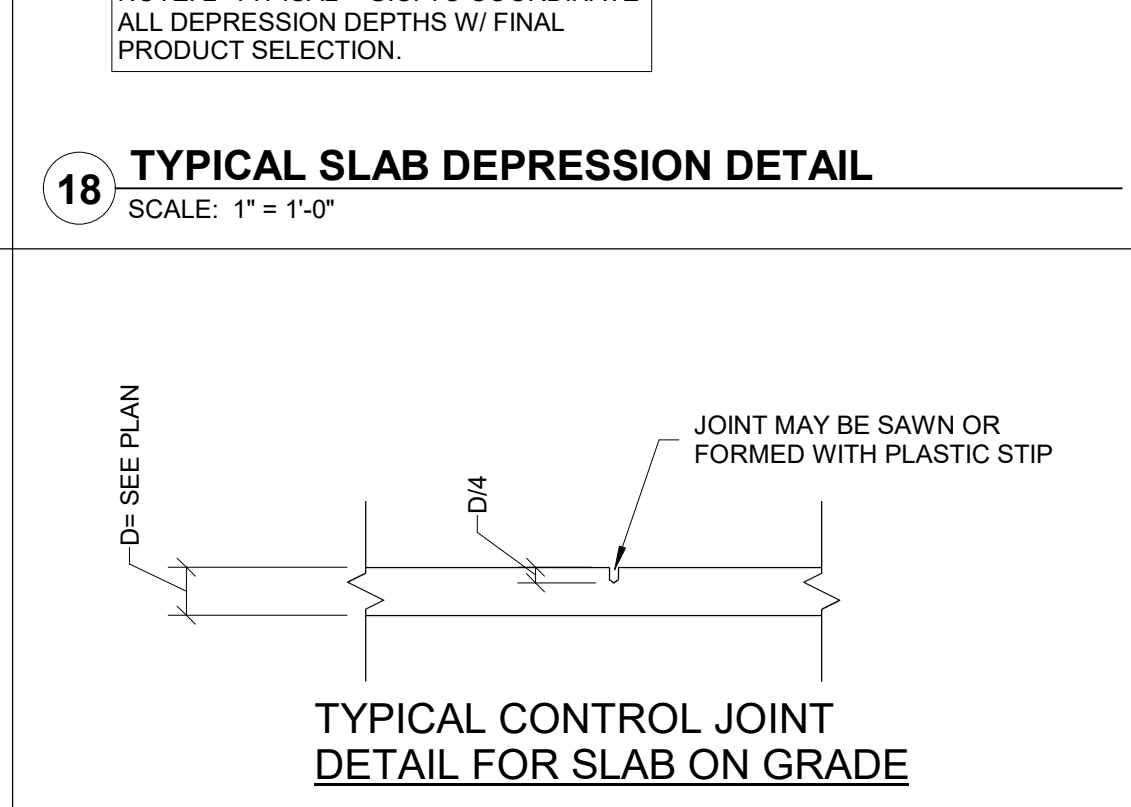
2 TYPICAL CONDITION
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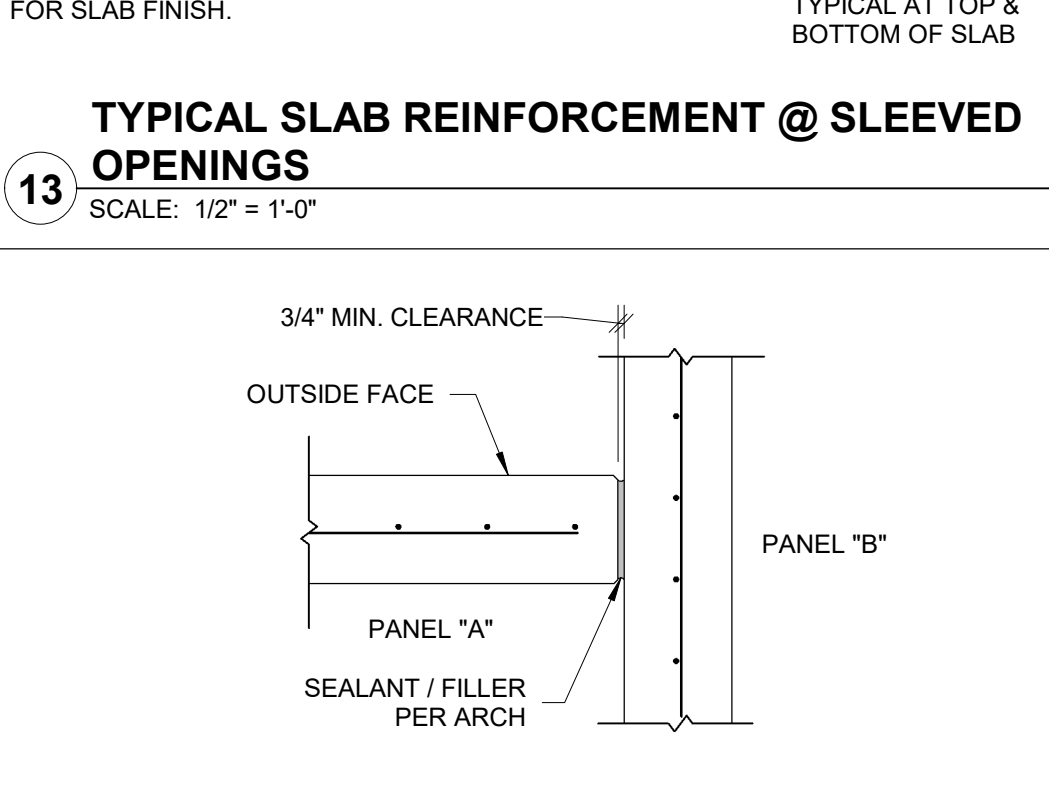
27 HOUSEKEEPING PAD FOR MECHANICAL UNIT
SCALE: 3/4" = 1'-0"



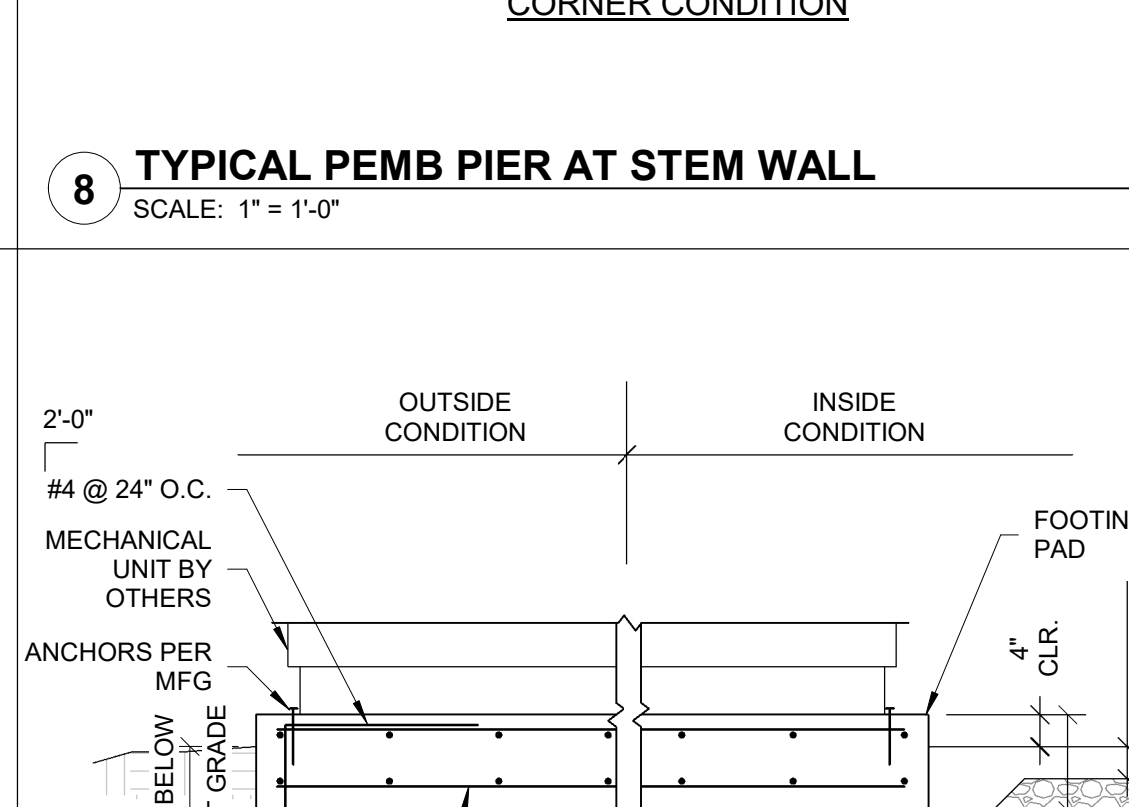
22 STEEL STRINGER ON SLAB ON GRADE
SCALE: 1/2" = 1'-0"



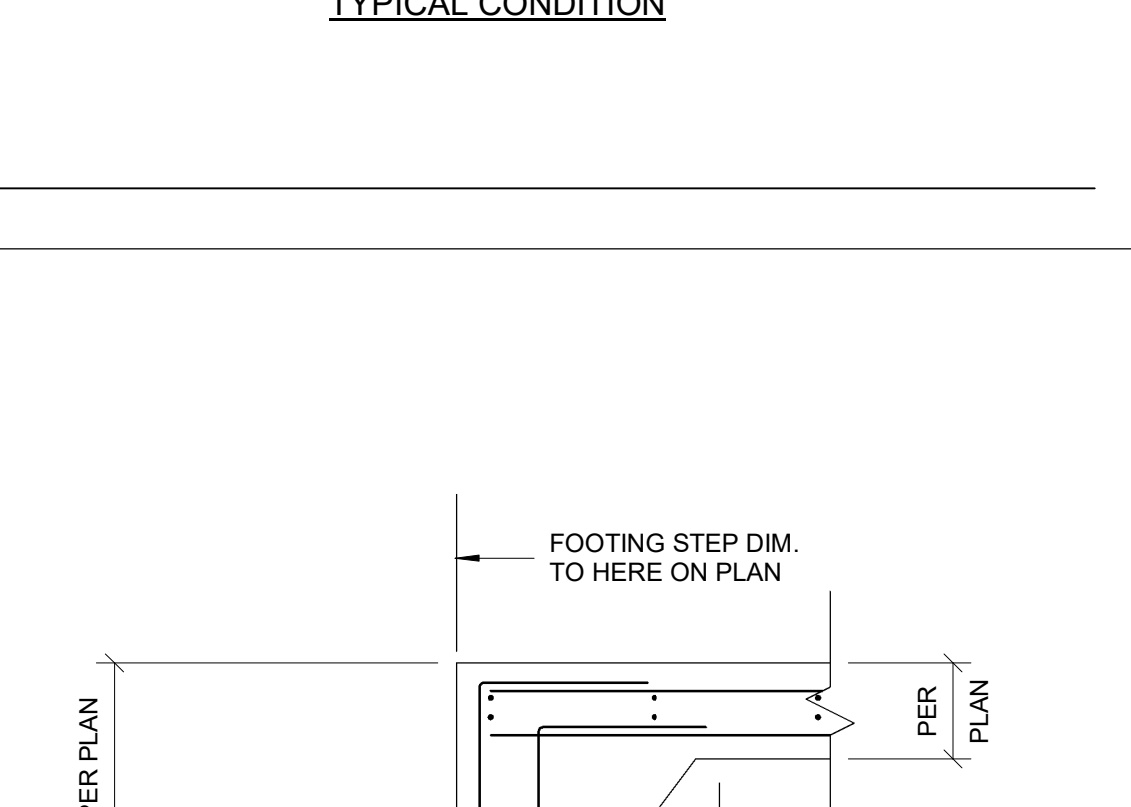
16 TYPICAL SLAB ON GRADE DETAILS
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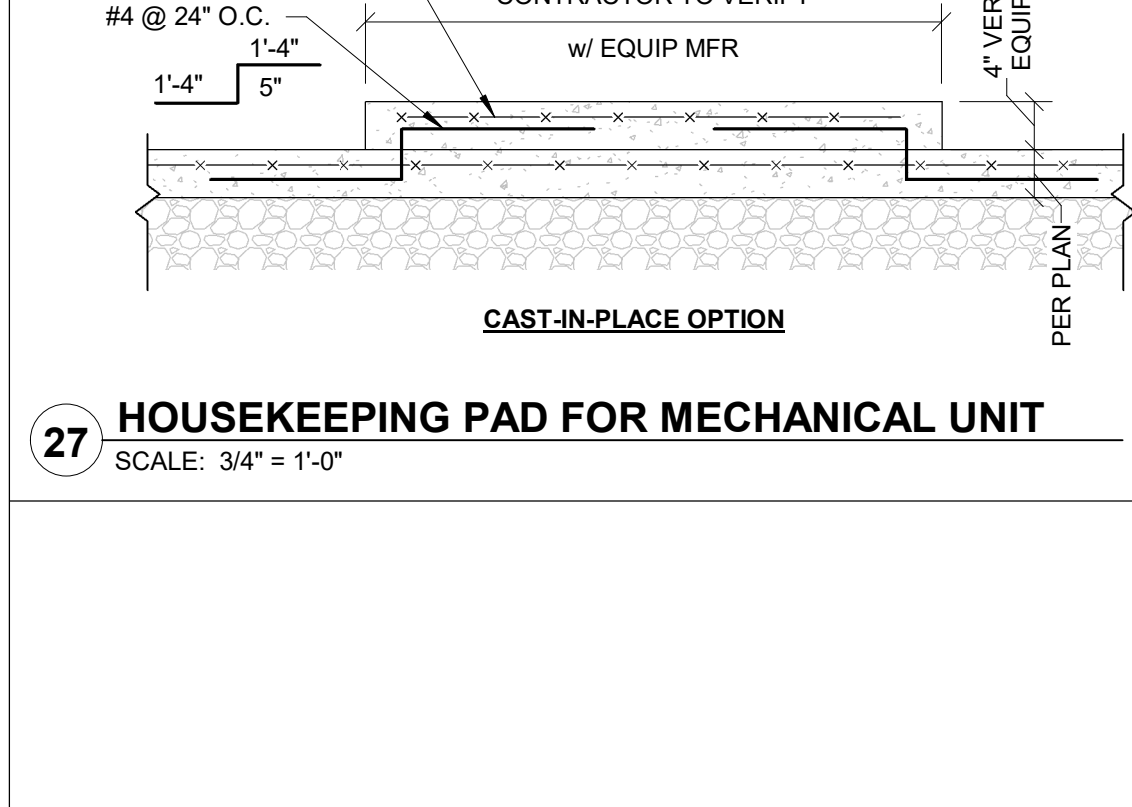
11 TYP FOUNDATION WALL CONTROL JOINT
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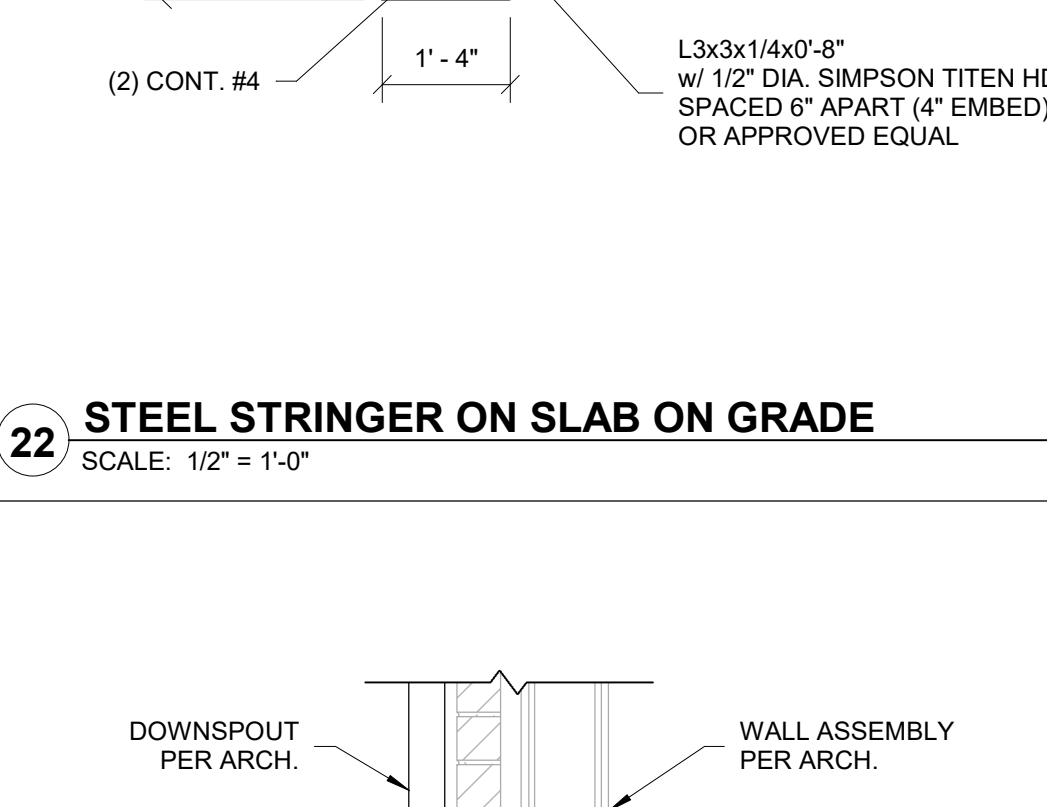
7 TYPICAL DETAIL @ OPENINGS IN CONCRETE WALL
SCALE: 1/2" = 1'-0"



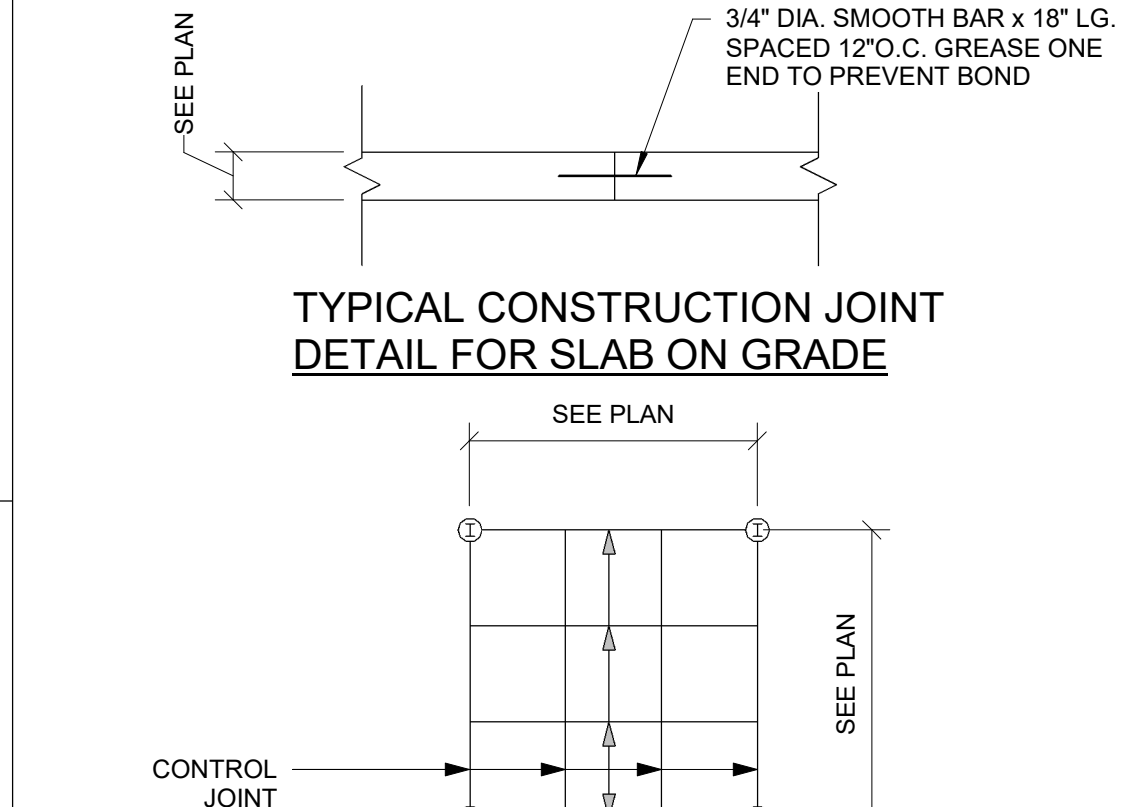
1 TYPICAL ISOLATION JOINT DETAIL @ COLUMN
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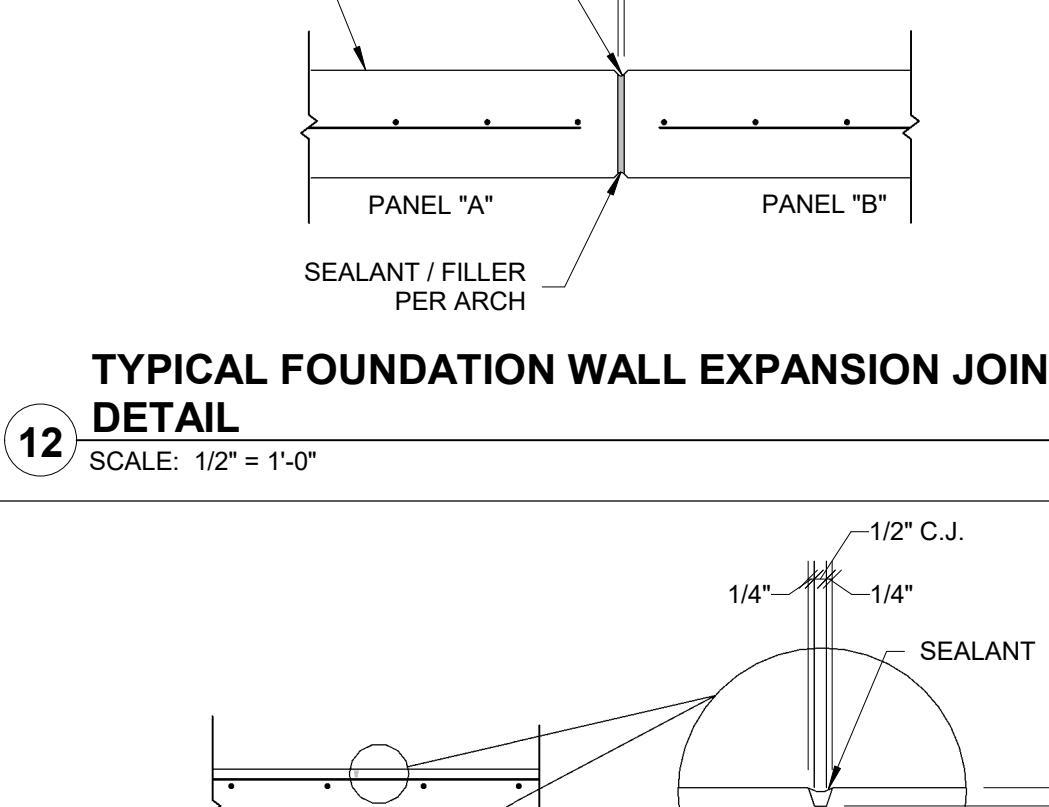
26 TYPICAL SLAB DETAIL @ INFLOOR ELECTRICAL BOXES
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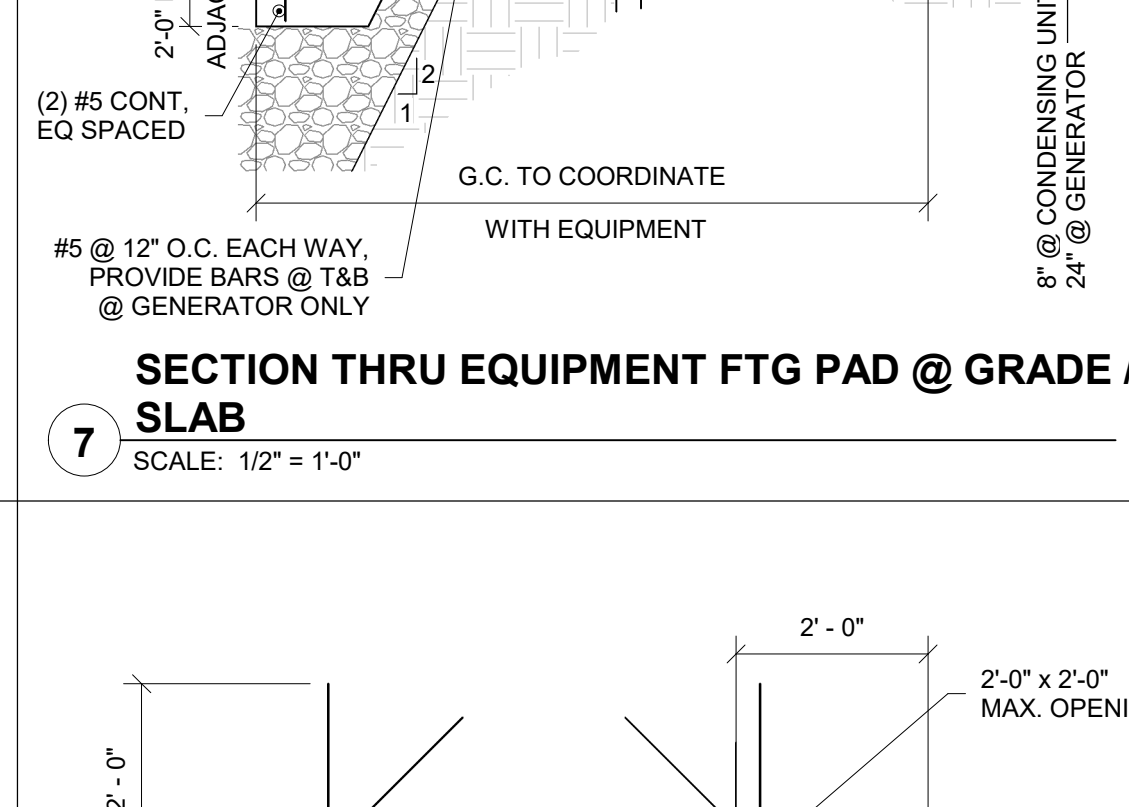
21 SECTION AT DOWNSPOUT BLOCKOUT
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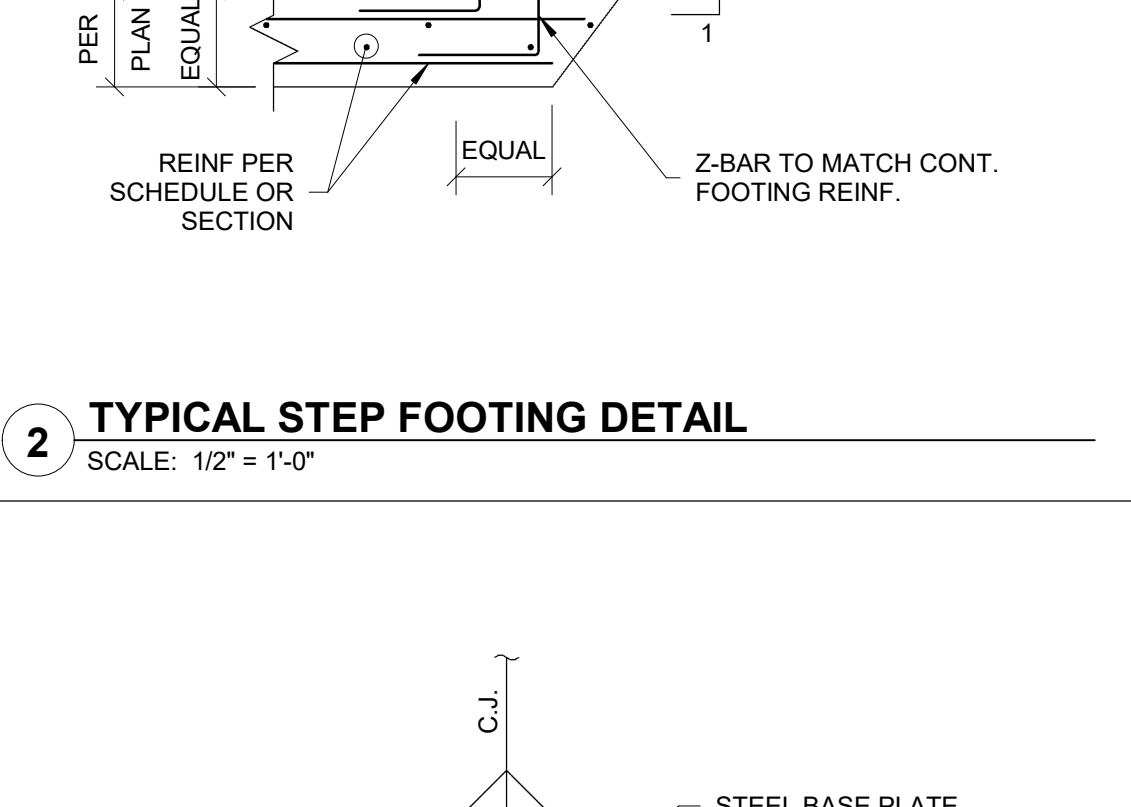
16 TYPICAL SLAB ON GRADE DETAILS
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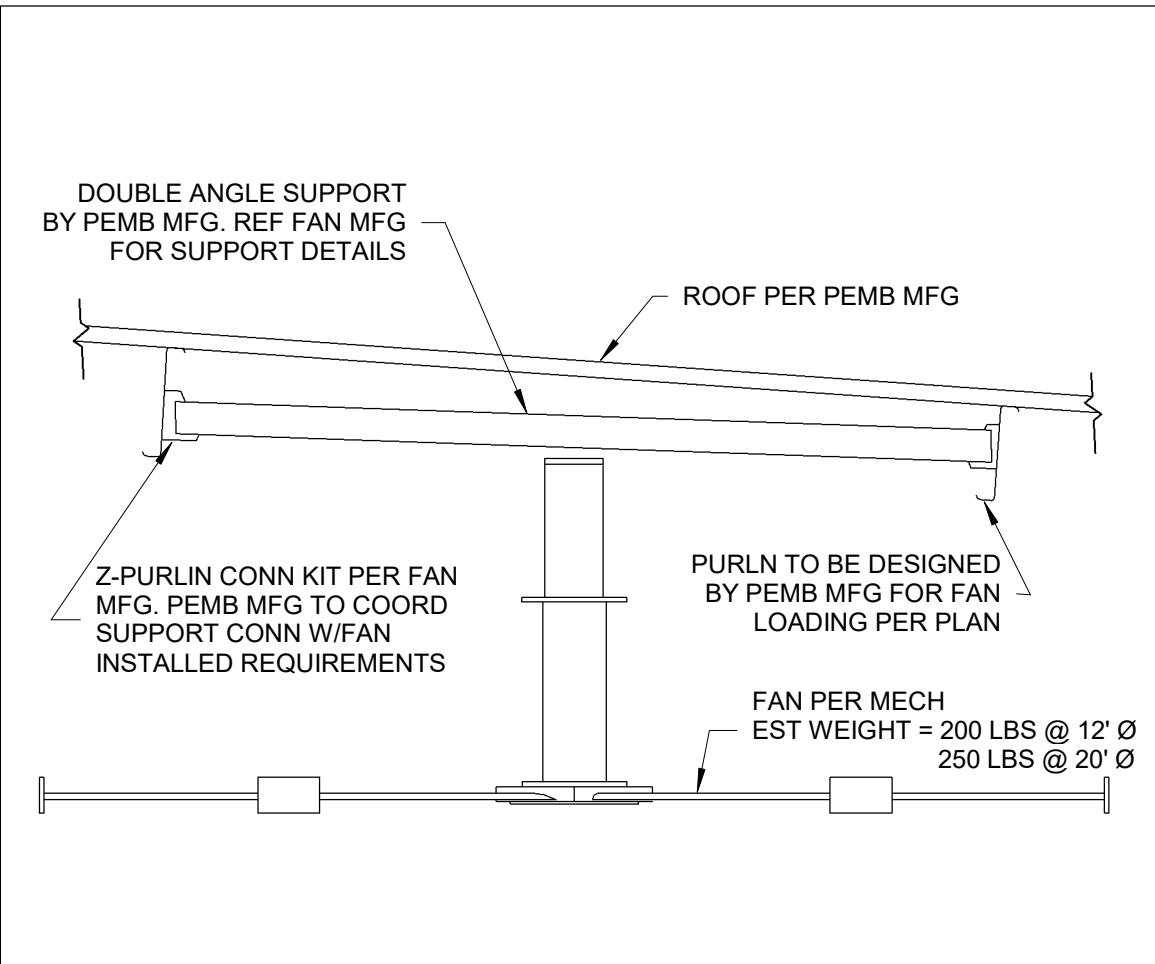
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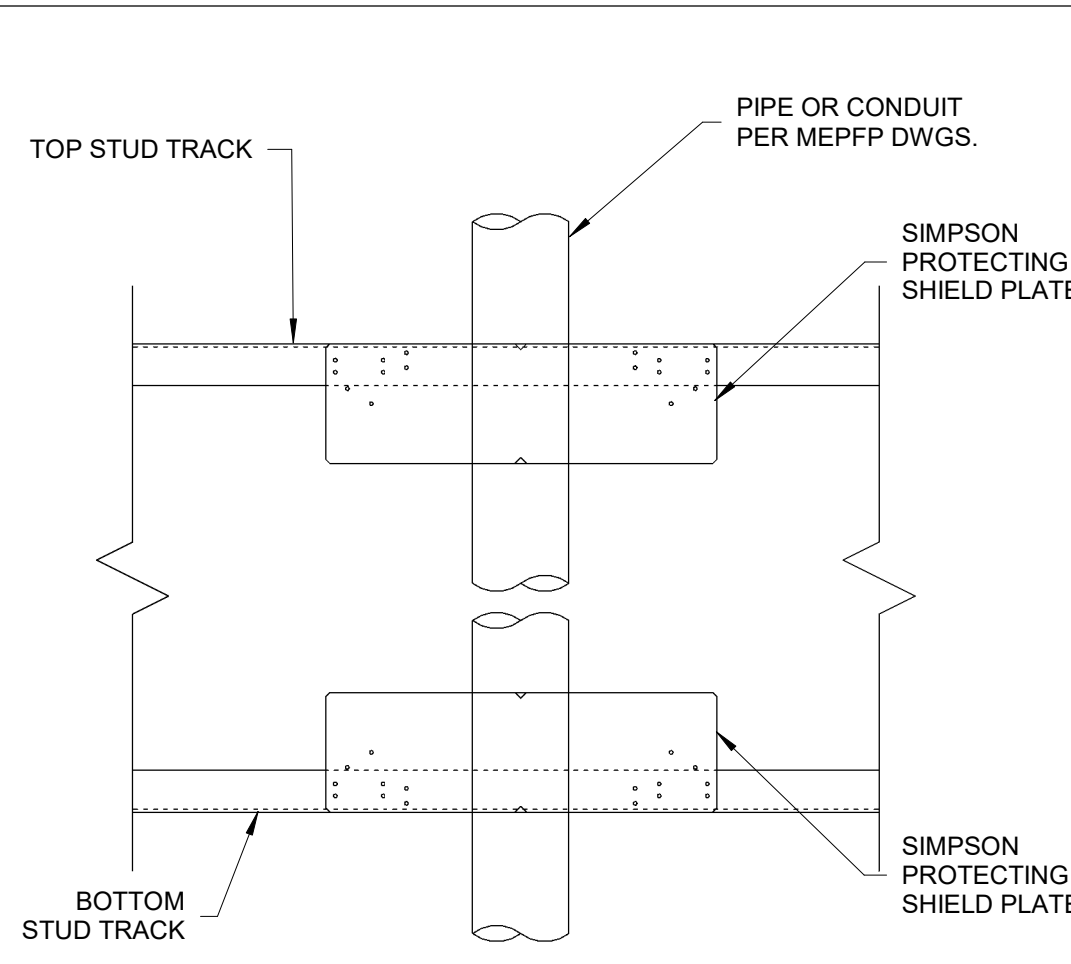
6 TYPICAL DETAIL @ OPENINGS IN CONCRETE WALL
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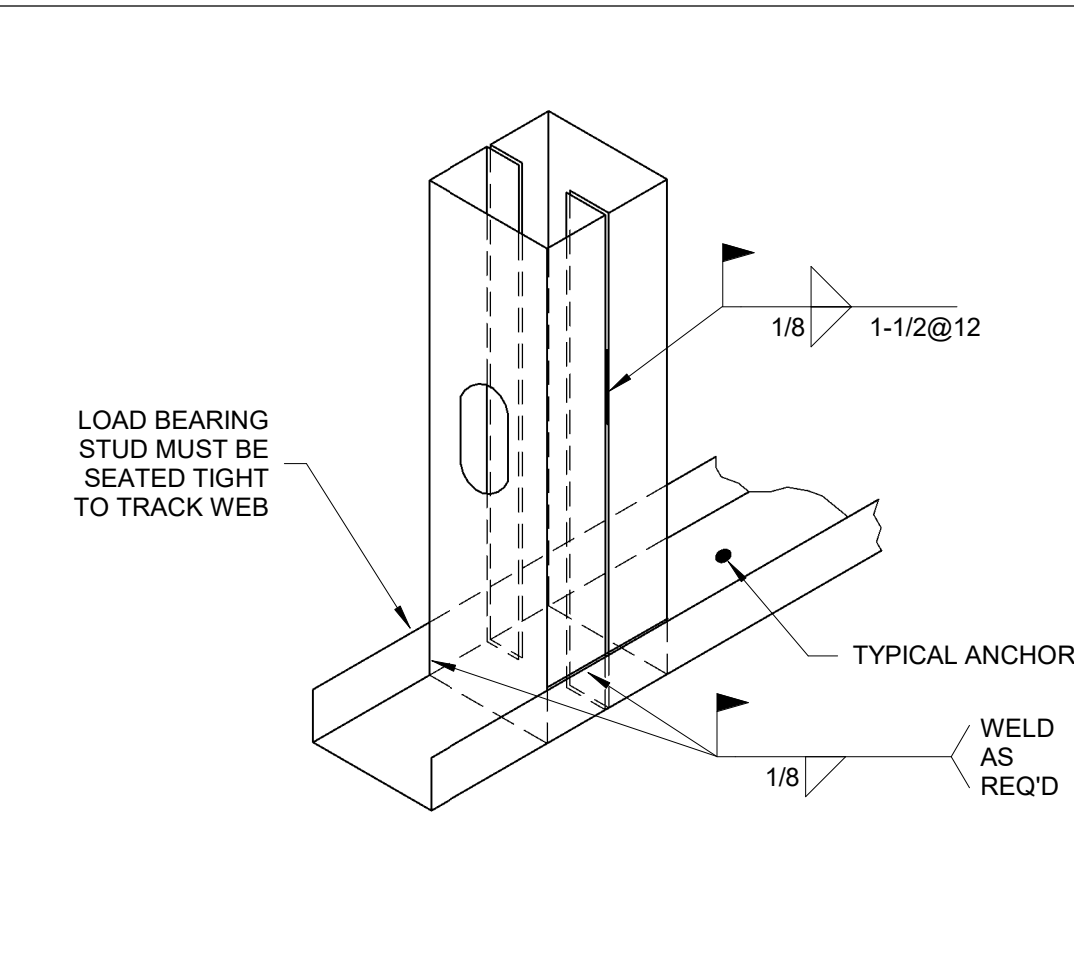
1 TYPICAL ISOLATION JOINT DETAIL @ COLUMN
SCALE: 1/2" = 1'-0"



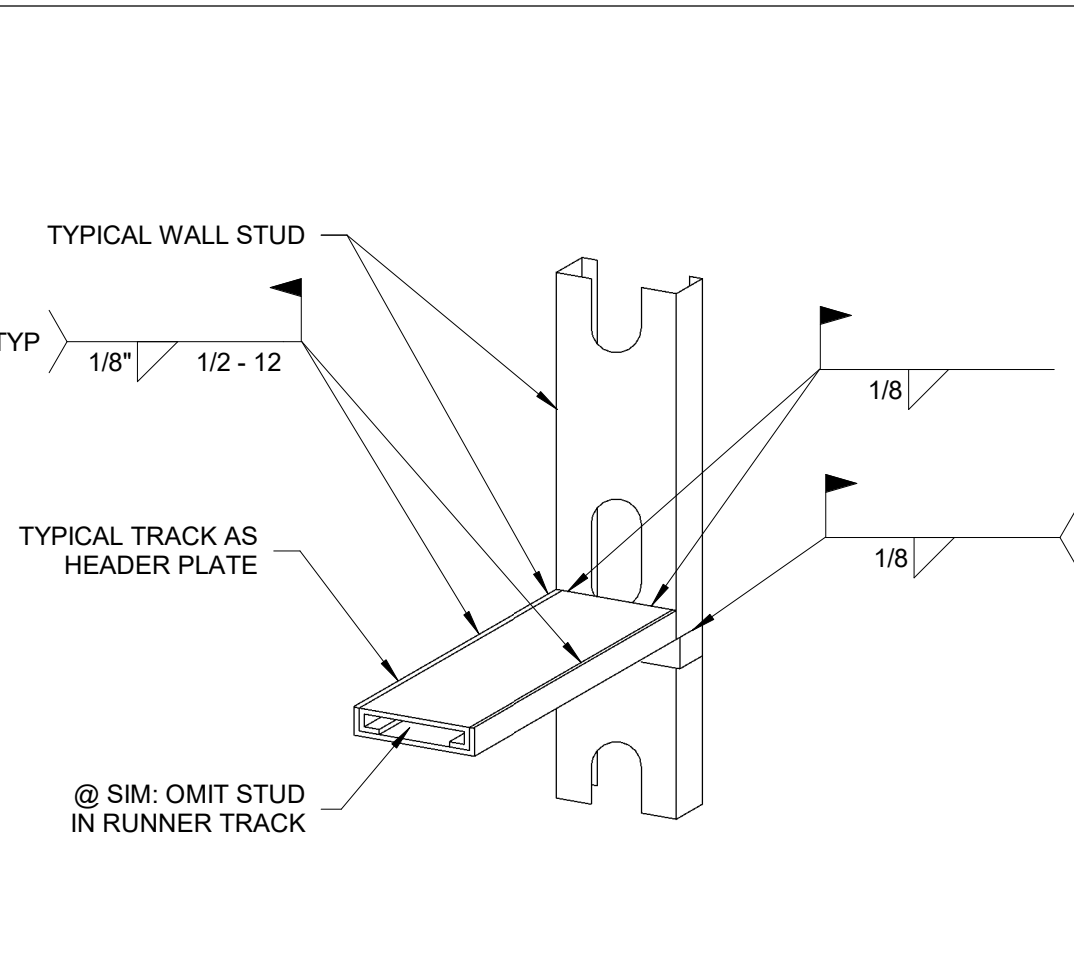
30 FRAMING SECTION @ DS FAN & ROOF
SCALE: 1/2" = 1'-0"



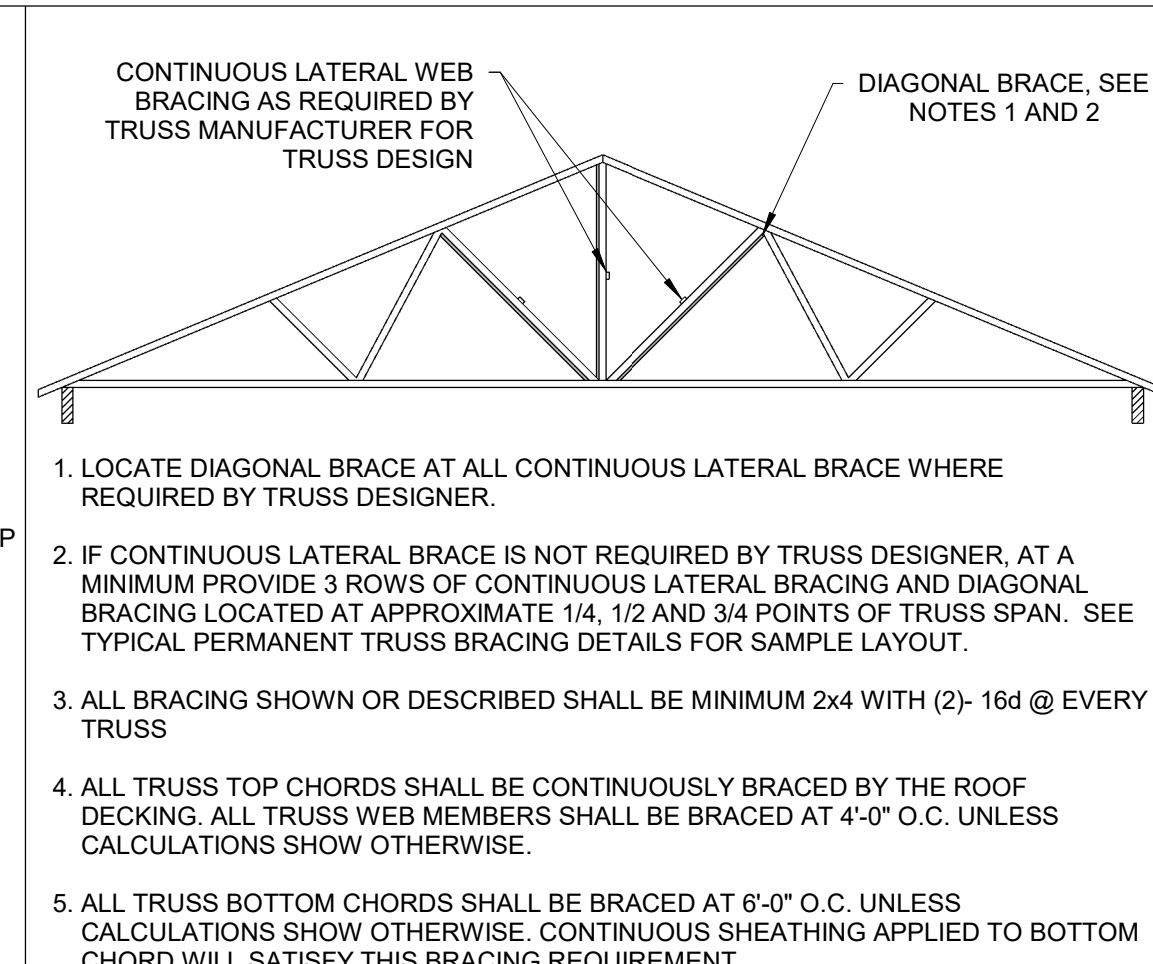
25 SIMPSON PROTECTING PLATE
NOT TO SCALE



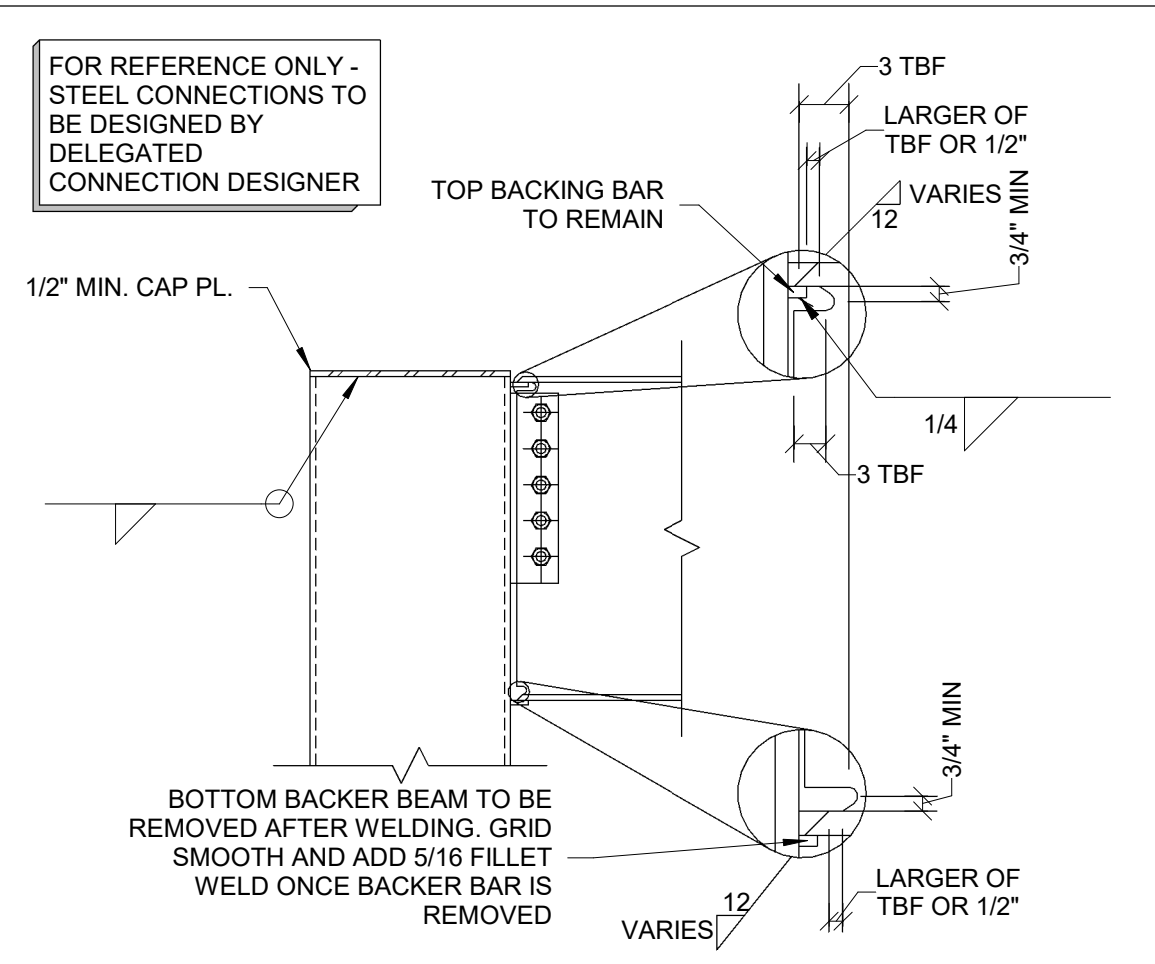
20 TYPICAL STUD @ TRACK CONNECTION @ OPENING
NOT TO SCALE



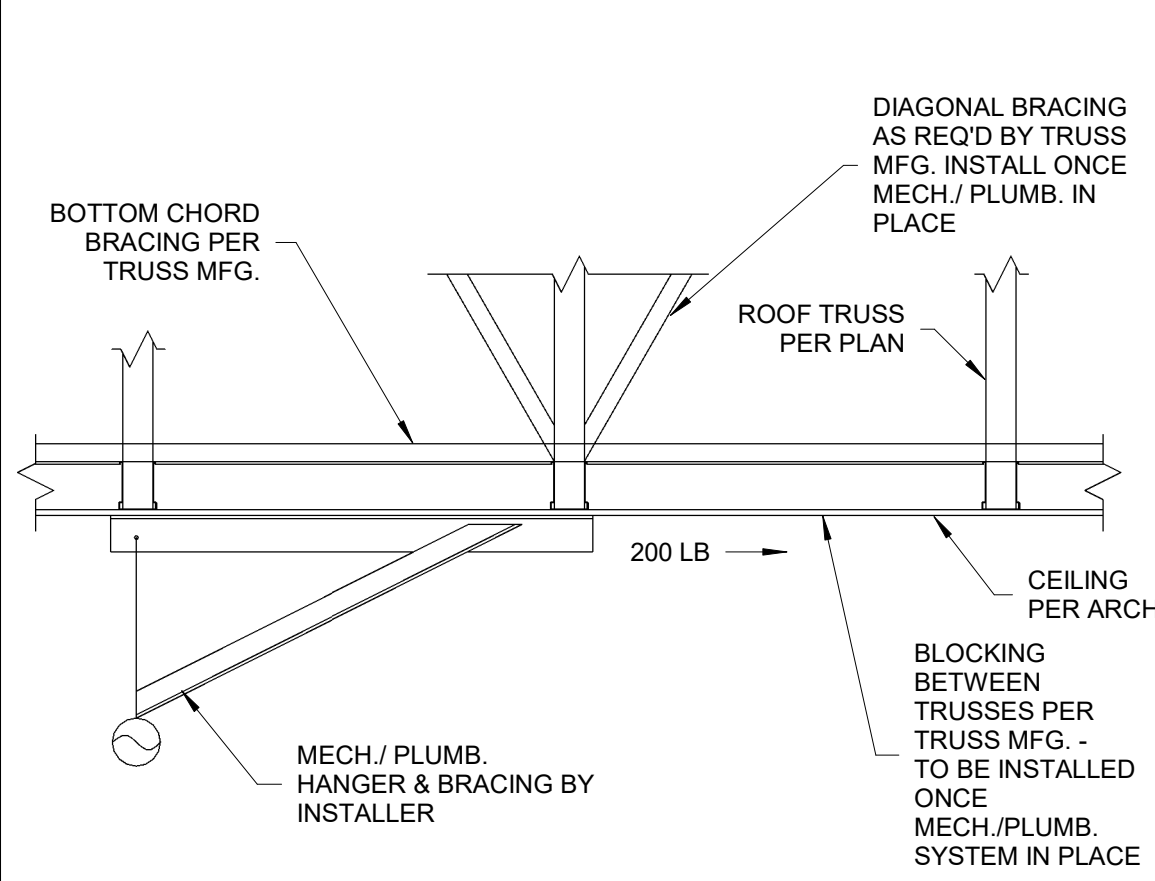
15 TYPICAL NON-LOAD BEARING HEADER
SCALE: 3/4" = 1'-0"



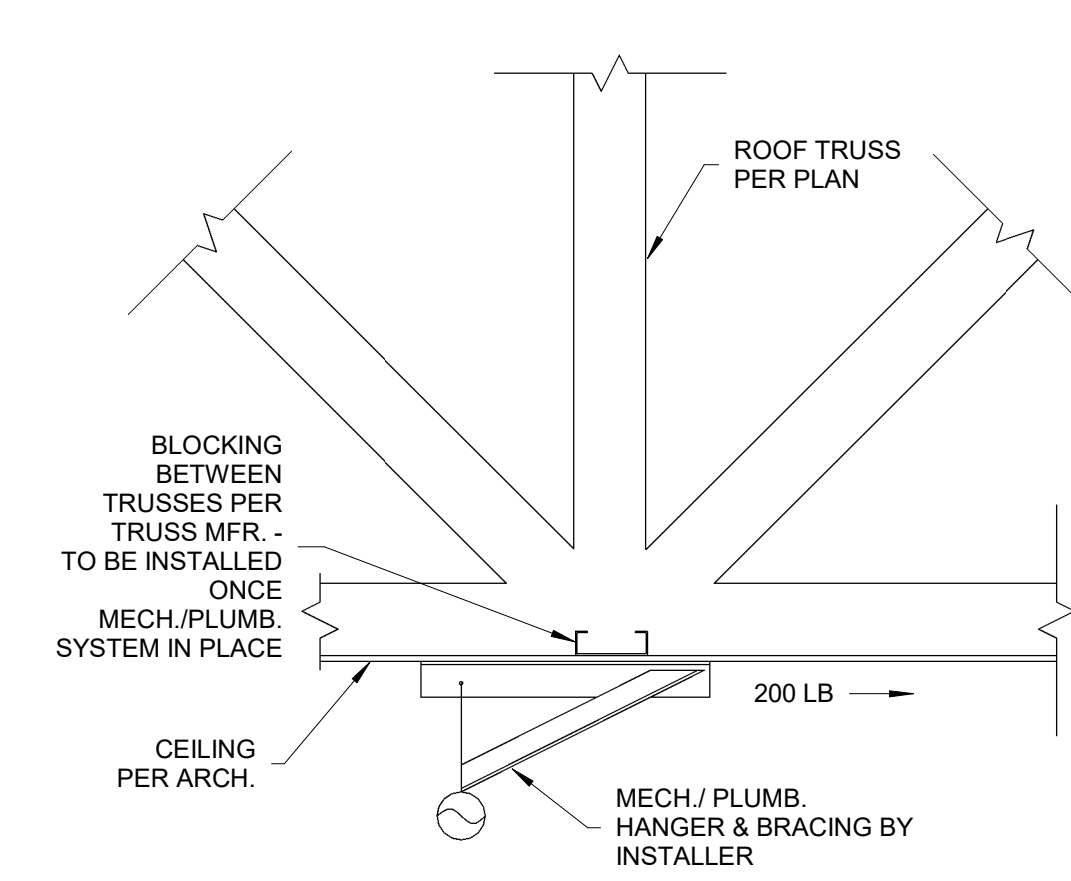
10 TYPICAL TRUSS BRACING
NOT TO SCALE



5 TYPICAL W-SHAPE TO HSS COLUMN MOMENT CONNECTION (FOR REF. ONLY)
NOT TO SCALE



29 MECH/PLUMB BRACING FORCE PERPENDICULAR TO TRUSS
SCALE: 3/4" = 1'-0"



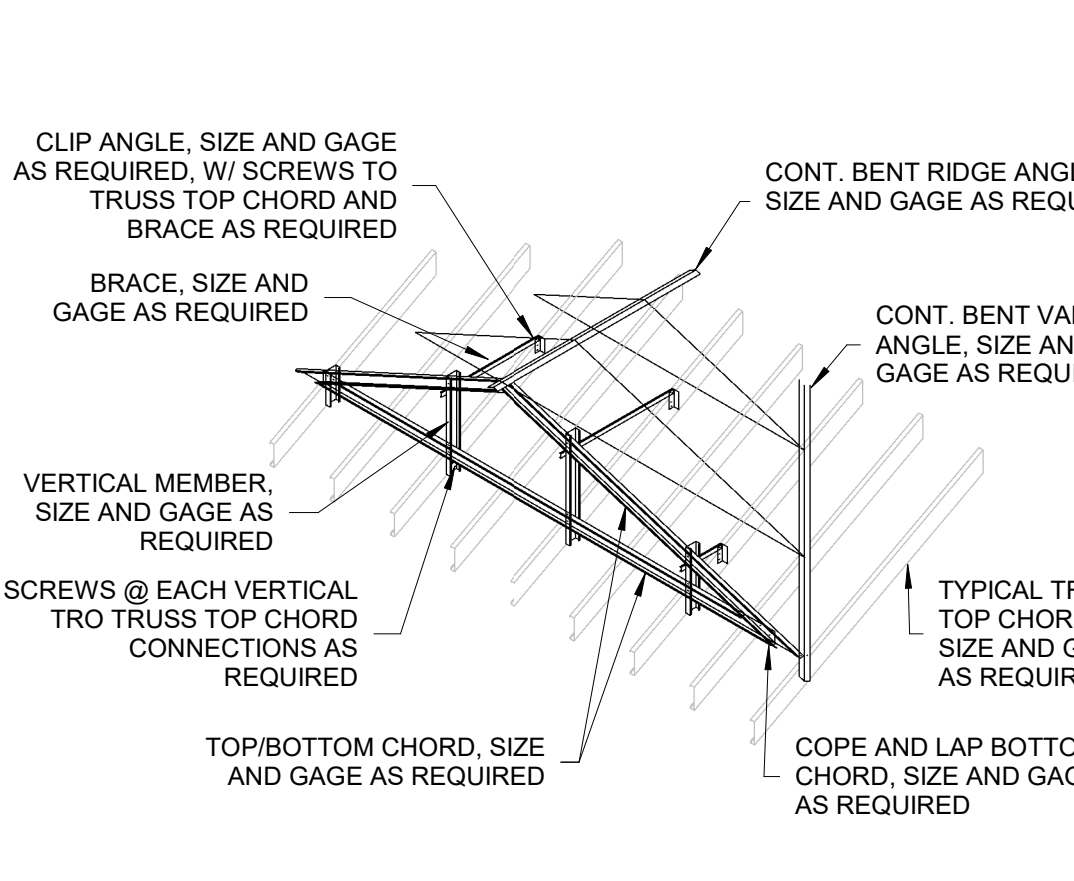
24 MECH/PLUMB BRACING FORCE PARALLEL TO TRUSS
SCALE: 3/4" = 1'-0"

BRICK VENEER LOOSE LINTEL SCHEDULE

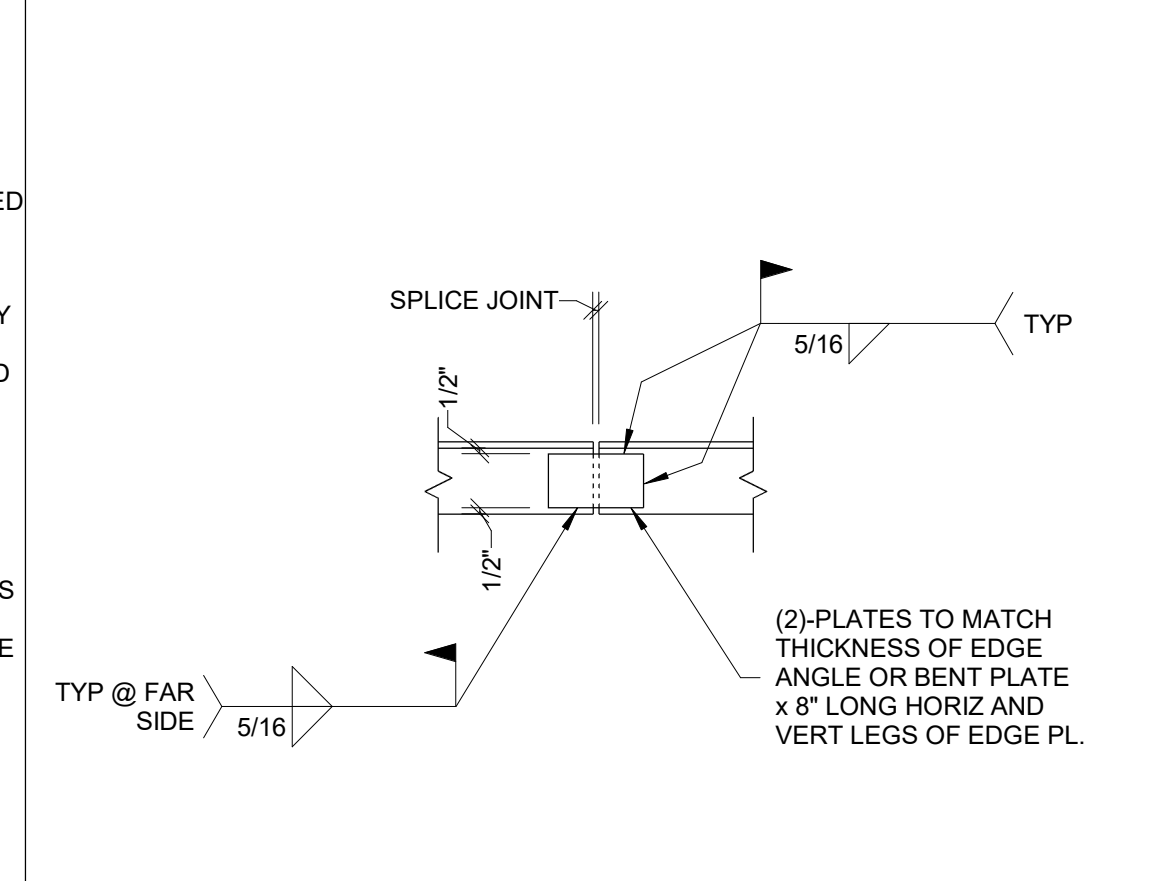
SIZE	OPENING SIZE	REMARKS
L4x4x5/16	UP TO 3'-8"	-
L6x4x5/16 (LLV)	3'-8" TO 6'-0"	-
L6x4x5/16 (LLV)	6'-0" TO 8'-0"	-

NOTE
1. PROVIDE 8" BEARING AT EACH END.
2. PROVIDE GALVANIZED FINISH.
3. SEE ARCH. DRAWINGS FOR LINTEL LOCATIONS.

19 BRICK VENEER LOOSE LINTEL SCHEDULE
SCALE: 1/8" = 1'-0"



14 TYPICAL LIGHT GAGE STEEL TRUSS OVERBUILDING FRAMING
NOT TO SCALE



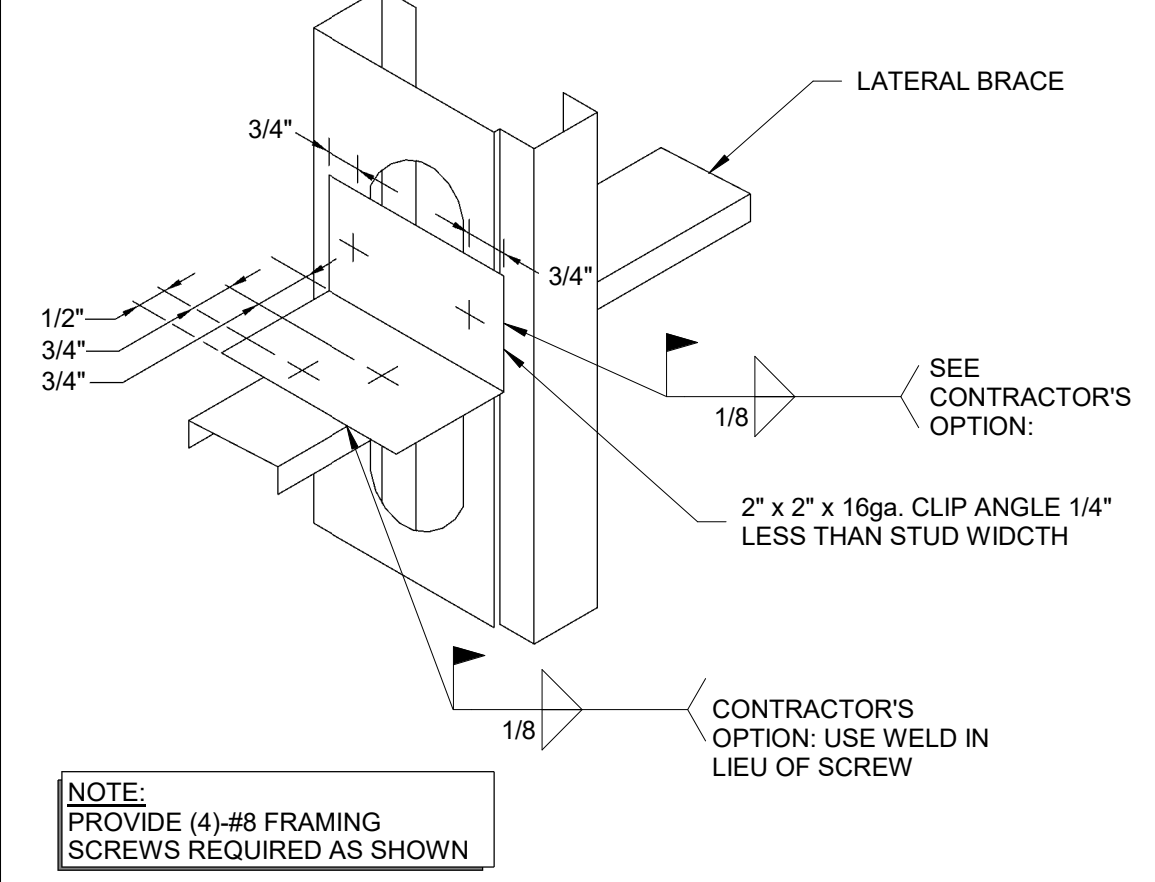
9 TYPICAL EDGE ANGLE SPLICE DETAIL
SCALE: 3/4" = 1'-0"

FOR REFERENCE ONLY - STEEL CONNECTIONS TO BE DESIGNED BY DELEGATED CONNECTION DESIGNER

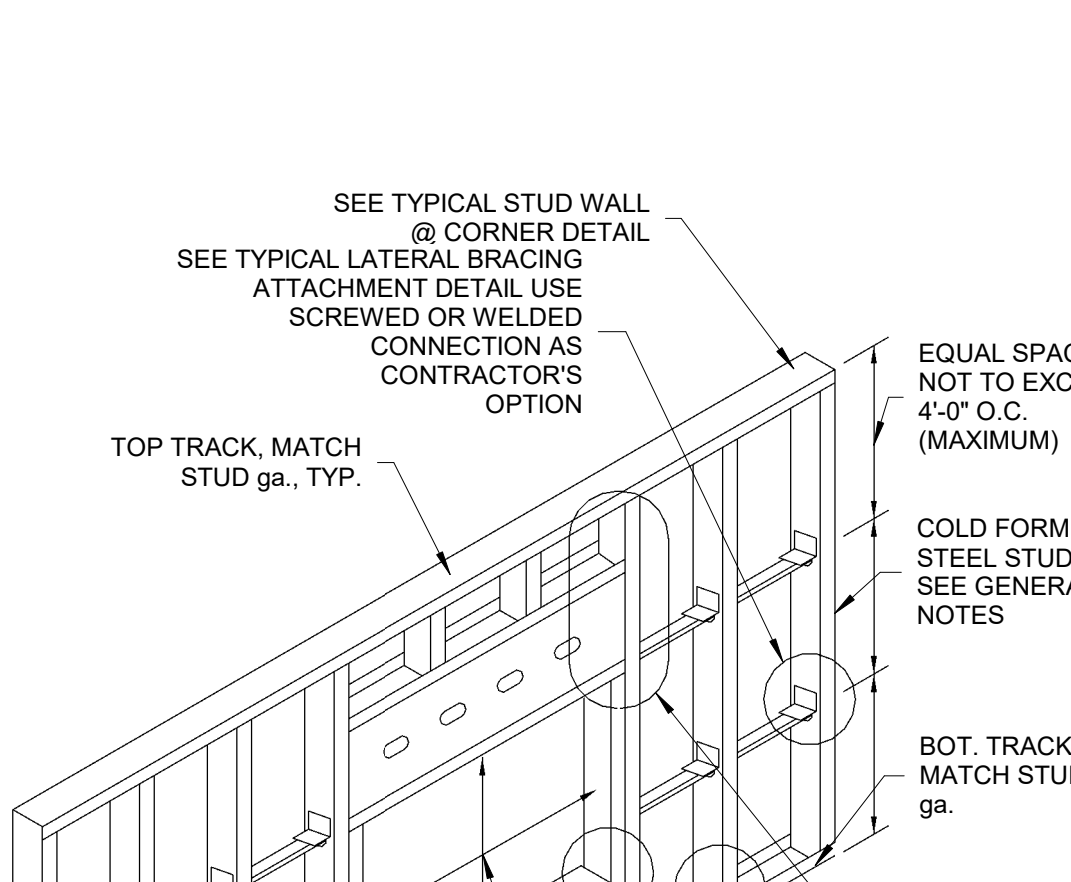
BEAM SIZE	5/16" STEEL PLATE LENGTH, L (IN.)	NUMBER OF 3/4" DIA. BOLTS
W10, 8	5 1/2"	2
W14, 12	8 1/2"	3
W18, 16	11 1/2"	4
W21, 18	14 1/2"	5
W27, 24	17 1/2"	6
W33, 30	20 1/2"	7
W36	23 1/2"	8

NOTES:
1. ADAPTED FROM TABLE 10-9a, AISC STEEL CONSTRUCTION MANUAL, FOURTEENTH EDITION.
2. ONLY A SINGLE VERTICAL ROW OF BOLTS IS PERMITTED.
3. THE DISTANCE FROM THE BOLT LINE TO THE WELD LINE, MUST BE EQUAL TO OR LESS THAN 3 1/2".
4. STD OR SSL HOLDS ARE PERMITTED TO BE USED.
5. THE HORIZONTAL EDGE DISTANCE MUST BE EQUAL TO OR GREATER THAN 1 1/2" FOR BOTH THE PLATE AND THE BEAM WEB.
6. THE VERTICAL EDGE DISTANCES MUST SATISFY AISC SPECIFICATION TABLE J3.4 REQUIREMENTS FOR BOTH THE PLATE AND THE BEAM WEB.
7. EDGE DISTANCES ARE MEASURED TO THE CENTER OF THE BOLT OR SLOT.
8. BOLT SPACING SHALL BE 3".
9. BOLTS SHALL BE ASTM A325-N OR X.

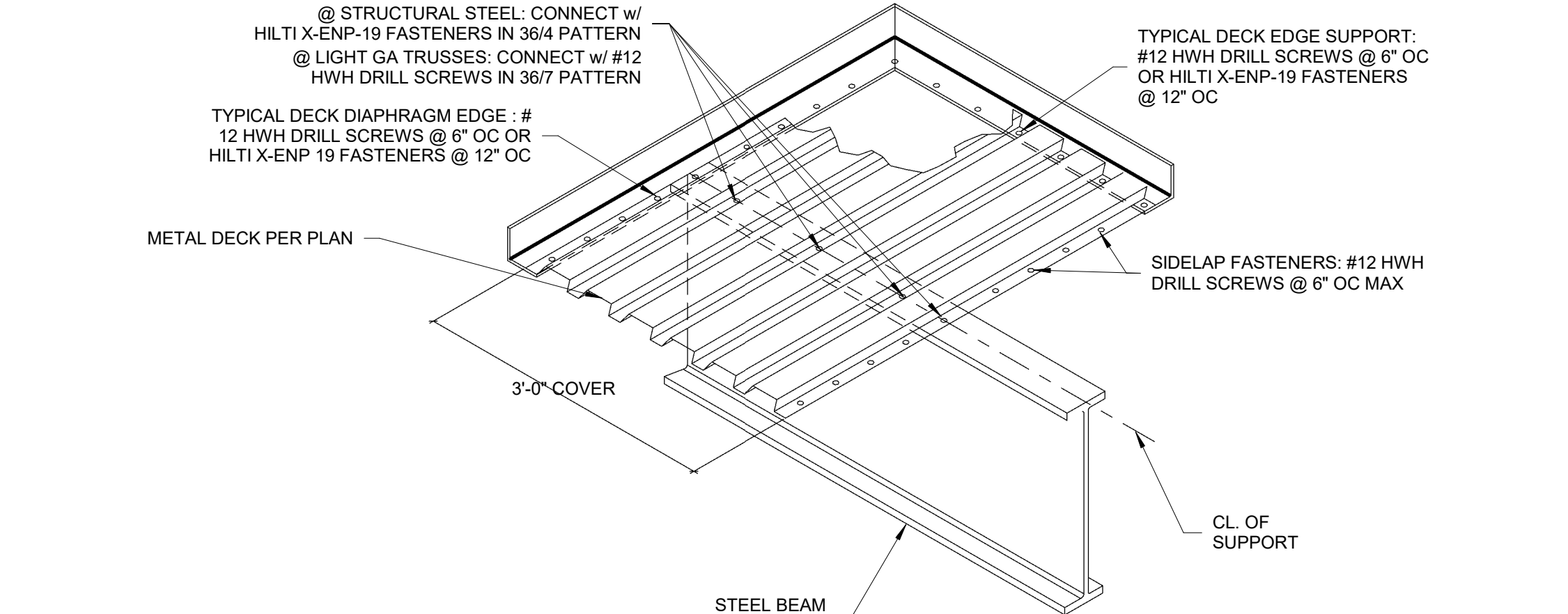
3 TYPICAL SINGLE-PLATE SHEAR CONNECTIONS (FOR REFERENCE ONLY)
SCALE: 3/4" = 1'-0"



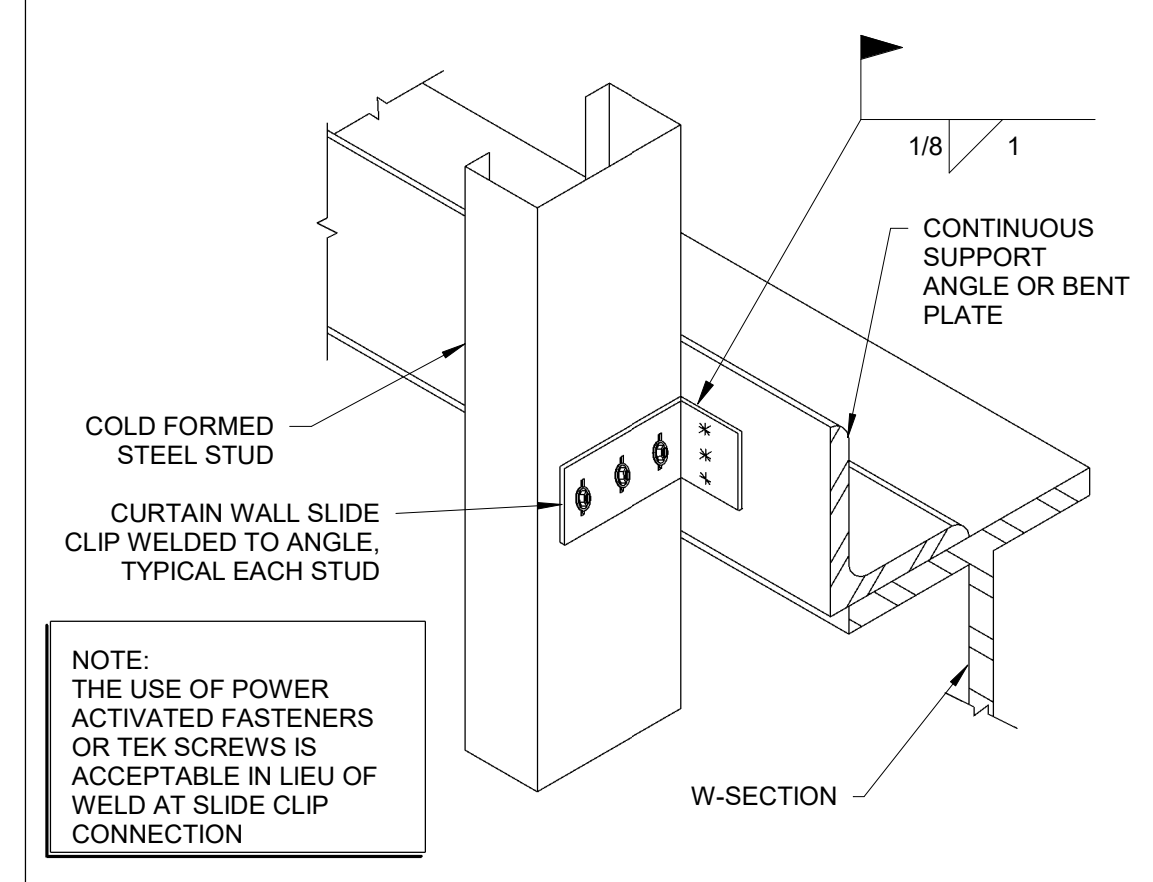
28 TYPICAL LATERAL BRACING ATTACHMENT
NOT TO SCALE



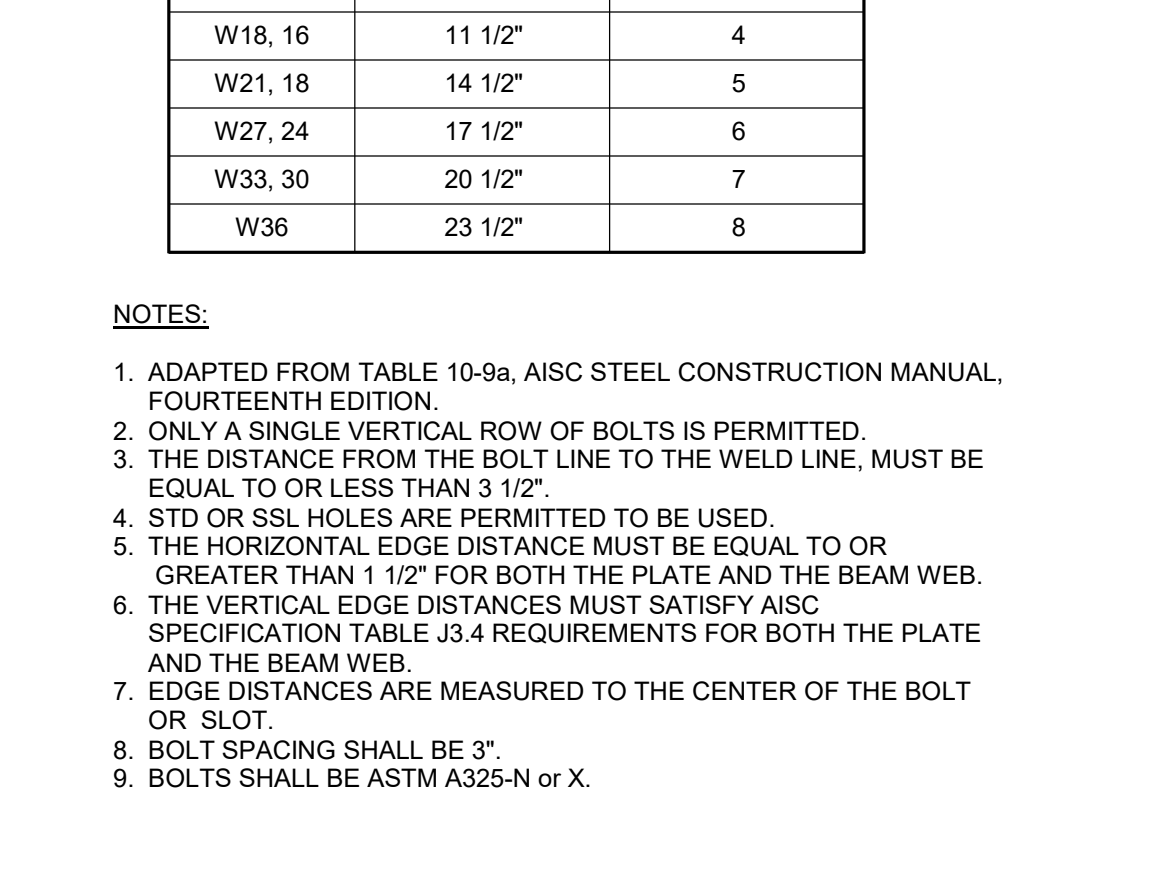
22 TYPICAL STUD WALL PANEL
NOT TO SCALE



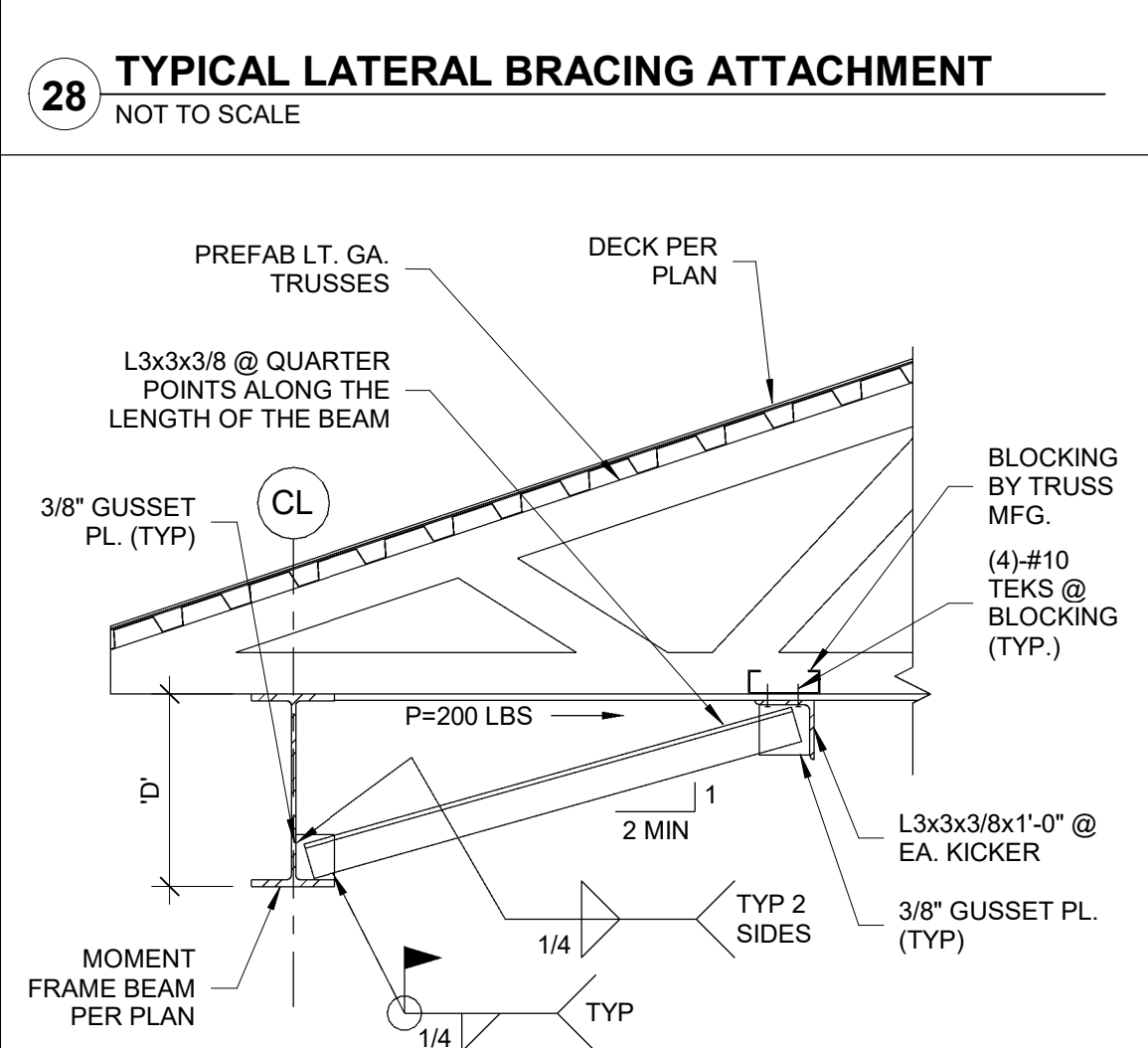
18 TYPICAL ROOF DECK MECHANICAL FASTENER ATTACHMENT - OFFICE
SCALE: 3/4" = 1'-0"



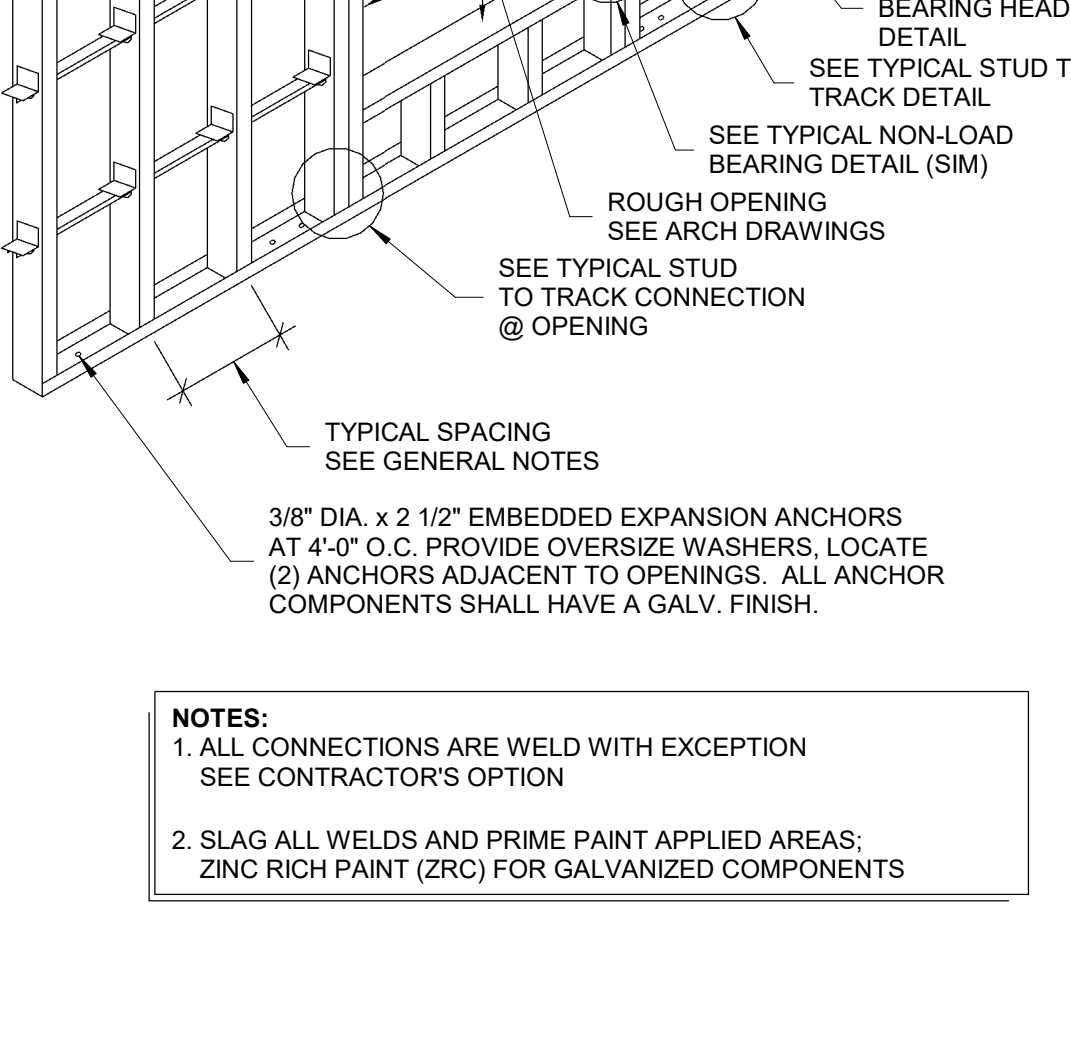
8 TYPICAL CURTAIN WALL SLIDE CLIP CONNECTION
NOT TO SCALE



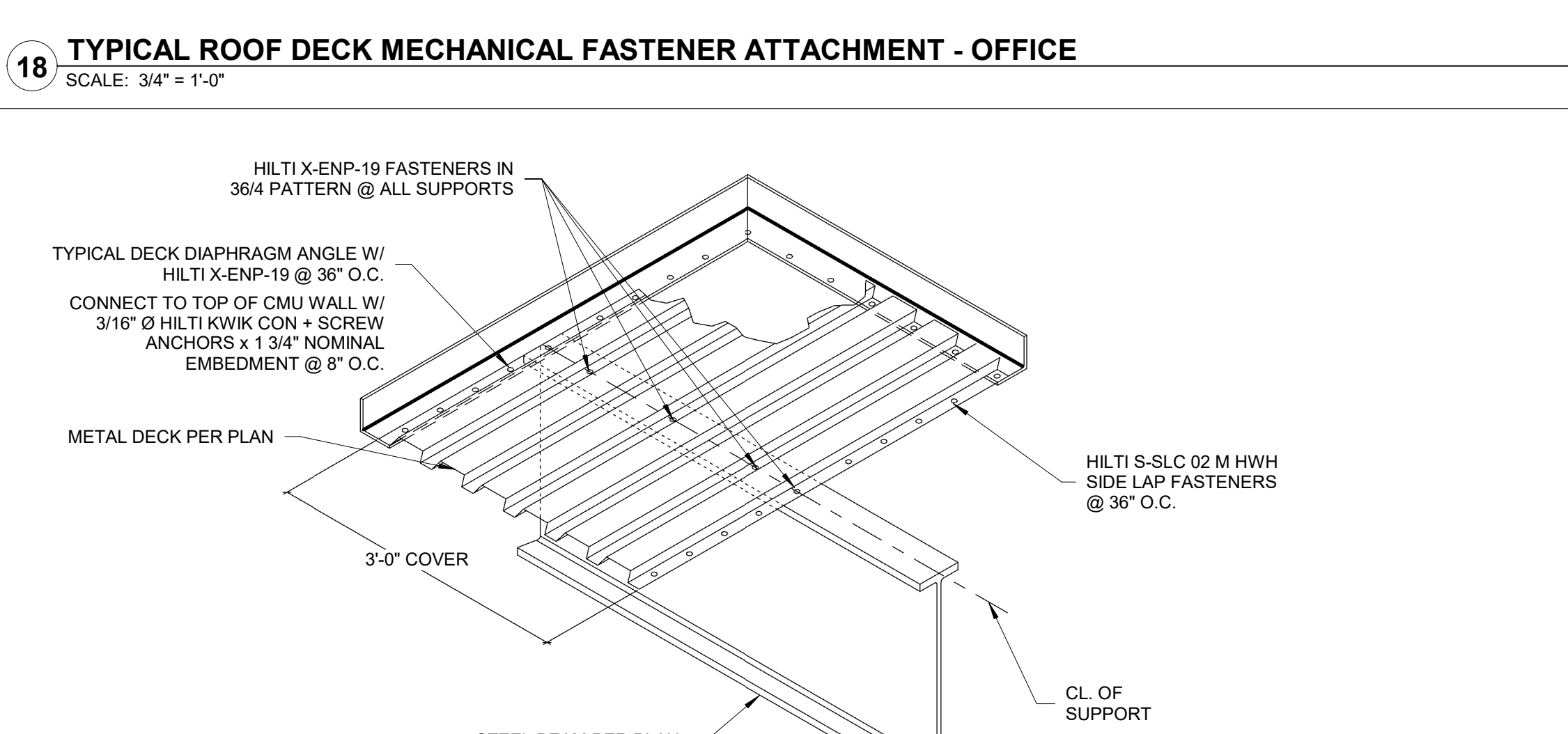
3 TYPICAL SINGLE-ANGLE ALL-BOLTED SHEAR CONNECTIONS (FOR REFERENCE ONLY)
SCALE: 3/4" = 1'-0"



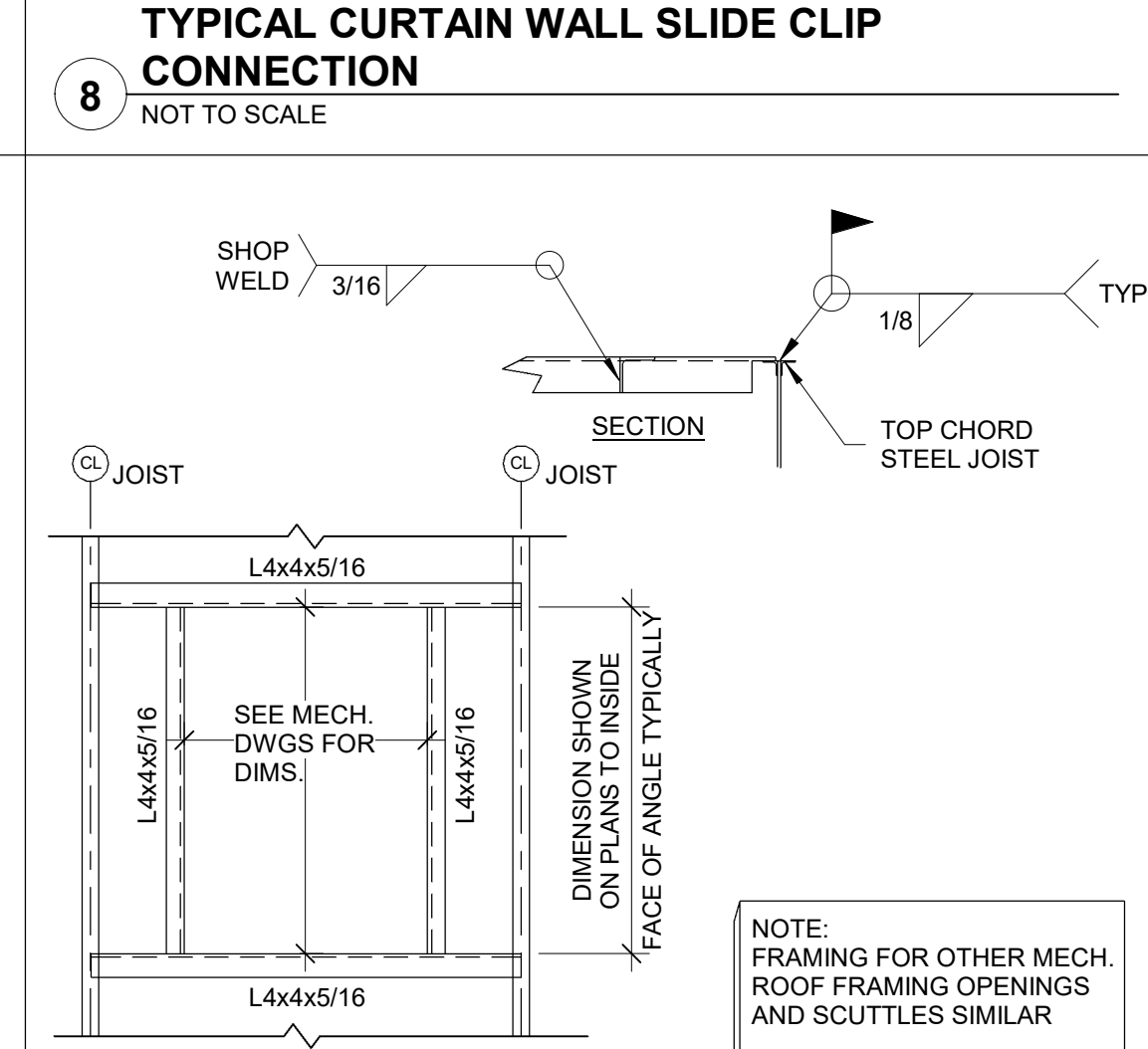
27 TYPICAL MOMENT FRAME BOTTOM FLANGE BRACING @ TRUSS
SCALE: 3/4" = 1'-0"



22 TYPICAL STUD WALL PANEL
NOT TO SCALE



17 TYPICAL FLOOR DECK ATTACHMENT AT MEZZANINES
SCALE: 3/4" = 1'-0"



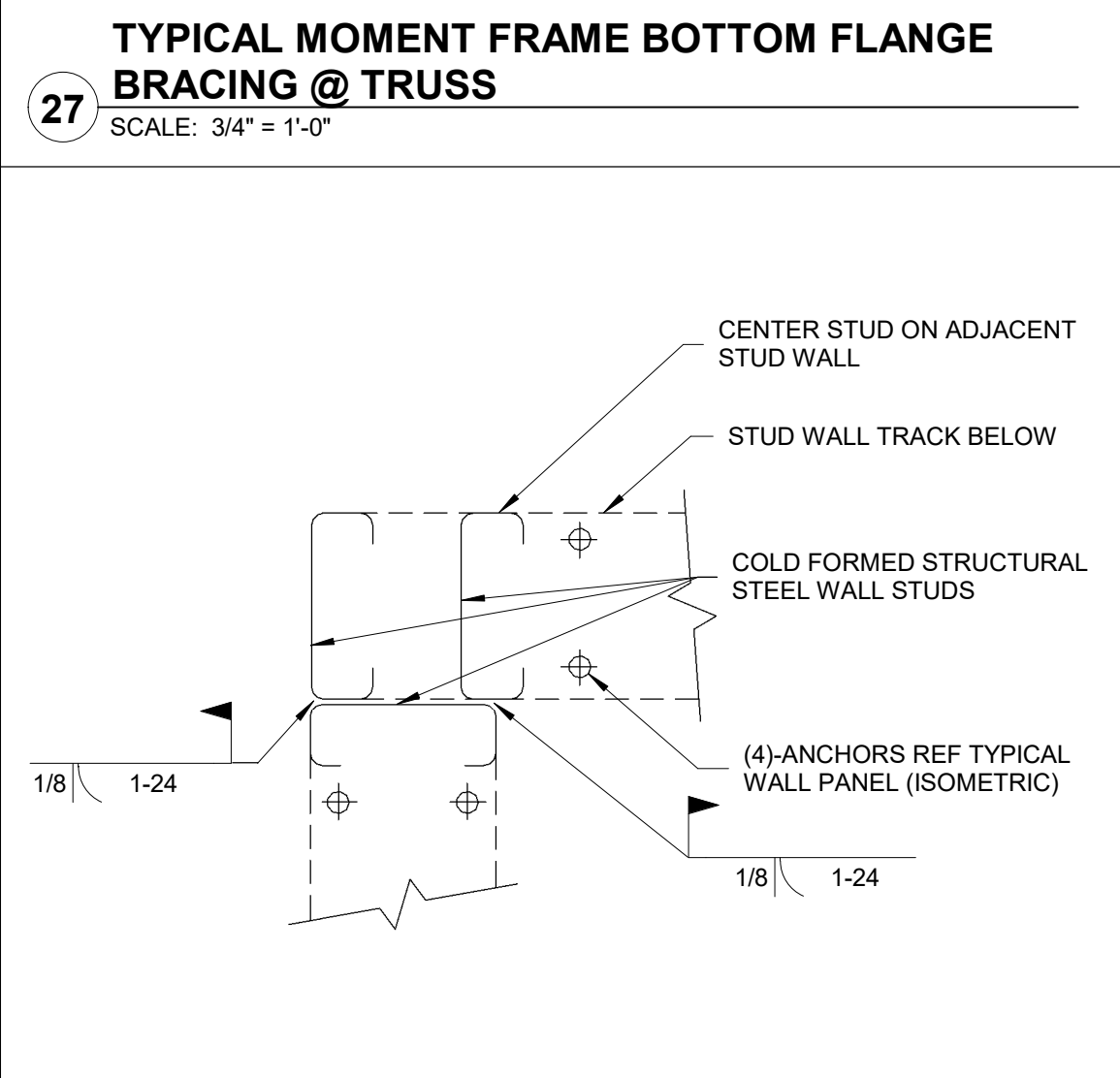
7 TYPICAL FRAMING FOR ROOF DECK OPENING
SCALE: 3/4" = 1'-0"

FOR REFERENCE ONLY - STEEL CONNECTIONS TO BE DESIGNED BY DELEGATED CONNECTION DESIGNER

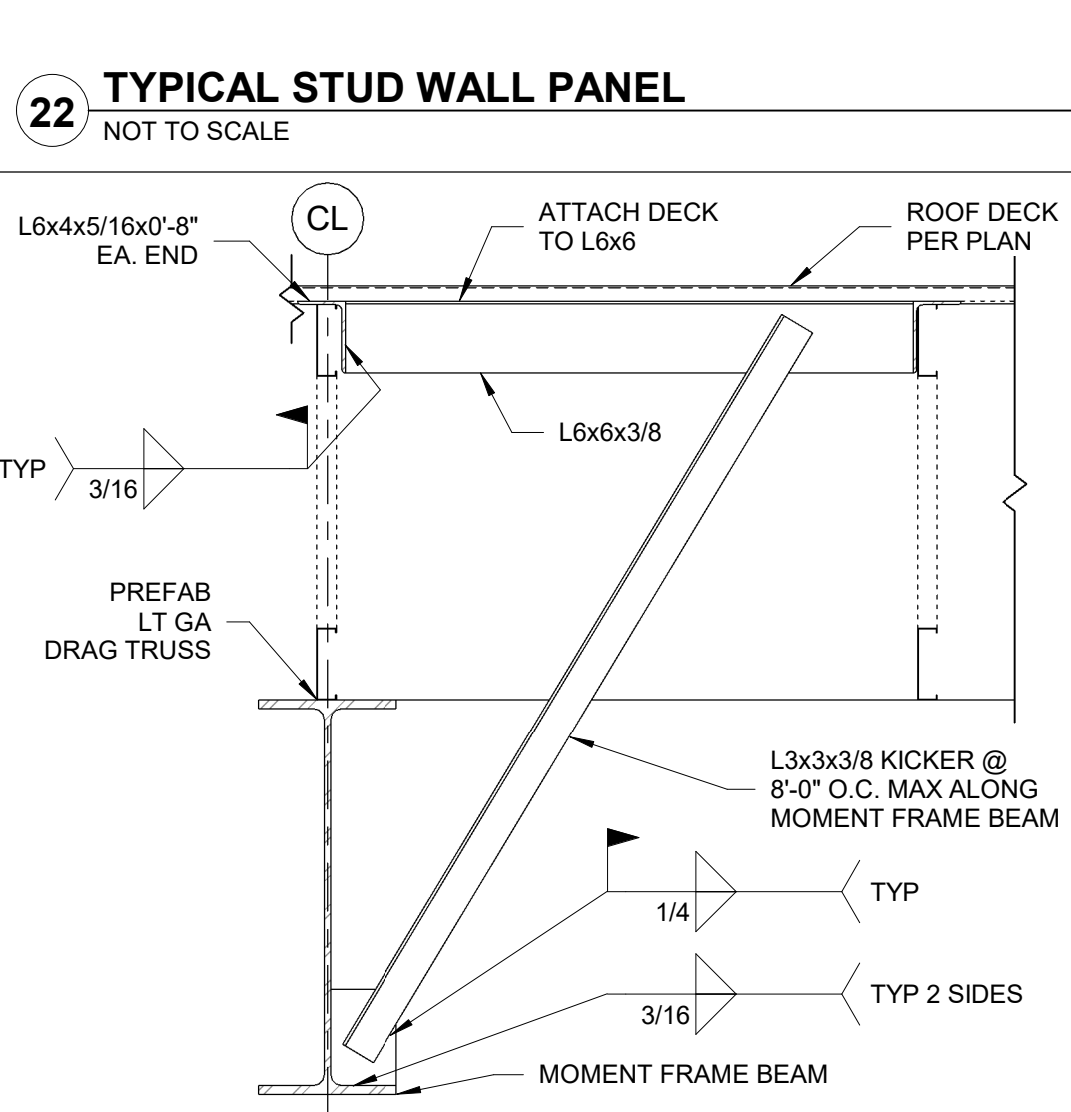
BEAM SIZE	5/16" STEEL PLATE LENGTH, L (IN.)	NUMBER OF 3/4" DIA. BOLTS
W10, 8	5 1/2"	2
W14, 12	8 1/2"	3
W18, 16	11 1/2"	4
W21, 18	14 1/2"	5
W27, 24	17 1/2"	6
W33, 30	20 1/2"	7
W36	23 1/2"	8

NOTES:
1. ADAPTED FROM TABLE 10-12, AISC STEEL CONSTRUCTION MANUAL, FOURTEENTH EDITION.
2. ALLOWABLE SHEAR STRENGTH OF BEAM WEB MUST BE CHECKED PER AISC TABLE 10-12. THE BEAM WEB MAY CONTROL THE CONNECTION CAPACITY. THIS TABLE MUST BE USED WITH CAUTION.
3. THE DISTANCE FROM THE BOLT LINE TO THE BEAM WEB MUST BE 2 1/4" OR 1 3/4".
4. THE HORIZONTAL EDGE DISTANCE FROM THE BEAM WEB MUST BE EITHER 1 1/2" OR 1 3/4".
5. THE VERTICAL EDGE DISTANCE AT THE WEB OF A COPED BEAM MUST BE GREATER THAN OR EQUAL TO 1 1/4".
6. EDGE DISTANCES ARE MEASURED TO THE CENTER OF THE BOLT OR SLOT.
7. BOLT SPACING SHALL BE 3".
8. BOLTS SHALL BE ASTM A325-N OR X.

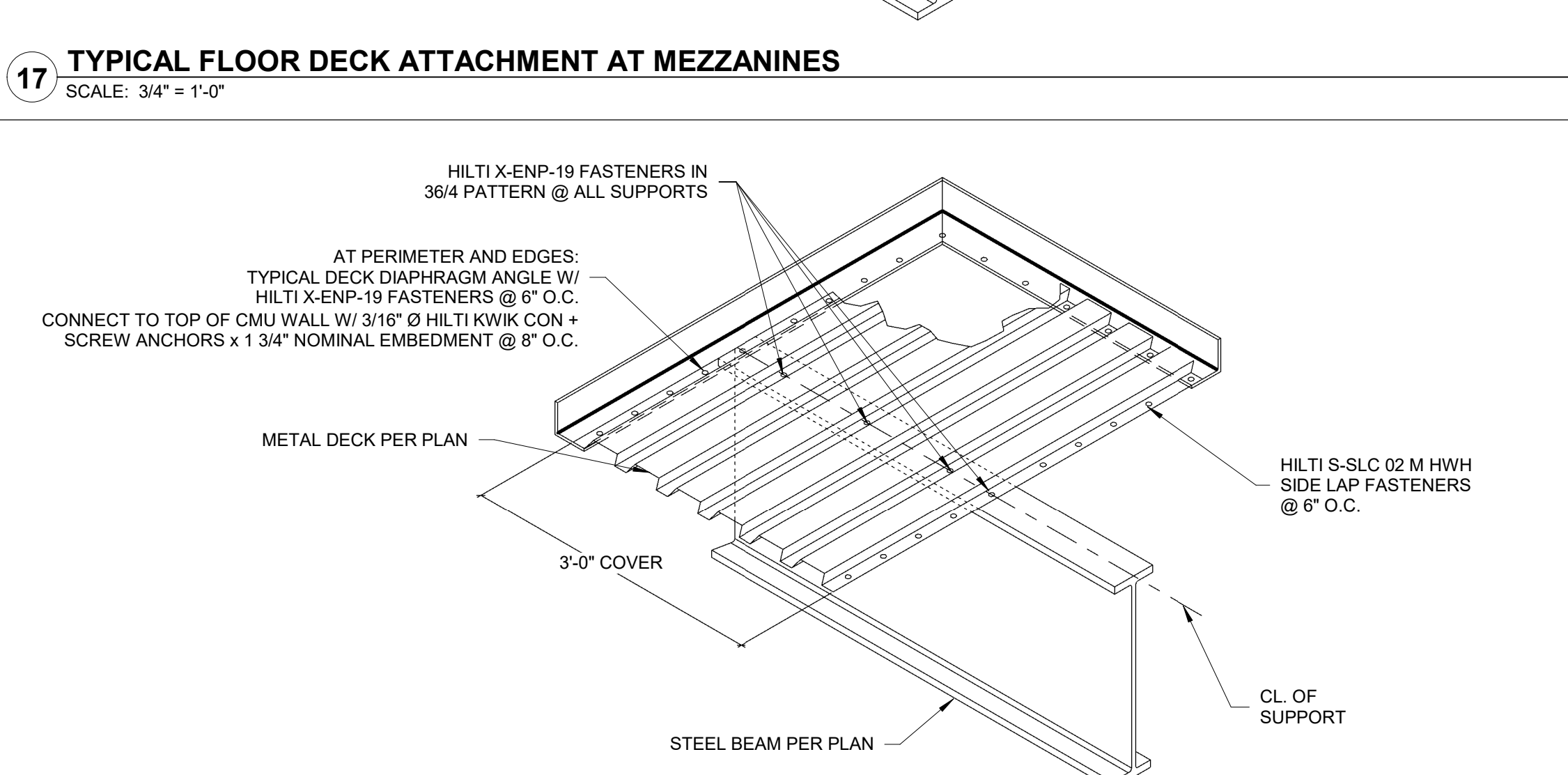
1 TYPICAL SINGLE-ANGLE ALL-BOLTED (FOR REFERENCE ONLY)
SCALE: 3/4" = 1'-0"



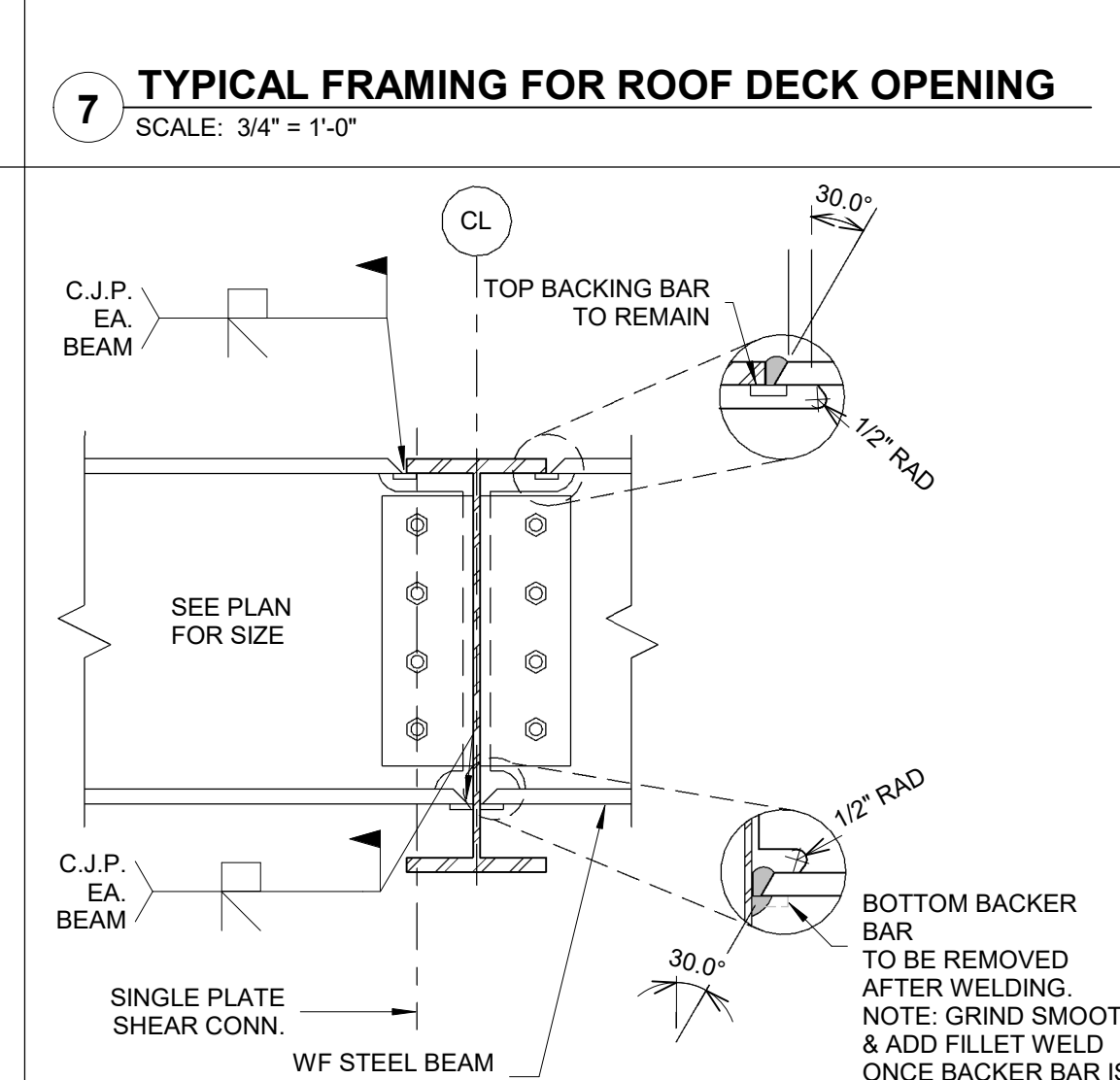
26 TYPICAL METAL STUD WALL @ CORNER
NOT TO SCALE



21 TYPICAL MOMENT FRAME BRACING @ METAL TRUSS-PARALLEL
SCALE: 3/4" = 1'-0"



16 TYPICAL FLOOR DECK ATTACHMENT AT HARDENED AREA
SCALE: 3/4" = 1'-0"



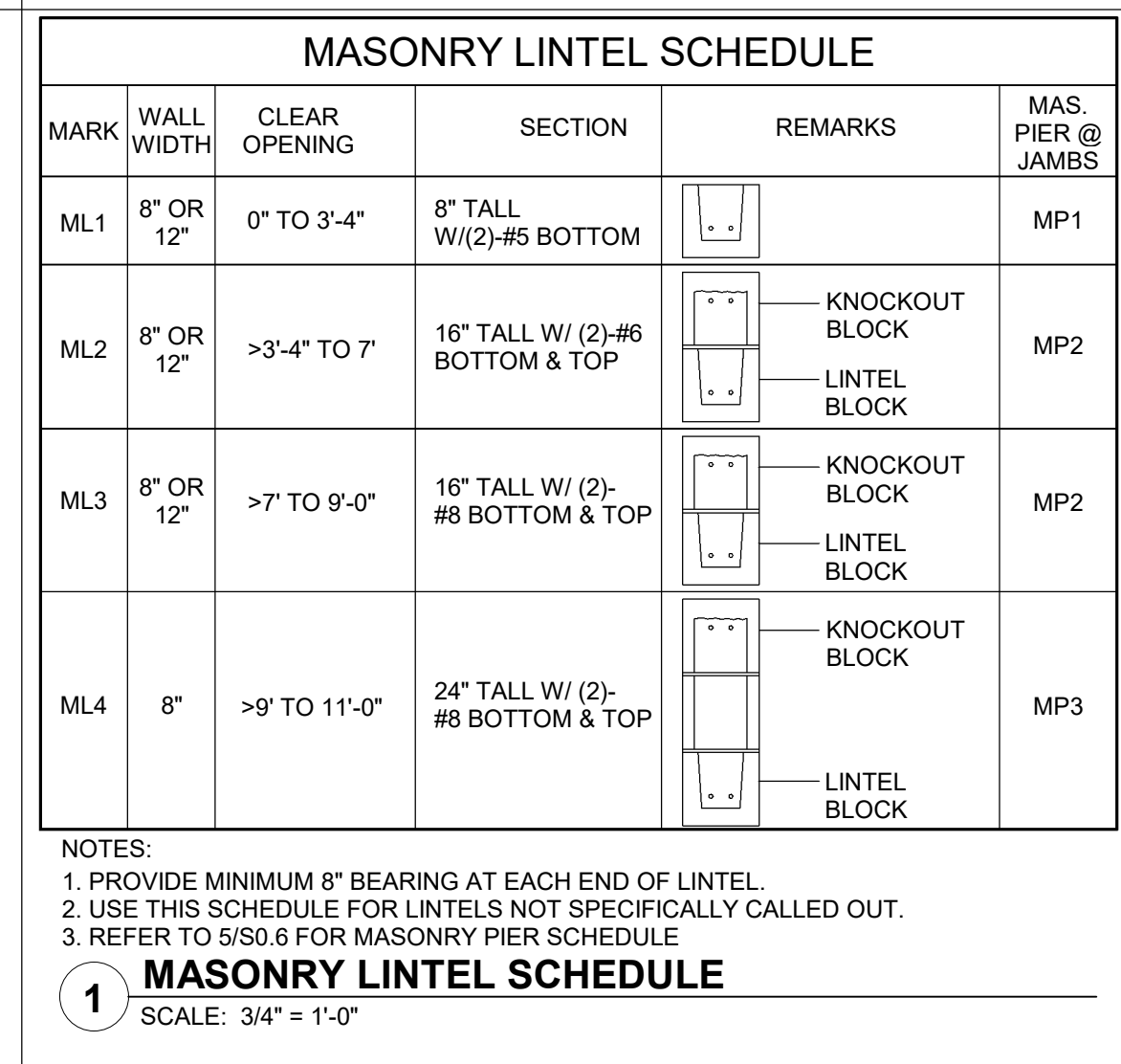
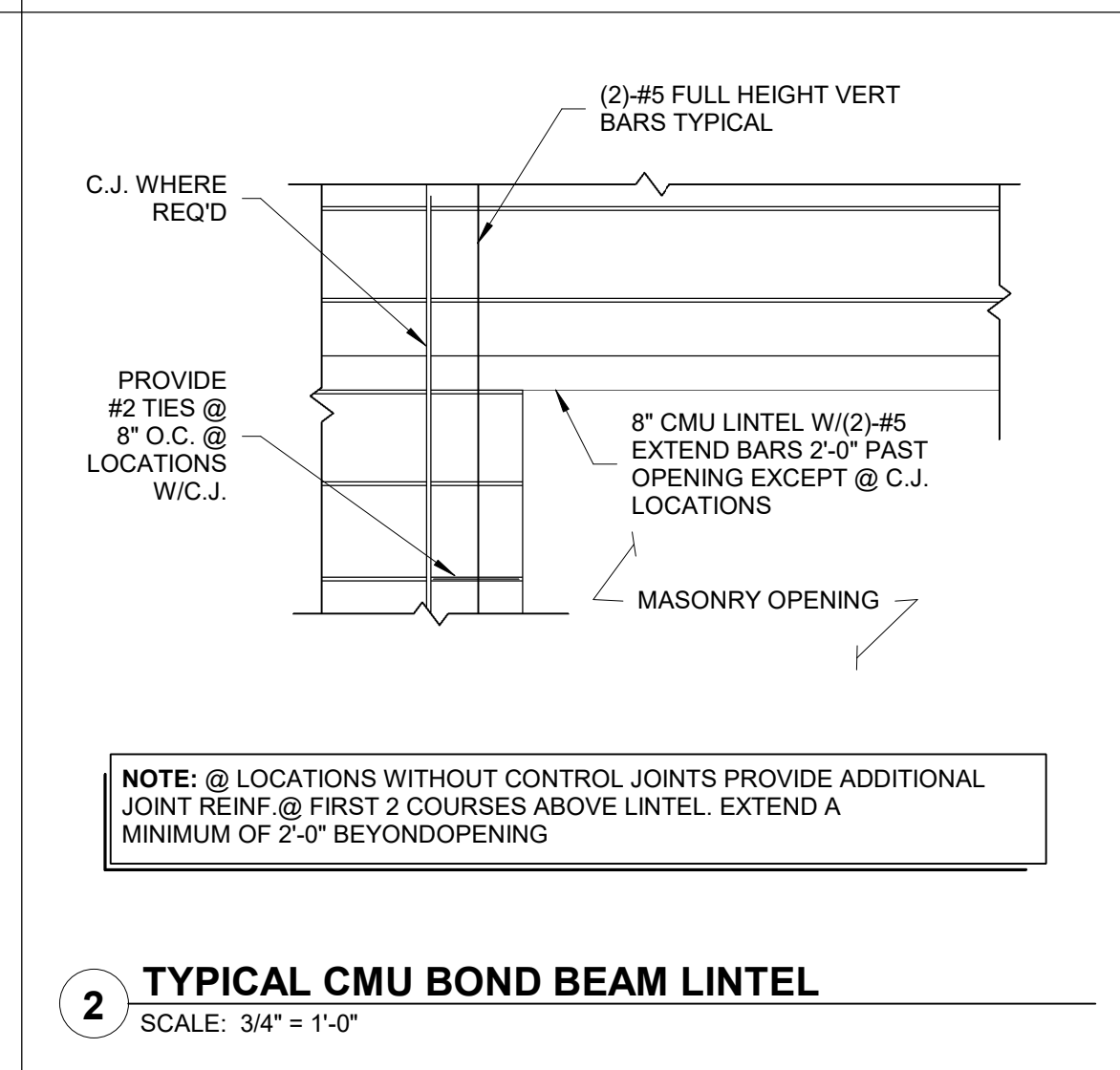
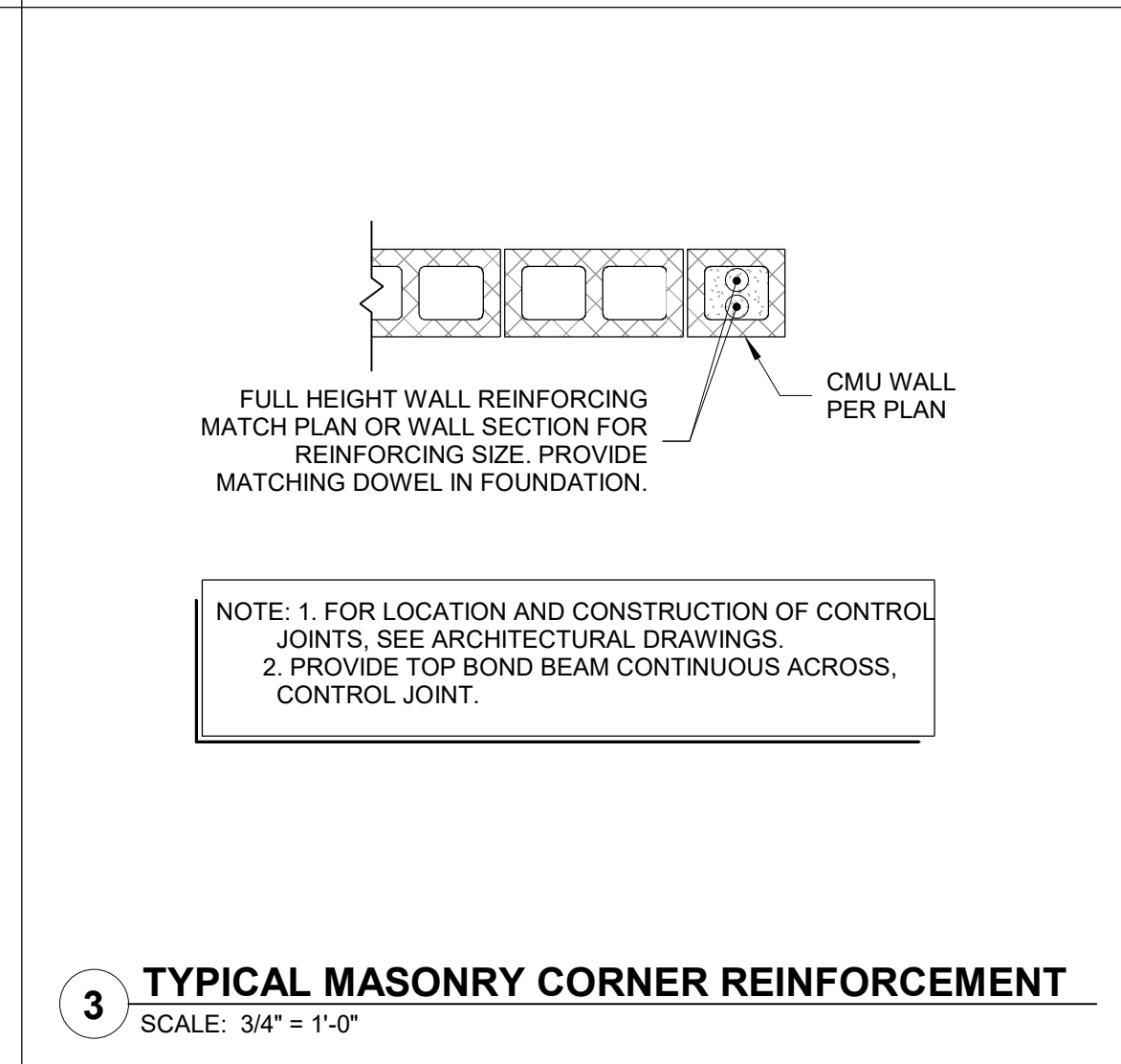
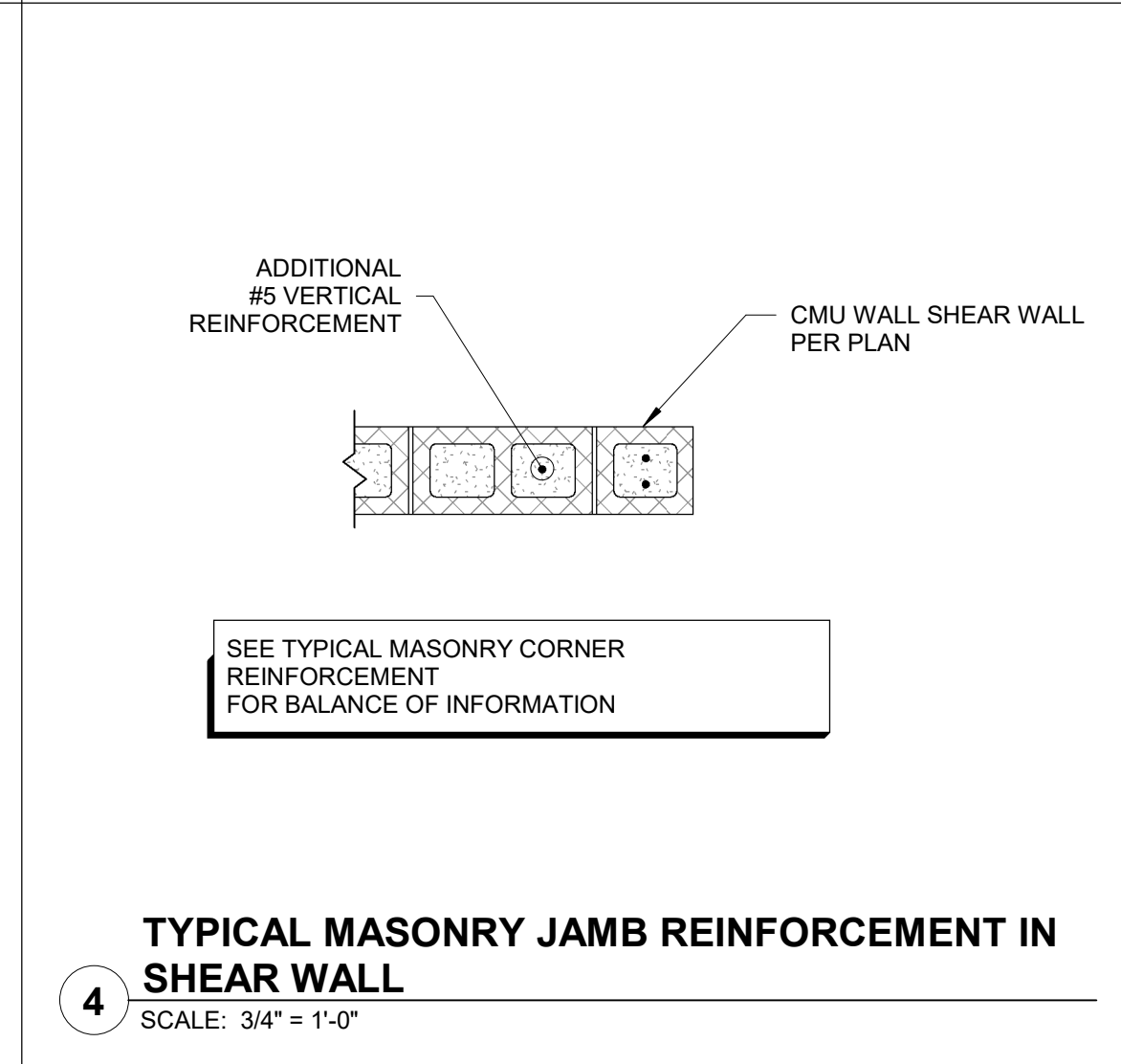
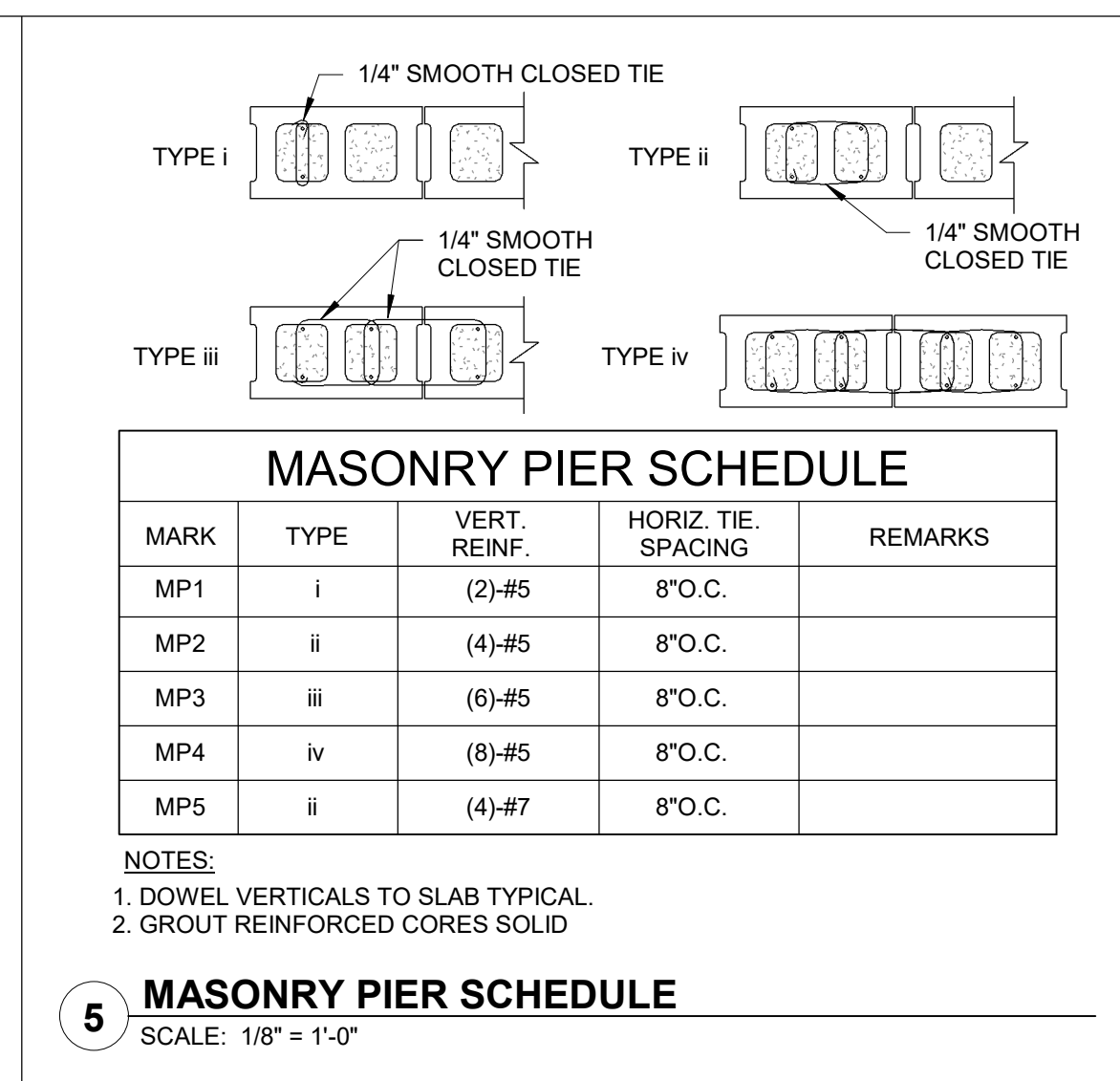
6 TYPICAL BEAM TO BEAM MOMENT CONNECTION
SCALE: 3/4" = 1'-0"

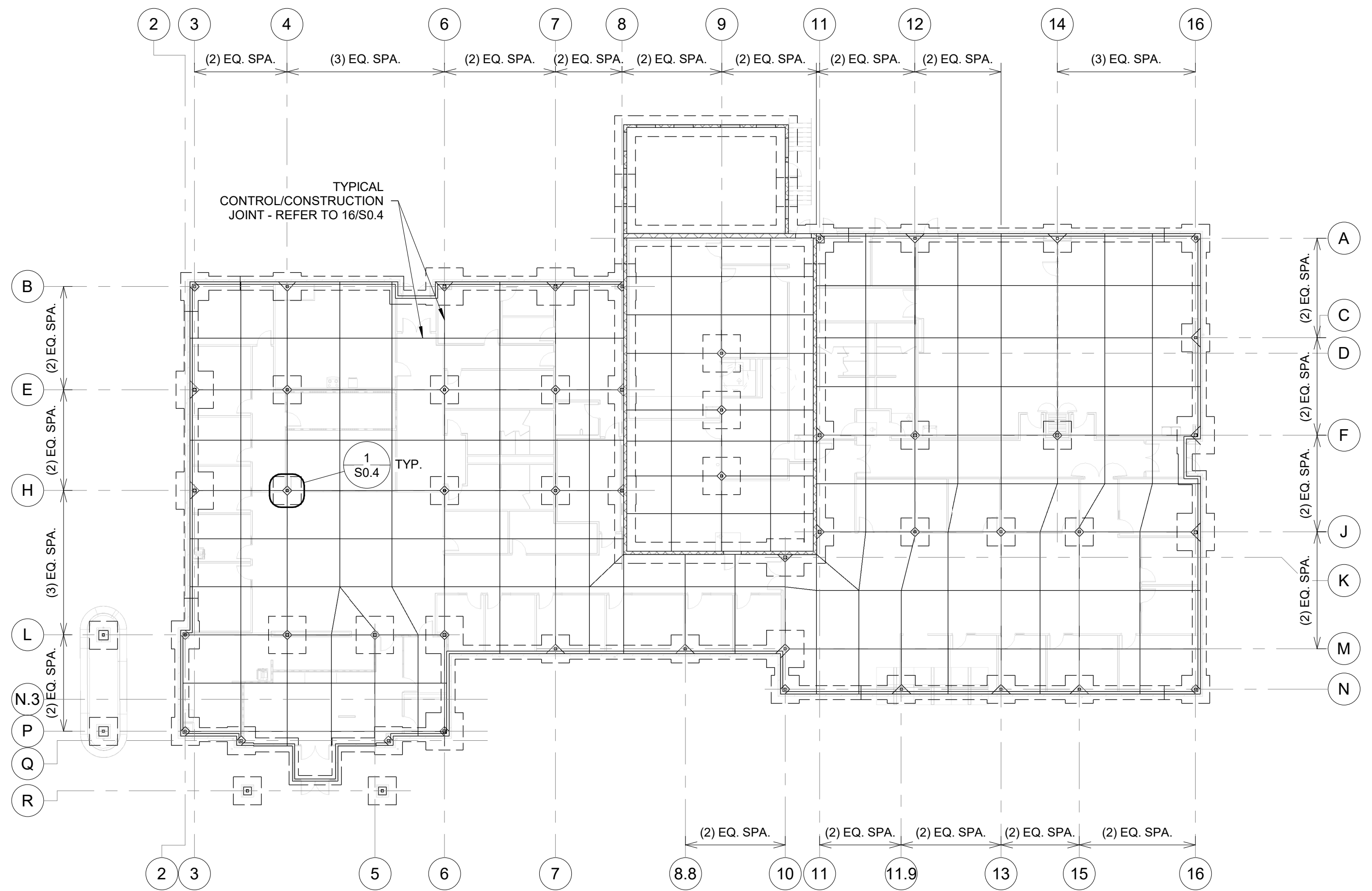
FOR REFERENCE ONLY - STEEL CONNECTIONS TO BE DESIGNED BY DELEGATED CONNECTION DESIGNER

BEAM SIZE	5/16" STEEL PLATE LENGTH, L (IN.)	NUMBER OF 3/4" DIA. BOLTS
W10, 8	5 1/2"	2
W14, 12	8 1/2"	3
W18, 16	11 1/2"	4
W21, 18	14 1/2"	5
W27, 24	17 1/2"	6
W33, 30	20 1/2"	7
W36	23 1/2"	8

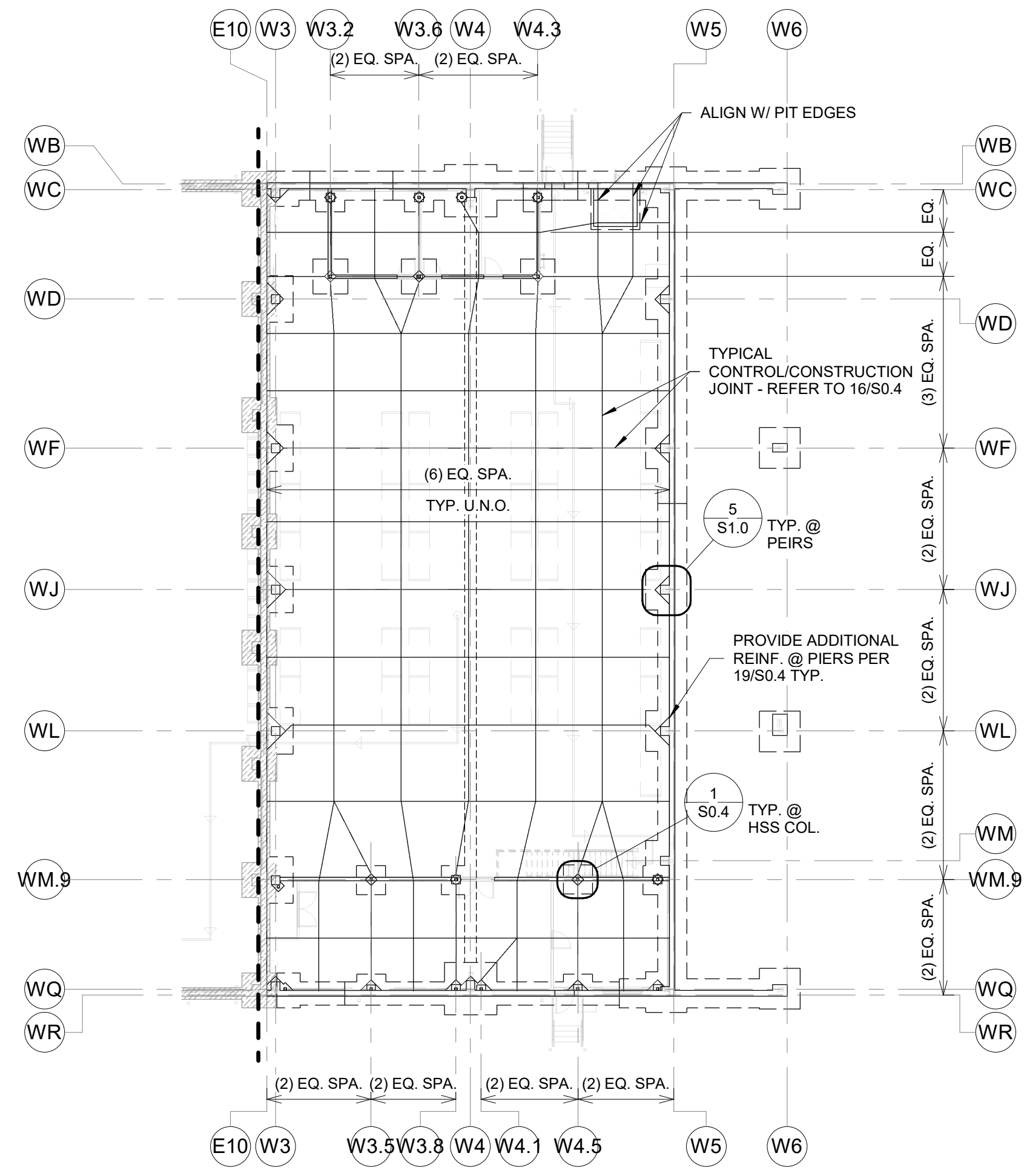
NOTES:
1. ADAPTED FROM TABLE 10-12, AISC STEEL CONSTRUCTION MANUAL, FOURTEENTH EDITION.
2. ALLOWABLE SHEAR STRENGTH OF BEAM WEB MUST BE CHECKED PER AISC TABLE 10-12. THE BEAM WEB MAY CONTROL THE CONNECTION CAPACITY. THIS TABLE MUST BE USED WITH CAUTION.
3. THE DISTANCE FROM THE BOLT LINE TO THE BEAM WEB MUST BE 2 1/4" OR 1 3/4".
4. THE HORIZONTAL EDGE DISTANCE FROM THE BEAM WEB MUST BE EITHER 1 1/2" OR 1 3/4".
5. THE VERTICAL EDGE DISTANCE AT THE WEB OF A COPED BEAM MUST BE GREATER THAN OR EQUAL TO 1 1/4".
6. EDGE DISTANCES ARE MEASURED TO THE CENTER OF THE BOLT OR SLOT.
7. BOLT SPACING SHALL BE 3".
8. BOLTS SHALL BE ASTM A325-N OR X.

1 TYPICAL SINGLE-ANGLE ALL-BOLTED (FOR REFERENCE ONLY)
SCALE: 3/4" = 1'-0"

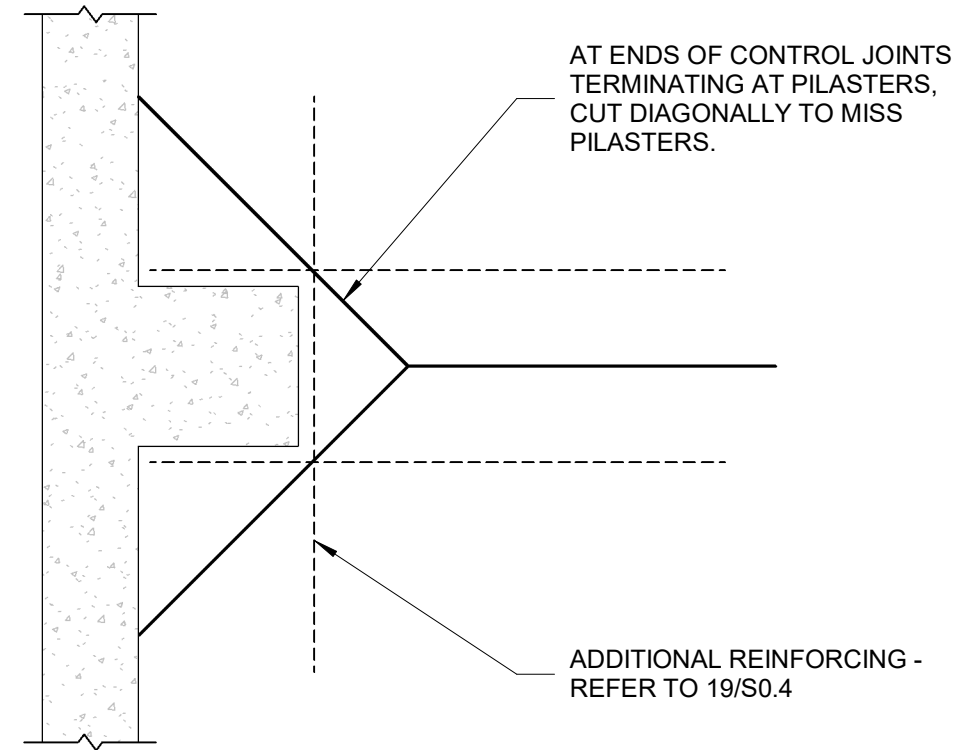




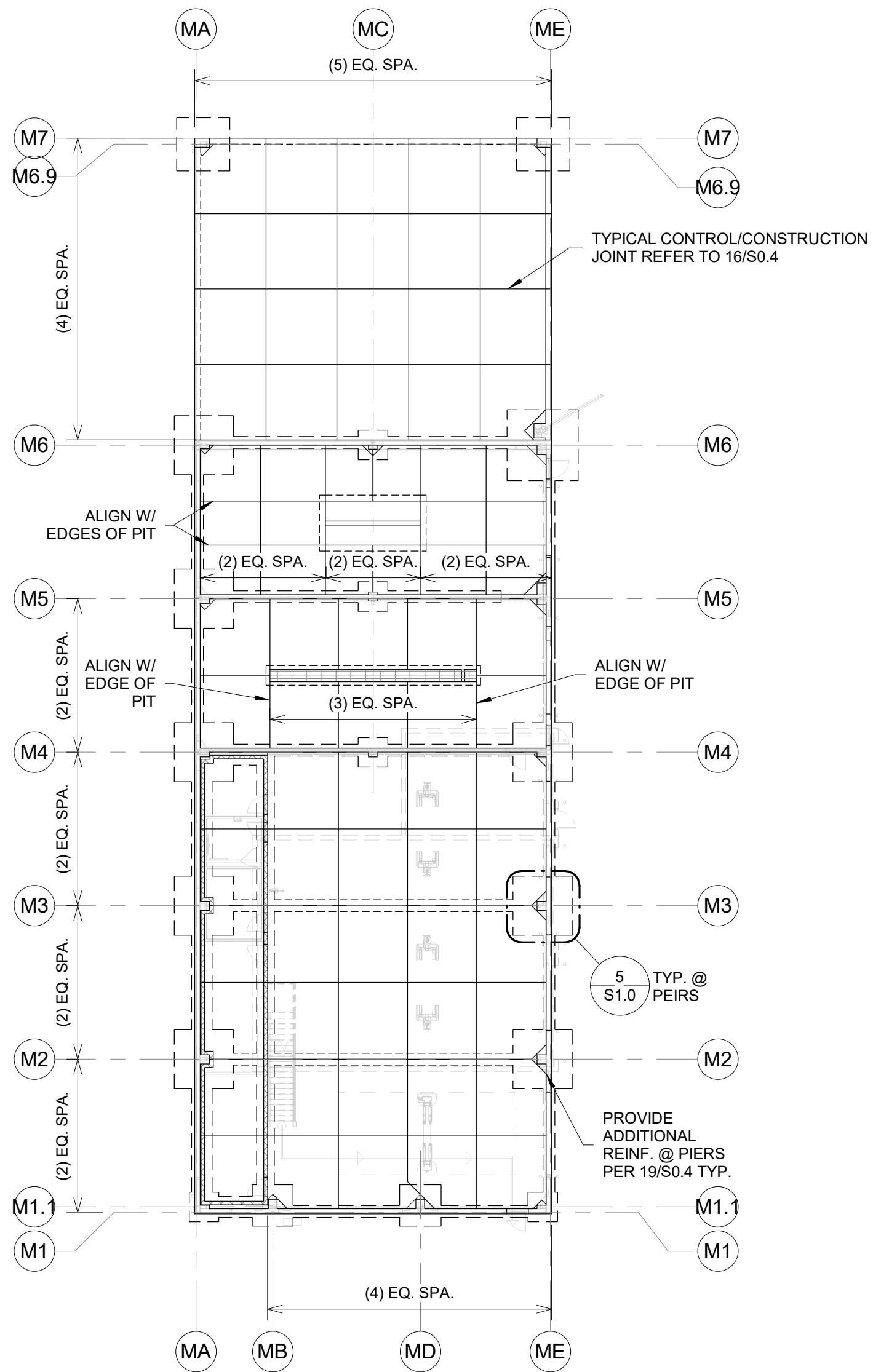
STRUCTURAL CONTROL JOINT PLAN - OFFICE
SCALE: 1" = 20'-0"
NORTH



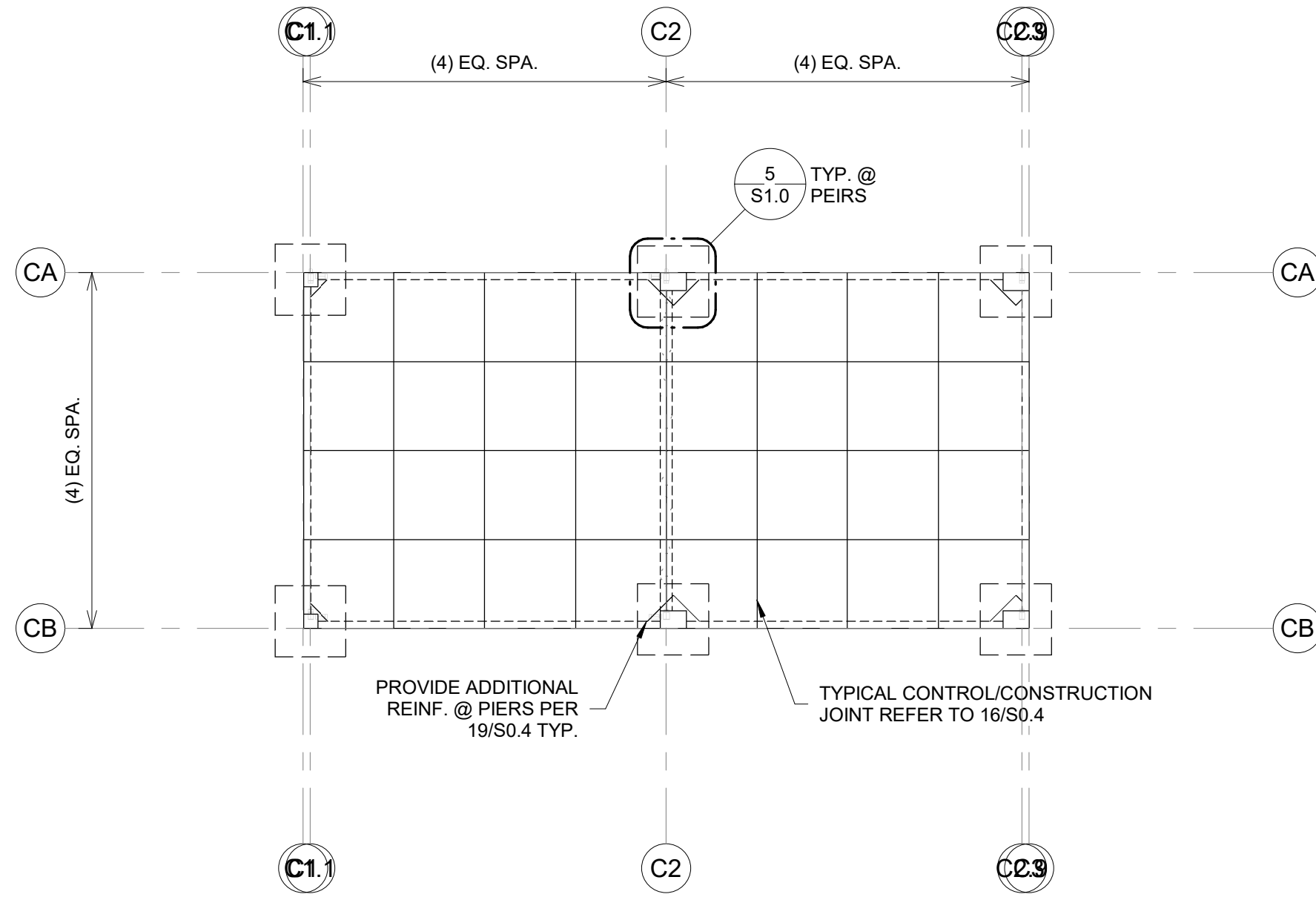
STRUCTURAL CONTROL JOINT PLAN - WAREHOUSE
SCALE: 1" = 20'-0"
NORTH



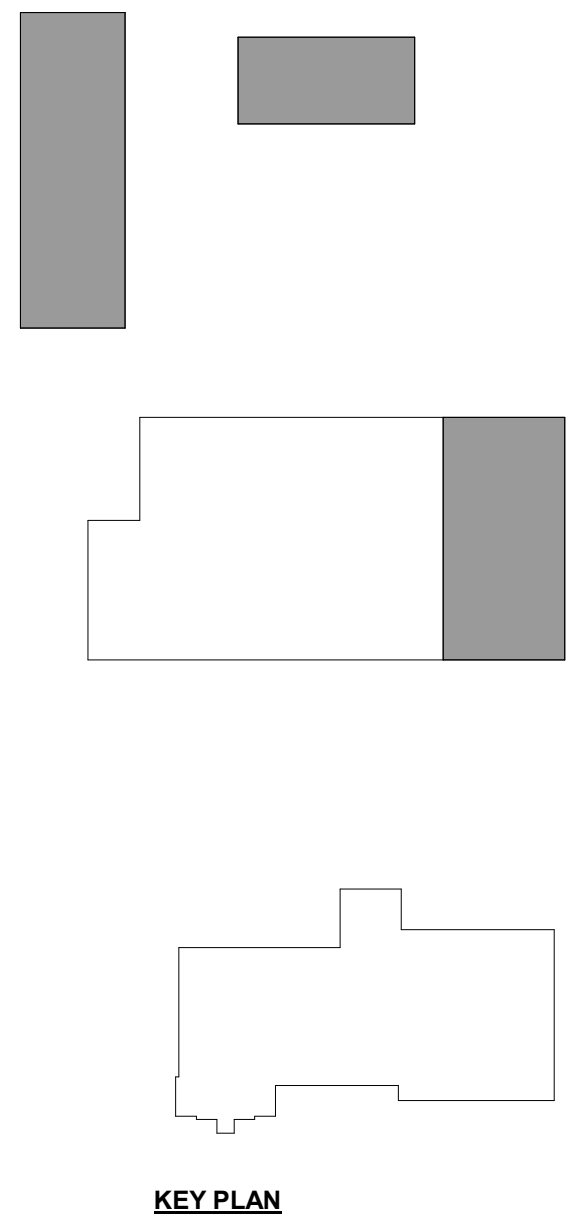
5 TYPICAL SLAB CONTROL JOINT AT PIER
SCALE: 1/2" = 1'-0"



STRUCTURAL CONTROL JOINT PLAN - MAINTENANCE
SCALE: 1" = 20'-0"
NORTH



STRUCTURAL CONTROL JOINT PLAN - COVERED STORAGE
SCALE: 1" = 20'-0"
NORTH



KEY PLAN

FOUNDATION PLAN NOTES:
1. REF. SHEET S1.1 FOR FOUNDATION PLAN NOTES.

M+H
ARCHITECTS
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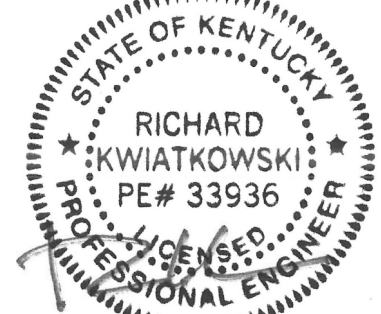
COOPERATIVE
BUILDING SOLUTIONS

TAYLOR COUNTY RECC

685 WATER TOWER BYPASS
CAMBELLSVILLE, KY 42718

RECC
A Taylor County Company

RICHARD E. KWIATKOWSKI
Professional Engineer
KY LIC# PE-33936
EXP: 06/30/2027



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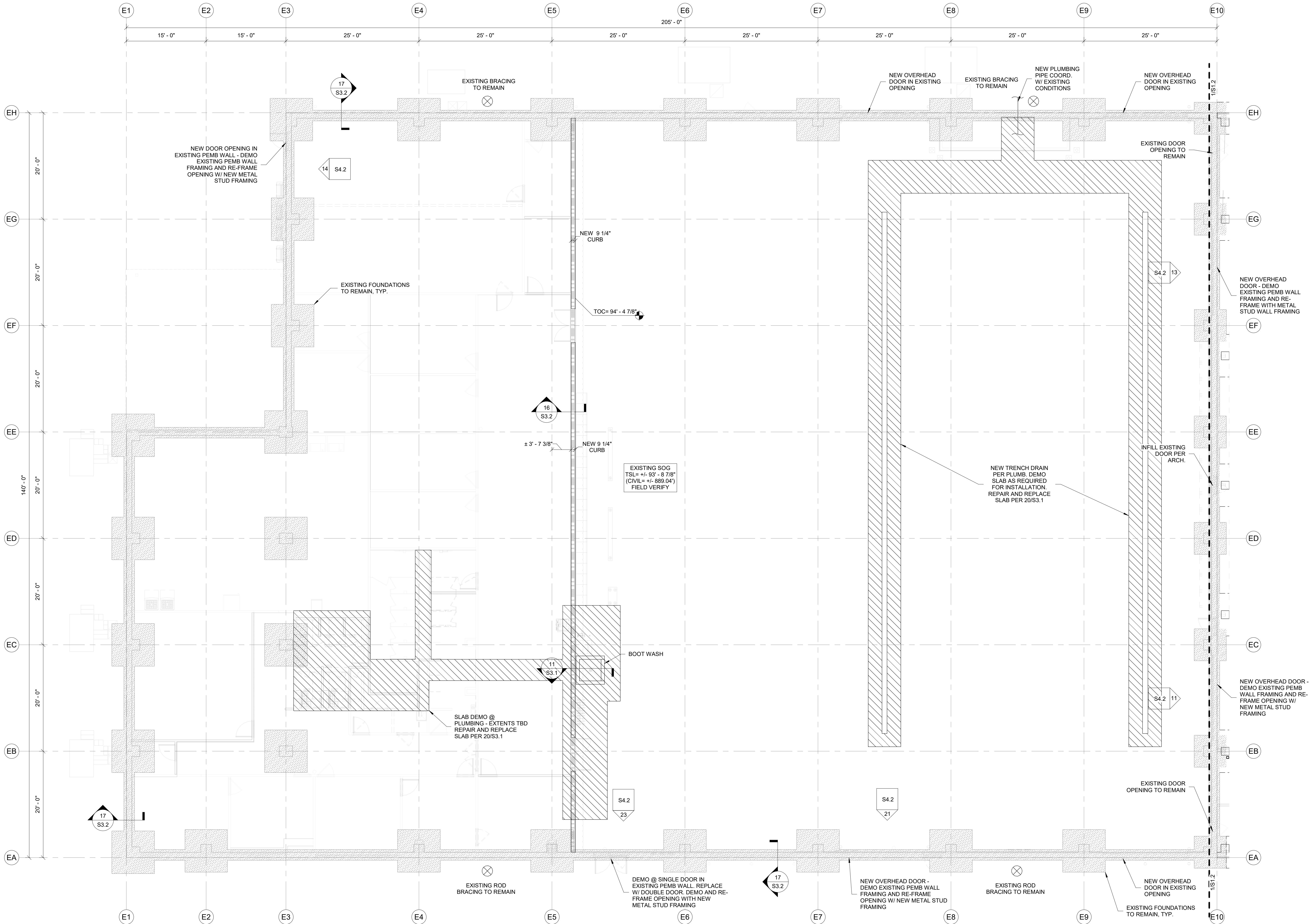
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Sheet Issue Date:	08/20/2026
Job Number:	250120.00
Drawn By:	Author
Checked By:	Checker

Drawing Title:
STRUCTURAL CONTROL JOINT PLANS

S1.0

SSC JOB# 2025456



- FOUNDATION PLAN NOTES:**
- REF. SHEET S0.1 - S0.2 FOR STRUCTURAL GENERAL NOTES.
 - REF. SHEET S1.1 FOR THE BELOW SCHEDULES:
A. FOUNDATION
B. STEEL COLUMN
C. PIER
 - REF. SHEET S0.4 FOR TYPICAL FOUNDATION AND SLAB ON GRADE DETAILS.
 - REFER TO THE ARCHITECTURAL DRAWINGS AND/OR COORDINATE WITH THE ARCHITECT REGARDING MISSING DIMENSIONS.
 - FINISHED FLOOR ELEVATION IS DEFINED ON EACH BUILDING FOUNDATION PLAN.
A. ELEVATION = 100'-0" CORRESPONDS TO AN ACTUAL ELEVATION OF 895.30'. COORDINATE ALL ELEVATIONS WITH CIVIL DRAWINGS.
 - SLAB ON GRADE:
A. OFFICE: 4" NORMAL WEIGHT CONCRETE SLAB ON GRADE REINFORCED WITH 6x6/W1.4 x W1.4 WWF PLACED IN UPPER THIRD OF SLAB
B. HARDENED AREA: 5" NORMAL WEIGHT CONCRETE SLAB ON GRADE REINFORCED WITH 6x6/W2.9 x W2.9 WWF PLACED IN UPPER THIRD OF SLAB
C. WAREHOUSE, MAINTENANCE, AND COVERED STORAGE BUILDINGS: 7" NORMAL WEIGHT CONCRETE SLAB ON GRADE REINFORCED WITH 6x6/W2.9 x W2.9 WWF PLACED IN UPPER THIRD OF SLAB, UNLESS NOTED OTHERWISE ON PLAN.
 - PLACE SLAB ON 15 MIL VAPOR BARRIER, OVER 4" MINIMUM CRUSHED & COMPACTED ROCK. COMPACT TO 95% STANDARD PROCTOR.
 - SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF RAMPS, DEPRESSED SLABS, STEPPED SLABS, STOOPS AND NON-BEARING PARTITION WALLS.
 - PLACE ALL EXTERIOR STOOP SLABS OVER COMPACTED CLEAN GRANULAR FILL MATERIAL PER TYPICAL DETAILS. DEPTH TO MATCH LOCAL FROST REQUIREMENTS.
 - EACH BUILDING HAS DIFFERENT TOP OF SLAB ELEVATIONS. REFER TO EACH BUILDING FOUNDATION SHEET FOR ELEVATIONS AND COORDINATE WITH CORRESPONDING CIVIL ELEVATIONS.
 - T/SLAB OF NEW SLAB ON GRADE TO MATCH T/SLAB EXISTING SLAB AT EXISTING OPERATIONS BUILDING AND NEW WAREHOUSE BUILDING. FIELD VERIFY EXISTING ELEVATIONS AT POINTS OF MATCHING CONTACT PRIOR TO FINAL GRADING.
 - TOP (EXTERIOR) ELEVATION = @ OFFICE (-1'-4") BELOW T/SL ELEVATION, @ WAREHOUSE, MAINTENANCE, COVERED STORAGE (-2'-0") BELOW T/SL ELEVATION UNLESS NOTED OTHERWISE ON PLAN.
 - TOP (INTERIOR) ELEVATION = (-1'-0") BELOW T/SL ELEVATION UNLESS NOTED OTHERWISE ON PLAN.
 - COORDINATE TOP OF FOOTING ELEVATIONS WITH CROSSING MECHANICAL PLUMBING LINE INVERTS AND ELECTRICAL LINE LOCATIONS. STEP FOOTINGS DOWN ON EITHER SIDE OF LINE AND SLEEVE THROUGH FOUNDATION WALLS.
 - PER CRSI - "MANUAL OF STANDARD PRACTICE": DO NOT STICK DOWELS INTO WET CONCRETE. TIE IN PLACE PRIOR TO START OF CONCRETE PLACEMENT.

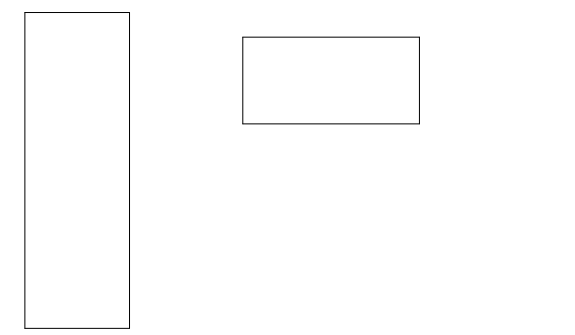
STRUCTURAL FOUNDATION SCHEDULE		
MARK	SIZE	REINFORCING
F5	5'-0" x 5'-0" x 1'-6"	(6) #6 EACH WAY (TOP & BOTTOM)
F5B	5'-0" x 5'-0" x 2'-0"	(6) #6 EACH WAY (TOP & BOTTOM)
F6	6'-0" x 5'-0" x 1'-6"	(6) #6 EACH WAY (TOP & BOTTOM)
F6A	6'-0" x 6'-0" x 2'-0"	(6) #7 EACH WAY (TOP & BOTTOM)
F6B	4'-0" x 6'-0" x 2'-0"	(4) #7 BARS LONG (TOP & BOTTOM), (6) #7 BARS SHORT (TOP & BOTTOM)
F7	7'-0" x 7'-0" x 2'-0"	(7) #7 EACH WAY (TOP & BOTTOM)
F8	8'-0" x 8'-0" x 1'-6"	(8) #6 EACH WAY (TOP & BOTTOM)
F8B	4'-0" x 8'-0" x 2'-0"	(4) #7 BARS LONG (TOP & BOTTOM), (8) #7 BARS SHORT (TOP & BOTTOM)
F9	9'-0" x 9'-0" x 2'-0"	(8) #7 EACH WAY (TOP & BOTTOM)
F10	10'-0" x 10'-0" x 2'-0"	(10) #7 EACH WAY (TOP & BOTTOM)
F12	12'-0" x 12'-0" x 2'-0"	(12) #7 EACH WAY (TOP & BOTTOM)

STRUCTURAL STEEL COLUMN SCHEDULE		
MARK	SIZE	COMMENTS
C5	HSS55x53x3/8	
C6	HSS60x63x3/8	
C8	HSS88x81x1/2	

NOTES:

1. USE BASE PLATE TYPE LISTED IN THE COLUMN SCHEDULE UNLESS DIRECTED OTHERWISE BY DETAIL REFERENCE OR SECTION CUT. SEE SHEET 6/S3.2 FOR BASE PLATE DETAILS.

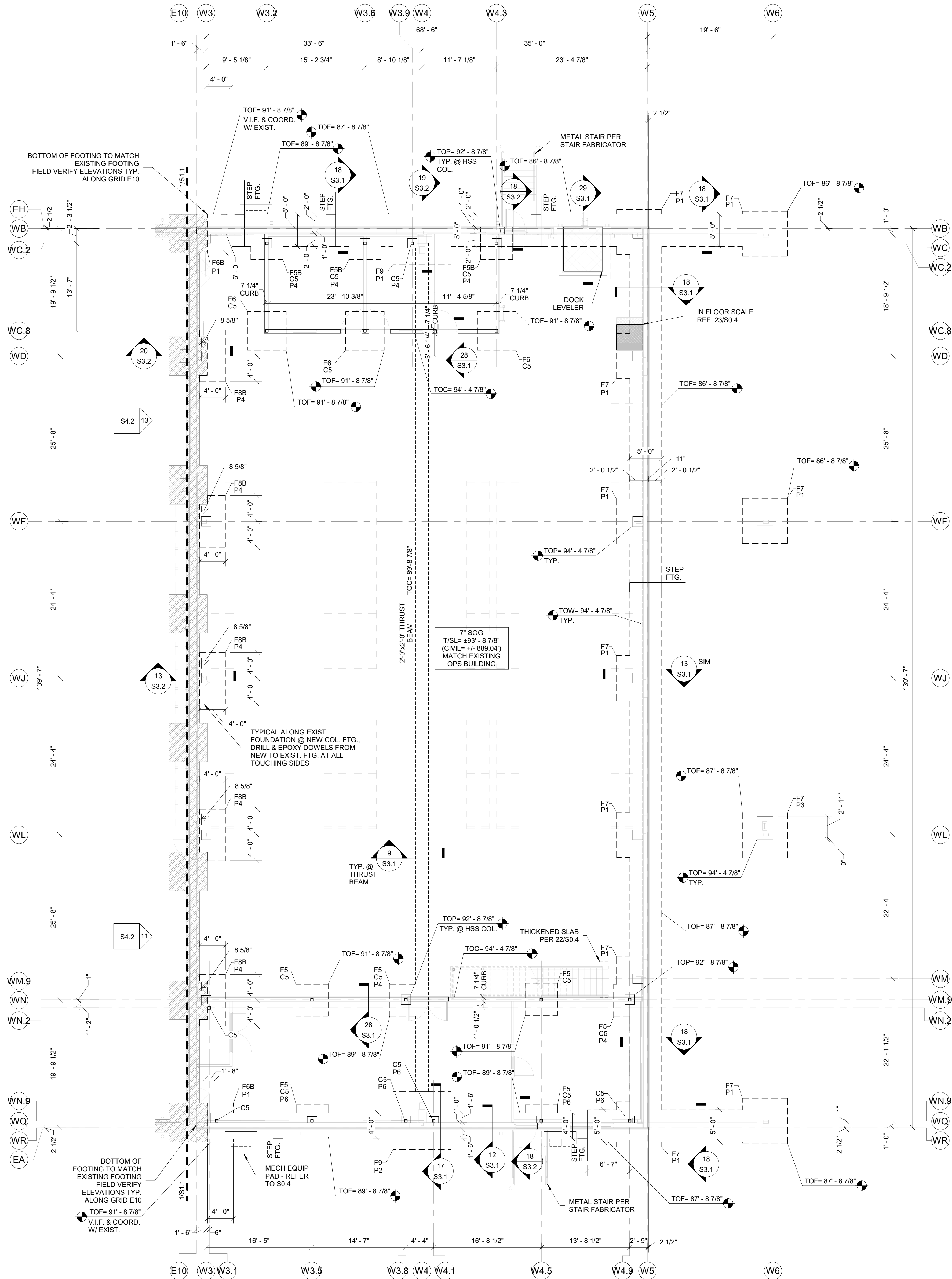
STRUCTURAL PIER SCHEDULE		
MARK	SIZE (B x L)	REINFORCING
P1	1'-6" x 2'-6"	(14) #6 VERTS. W/ #3 TIES, TOP (3) TIES @ 3" O.C. BALANCE @ 12" O.C.
P2	1'-6" x 2'-6"	(14) #6 VERTS. W/ #3 TIES, TOP (3) TIES @ 3" O.C. BALANCE @ 12" O.C.
P3	2'-6" x 3'-8"	(20) #6 VERTS. W/ #3 TIES, TOP (3) TIES @ 3" O.C. BALANCE @ 12" O.C.
P4	1'-6" x 1'-6"	(8) #6 VERTS. W/ #3 TIES, TOP (3) TIES @ 3" O.C. BALANCE @ 12" O.C.
P5	1'-8" x 1'-8"	(8) #6 VERTS W/ (3) #3 TIES @ 3" OC AT TOP & 12" OC THEREAFTER
P6	1'-6" x 1'-4"	(8) #6 VERTS W/ (3) #3 TIES @ 3" OC AT TOP & 12" OC THEREAFTER
P7	2'-0" x 2'-0"	(4) #6 VERTS. W/ #3 TIES @ 12" O.C. - REFER TO 22/S3.1
P8	2'-6" x 2'-6"	(20) #6 VERTS. W/ #3 TIES, TOP (3) TIES @ 3" O.C. BALANCE @ 12" O.C.



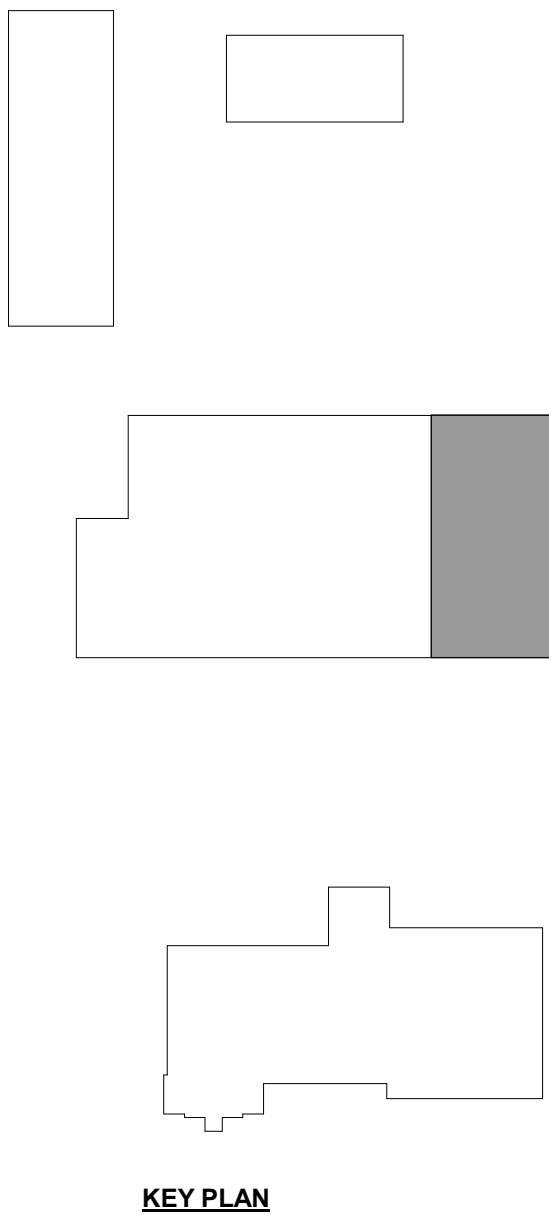
OPERATIONS

KEY PLAN

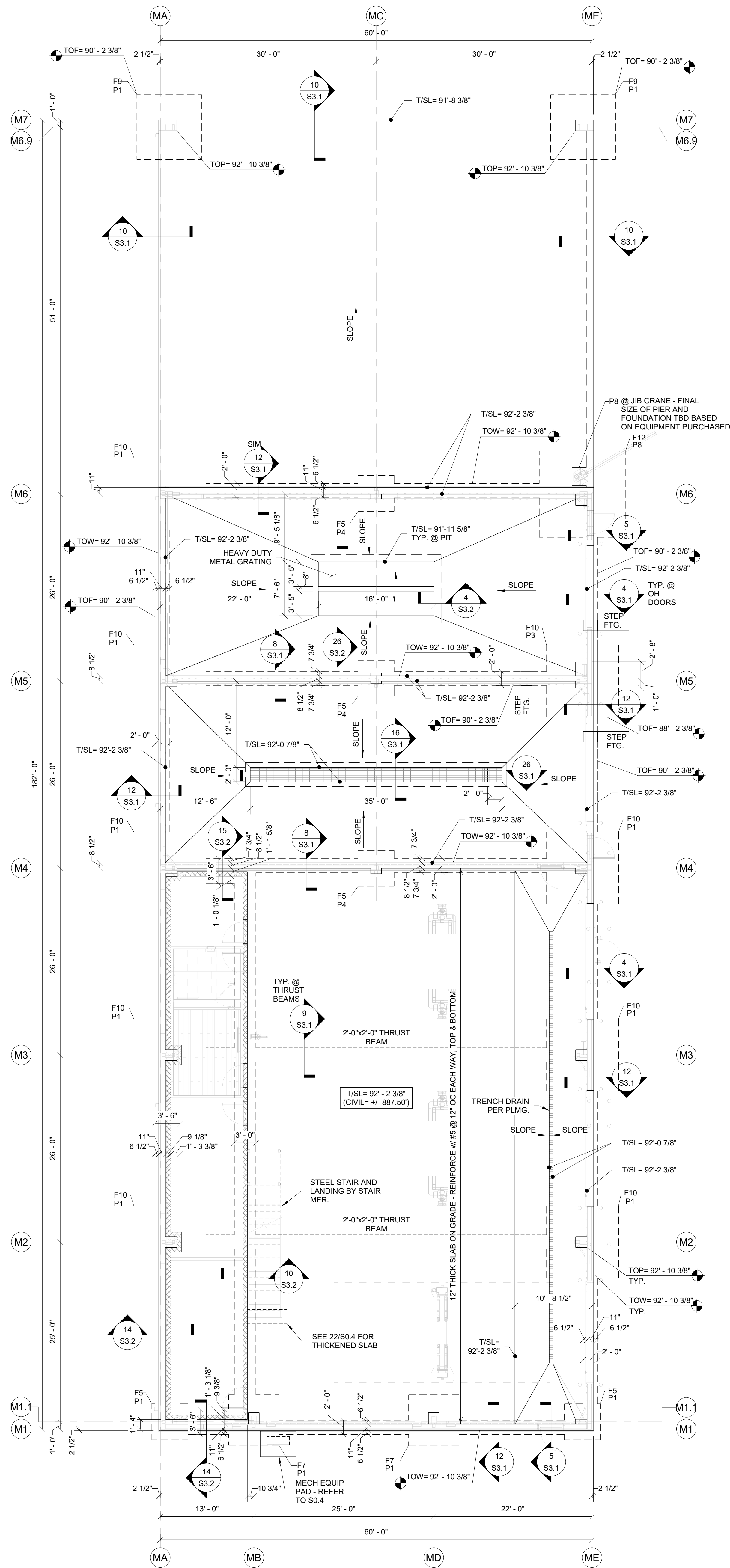
STRUCTURAL FOUNDATION PLAN - OPERATIONS BUILDING
SCALE: 1/8" = 1'-0"
NORTH



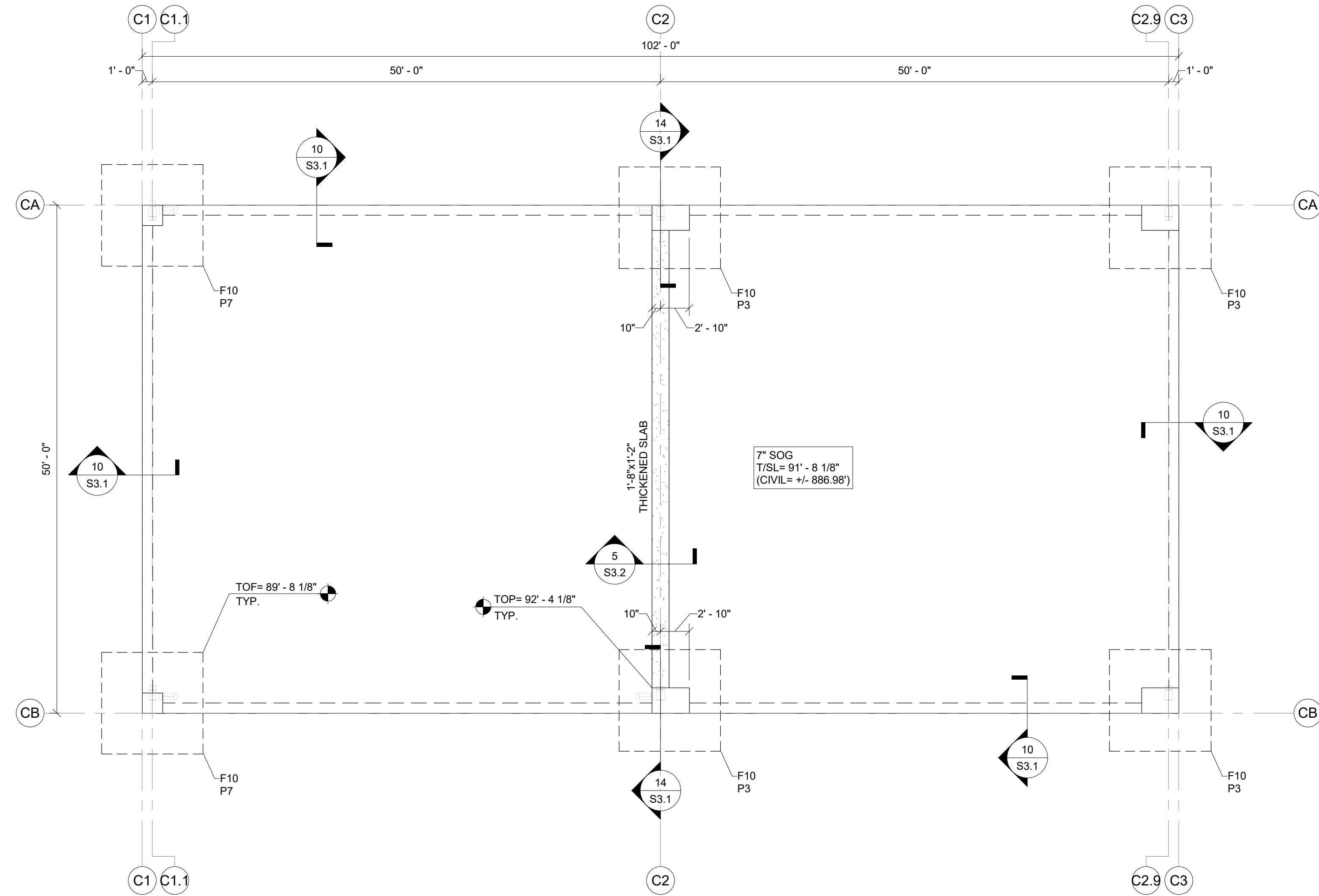
STRUCTURAL FOUNDATION PLAN - WAREHOUSE
SCALE: 1/8" = 1'-0"
NORTH



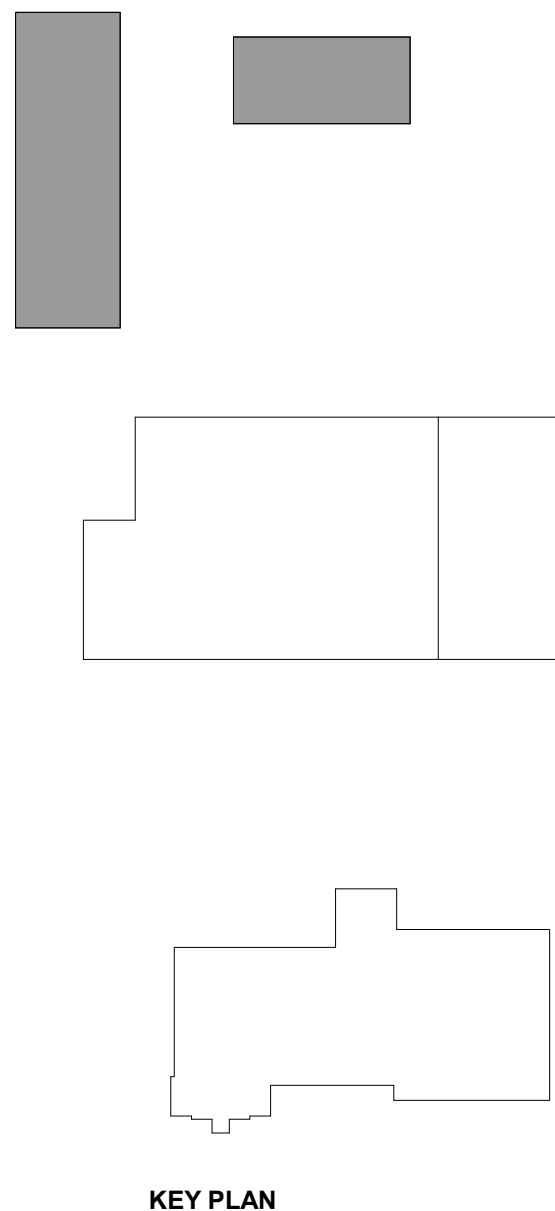
SSC JOB# 2025456



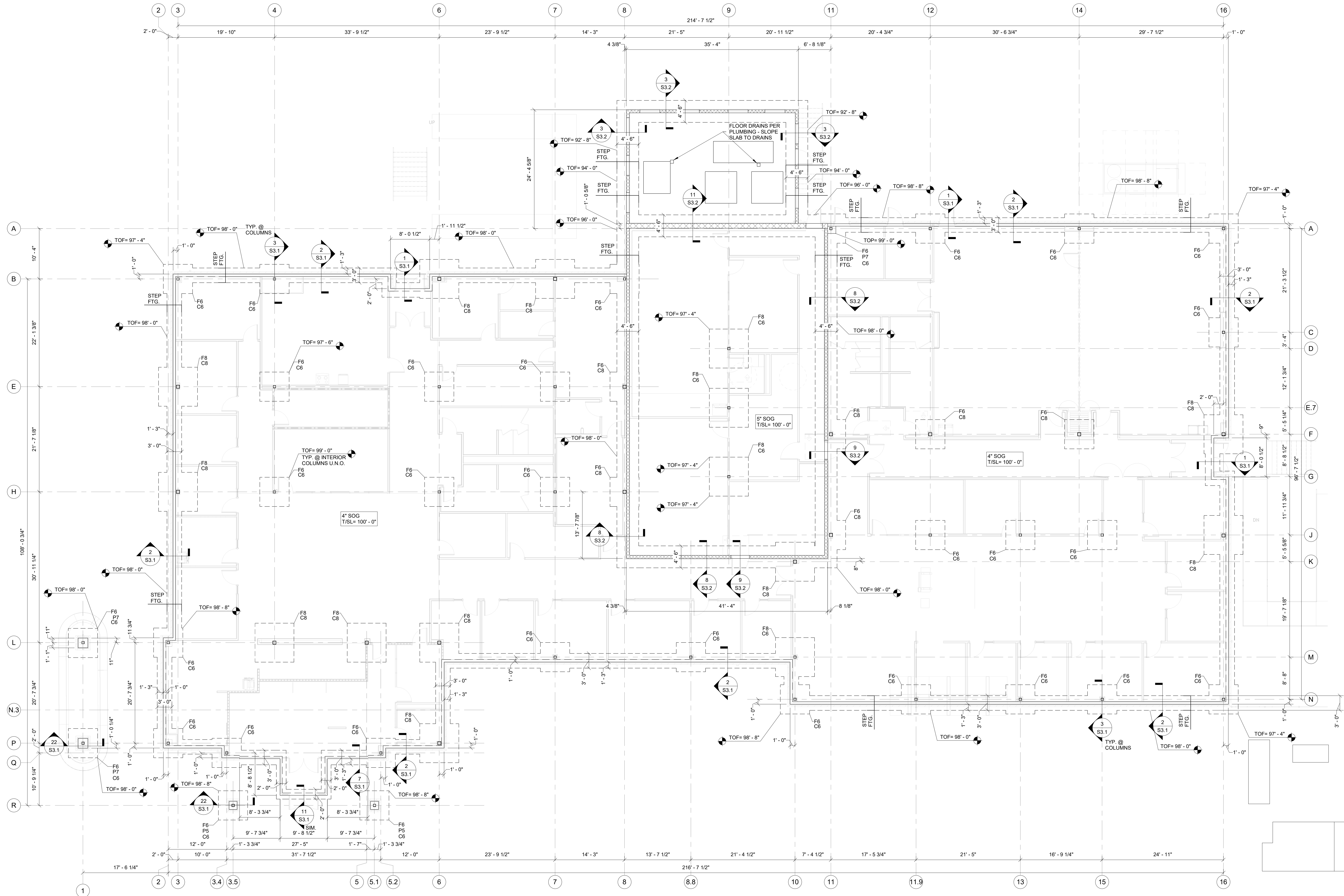
STRUCTURAL FOUNDATION PLAN - MAINTENANCE
SCALE: 1/8" = 1'-0"
NORTH



STRUCTURAL FOUNDATION PLAN - COVERED STORAGE
SCALE: 1/8" = 1'-0"
NORTH



KEY PLAN





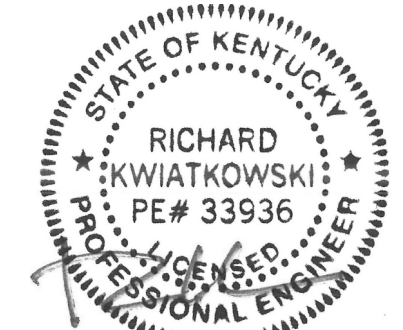
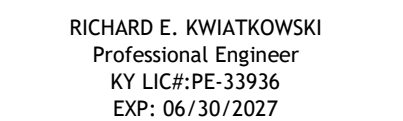
CONSULTANT:
SSC Engineering, Inc.
Mechanical • Electrical • Plumbing • Fire Protection • Structural



COOPERATIVE
BUILDING SOLUTIONS

TAYLOR COUNTY RECC

685 WATER TOWER BYPASS
CAMPBELLSVILLE, KY 42718



REVISION:

100% CONSTRUCTION DOCUMENTS	08/20/20
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Drawing Title:

S1.5



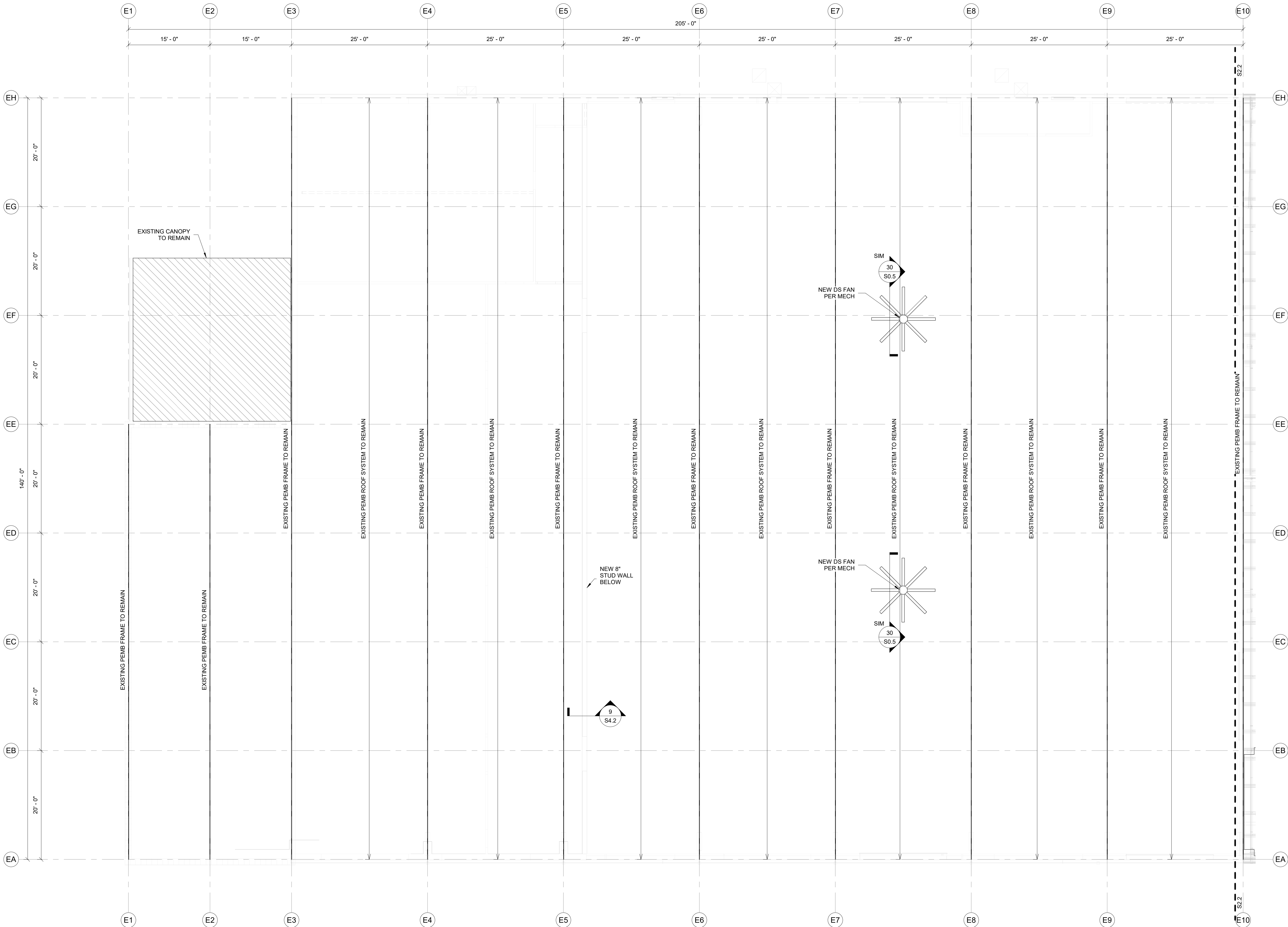
GENERAL

- ### STEEL FLOOR BEAMS

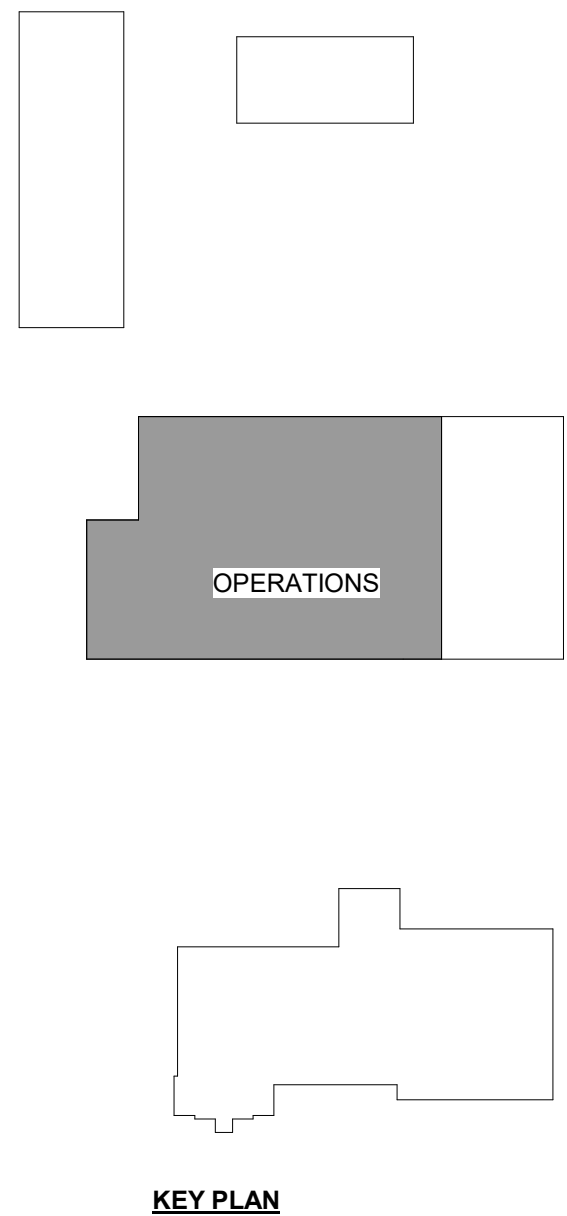
10. TIE/ELEVATION = BOTTOM OF SLAB ELEVATION, U.N.O..
11. SERVICE REACTIONS FOR STEEL BEAMS ARE SHOWN AS (12K) ON PLAN NEAR END OF BEAM. WHERE NOT NOTED, STEEL SUPPLIER TO PROVIDE CONNECTIONS PER STRUCTURAL GENERAL NOTES.
12. ADDITIONAL TENSION CONNECTION DESIGN LOADS INDICATED [10K] ON PLAN.
13. SEE DETAIL 6/SO 6 FOR STEEL BEAM BEARING PLATES AT HARDENED AREA MASONRY WALLS.
14. SEE DETAIL 12/SO 6 FOR STEEL BEAM BEARING PLATES AT MEZZANINE MASONRY WALLS.
15. AT BEARING WALLS LOCATE MECHANICAL OPENINGS MINIMUM 16 INCHES FROM BEAM BEARING LOCATIONS. SEE MECHANICAL DRAWINGS FOR OPENING LOCATIONS NOT SHOWN.
16. ► DENOTES MOMENTY LINE CONNECTION (CONNECTION DESIGN BY FABRICATOR)
17. REFER TO MASONRY LINTEL (ML/L) SCHEDULES ON SHEET 50.6.



SSC JOB# 2025456



STRUCTURAL ROOF FRAMING PLAN - OPERATIONS BUILDING
SCALE: 1/8" = 1'-0"
NORTH



PEMB ROOF FRAMING PLAN NOTES:
1. GENERAL
A. REF. SHEET S0.1 - S0.2 FOR STRUCTURAL GENERAL NOTES.
B. REFER TO THE ARCHITECTURAL DRAWINGS AND/OR COORDINATE WITH THE ARCHITECT REGARDING MISSING DIMENSIONS.

M+H
ARCHITECTS
ARCHITECT:
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Michael K. Swim Consulting Engineer, Inc.
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636-530-7770
Corporate License No.: 001244

COOPERATIVE
BUILDING SOLUTIONS

TAYLOR COUNTY RECC
685 WATER TOWER BYPASS
CAMBELLSVILLE, KY 42718

RECC
A Taylor County Company

RICHARD E. KWIATKOWSKI
Professional Engineer
KY LIC# PE-33936
EXP: 06/30/2027
STATE OF KENTUCKY
RICHARD E. KWIATKOWSKI
PE# 33936
Professional Engineer

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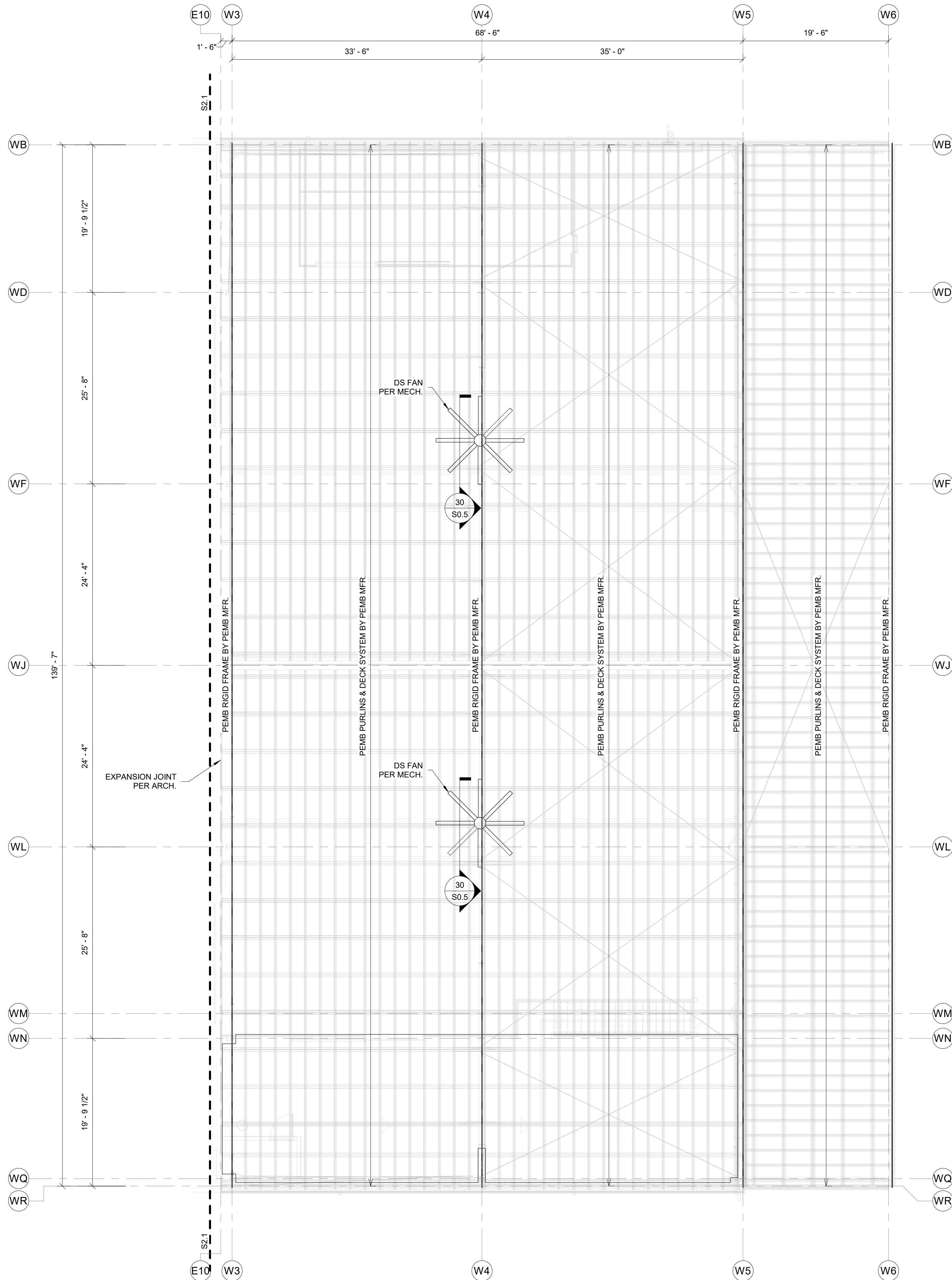
REVISION:
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Sheet Issue Date: 08/20/2026
Job Number: 250120.00
Drawn By: Author
Checked By: Checker

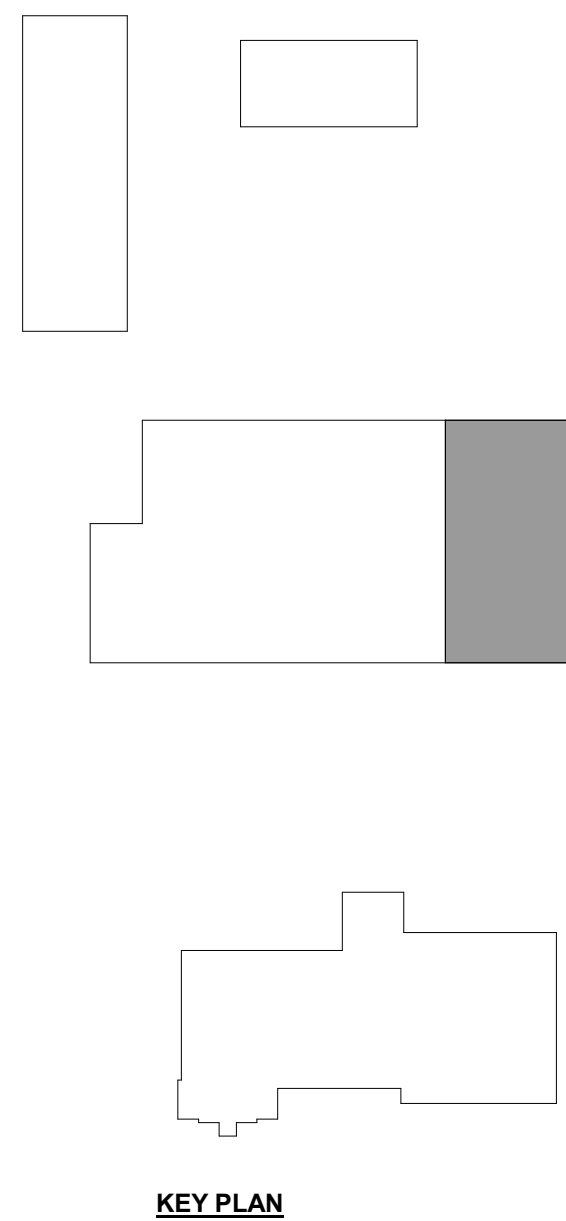
Drawings Title:
STRUCTURAL ROOF FRAMING PLAN - OPERATIONS BUILDING

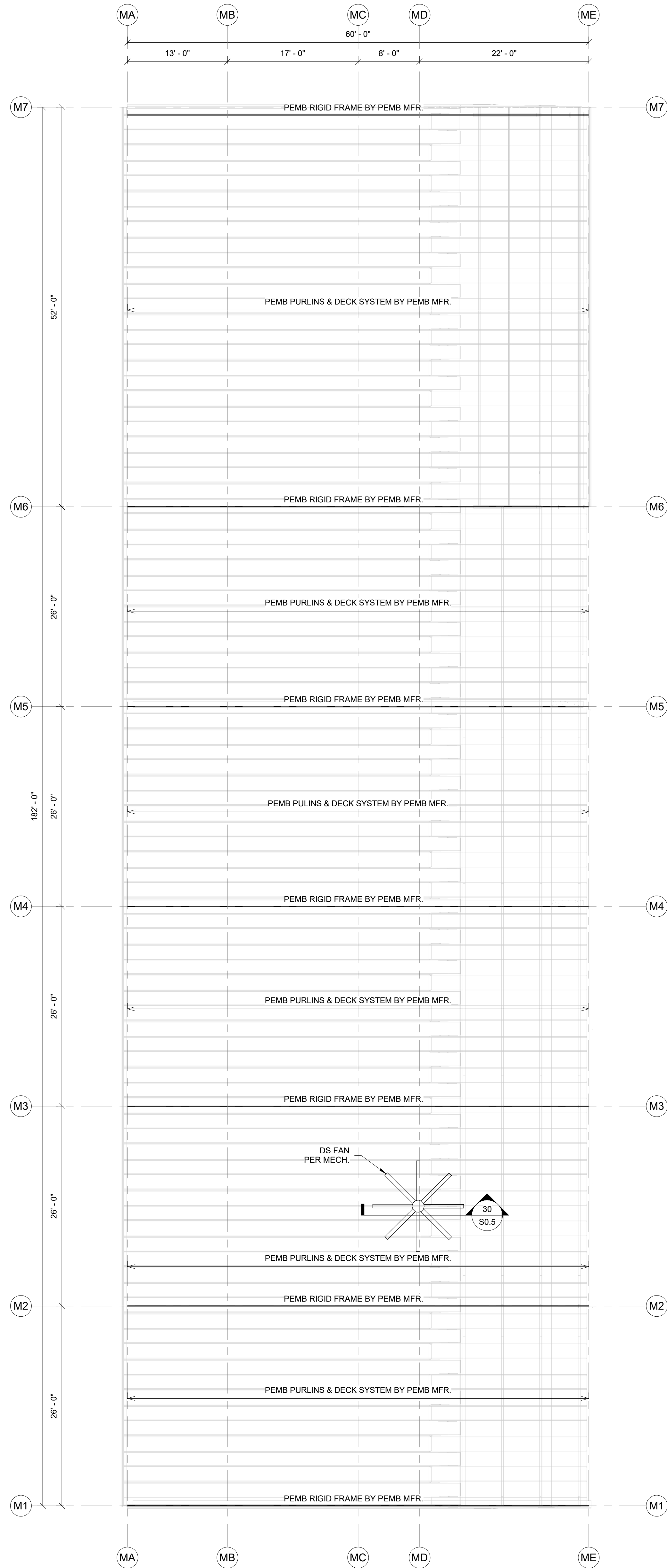
S2.1

SSC JOB# 2025456

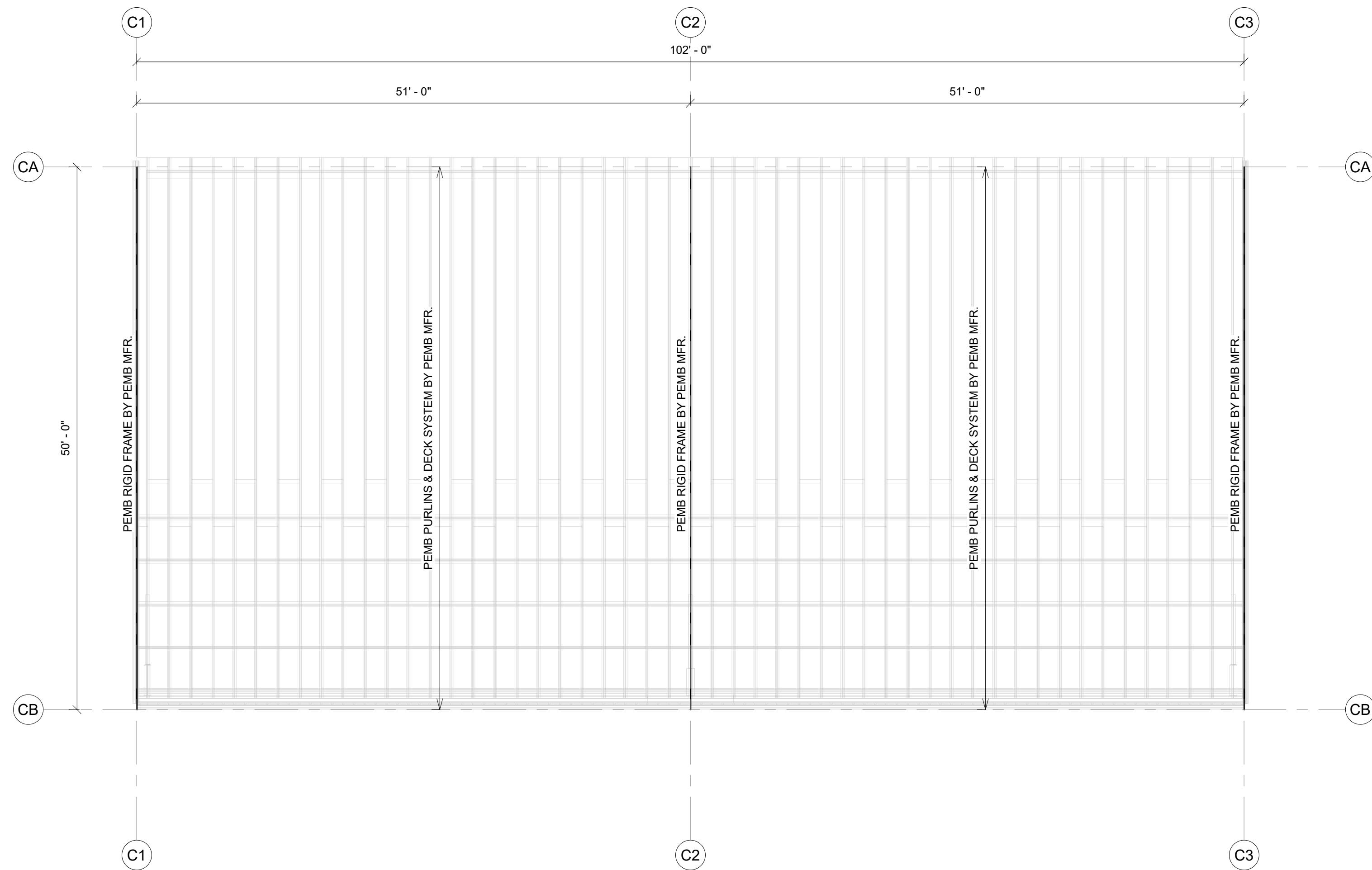


STRUCTURAL ROOF FRAMING PLAN - WAREHOUSE
SCALE: 1/8" = 1'-0"
NORTH

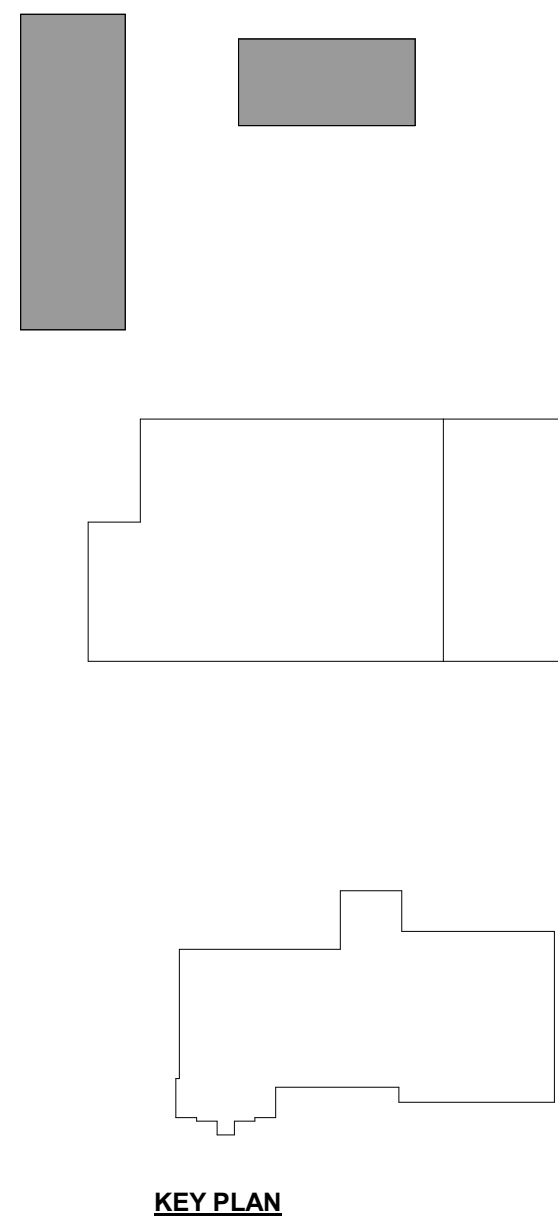




STRUCTURAL ROOF FRAMING PLAN - MAINTENANCE
SCALE: 1/8" = 1'-0"
NORTH



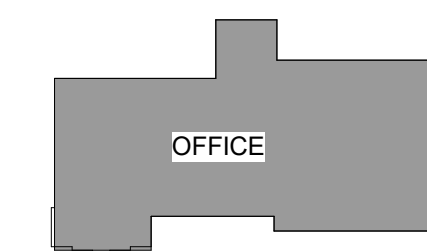
STRUCTURAL ROOF FRAMING PLAN - COVERED STORAGE
SCALE: 1/8" = 1'-0"
NORTH



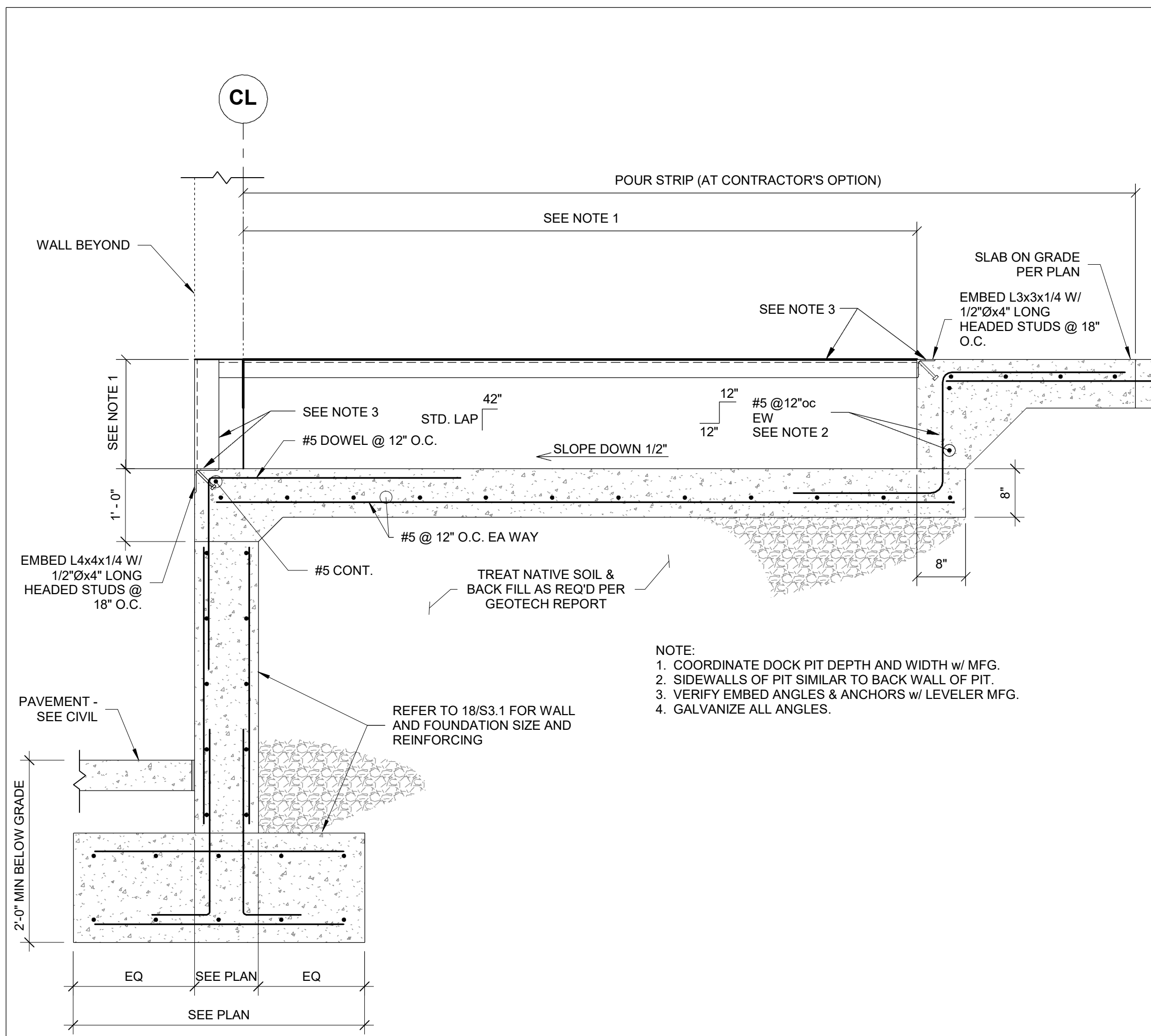
KEY PLAN

2. STEEL ROOF BEAMS

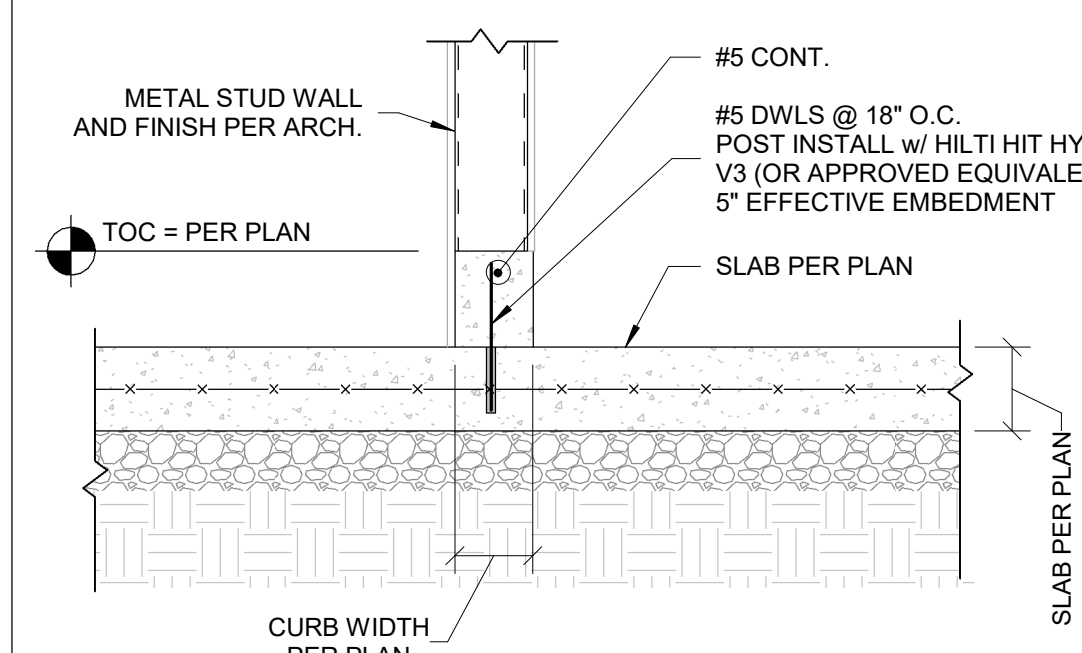
- A. TOP OF STEEL (TOS) ELEVATION: = 113' - 4" U.N.O.
- B. SERVICE REQUIREMENTS FOR STEEL BEAMS ARE SHOWN ON PLAN NEAR END OF BEAM. WHERE NOT NOTED, STEEL SUPPLIER TO PROVIDE CONNECTIONS PER STEEL GENERAL NOTES.
- C. [ADDITIONAL TENSION CONNECTION DESIGN LOADS INDICATED ON PLAN.]
- D. AT BEARING WALLS LOCATE MECHANICAL OPENINGS MINIMUM 16 INCHES FROM BEAM BEARING LOCATION. SEE MECHANICAL DRAWINGS FOR OPENING LOCATIONS NOT SHOWN.



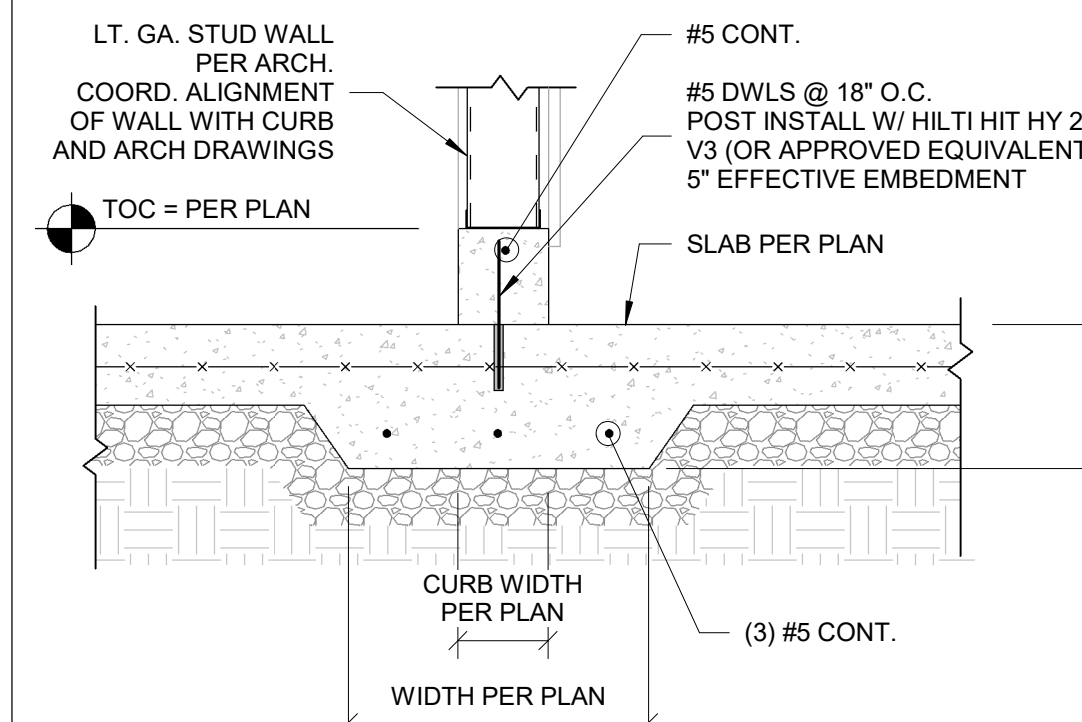
 **STRUCTURAL ROOF FRAMING PLAN - OFFICE**
SCALE: 1/8" = 1'-0"



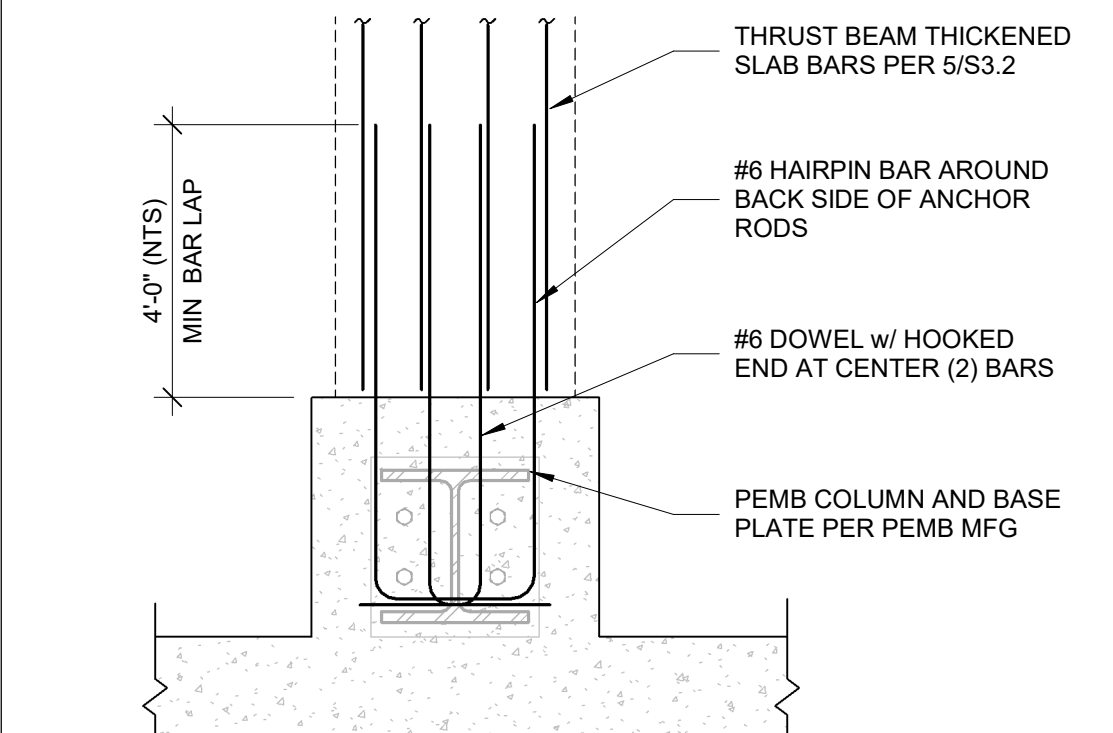
29 FND SECTION @ LOADING DOCK
SCALE: 3/4" = 1'-0"



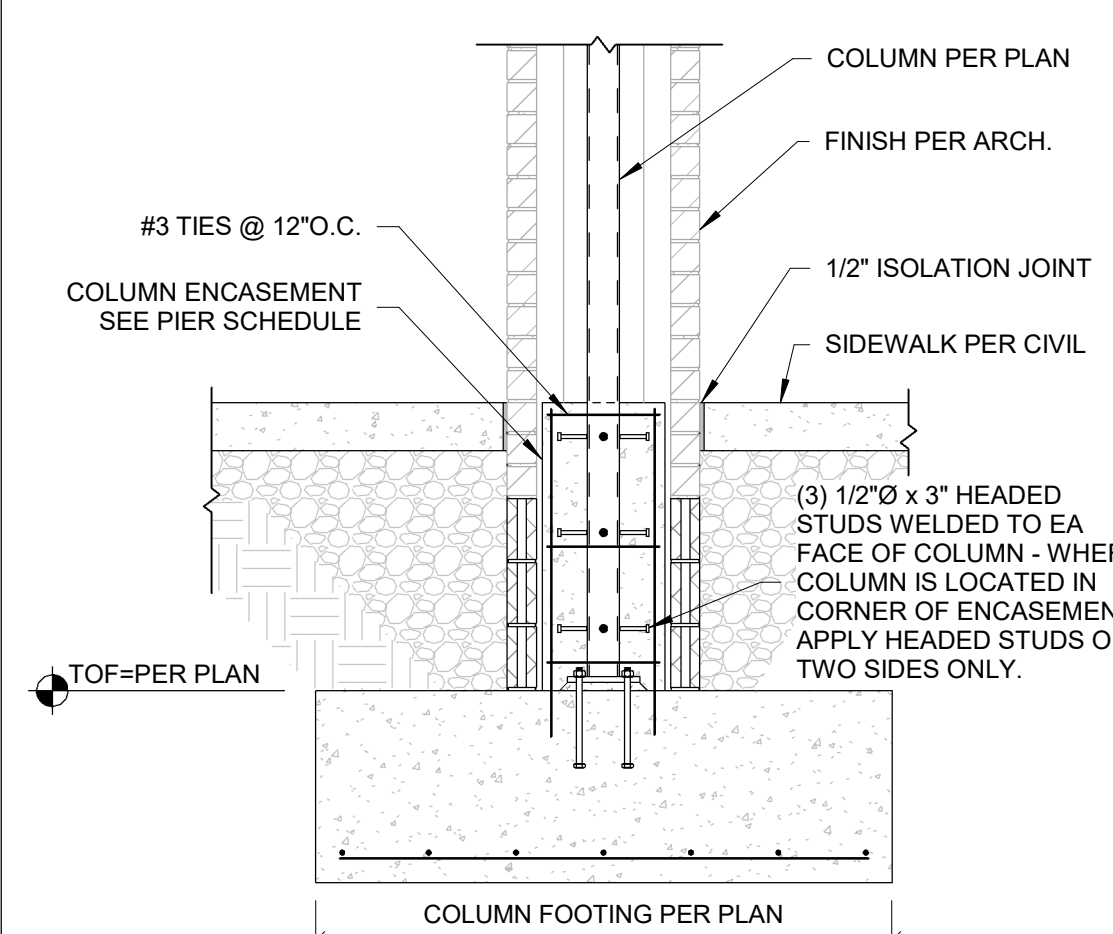
28 WALL ON INTERIOR CURB DETAIL
SCALE: 3/4" = 1'-0"



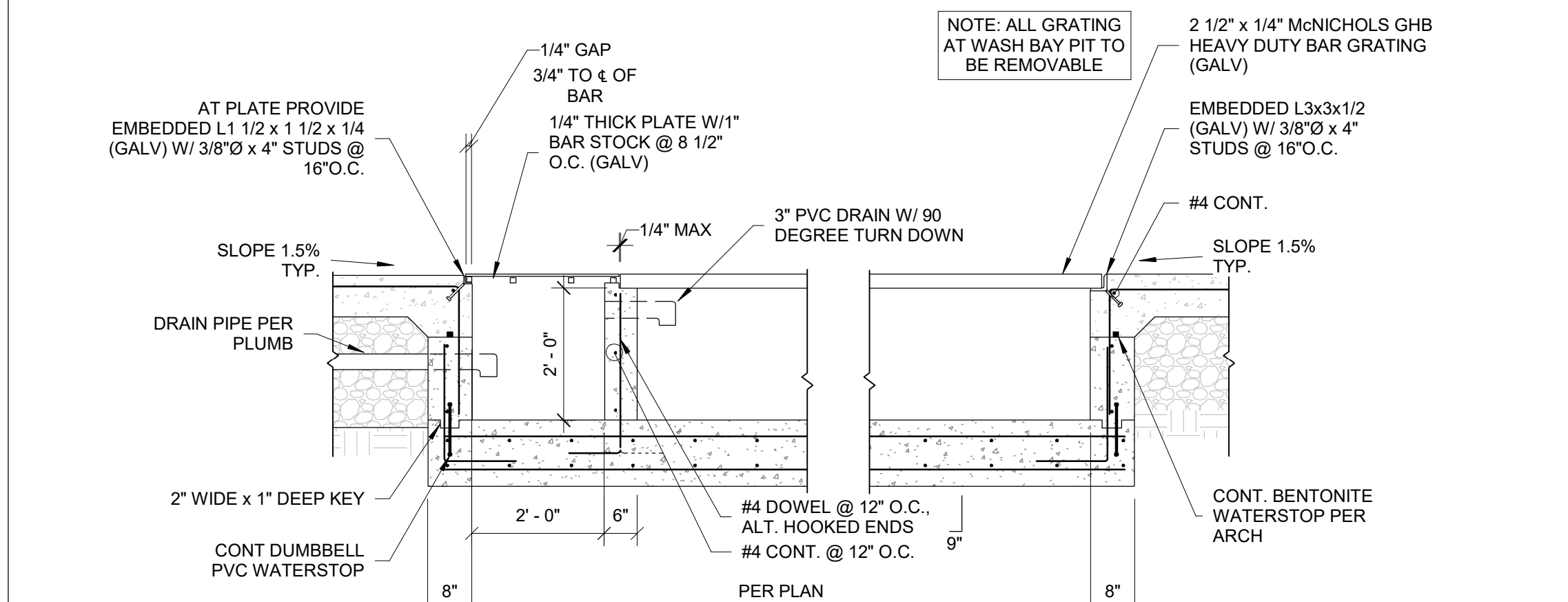
27 STUD WALL ON INTERIOR CURB DETAIL
SCALE: 3/4" = 1'-0"



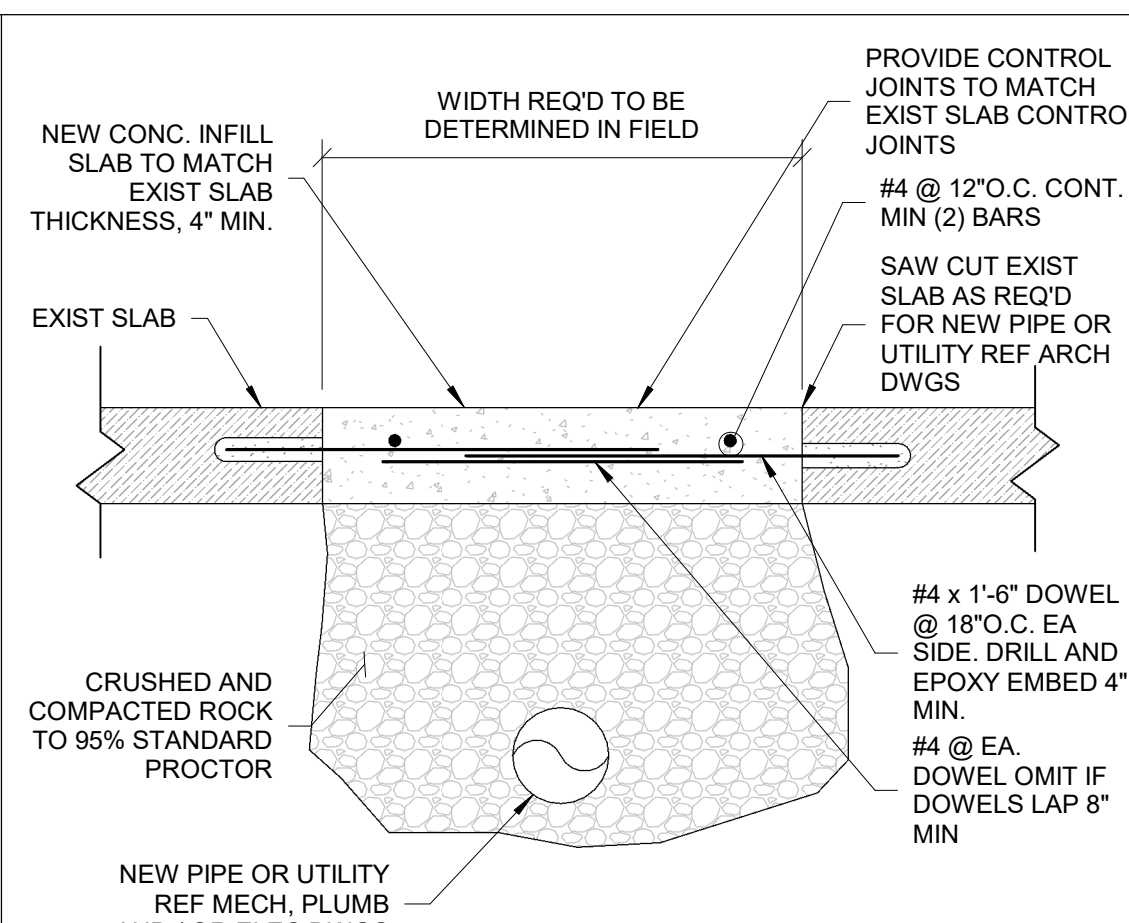
23 PLAN DETAIL @ HAIRPINS
SCALE: 3/4" = 1'-0"



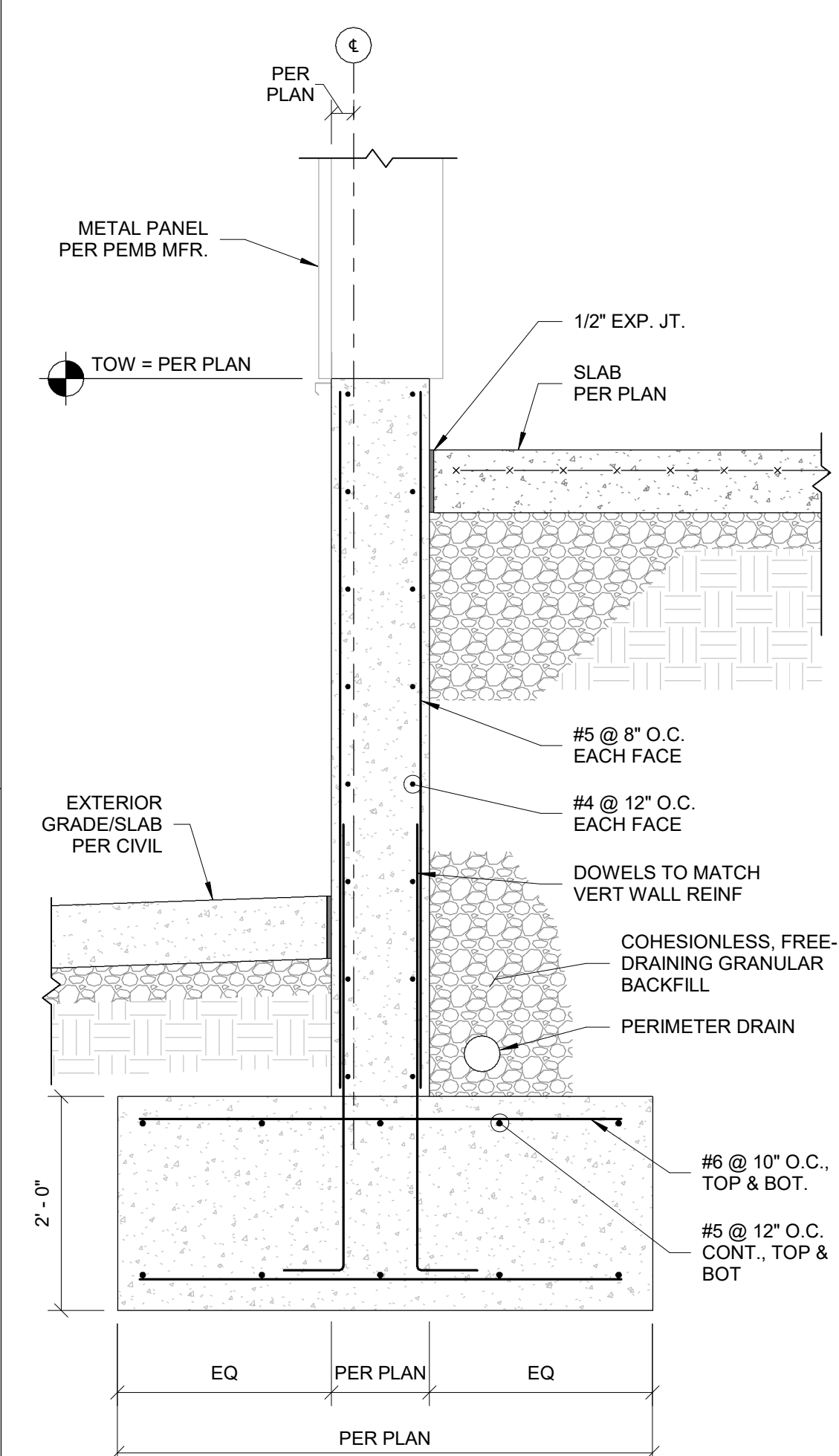
22 SECTION @ HSS CANOPY COLUMN
SCALE: 1/2" = 1'-0"



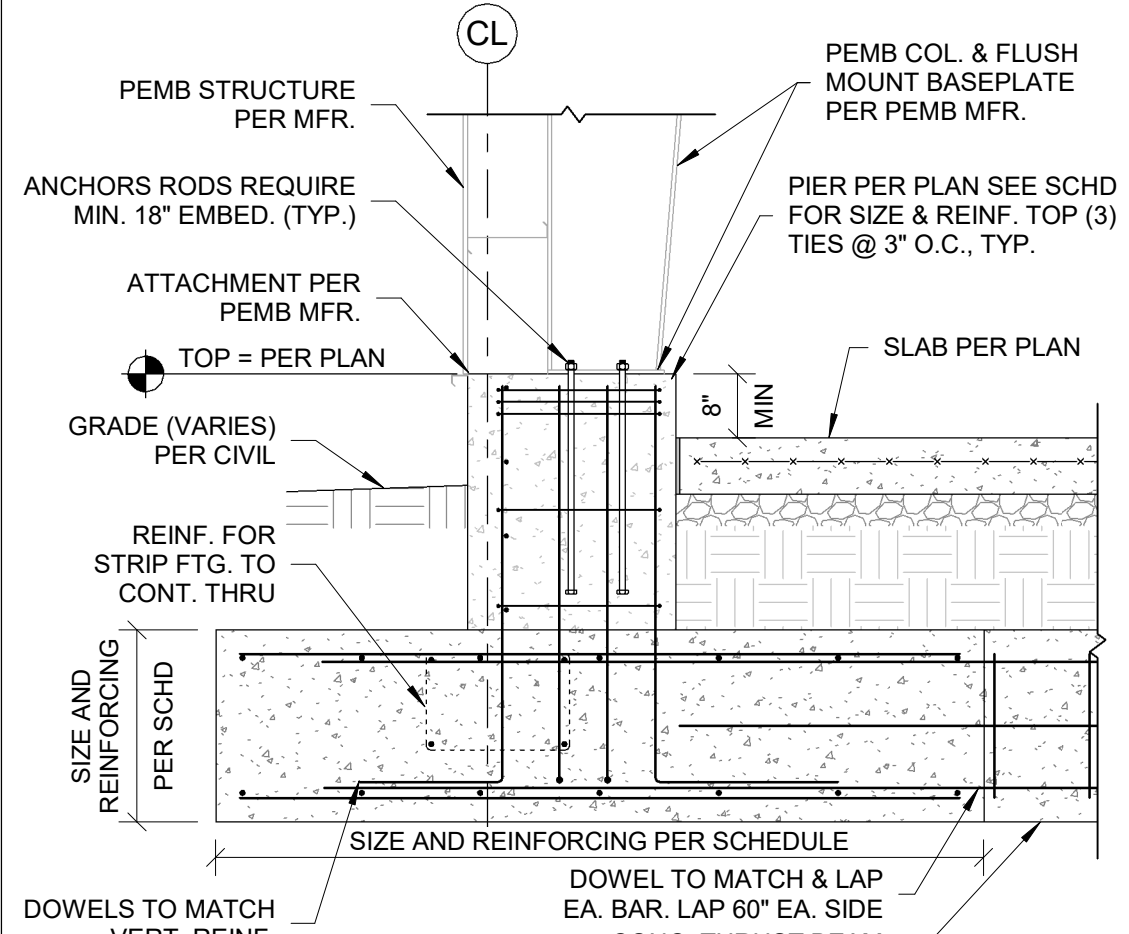
26 SECTION @ WASH BAY PIT
SCALE: 1/2" = 1'-0"



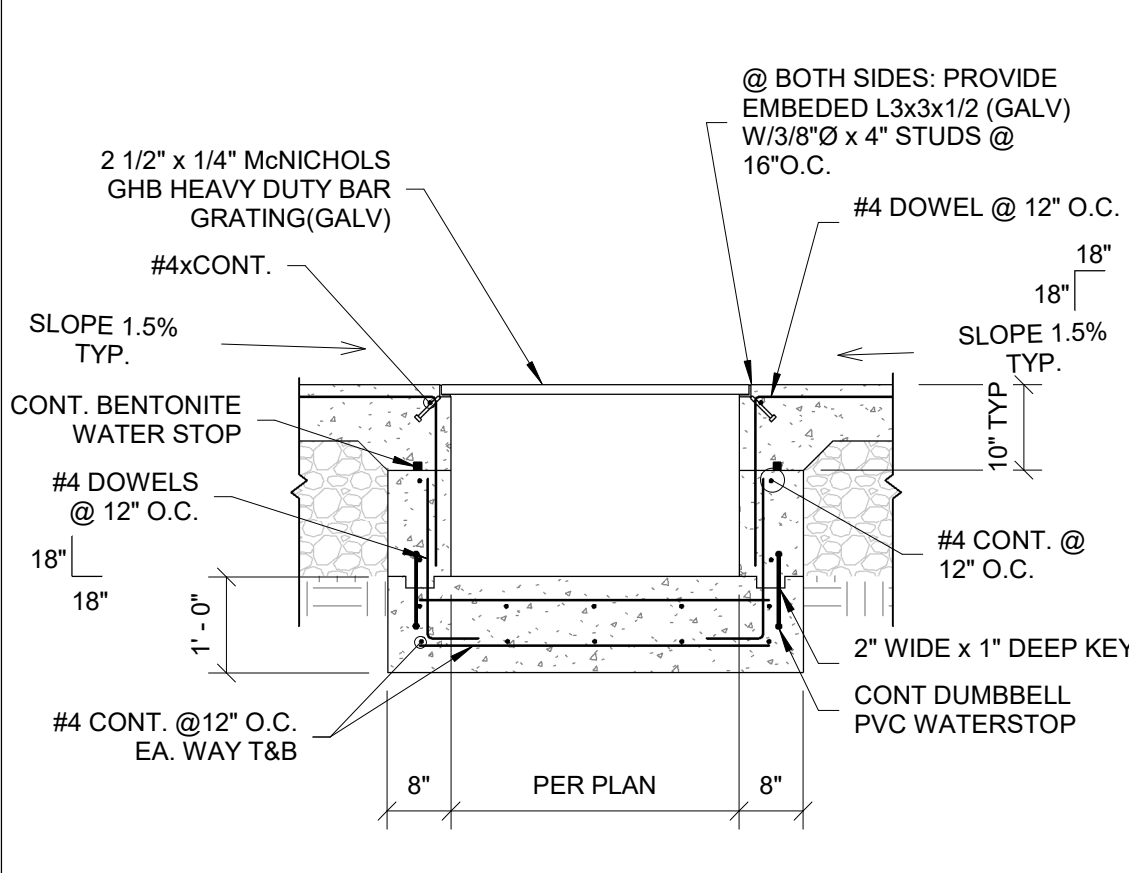
20 SLAB REPAIR AT NEW PIPE LOCATIONS
NOT TO SCALE



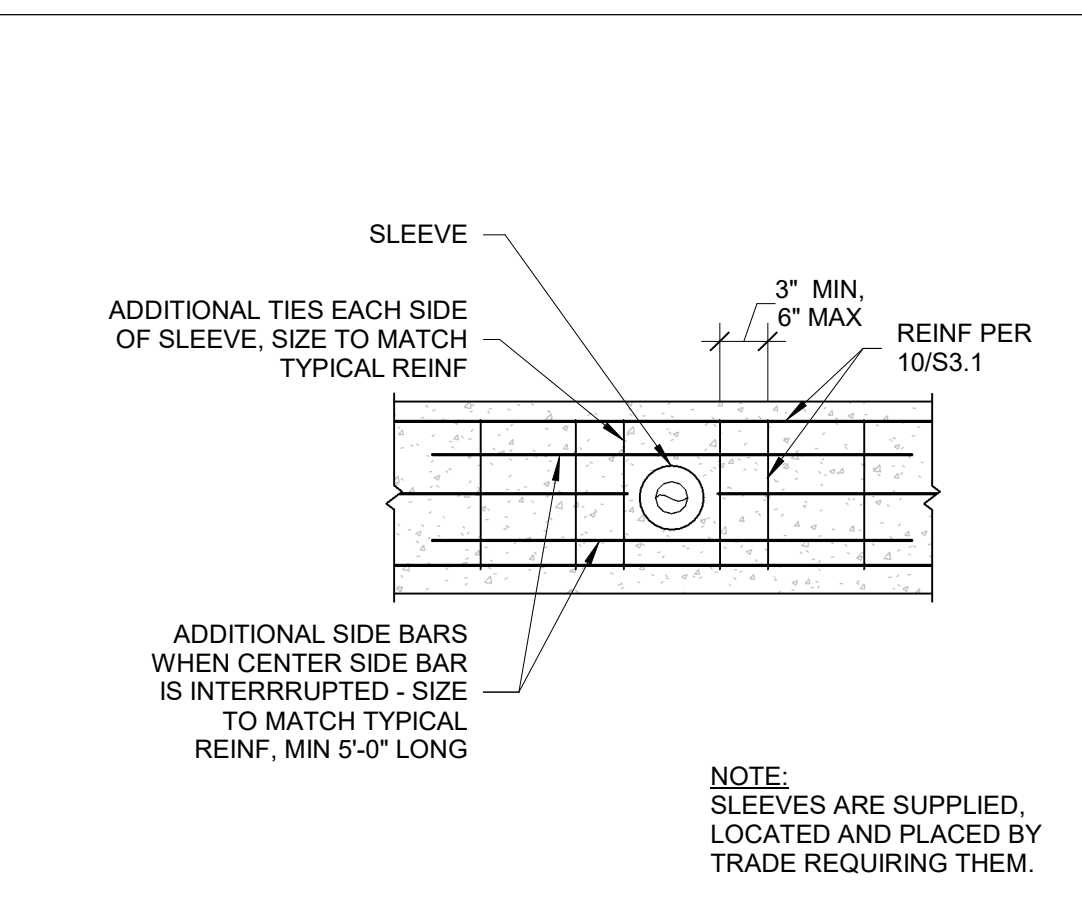
18 PEMB WALL ON RETAINING WALL
SCALE: 3/4" = 1'-0"



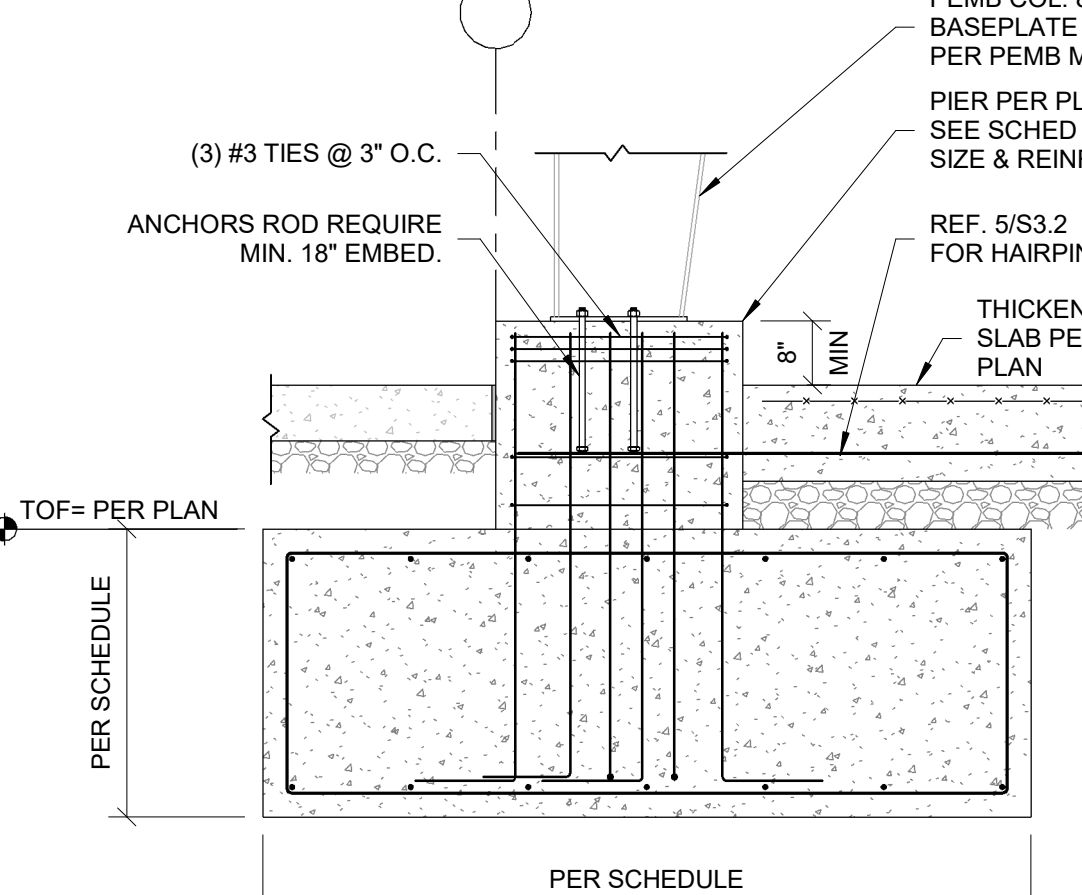
17 SECTION @ PEMB COL FTG
SCALE: 1/2" = 1'-0"



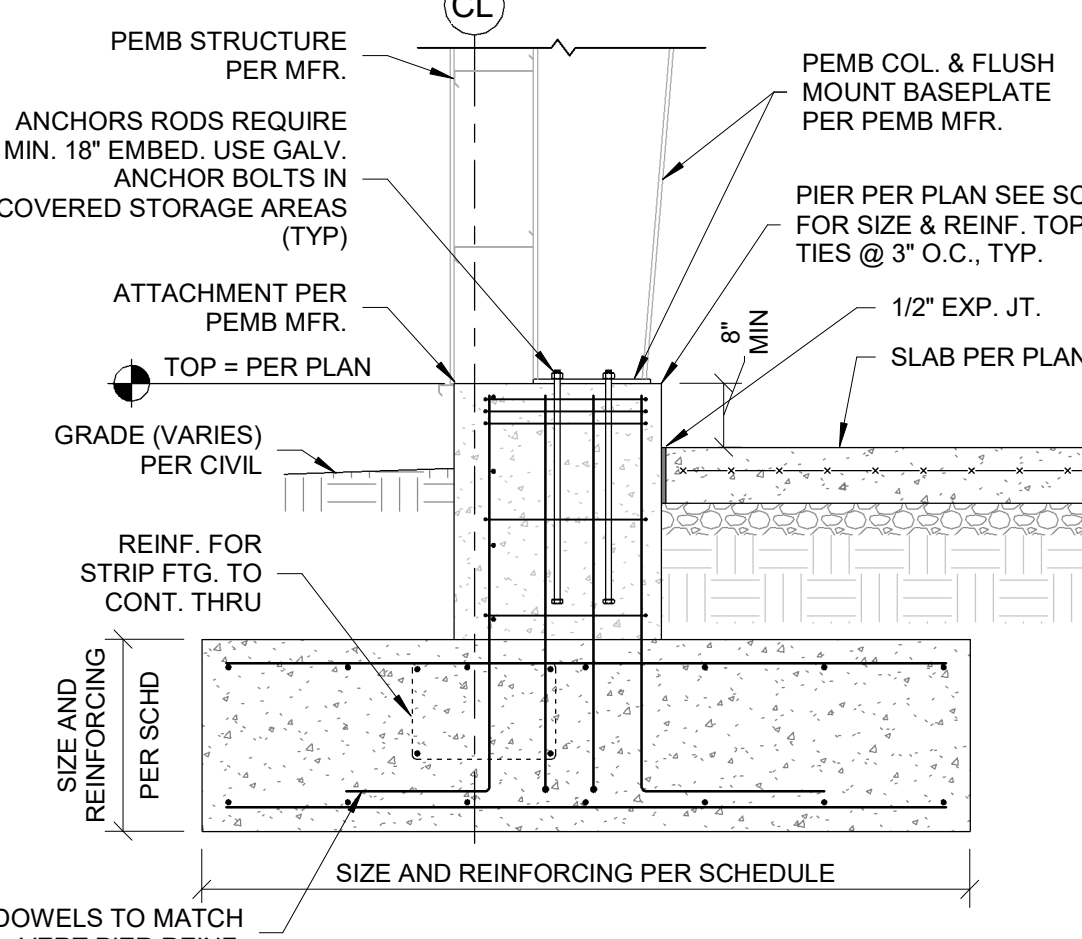
16 SECTION @ WASH BAY PIT
SCALE: 1/2" = 1'-0"



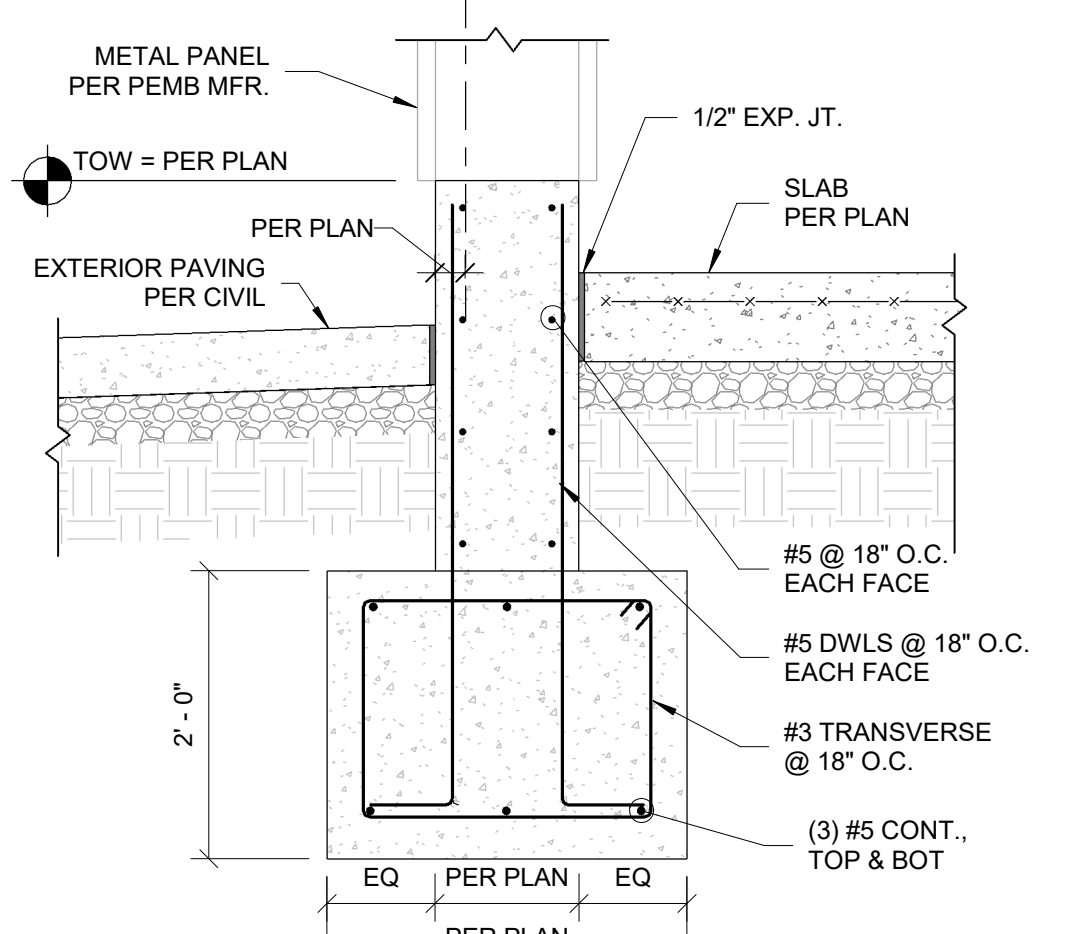
15 GRADE BEAM REINFORCEMENT @ SLEEVE
SCALE: 1/2" = 1'-0"



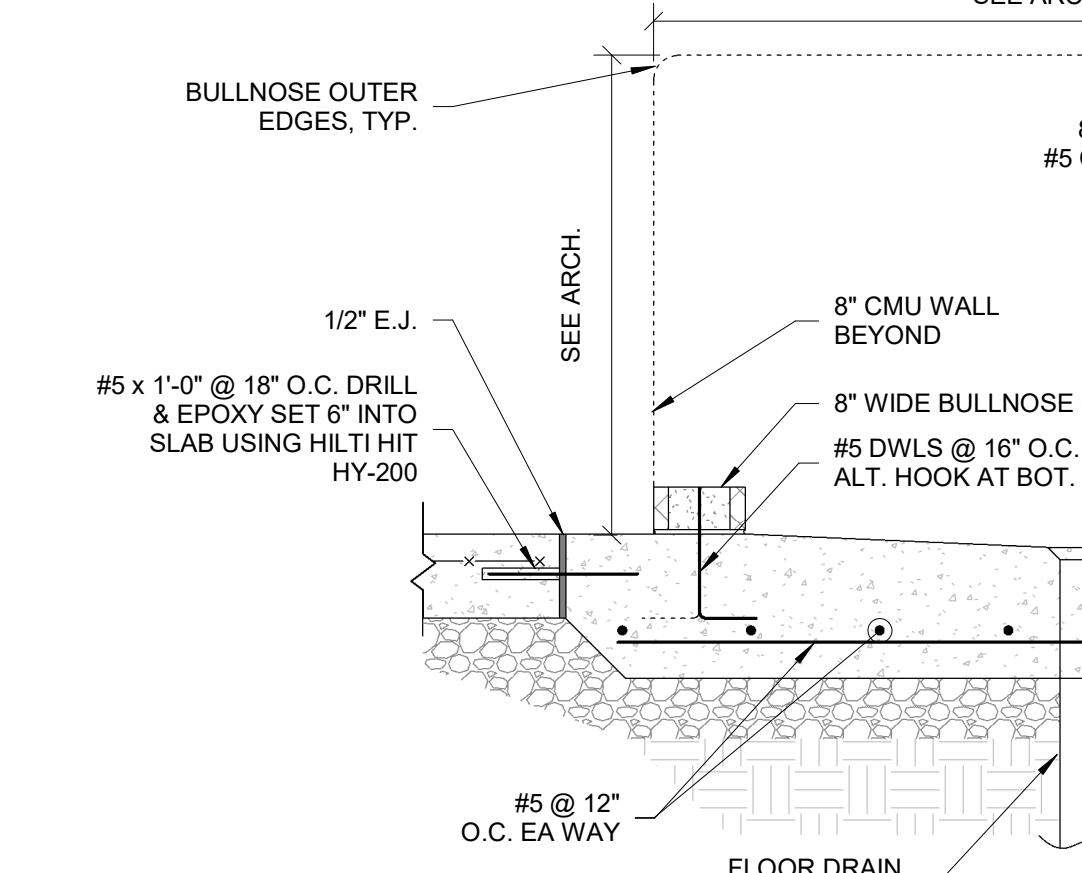
14 FND SECTION @ COVERED STORAGE
SCALE: 1/2" = 1'-0"



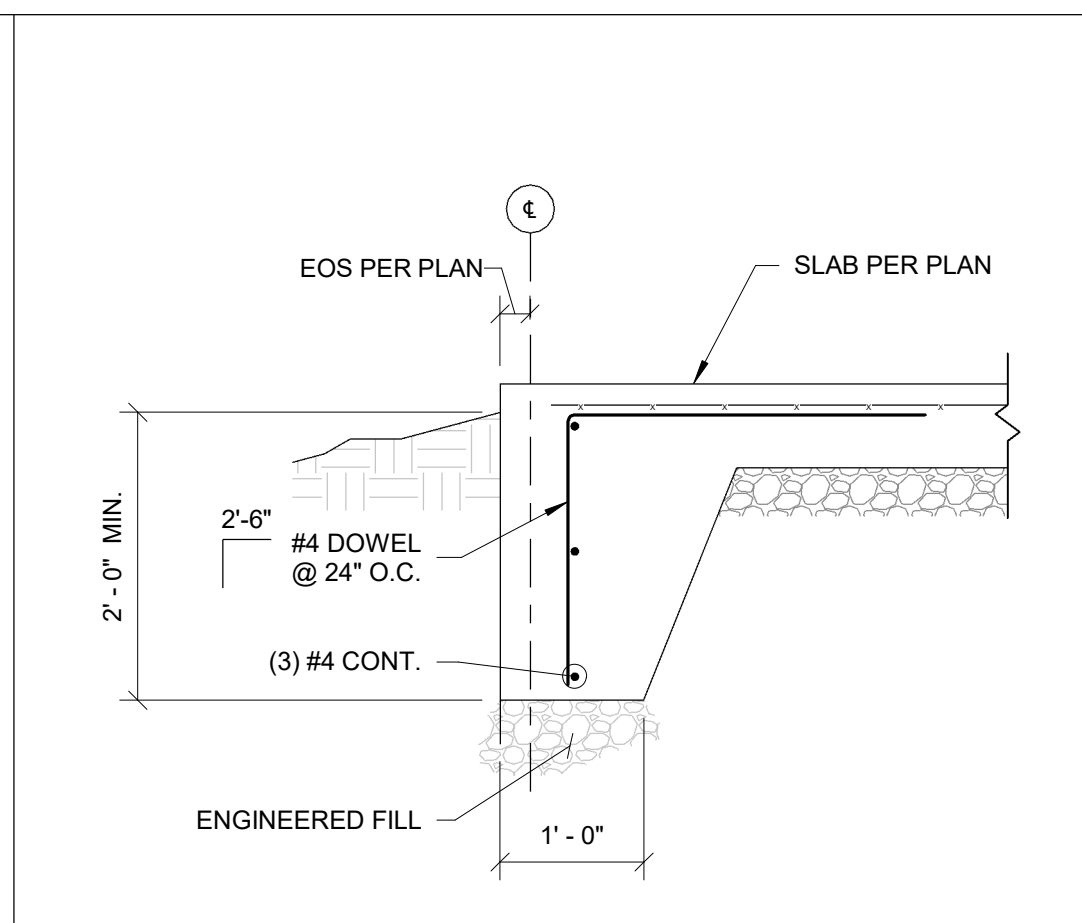
13 SECTION @ PEMB COL FTG
SCALE: 1/2" = 1'-0"



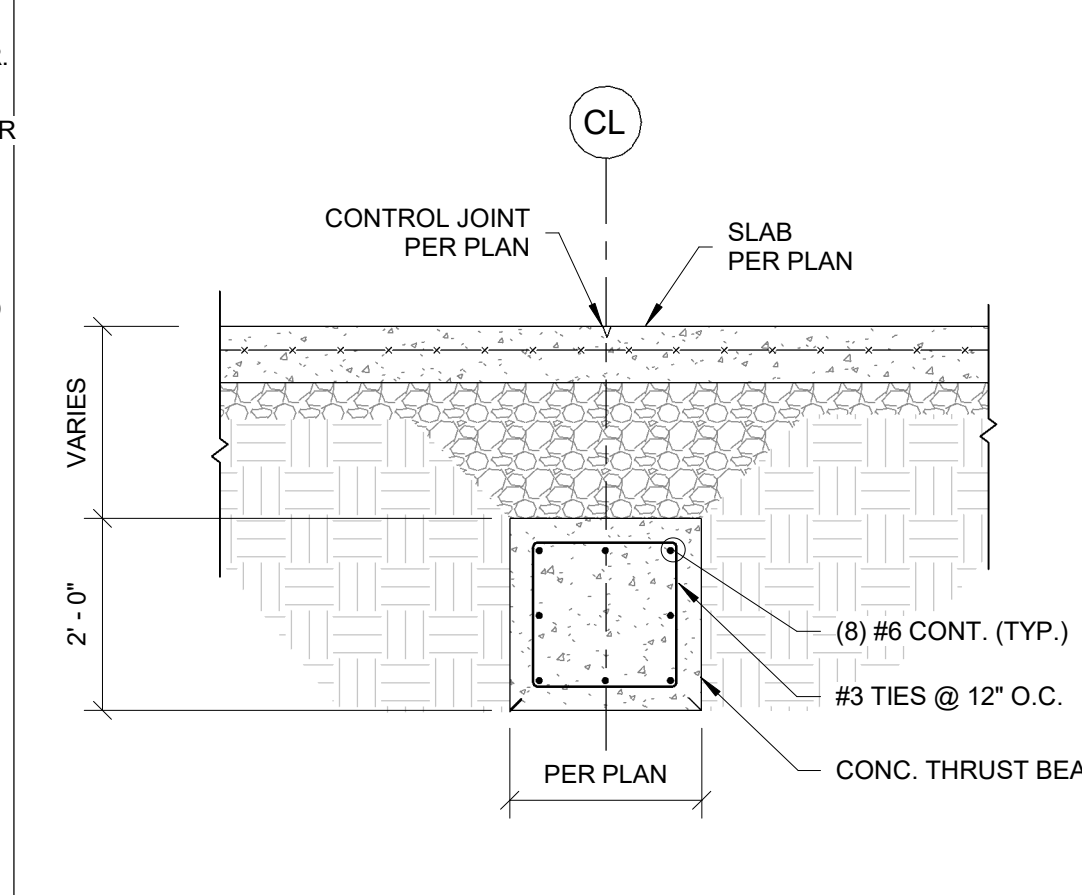
12 PEMB WALL ON CURB DETAIL
SCALE: 3/4" = 1'-0"



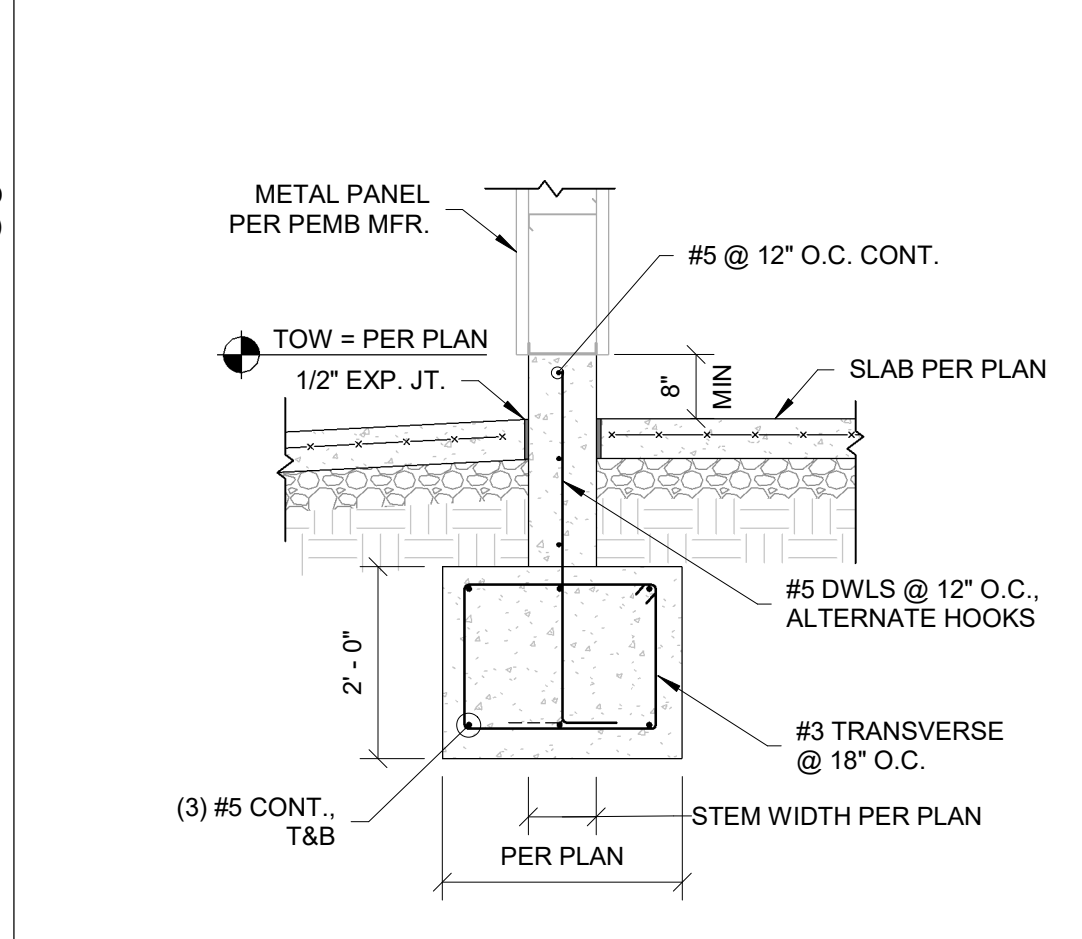
11 SECTION @ BOOT WASH
SCALE: 3/4" = 1'-0"



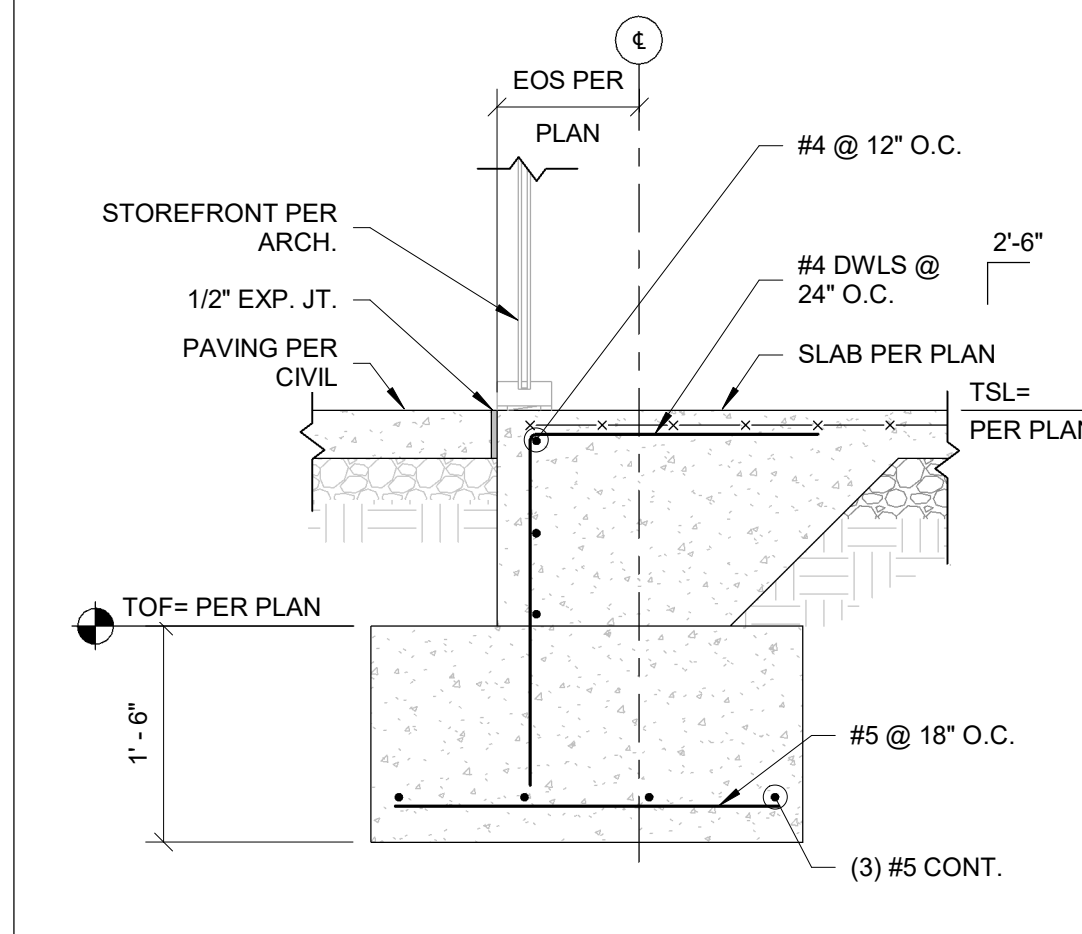
10 TURN DOWN SLAB EDGE DETAIL
SCALE: 3/4" = 1'-0"



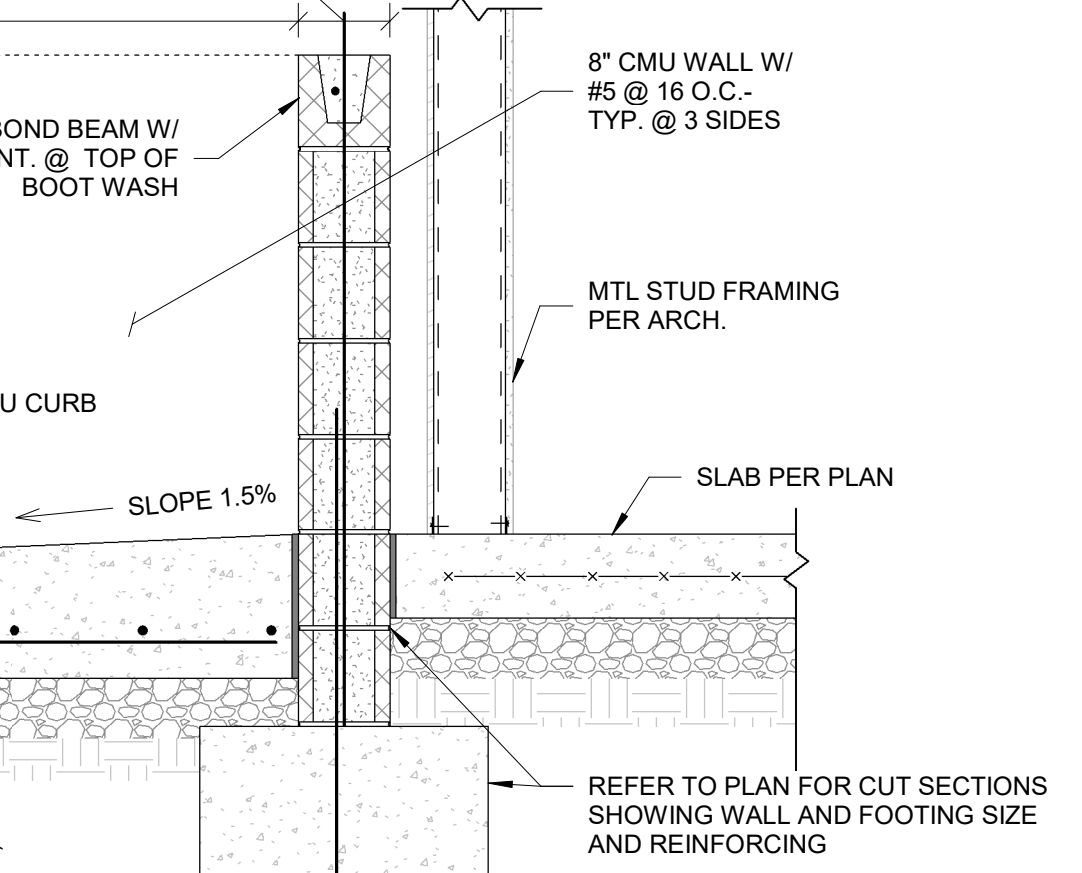
9 SECTION @ THRUST BEAM
SCALE: 1/2" = 1'-0"



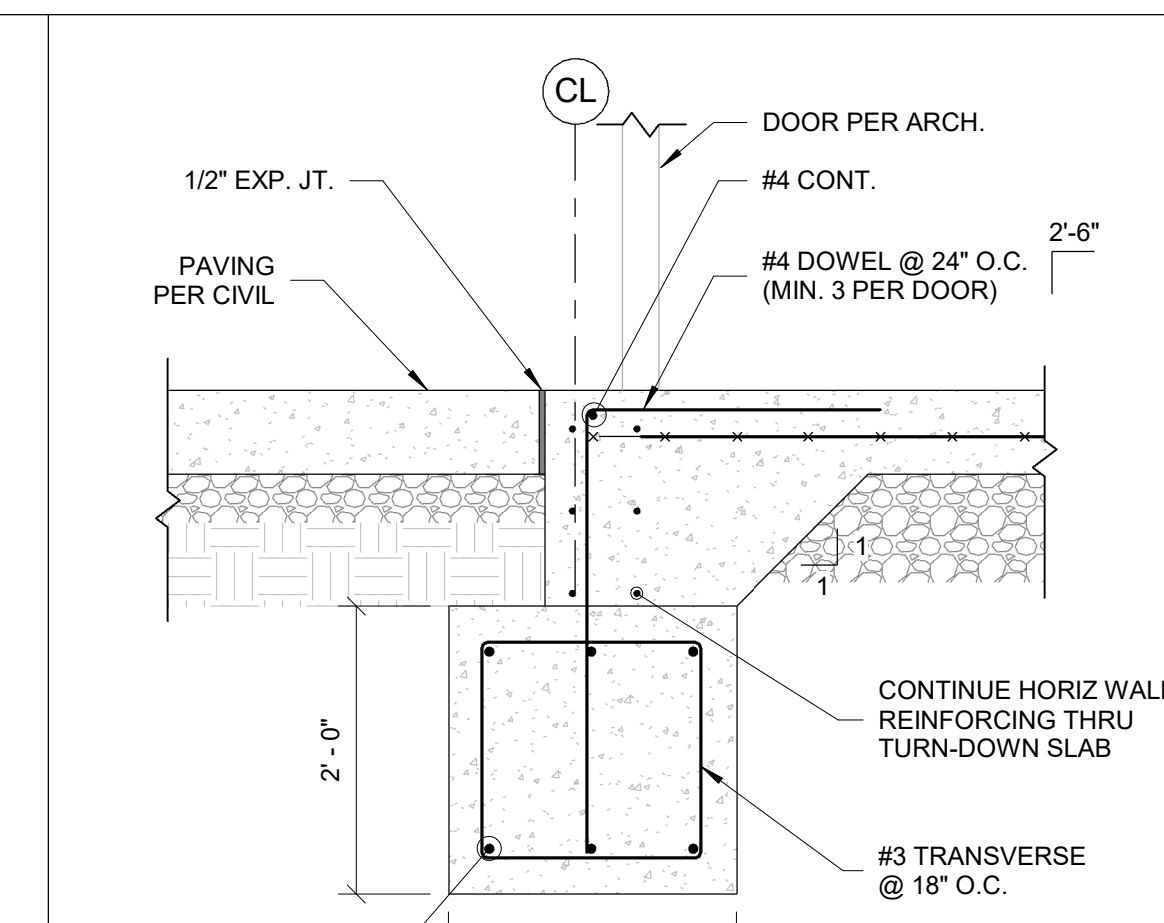
8 SECTION @ INTERIOR PEMB WALL
SCALE: 1/2" = 1'-0"



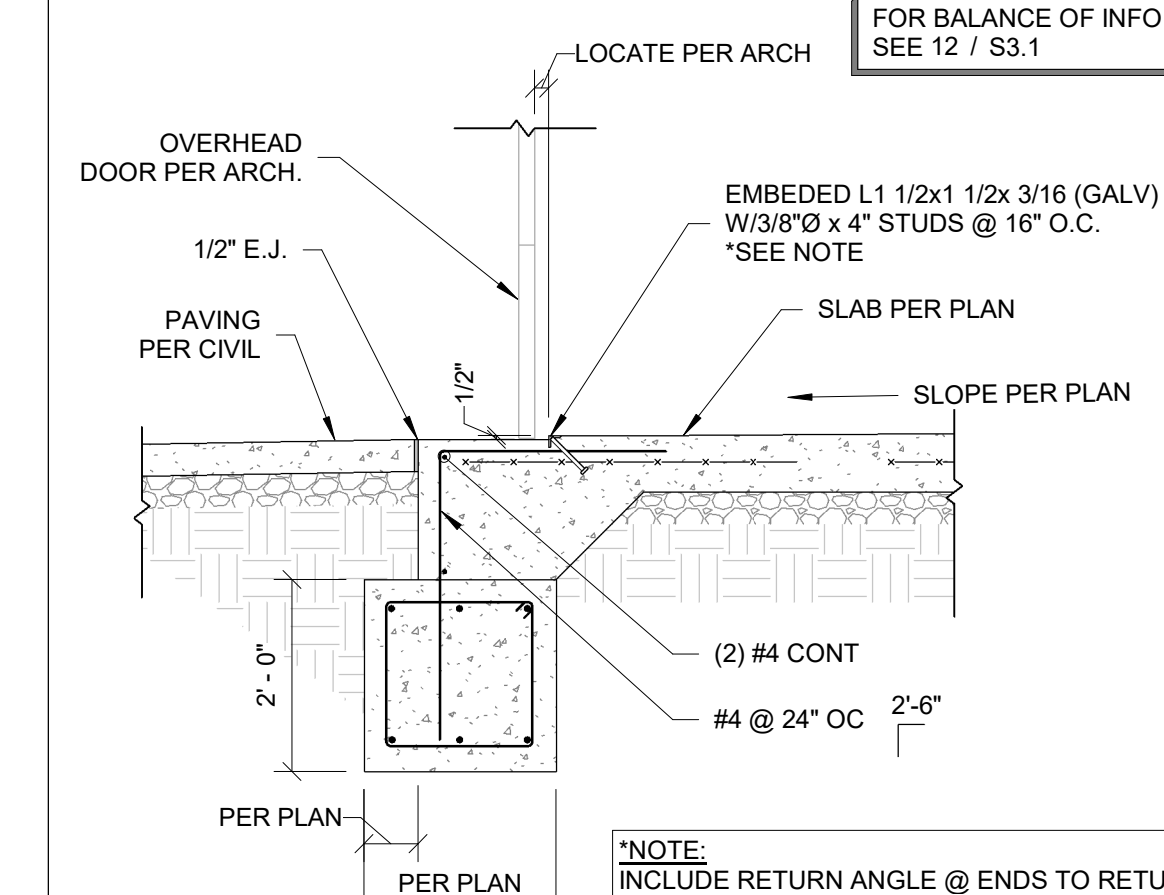
7 TYP. FND SECTION @ STOREFRONT - OFFICE
SCALE: 3/4" = 1'-0"



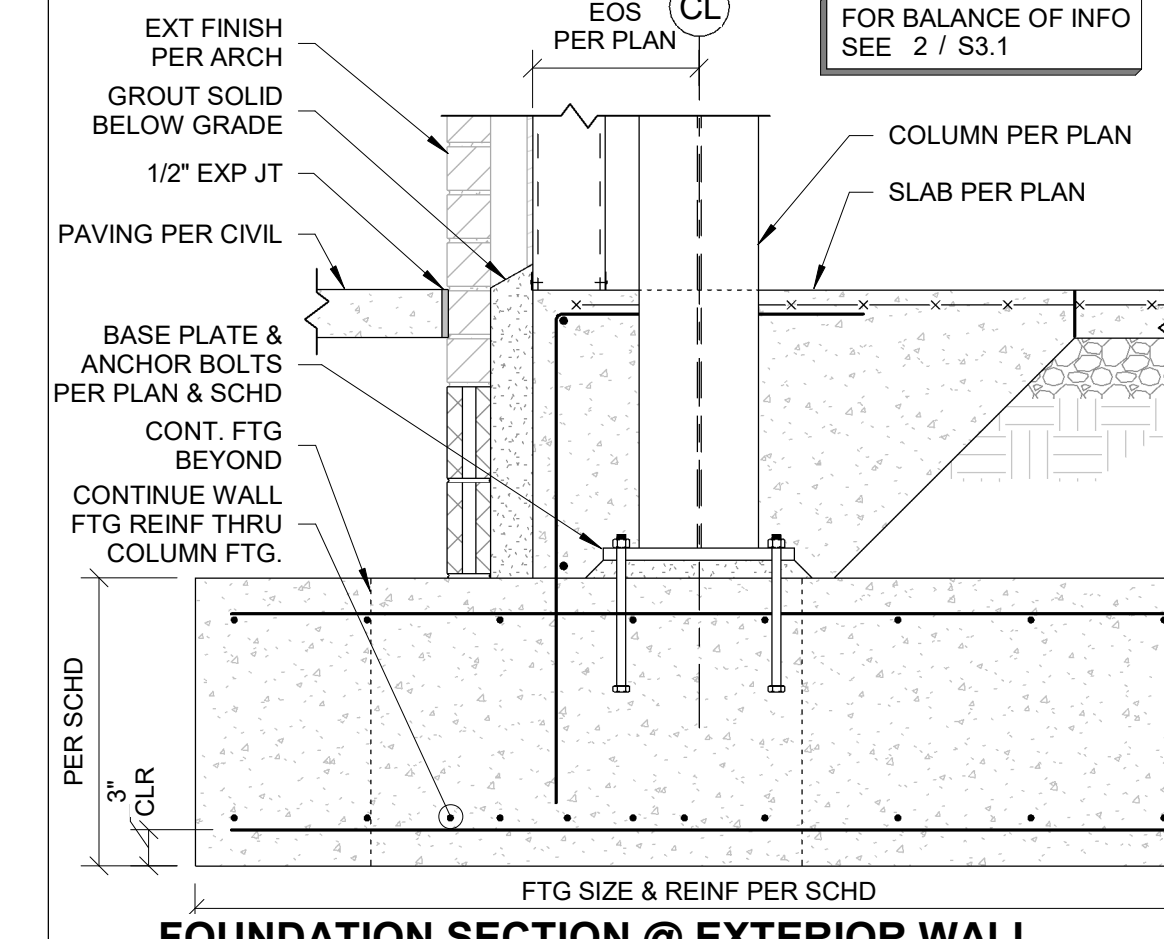
11 SECTION @ BOOT WASH
SCALE: 3/4" = 1'-0"



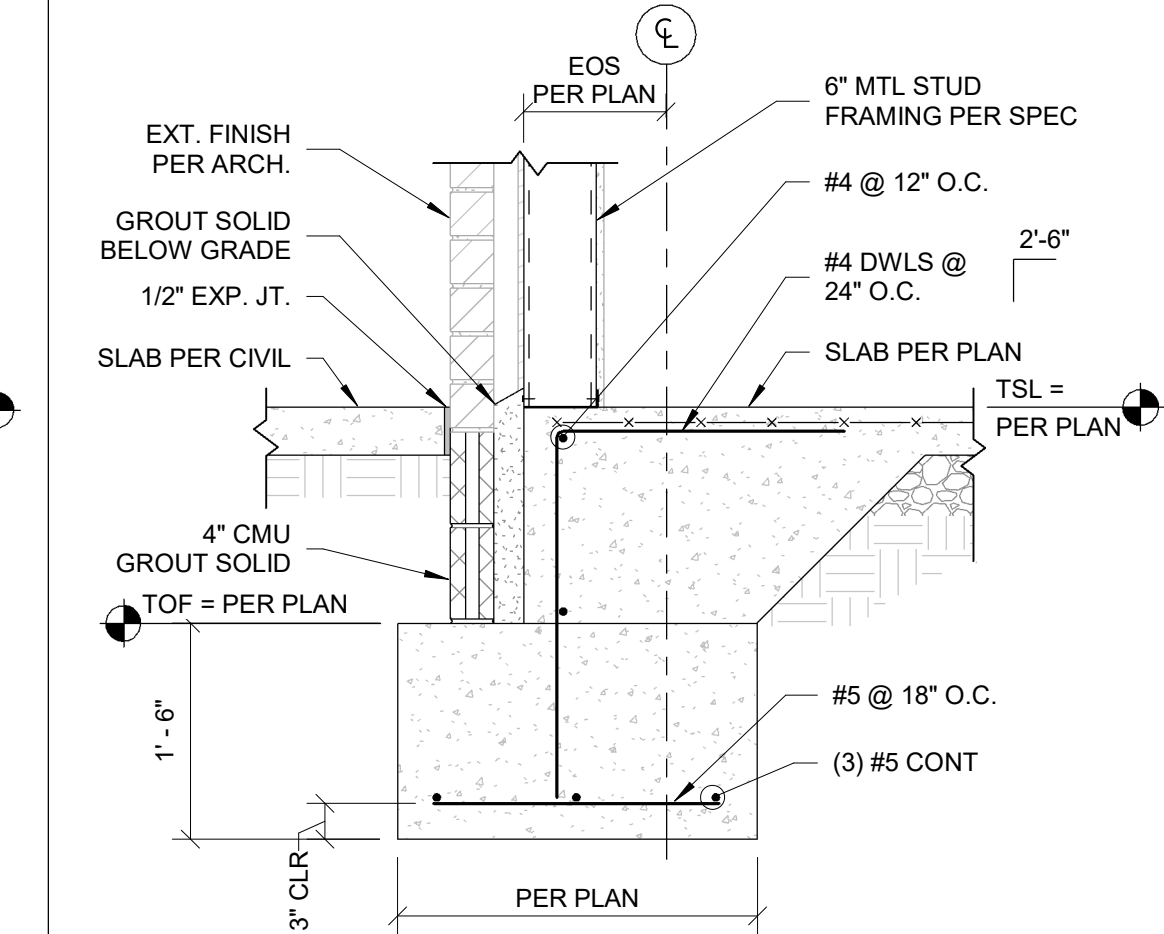
5 SECTION @ PEMB MAN DOOR
SCALE: 3/4" = 1'-0"



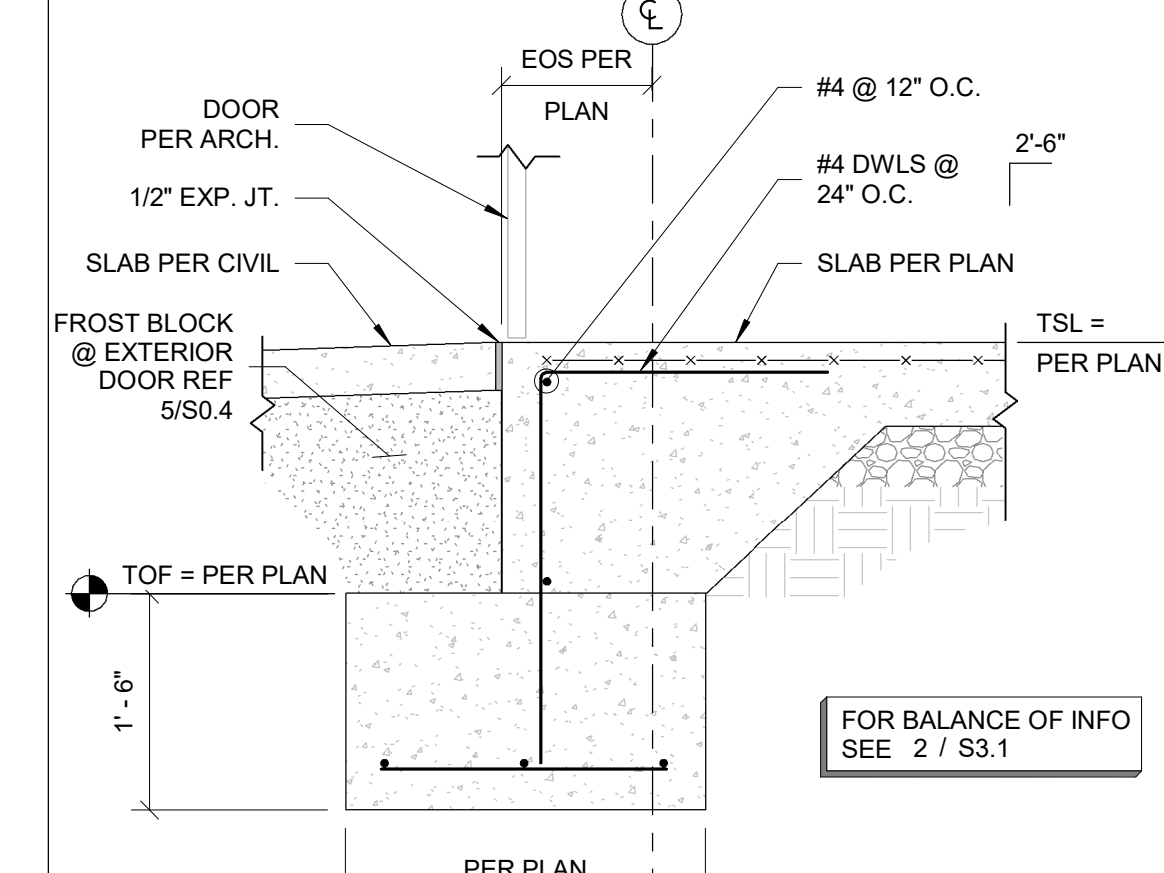
4 SECTION @ OVERHEAD DOOR
SCALE: 1/2" = 1'-0"



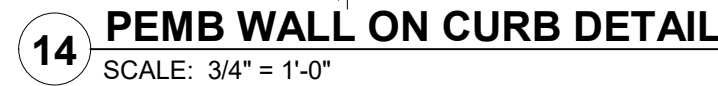
3 FOUNDATION SECTION @ EXTERIOR WALL - OFFICE
SCALE: 3/4" = 1'-0"



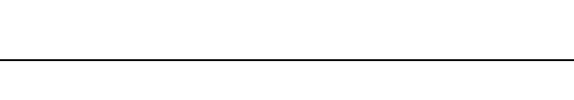
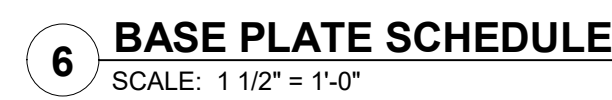
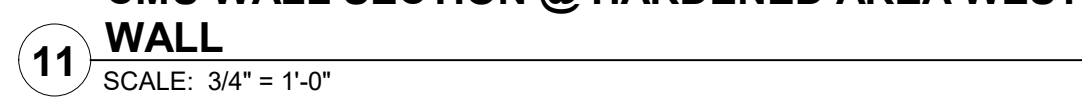
2 TYP FND SECTION @ EXT WALL - OFFICE
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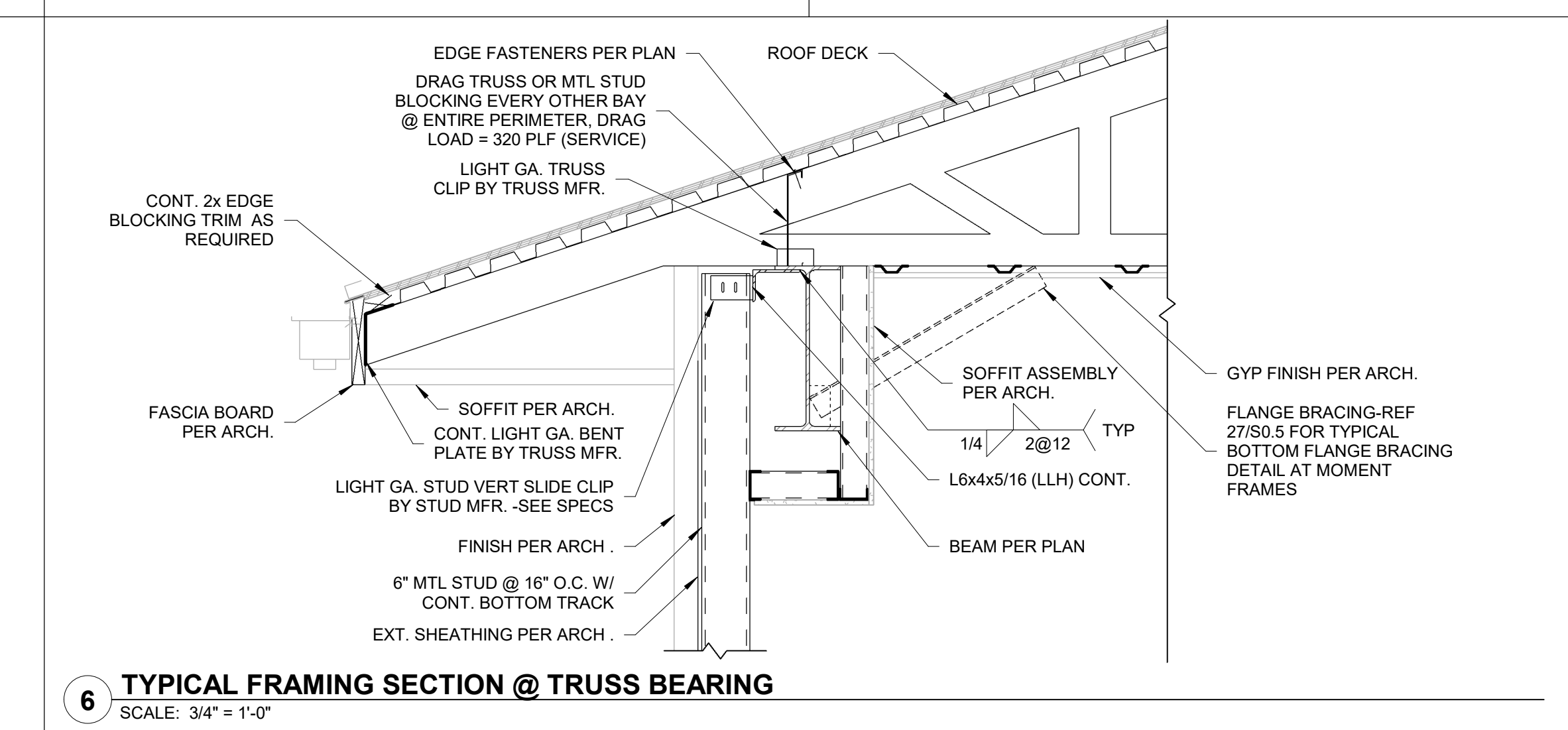
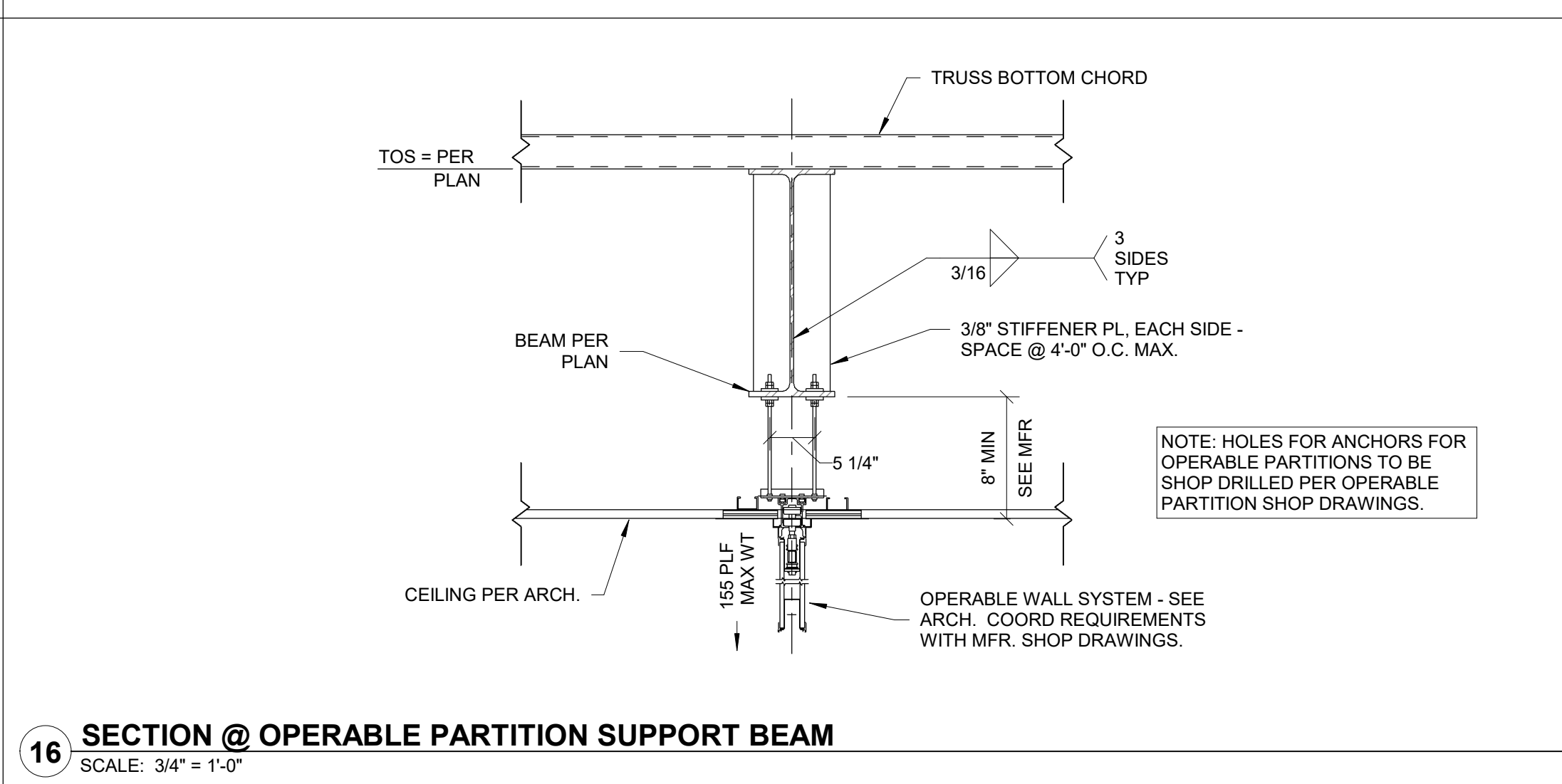
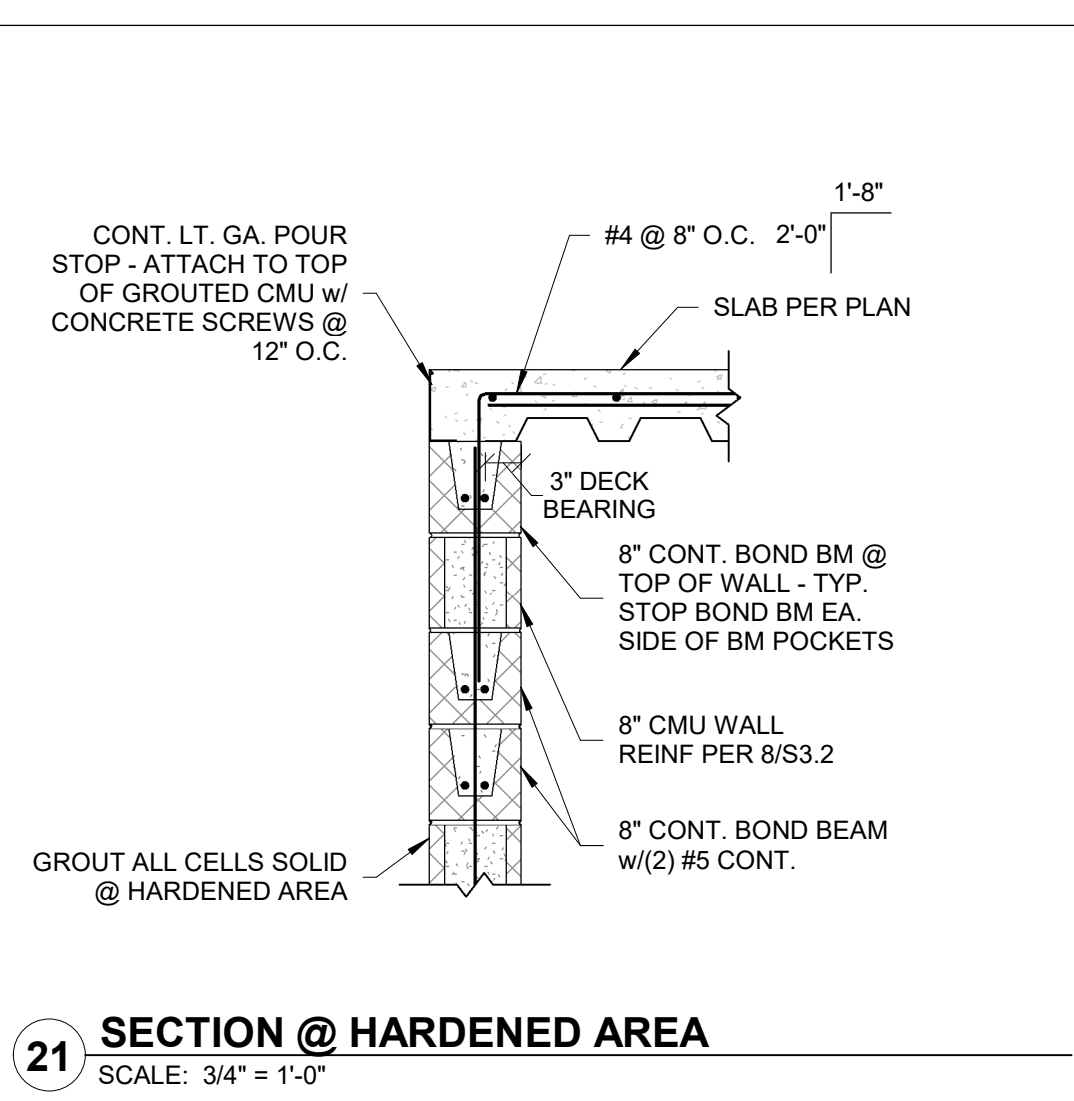
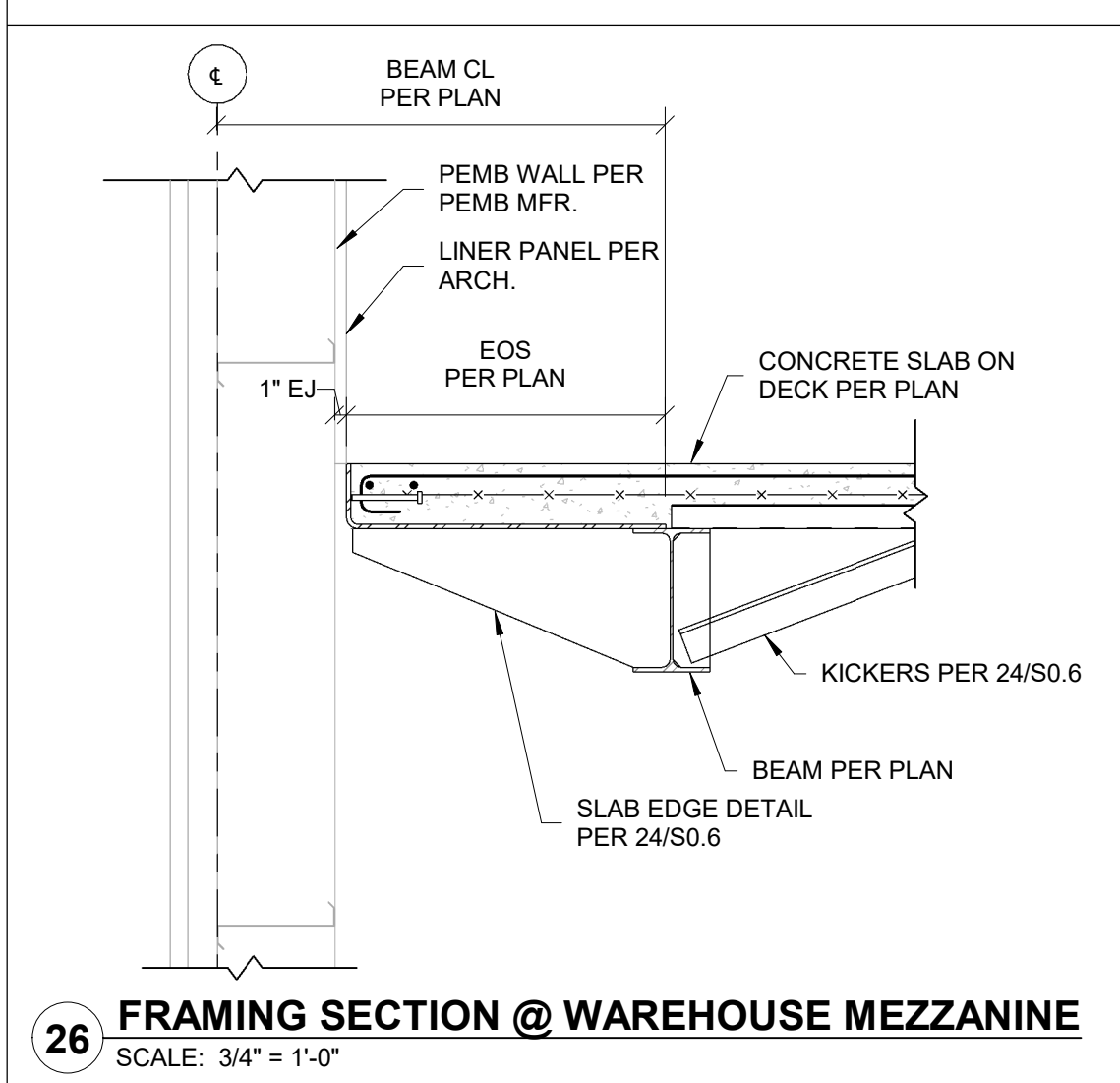
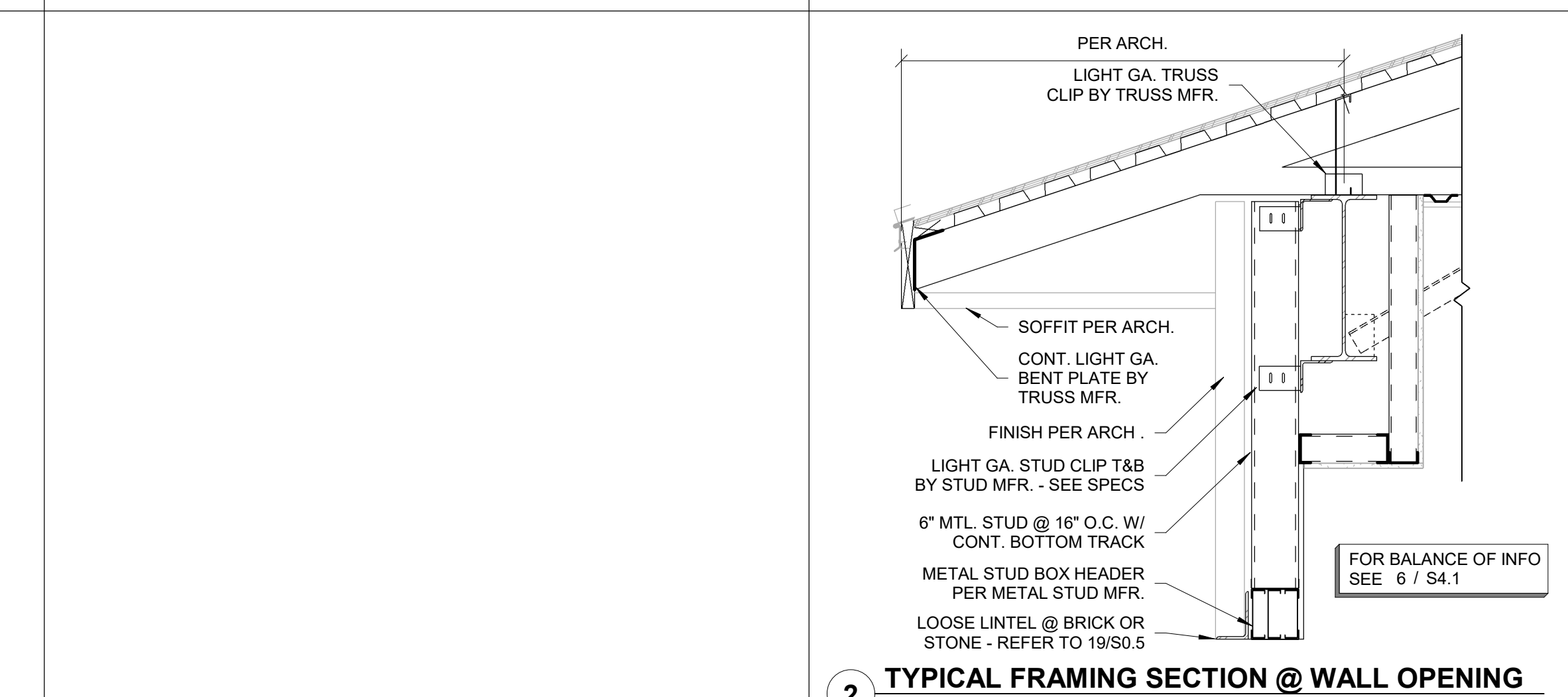
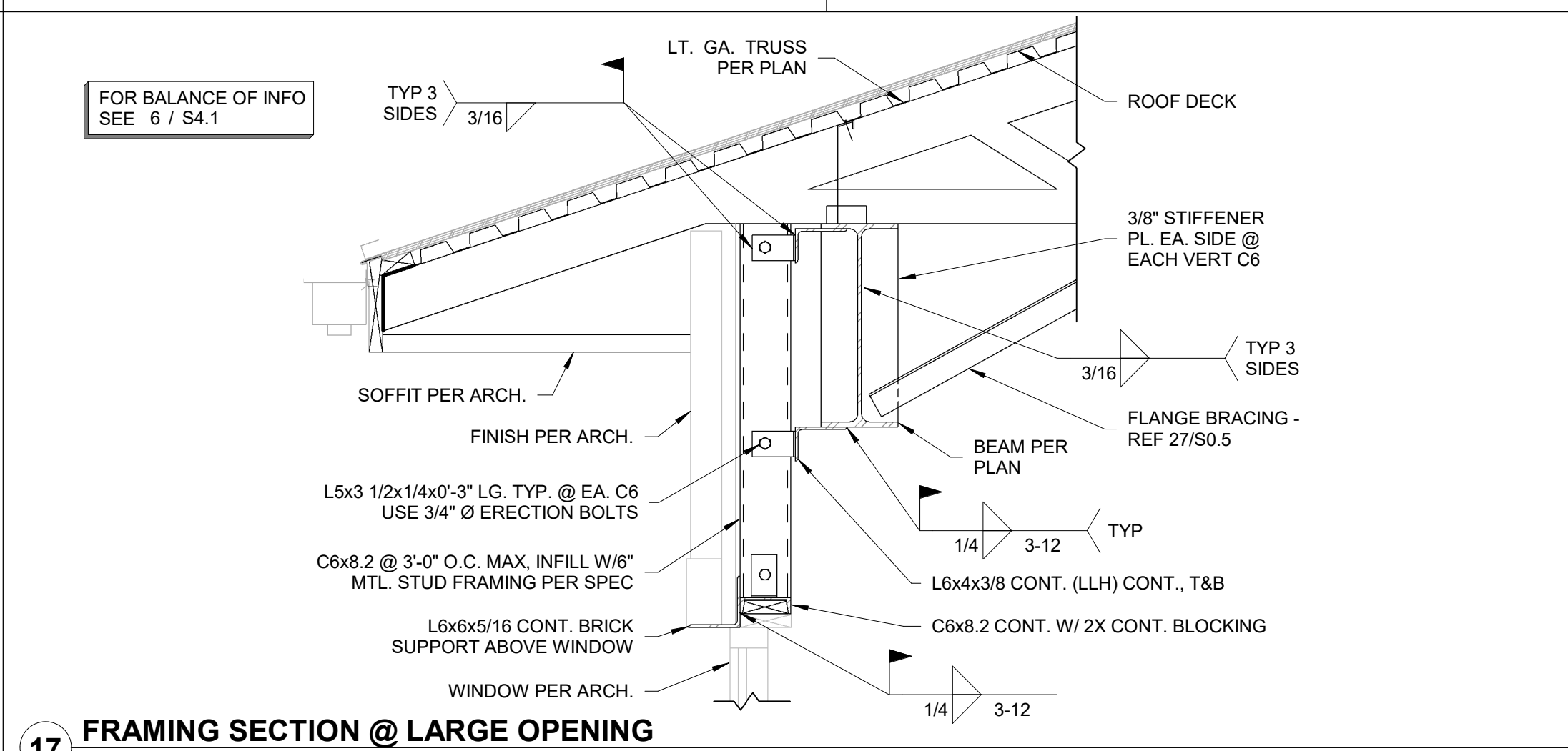
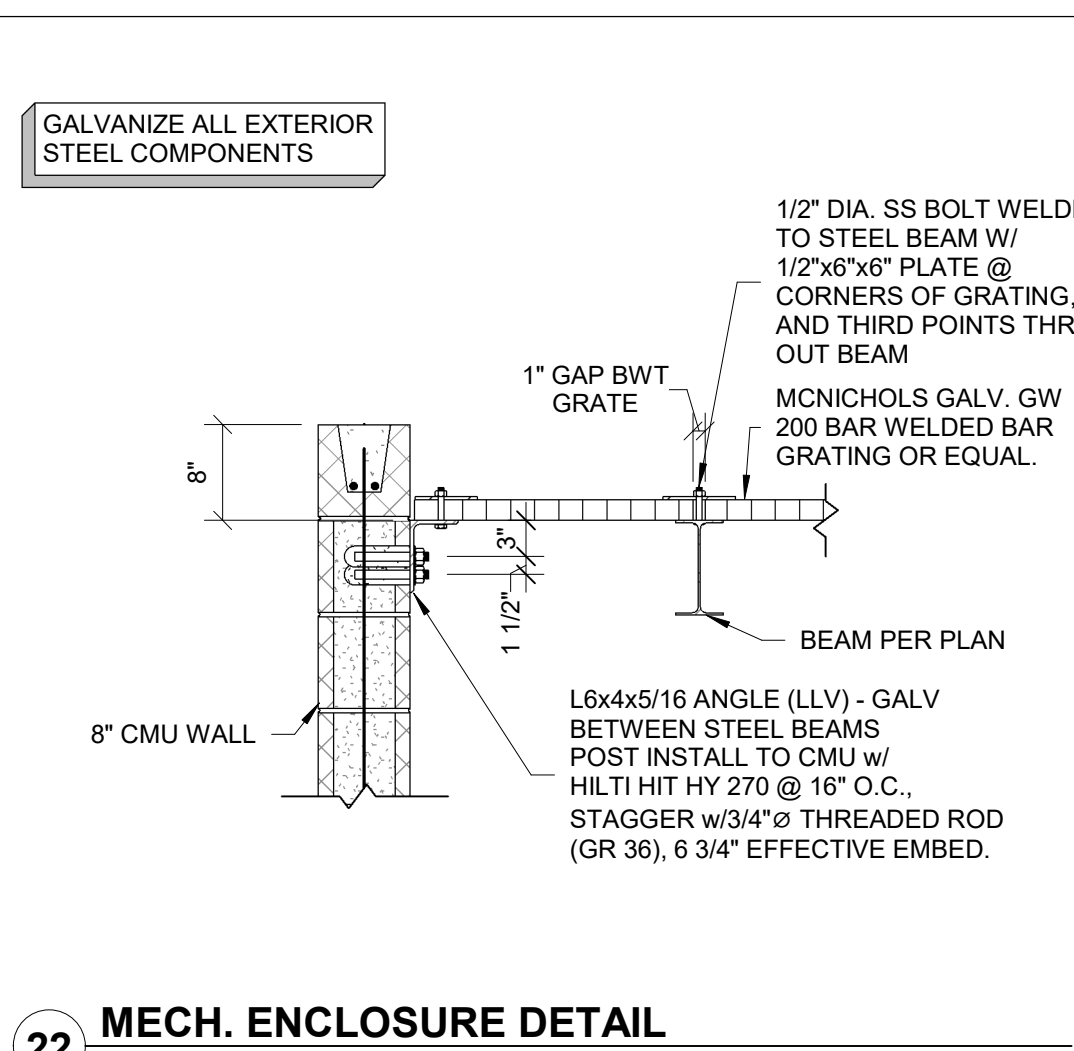
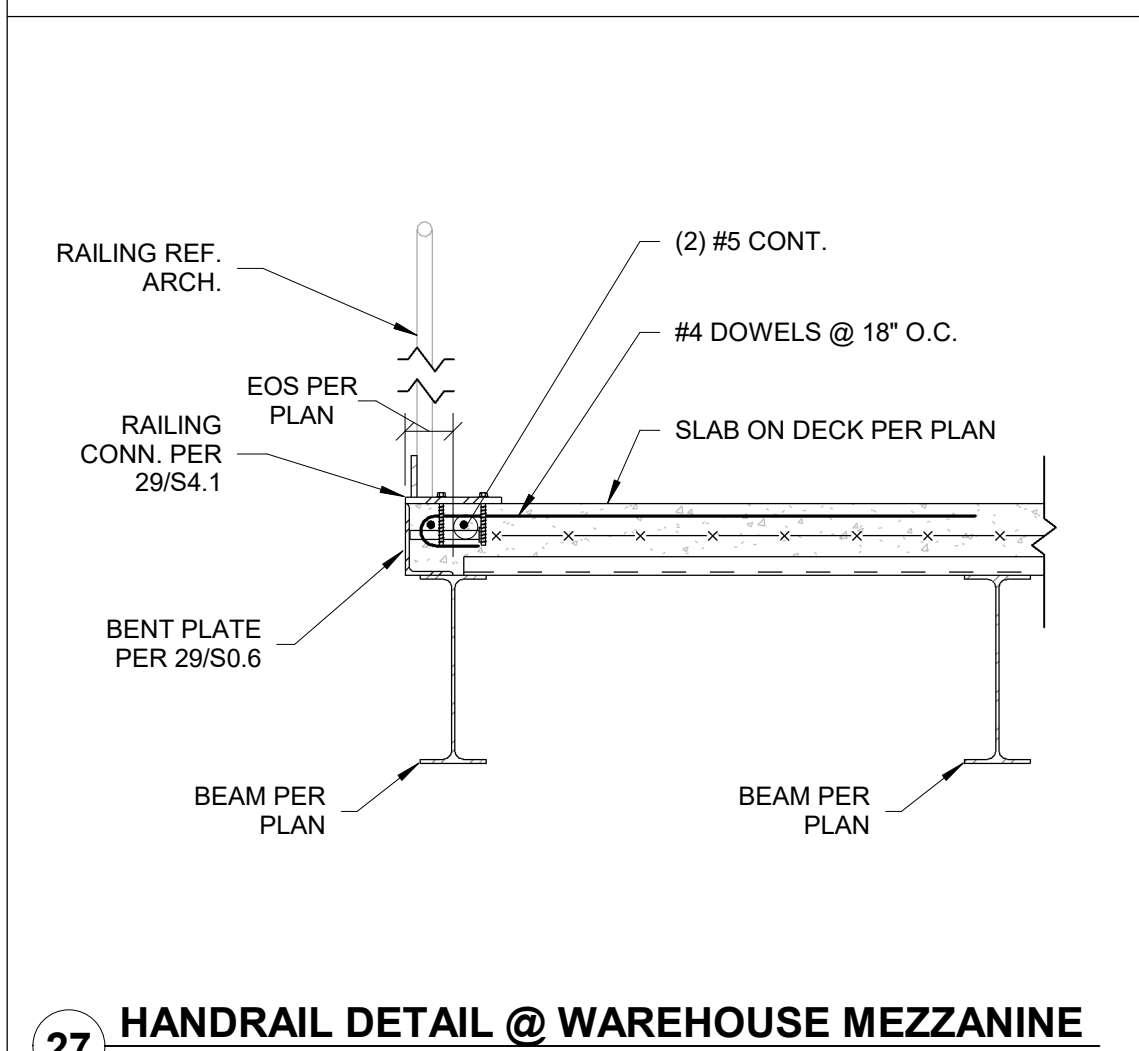
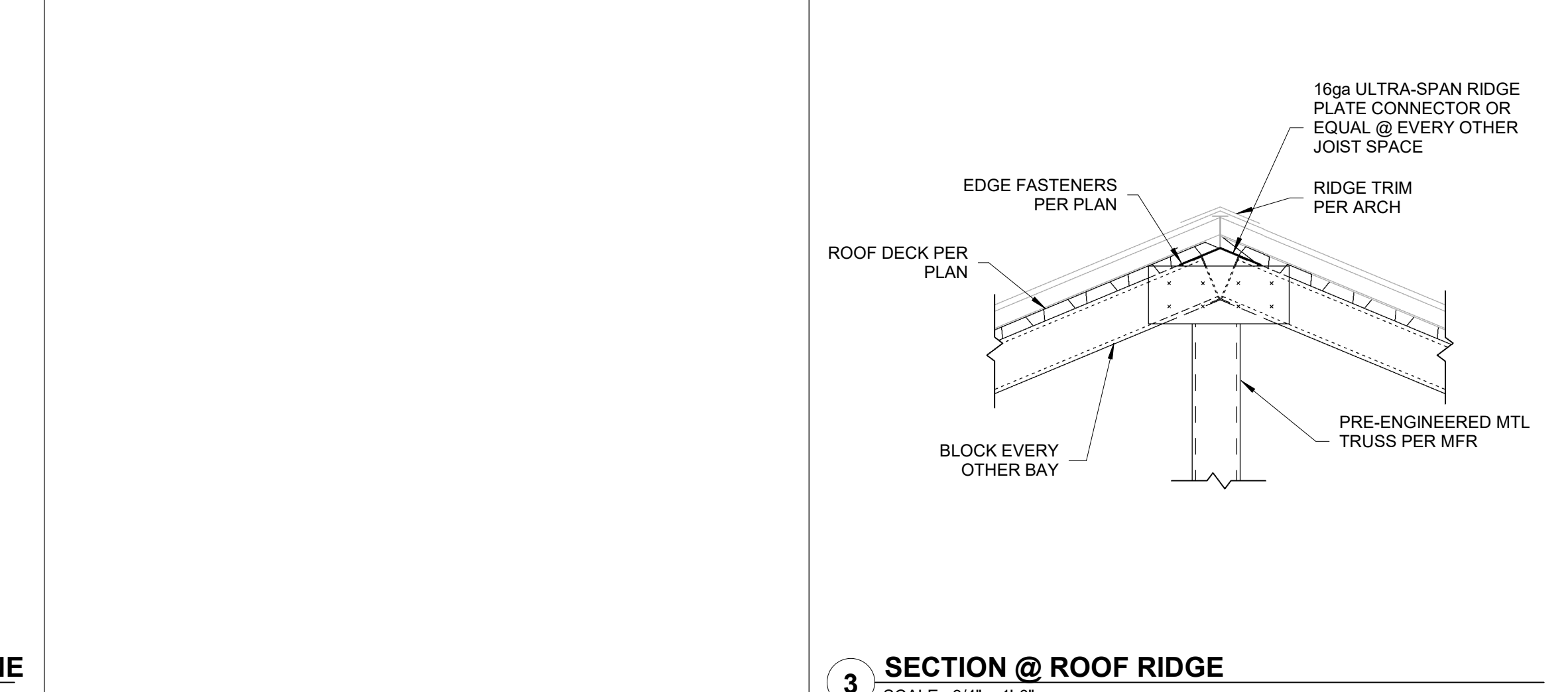
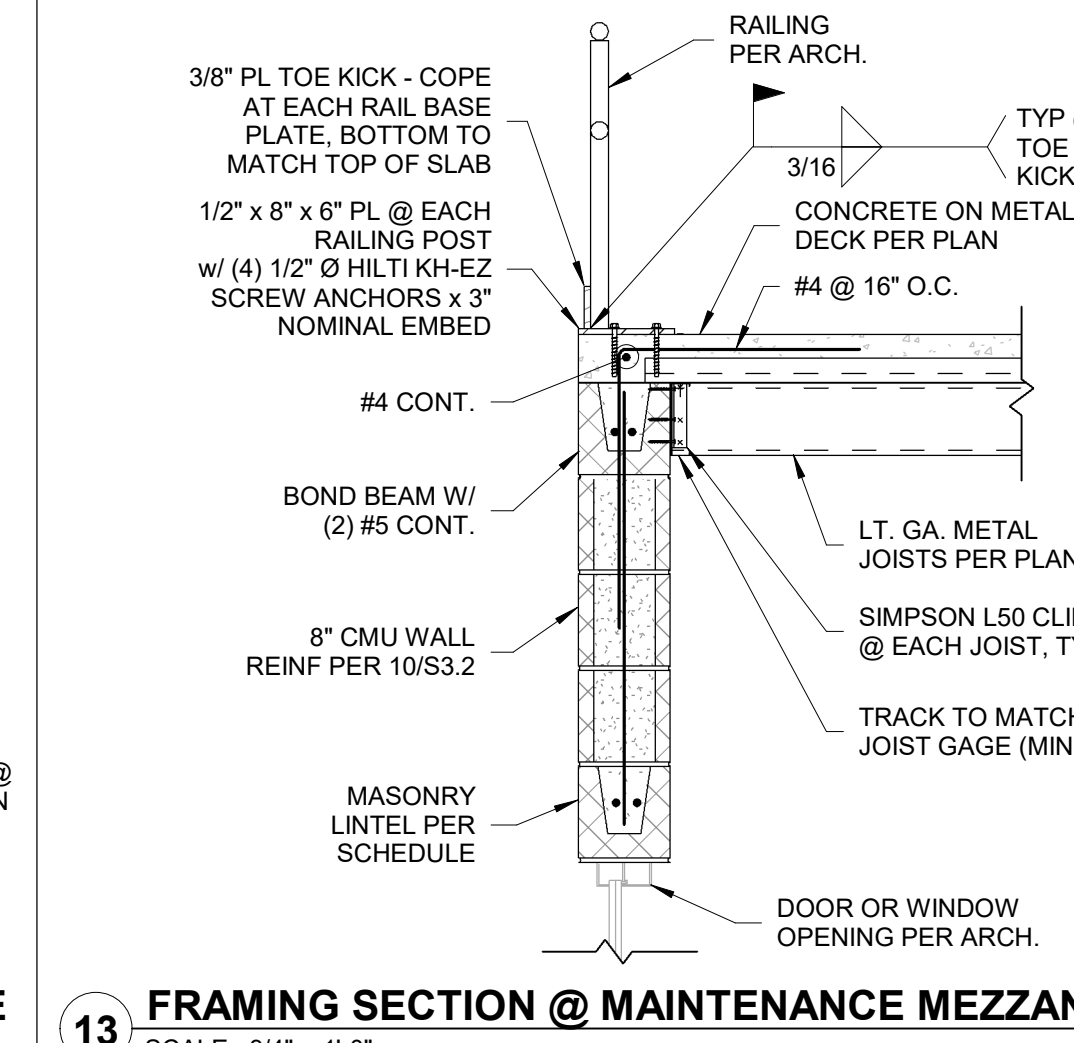
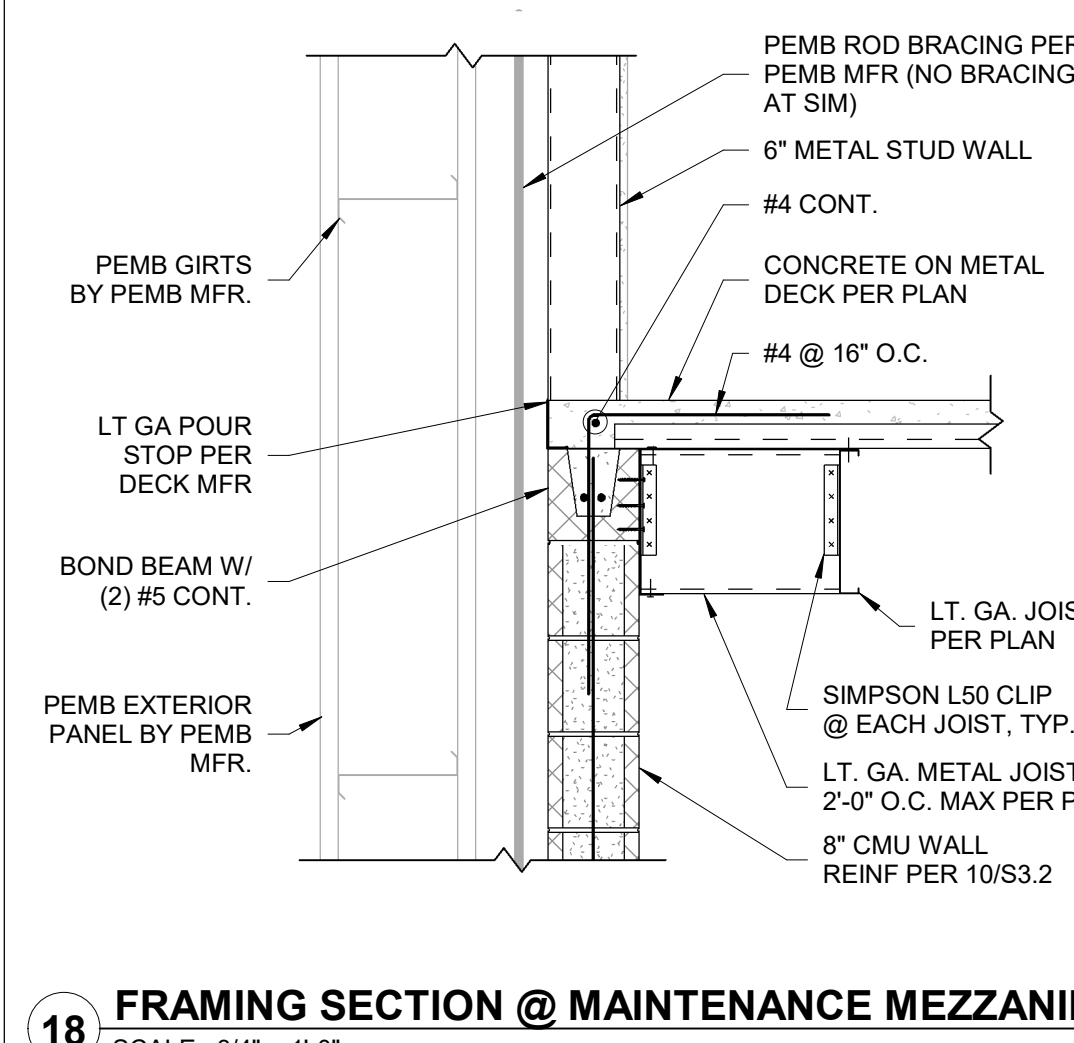
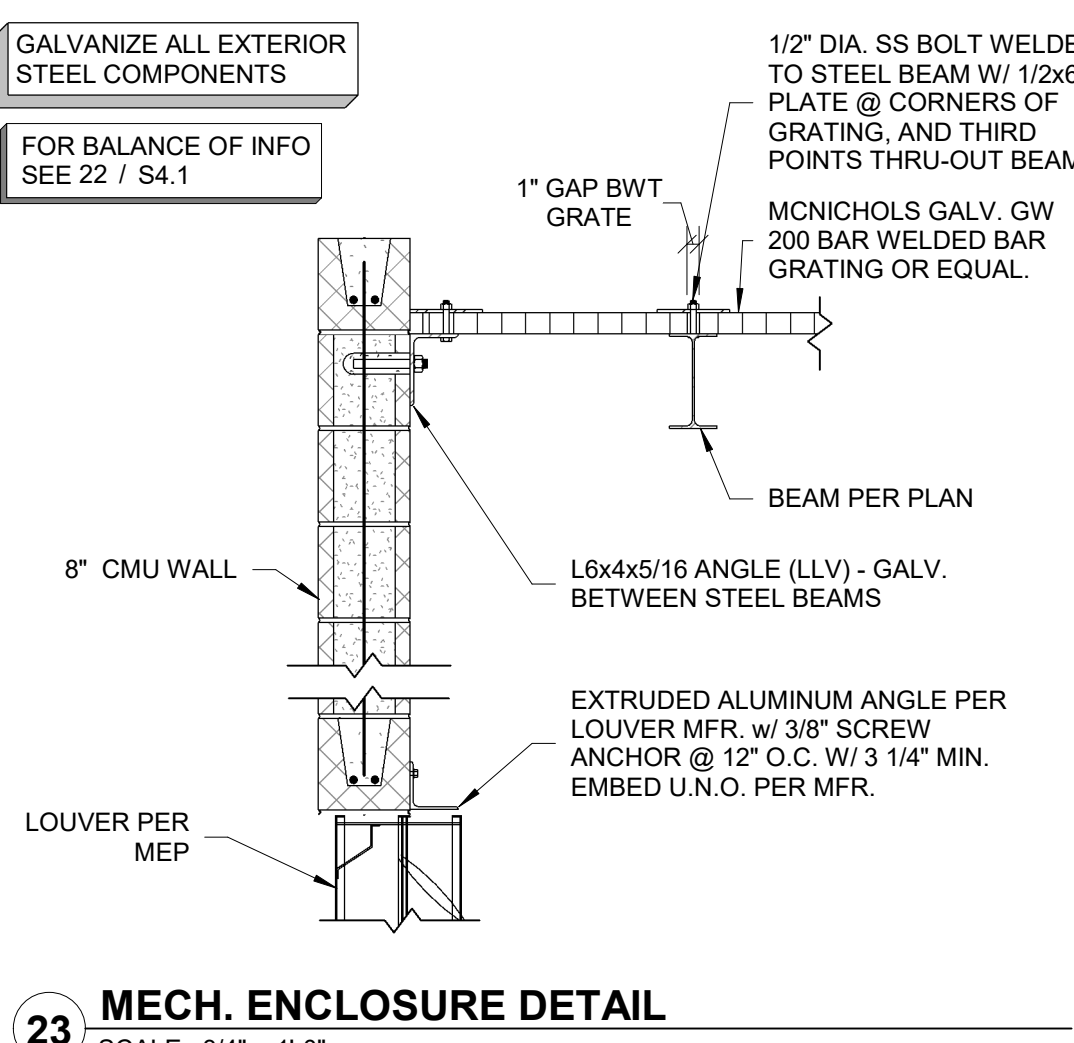
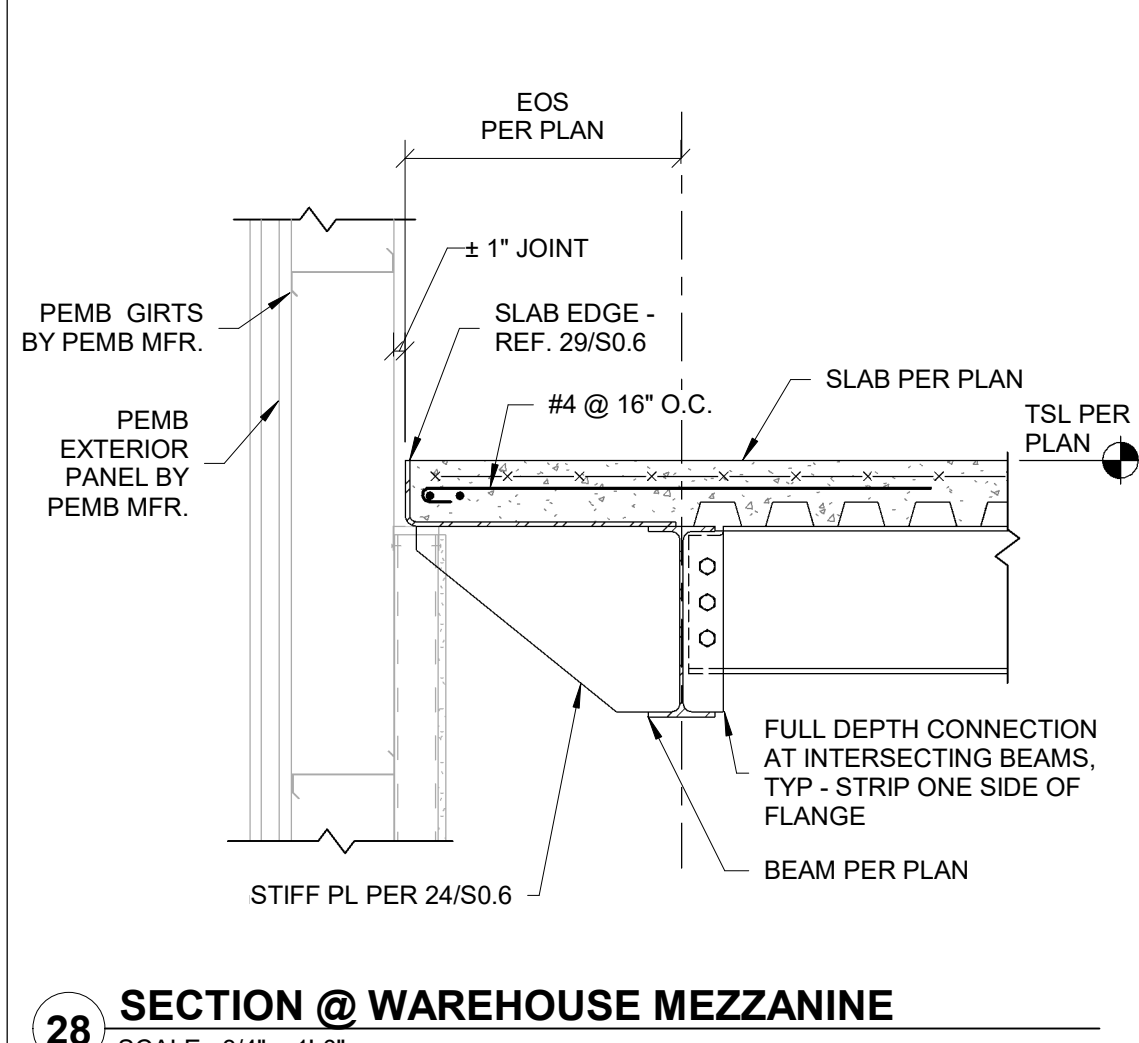
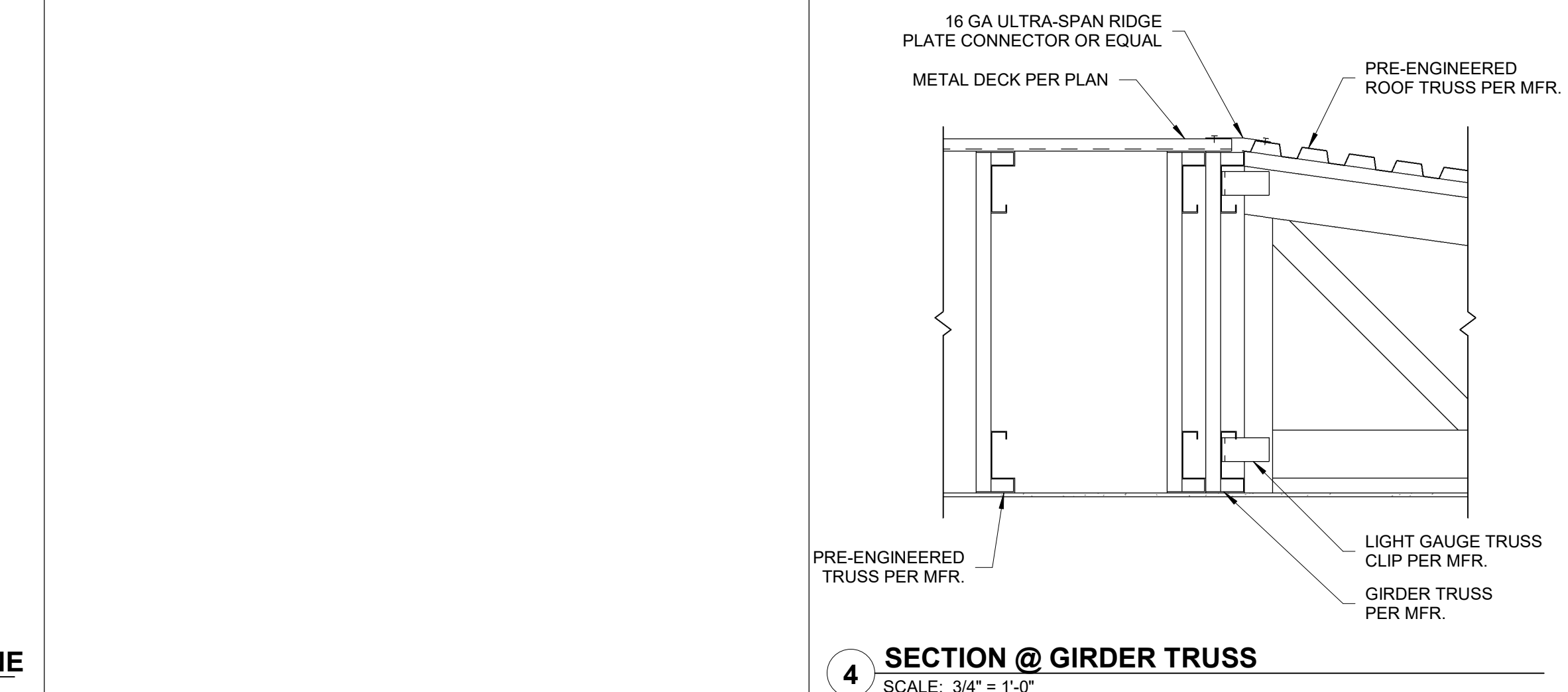
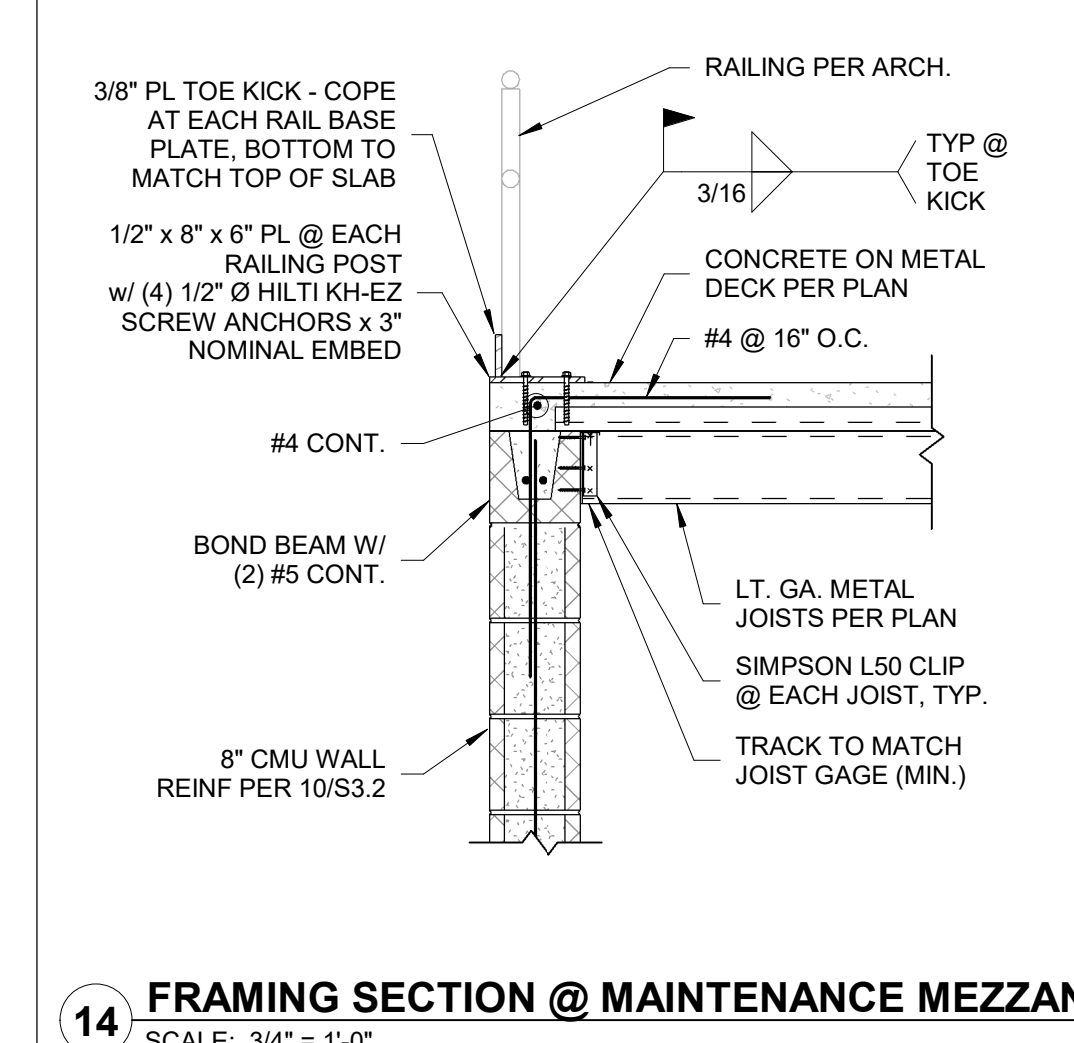
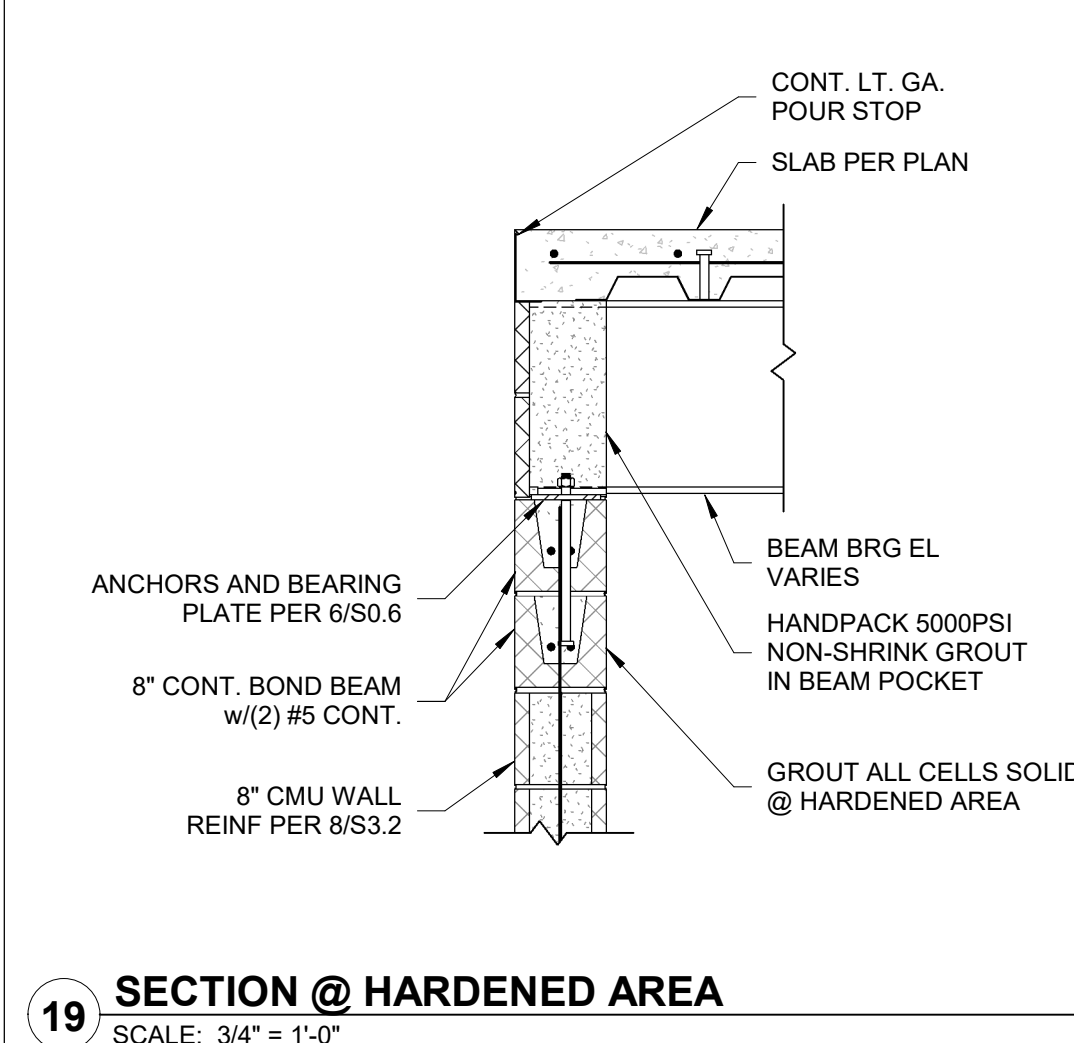
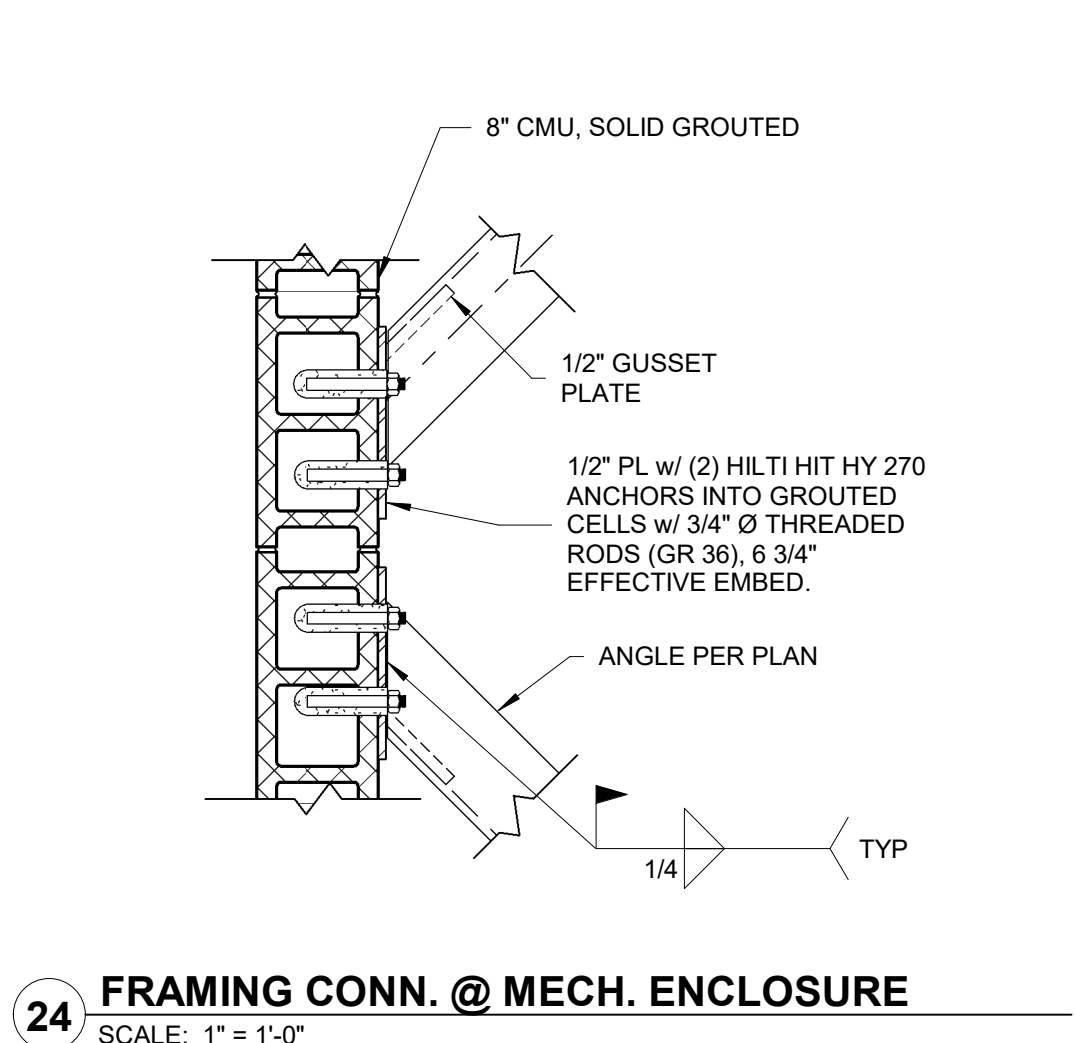
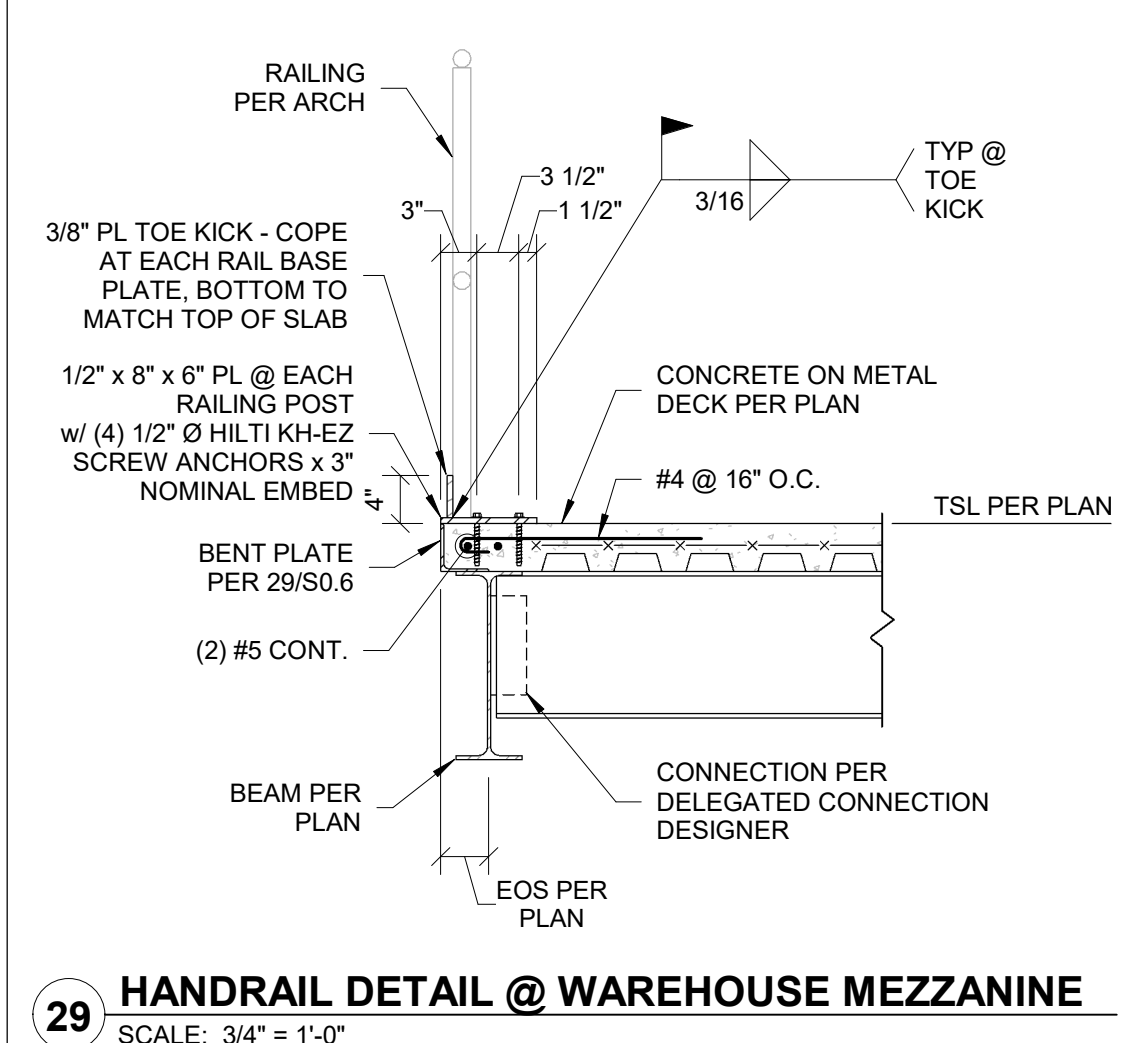
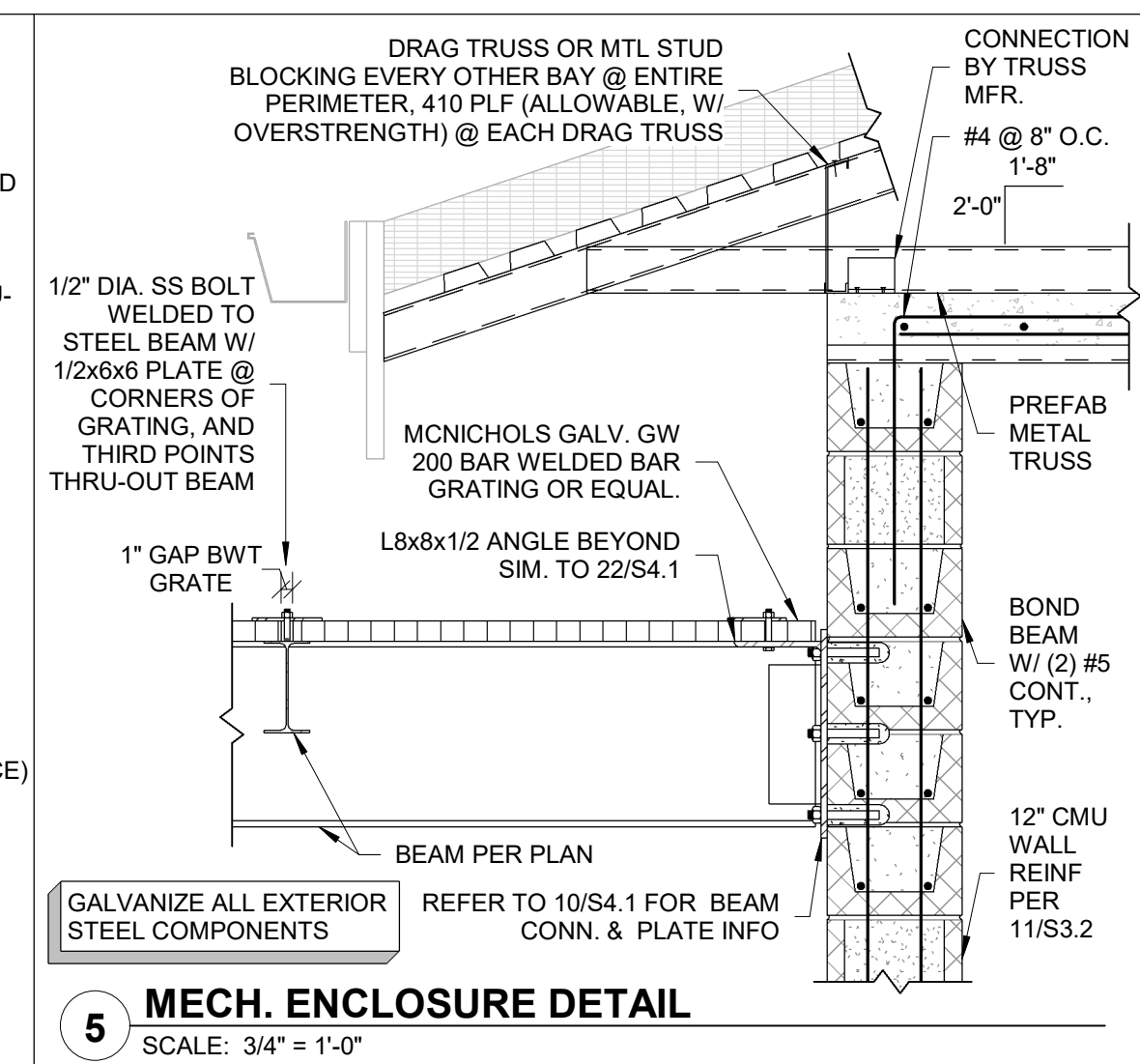
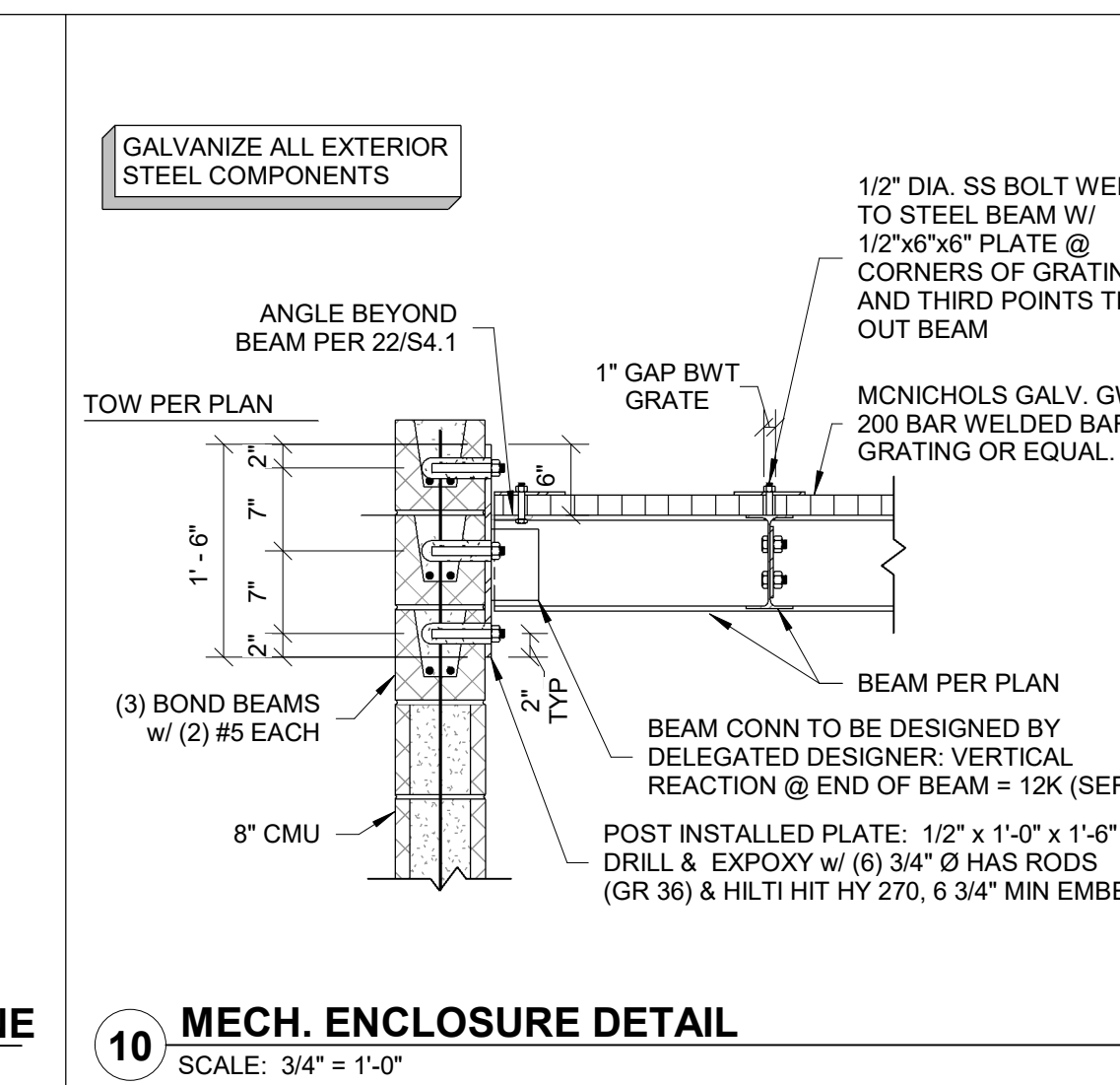
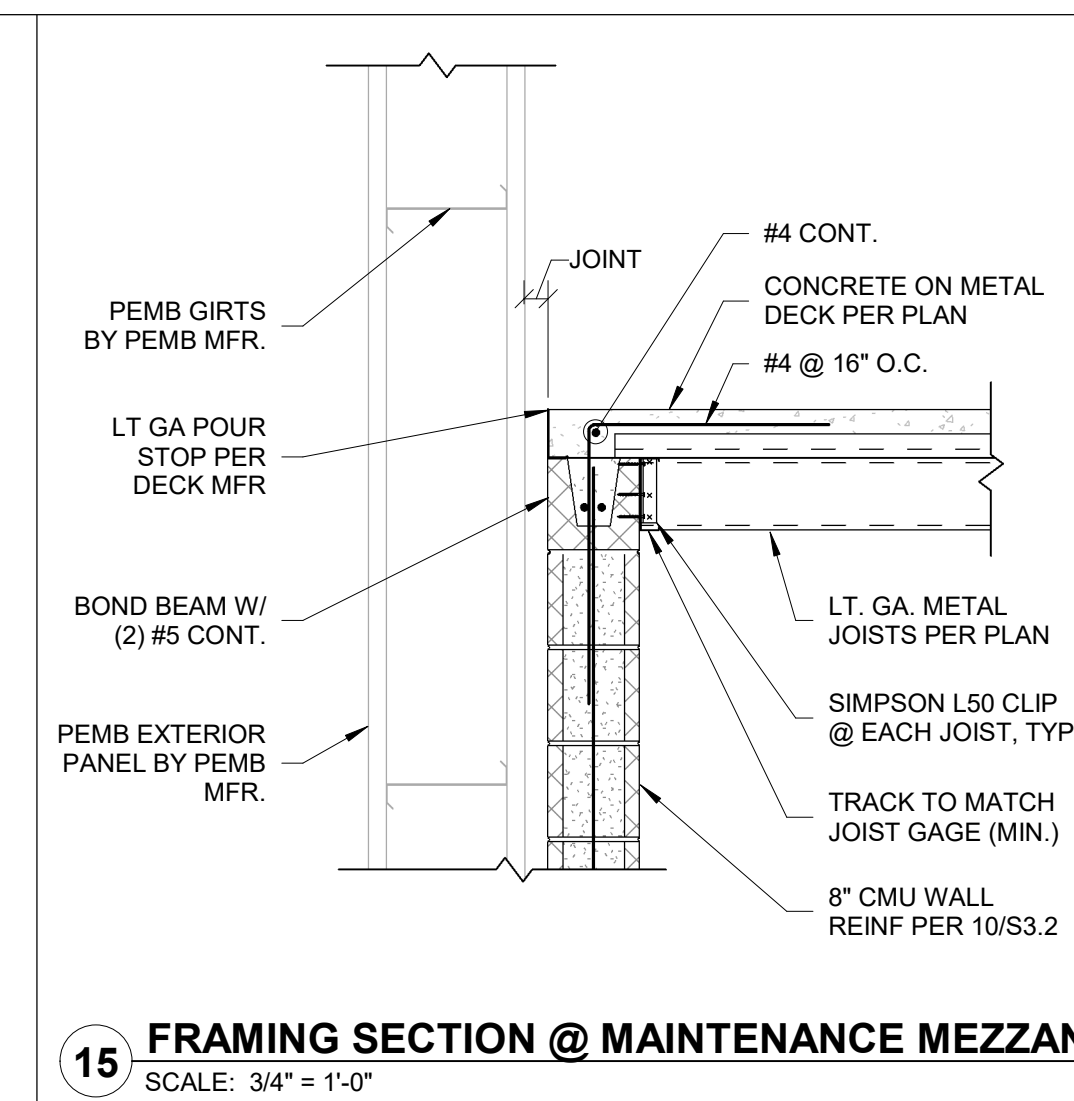
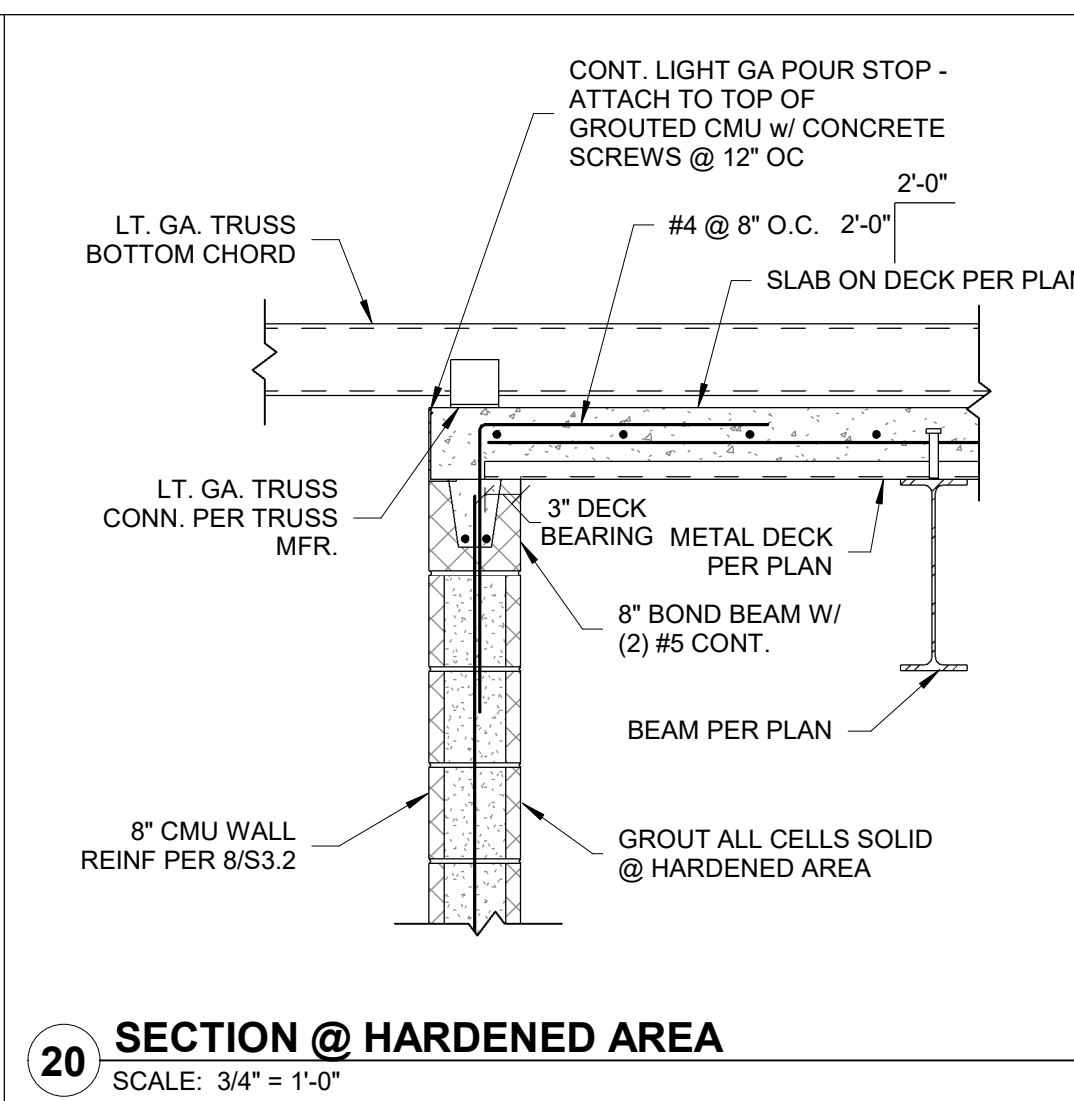
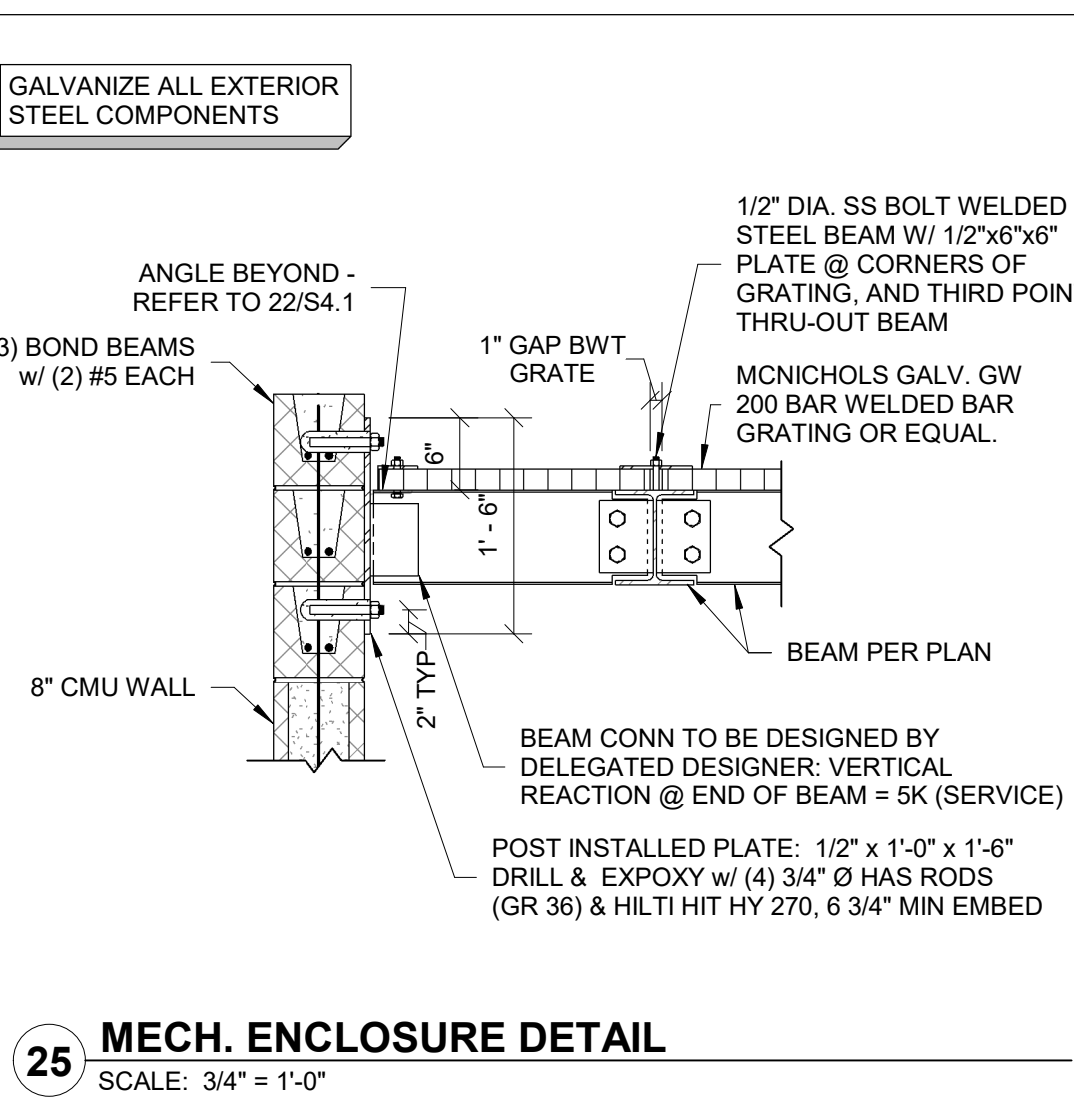
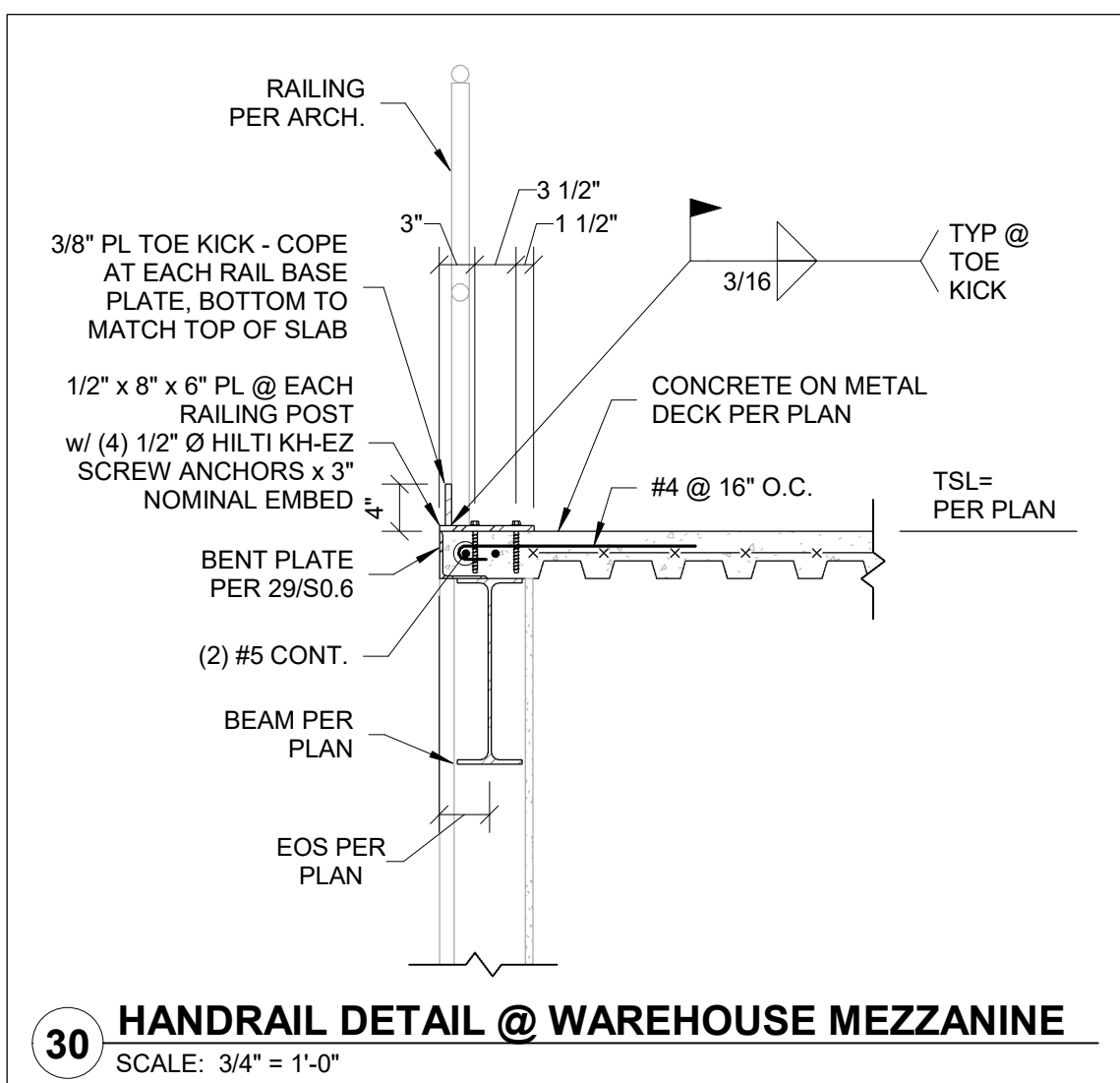


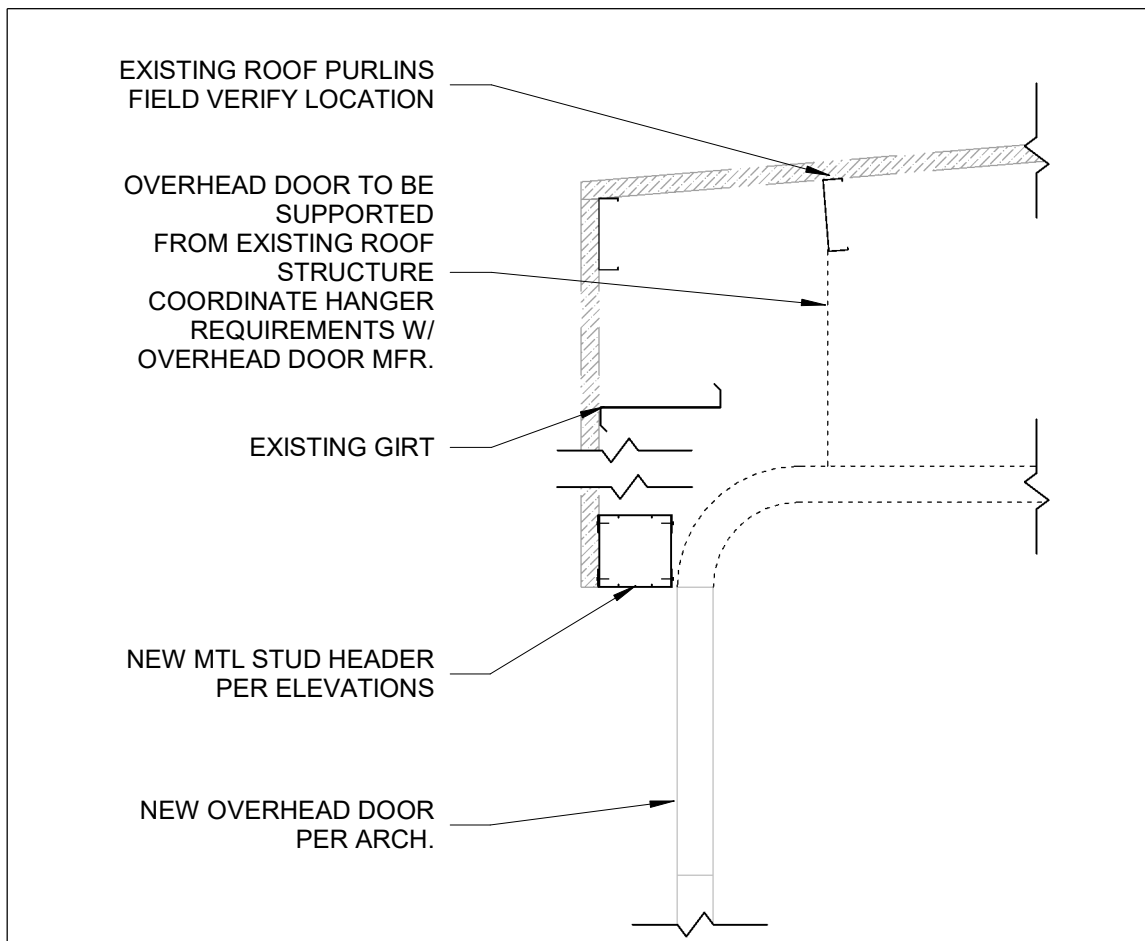
1 TYP FND SECTION @ DOOR - OFFICE
SCALE: 3/4" = 1'-0"



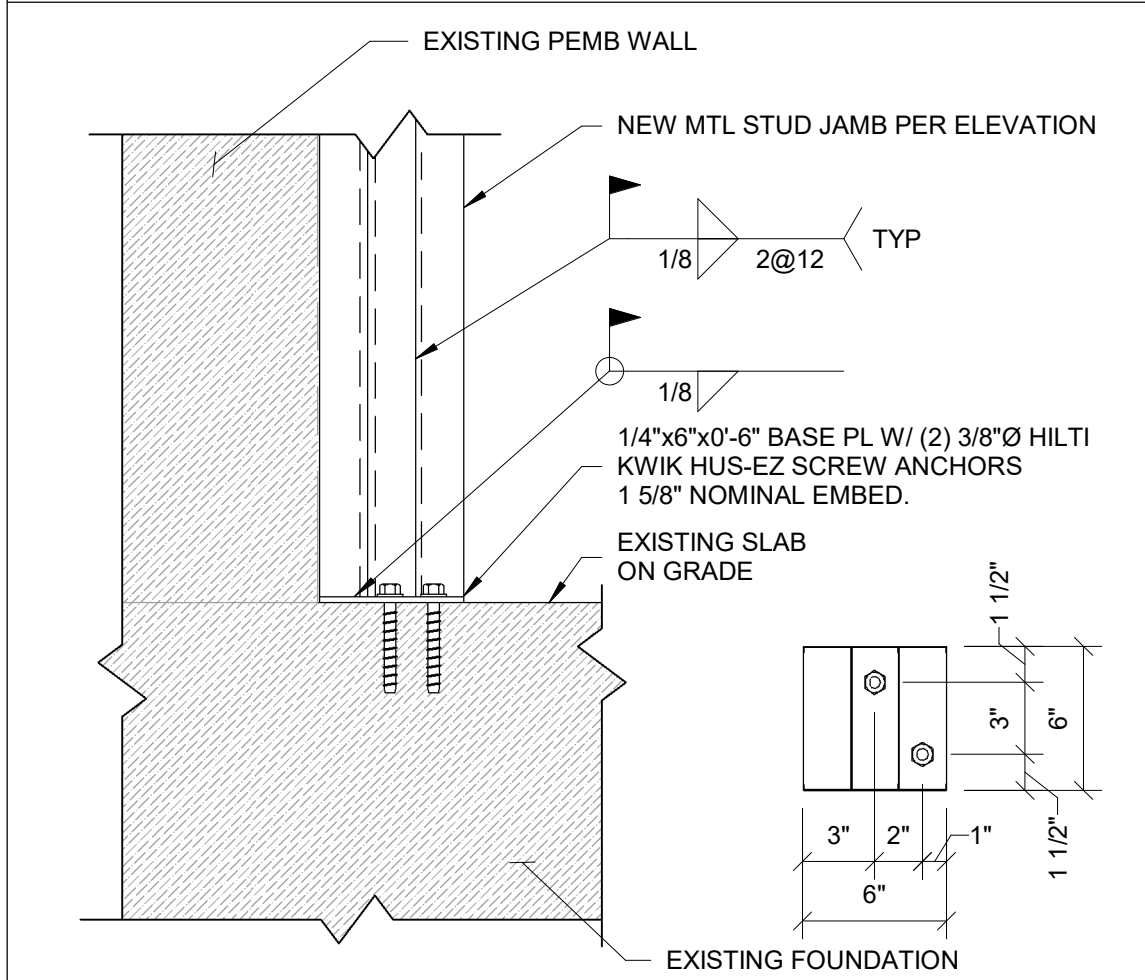
U.N.O. ALL ANCHOR RODS TO BE F1554 (GR 36)



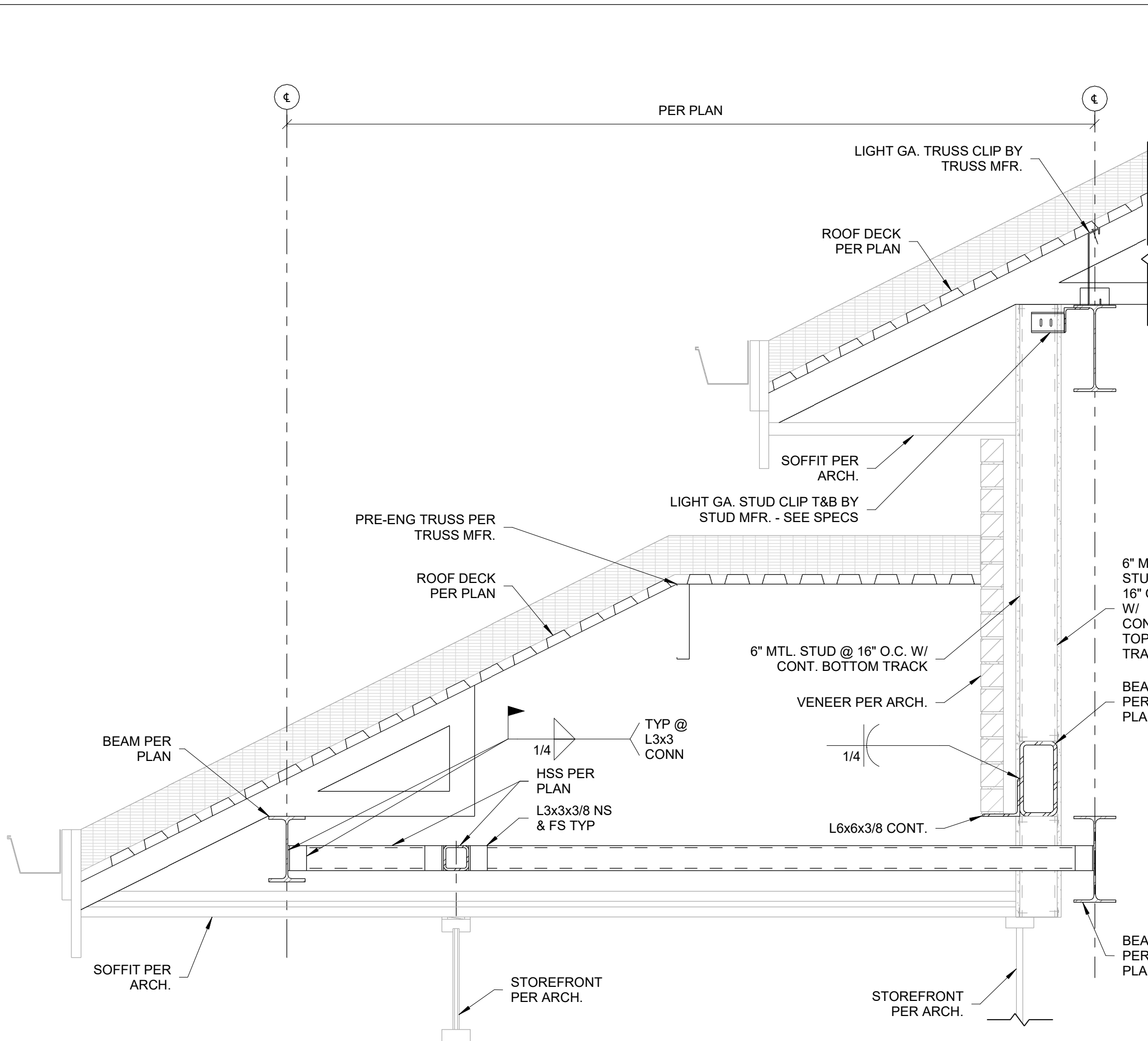




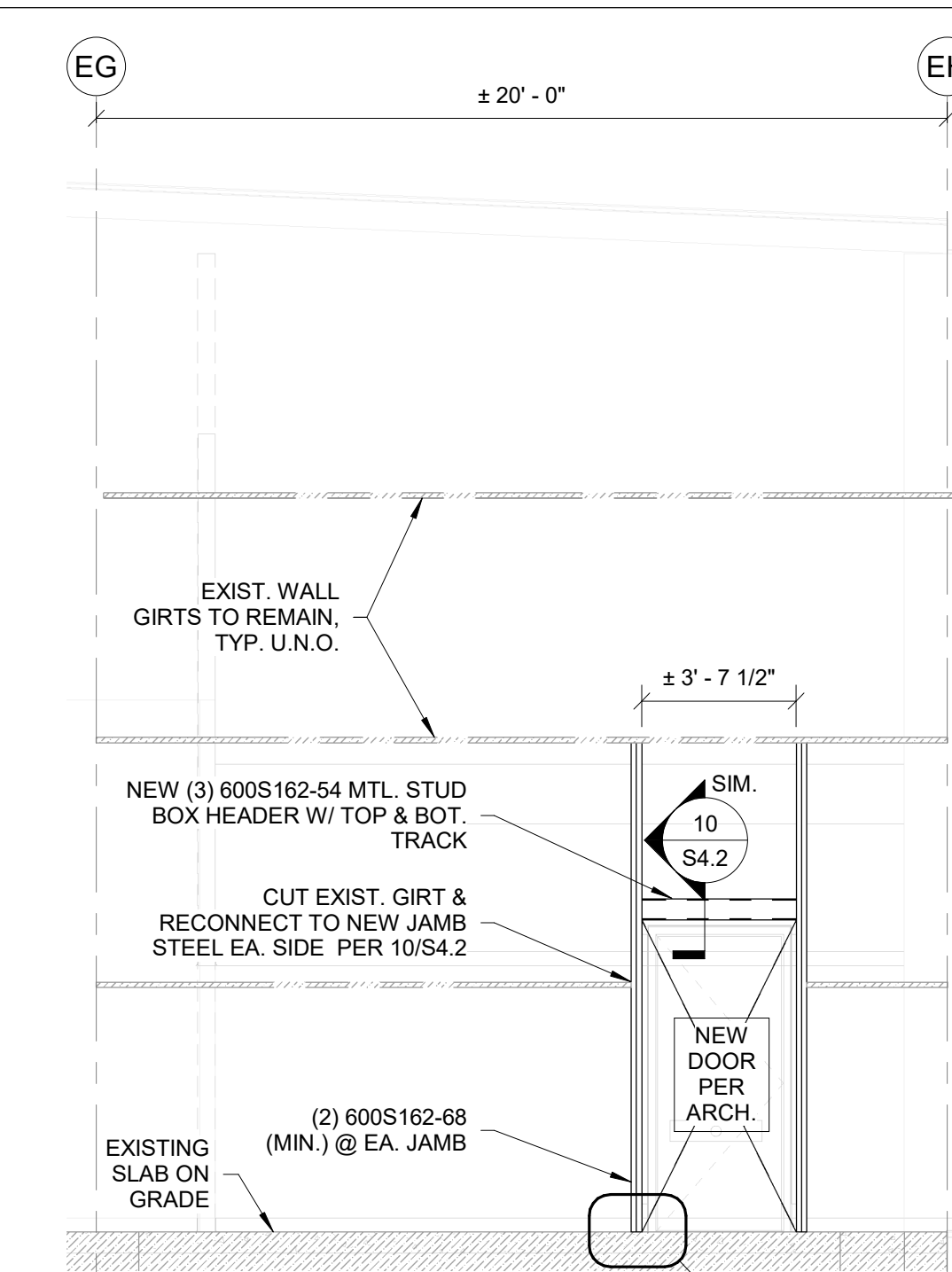
30 SECTION AT NEW OVERHEAD DOOR @ EXISTING
SCALE: 3/4" = 1'-0"



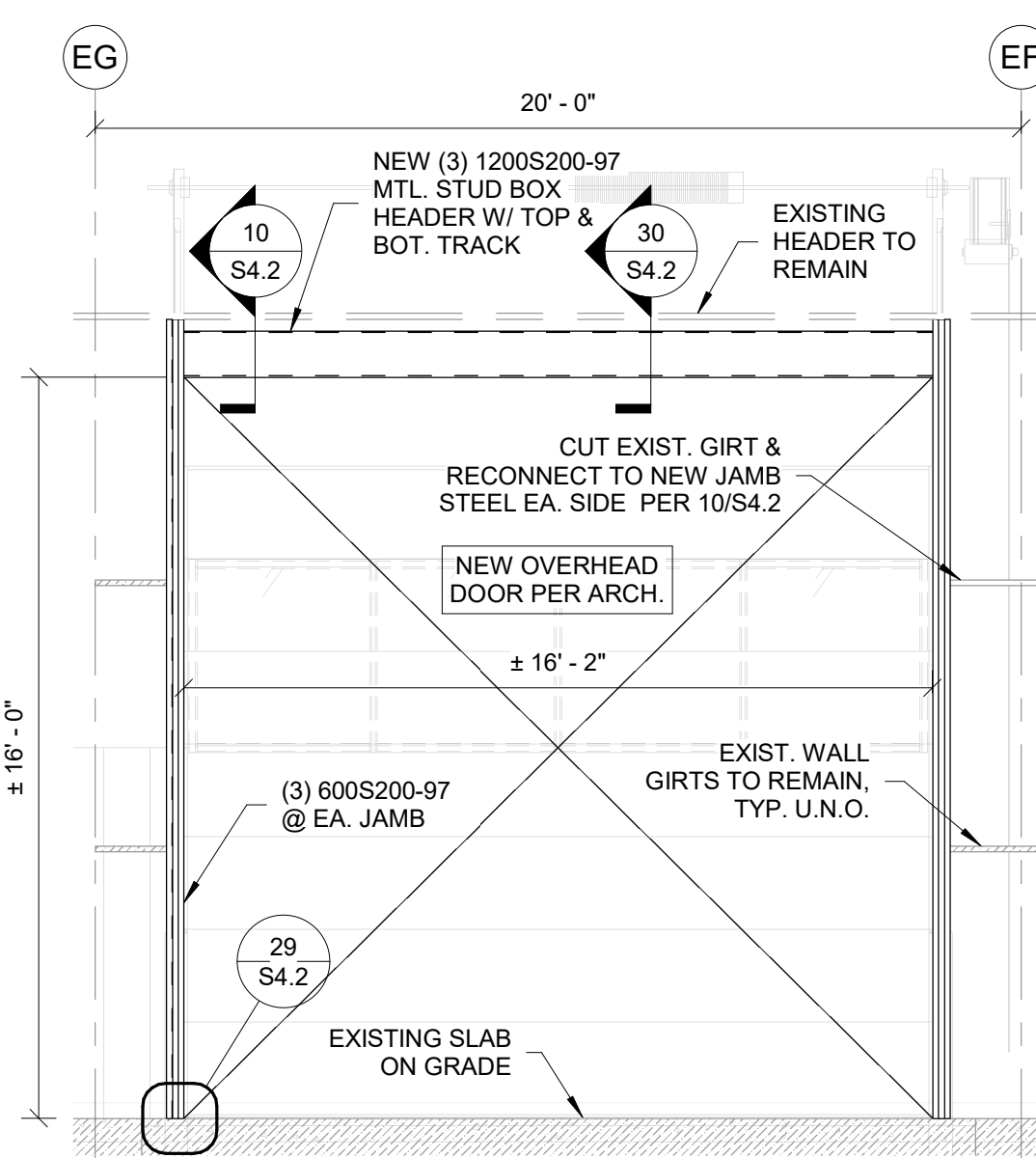
29 SECTION AT NEW OVERHEAD DOOR @ EXISTING
SCALE: 1 1/2" = 1'-0"



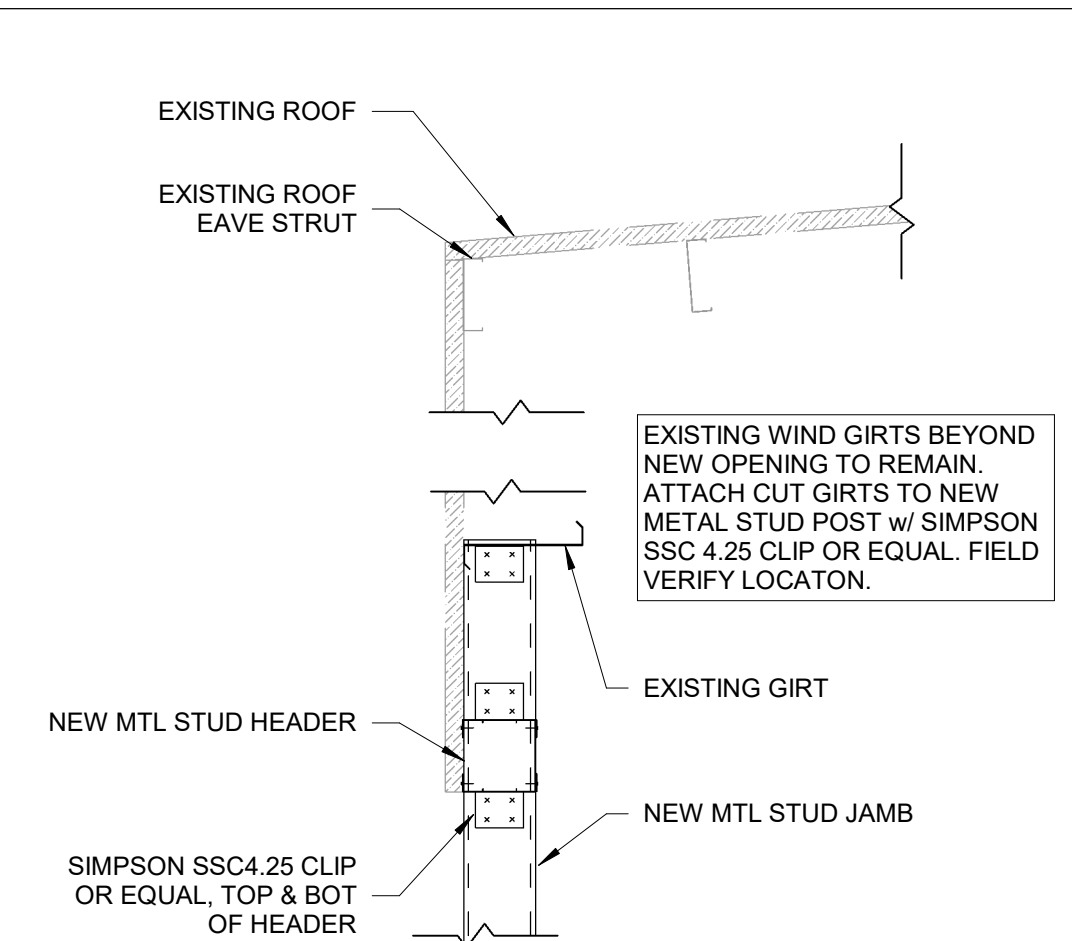
24 FRAMING SECTION @ FRONT ENTRANCE
SCALE: 3/4" = 1'-0"



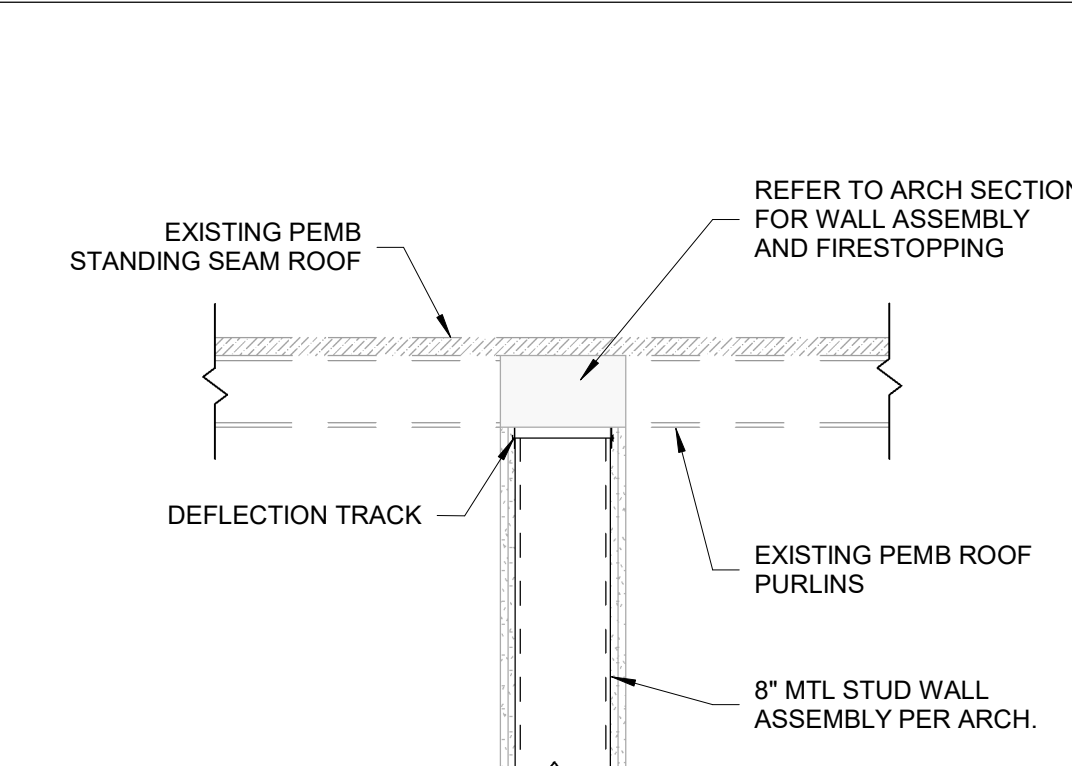
14 SECTION @ OPERATIONS BLDG. NEW DOOR
SCALE: 1/4" = 1'-0"



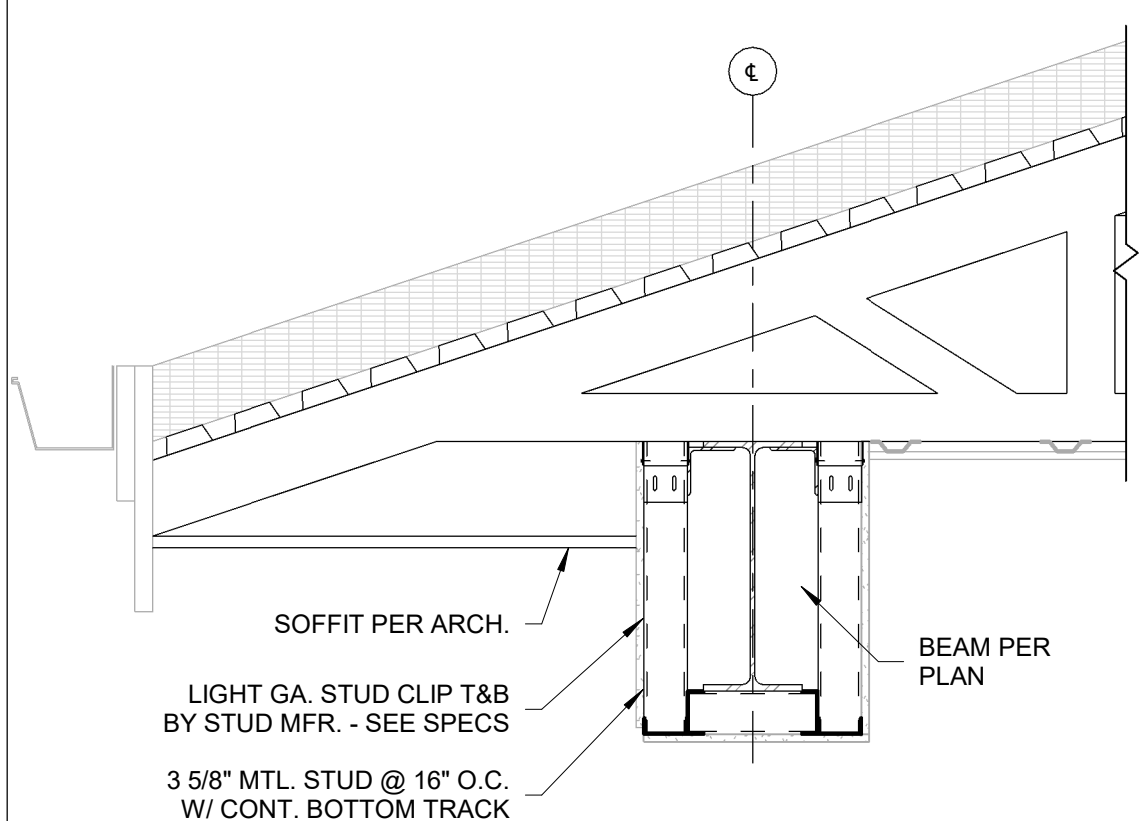
13 SECTION @ OPERATION BLDG. NEW DOOR OPENING
SCALE: 1/4" = 1'-0"



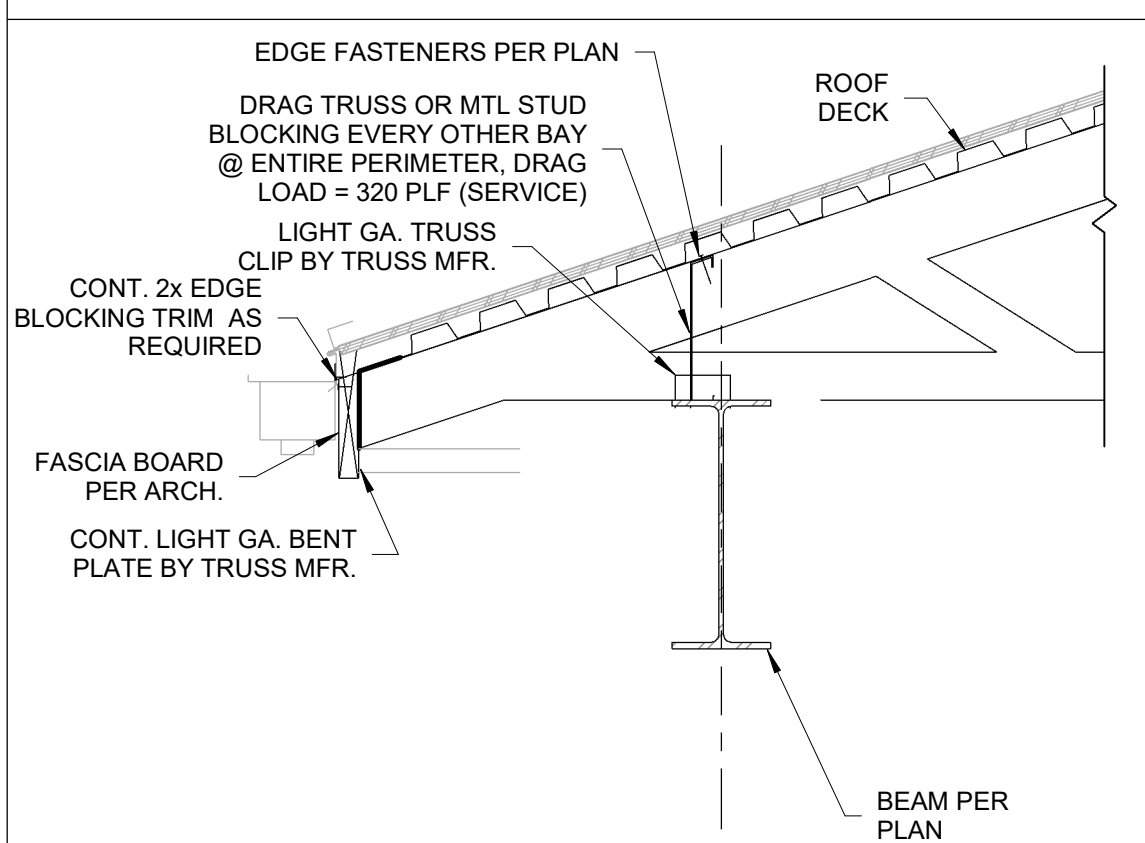
10 SECTION @ NEW OVERHEAD DOOR @ EXISTING
SCALE: 3/4" = 1'-0"



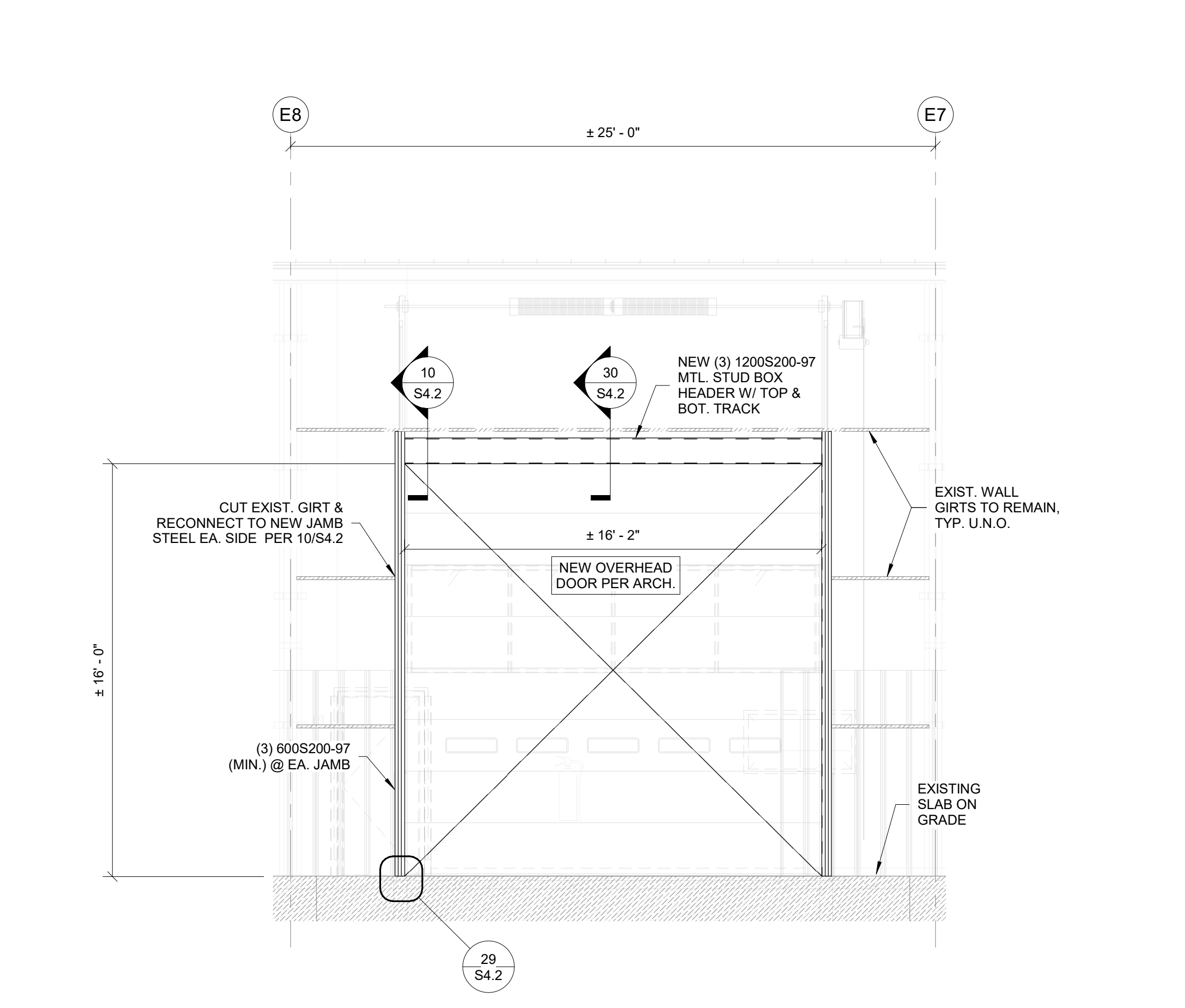
9 SECTION @ NEW RATED WALL
SCALE: 3/4" = 1'-0"



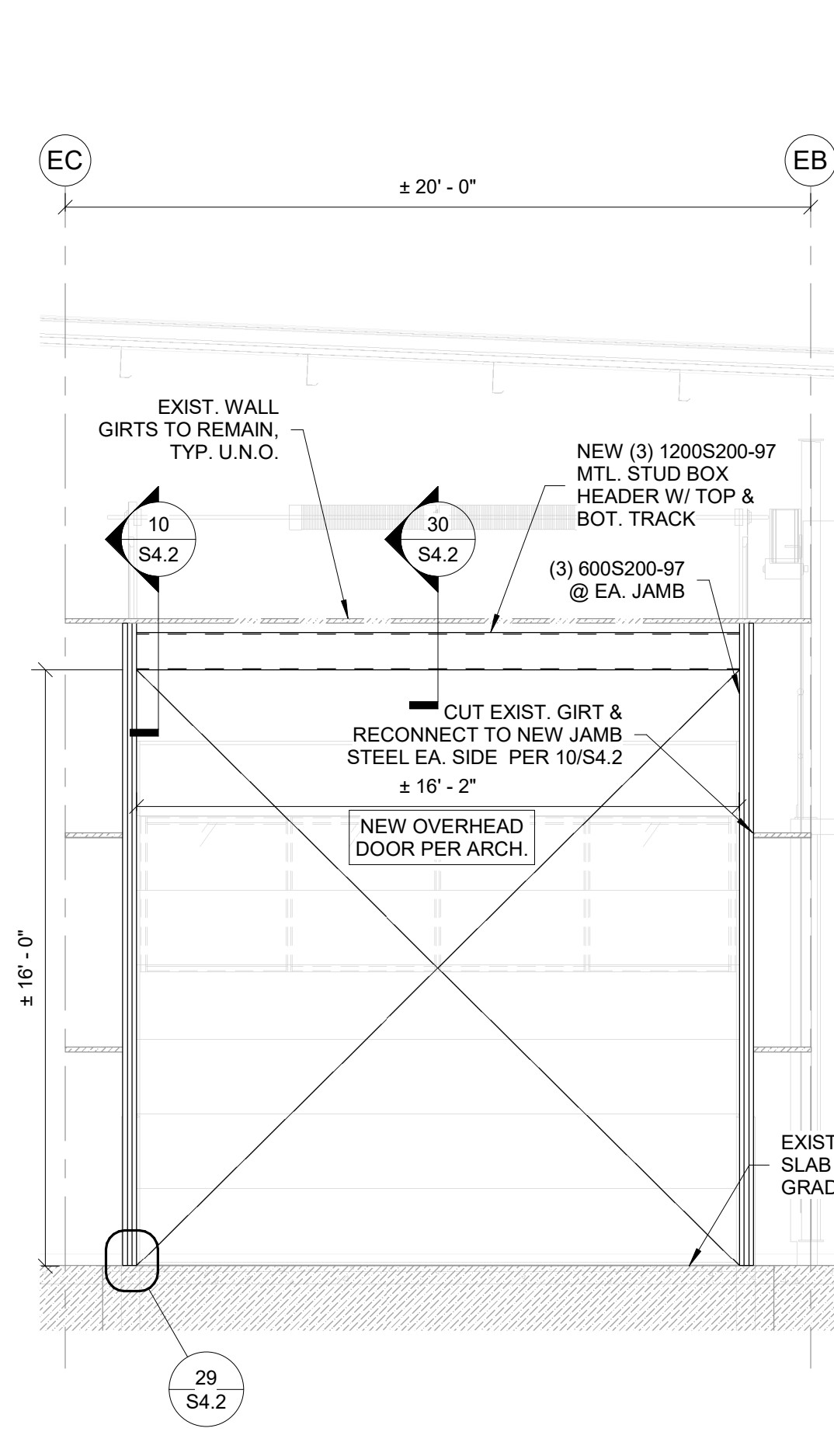
27 SECTION @ OFFICE ENTRANCE
SCALE: 3/4" = 1'-0"



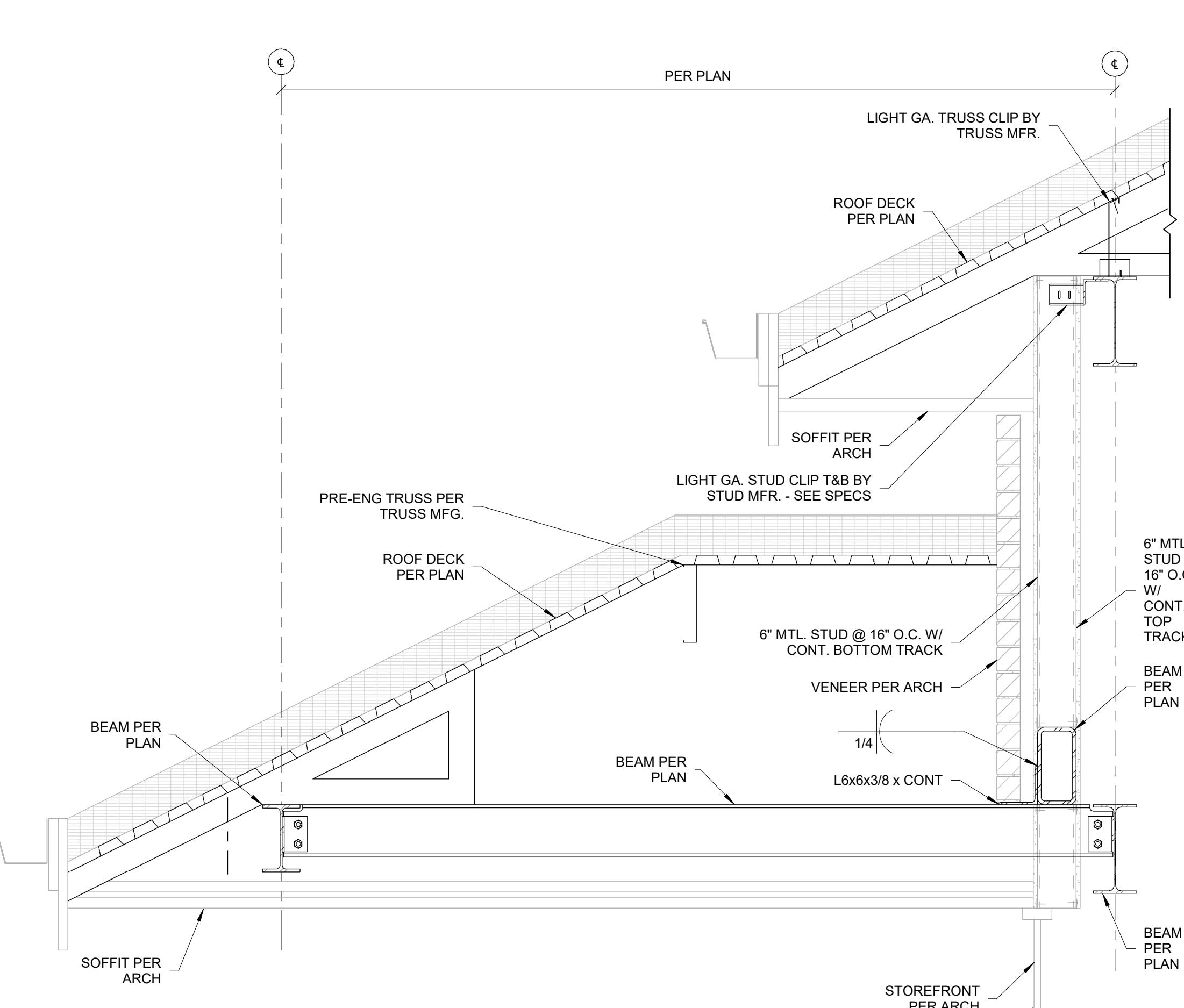
26 FRAMING SECTION @ ENTRANCES
SCALE: 3/4" = 1'-0"



21 SECTION @ OPERATIONS BLDG. NEW OVERHEAD DOOR
SCALE: 1/4" = 1'-0"



11 SECTION @ OPERATIONS BLDG. NEW OVERHEAD DOOR
SCALE: 1/4" = 1'-0"



6 FRAMING SECTION @ FRONT ENTRANCE
SCALE: 3/4" = 1'-0"