COMMONWEALTH OF KENTUCKY
BEFORE THE PUBLIC SERVICE COMMISSION

In the Matter of:

THE APPLICATION OF APC TOWERS IV, LLC, AND CELLCO PARTNERSHIP D/B/A VERIZON WIRELESS FOR ISSUANCE OF A CERTIFICATE OF PUBLIC CONVENIENCE AND NECESSITY TO CONSTRUCT A WIRELESS COMMUNICATIONS FACILITY IN THE COMMONWEALTH OF KENTUCKY IN THE COUNTY OF RUSSELL	)	CASE NO. 2026-00027

SITE NAME: JABEZ

* * * * * * *

RESPONSE TO PSC MARCH 26, 2026, NOTICE OF DEFICIENCY
CELLCO PARTNERSHIP, D/B/A VERIZON WIRELESS AND APC TOWERS IV, LLC

Public Service Commission (PSC) Staff's Rejection to application:

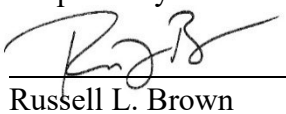
1. 807 KAR 5:063 Section 1(1)(d): The geotechnical investigation report has the seal of a professional engineer that is registered in Kentucky but does not have the signature of the professional engineer that is registered in Kentucky.
2. Attached as **Exhibit 1** please find updated Exhibit H – Geotechnical Investigation Report with the appropriate signature(s).
3. Attached hereto as **Exhibit 2** please find an Affidavit of Certification for all information contained in this application.
4. All Exhibits to this Application are hereby incorporated by reference as if fully set out as part of the Application.
5. All responses and requests associated with this Application may be directed to:

Russell L. Brown
Clark, Quinn, Moses, Scott & Grahn, LLP
320 North Meridian Street, Suite 1100
Indianapolis, IN 46204
Phone: (317) 637-1321

FAX: (317) 687-2344
Email: rbrown@clarkquinnlaw.com

WHEREFORE, Co-Applicants respectfully request that the PSC accept the foregoing Application for filing and, having met the requirements of KRS §§278.020(1), 278.650, and 278 .665 and all applicable rules and regulations of the PSC, grant a Certificate of Public Convenience and Necessity to construct and operate the WCF at the location set forth herein.

Respectfully submitted,



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Email: rbrown@clarkquinnlaw.com
Attorney for Cellco Partnership d/b/a Verizon Wireless

LIST OF EXHIBITS

1 Geotechnical Investigation Report with signature(s)
2 Affidavit of Certification

GEOTECHNICAL REPORT OF SUBSURFACE INVESTIGATION

January 12, 2026

PROPOSED GUYED TOWER
CK JABEZ
(KY-4128)

KY Highway 96
Nancy, KY 42602

36.985580, -84.891529

Prepared for:

APC Towers

Prepared by:

ENGINEERED TOWER SOLUTIONS

Raphael Mohamed
Digitally signed by Raphael Mohamed
DN: CN=Raphael Mohamed, dnQualifier=A01410C00000198AEAA697700124F74, O=Engineered Tower Solutions, C=US
Date: 2026.03.27 12:47:13-04'00'

STATE OF KENTUCKY
RAPHAEL MOHAMED
24429
LICENSED PROFESSIONAL ENGINEER

Matt Nesbit

Matt Nesbit, P.E.
Geotechnical Engineer II

Raphael Mohamed, P.E.
Registered KY 24429

Reviewed by: Jorge Varela, P.E.
Senior Geotechnical Engineer

Engineered Tower Solutions, PLLC - 3227 Wellington Court - Raleigh, NC 27615
(919) 782-2710

ENGINEERED TOWER SOLUTIONS

Project Summary

Item	Description
Project Description	A geotechnical exploration and report have been prepared for this proposed 255-foot guyed tower with 10-foot lighting arrestor. Included in this report are the results of the field exploration and the recommendations for the design of the foundation system.
Site Coordinates	Latitude: 36.985580 Longitude: -84.891529
Site Condition	The proposed tower will be installed at KY Highway 96 in Nancy, Kentucky.
Frost Depth	Based on the TIA Standard (TIA-222-H), dated October 2017, the recommended design frost penetration depth to be used for Pulaski County, KY is 30 inches (2.5 ft).
Groundwater	Groundwater was not encountered at the time of drilling. Please note that subsurface water levels will fluctuate with seasonal and cyclical temperatures and precipitation and can be higher or lower at other times.
Proposed Foundation	We assume the proposed foundation will be supported with either pad and pier or drilled shaft (caisson).

Field Exploration

Item	Description
Date	January 6th, 2026
Number of Borings	4
Location	B-1: Tower Center (Latitude: 36.985580 Longitude: -84.891529). Borings B-2 through B-4 at location of anchors.
Equipment Used	CME 45
Advancement Method	Hollow Stem Auger (HSA) and Rock Coring
Sampling Method	ASTM D-1586 with 1.5 I.D. Split Spoon Sampler ASTM D2113 Standard Practice for Rock Core Drilling and Sampling of Rock for Site Exploration

Laboratory Classification and Testing

Standard	Description
ASTM D2488	Standard Practice for Description and Identification of Soils

Subsurface Profile

Based on the results of our borings, the soils beneath the surface can be summarized in the table below:

Material Encountered	Approximate Depth to Bottom of Stratum	Description	Consistency / Density
CLAY	10	Brown and red sandy lean clay	Medium Stiff to Hard
PWR	15	Partially Weathered Rock sampled as limestone	--
ROCK	20.5	Highly fractured and moderately to highly weathered limestone with thin interbeds of shale.	--

Detailed descriptions of conditions encountered at each exploration point are indicated on the individual logs in the Appendix B. Stratification boundaries on the boring logs represent the approximate location of changes in soil types; in-situ, the transition between materials may be gradual.

Groundwater was not encountered at the time of drilling. Groundwater levels will fluctuate with seasonal and climatic changes and may be different at other times.

Earthwork Recommendations – Equipment Mat

Earthwork is anticipated to include excavations and fill placement. The following sections provide recommendations for use in the preparation of the equipment mat foundation area and access drive.

Site Preparation

The subgrade should be evaluated under the direction of the Geotechnical Engineer. Areas where soft material are present or excessively wet or dry material should either be removed, or moisture conditioned and recompacted.

Fill Material Types

Soil Type	USCS Classification	Acceptable Parameters (for Structural Fill)
Imported Low- to Moderate- Plasticity Soil [2]	CL, ML, SC or SM	All locations and elevations
Sand / Gravel with greater than 12% fines	GW/GP, SW/SP	Crushed stone base course may be used for the access roadway or beneath shallow foundations as a replacement material for overexcavated soils.
Near-Surface On-site soils [2]	CL	On-site soils generally appear suitable for use as fill when they contain at least 12% fines (clay and/or silt) and are compacted at an appropriate moisture content.

1. Controlled, compacted fill should consist of approved materials that are free of organic matter and debris. A sample of each material type should be submitted to the geotechnical engineer for evaluation.
2. Low- to moderate-plasticity cohesive soil or granular soil having at least 12% fines

Fill Compaction Requirements

Item	Structural Fill	General Fill
Maximum Lift Thickness	8 inches or less in loose thickness when heavy, self-propelled compaction equipment is used	Same as Structural fill
Minimum Compaction Requirements [1, 2]	98% of max. below foundations and within 1 foot of finished pavement subgrade 95% of max. above foundations, below floor slabs, and more than 1 foot below finished pavement subgrade	92% of max.
Water Content Range [1]	Low plasticity cohesive: -2% to +3% of optimum High plasticity cohesive: 0 to +4% of optimum Granular: -3% to +3% of optimum	As required to achieve min. compaction requirements

1. Maximum density and optimum water content as determined by the standard Proctor test (ASTM D 698).
2. High plasticity cohesive fill should not be compacted to more than 100% of standard Proctor maximum dry density.

Excavations

Groundwater was not encountered at the time of drilling. Although not expected, if encountered in deep trench excavations during construction, groundwater or perched groundwater will require dewatering until backfilling operations are complete.

All excavations that may be required should, at a minimum, comply with applicable local, state and federal safety regulations, including the current OSHA Excavation and Trench Safety Standards to provide stability and safe working conditions.

Slopes

For permanent slopes in unreinforced compacted fill areas, we recommended maximum configurations of 3:1 (Horizontal: Vertical) for the cohesive soils (clay) found at the site.

If steeper slopes are required for site development, stability analyses should be completed to design the grading plan. The face of all slopes should be compacted to the minimum specification for fill embankments. Fill slopes should be overbuilt and trimmed to compacted material.

Highly Plastic Soils

Highly plastic soils were encountered in the exploration. Highly plastic soils are prone to **shrink-swell behavior** with fluctuations in moisture content which can cause distress in pavements, slabs, and foundations. Two feet of vertical separation should be created between the existing highly plastic near surface soils and final design subgrades. This can be accomplished by over-excavation and replacement, raising site grades or a combination of both. Fill or backfill used to create the recommended separation should consist of imported low to moderate plasticity soil with at least 20% fines (silt and clay).

Earthwork Construction Considerations

The near-surface, on-site soils will lose strength when exposed to moisture. To the extent practical, earthwork should be performed during drier periods of weather. Increased remedial measures due to wet and soft or otherwise unsuitable conditions should be expected if earthwork is performed during colder and wetter periods of weather.

A qualified geotechnical engineer should be retained during the earthwork phase of the project to observe earthwork and to perform necessary tests and observations during subgrade preparation; to monitor proof-rolling, placement and compaction of controlled compacted fills, and backfilling of excavations to the completed subgrade.

Foundations Recommendations

The following recommendations are made based on our review of the test boring data and our past experience with similar projects and subsurface conditions. Ultimate soil strength parameters are presented on table below.

Ultimate Strength Parameters

Boring #	Depth (ft)	Unified Soil Classification	Total Unit Weight (pcf)	Friction Angle (degrees)	Cohesion (psf)
B-1	0.0 – 2.0	CL	105	--	500
	2.0 – 4.0	CL	115	--	2,400
	4.0 – 6.0	CL	115	--	1,800
	6.0 – 8.0	CL	115	--	1,800
	8.0 – 10.0	CL	115	--	1,600
	10.0 – 15.5	PWR	130	38	--
	15.5 – 20.5	ROCK	145	45	--
B-2	0.0 – 2.0	CL	105	--	600
	2.0 – 4.0	CL	105	--	500
	4.0 – 6.0	CL	115	--	2,200
	6.0 – 8.0	CL	115	--	4,700
	8.0 – 12.5	CL	105	--	700
B-3	0.0 – 2.0	CL	105	--	800
	2.0 – 4.0	CL	115	--	1,600
	4.0 – 6.0	CL	115	--	1,000
	6.0 – 8.0	CL	115	--	1,900
	8.0 – 11.5	SC	130	32	--

Boring #	Depth (ft)	Unified Soil Classification	Total Unit Weight (pcf)	Friction Angle (degrees)	Cohesion (psf)
B-4	0.0 – 2.0	CL	105	--	800
	2.0 – 4.0	CL	115	--	1,500
	4.0 – 6.0	CL	115	--	2,100
	6.0 – 8.0	CL	115	--	2,100
	8.0 – 11.3	CH	115	--	1,100

1. Groundwater was not encountered at the time of drilling. Utilize bouyon unit weight below this depth

Based on the subsurface conditions and typical design foundation loads for similar guyed towers, we recommend that either a caisson (drilled shaft) or a pad/pier be used to support the new tower.

Modulus of Subgrade Reaction

A vertical and horizontal modulus of subgrade reaction may be derived using the following equations and soils parameters expressed in the above table:

$$k_{s\text{-}v} = 12 \cdot SF \cdot q_a$$

$$k_{s\text{-}h} = k_{s\text{-}v} \cdot B$$

Where:

q_a = Allowable Bearing Capacity (ksf)
SF = Safety Factor
B = Base width (ft), use 1 if B< 1ft
$k_{s\text{-}v}$ =Vertical Modulus of Subgrade Reaction (kcf)
$k_{s\text{-}h}$ = Horizontal Modulus of Subgrade Reaction (ksf)

Caisson (Drilled Shaft)

Should caissons (drilled shafts) be used, the caissons (drilled shafts) will achieve compressive (downward) and tensile (uplift) resistance through skin friction along the sides of the shafts. In addition to skin friction, bearing resistance at the caisson's tip will contribute to compressive capacity. We recommend the values given the table below be used for this project. Please note the tip bearing capacity and skin frictions are net ultimate and ultimate values respectively. Appropriate factors of safety or resistance factors should be used. Lateral loads can be resisted by the lateral stiffness of the soil. Parameters for analysis of the laterally loaded caisson are also given the table below.

Caisson (Drilled Shaft) Parameters

Depth (ft)	Net Ultimate Tip Bearing Capacity (ksf)	Ultimate Skin Friction[1] (ksf)		Lateral Modulus (pci)	ε_{50} (in/in)
		Compressive	Uplift		
0.0 – 2.0	--	--	--	--	--
2.0 – 4.0	--	1.2	1.2	1,000	0.005
4.0 – 6.0	--	0.9	0.9	1,000	0.005
6.0 – 8.0	--	0.9	0.9	1,000	0.005
8.0 – 10.0	14	0.8	0.8	1,000	0.005
10.0 – 15.0	40	0.8	0.8	225	--
15.0 – 20.5	40	1.4	1.4	225	--

1. We recommend the skin friction be ignored for the top 3 ft of the caisson

Based on the subsurface soil conditions, excavations for the caissons (drilled shafts) should be possible using a large, truck-mounted, hydraulic-advanced drill rig. All debris, loose or disturbed soil should be removed from the excavation prior to placing reinforced steel and/or concrete. Reinforcing steel and/or concrete should be placed immediately upon completion of the excavation.

The excavations may be susceptible to caving. Drilling fluid or casing could be used to assist in keeping the drilled hole open. If casing is used, we recommend it be removed from the excavation as concrete is being placed. Continuous vibration or other approved methods should be used during

casing withdrawal to reduce the potential for void-space formation within the concrete. If water is present during concrete placement and/or drilling fluids are used to maintain hole stability, concrete should be pumped or otherwise discharged to the bottom of the hole via a hose or tremie pipe. The end of the hose or tremie pipe must remain below the top surface of any water, drilling fluid and the in-place concrete at all times. Additionally, concrete should be consolidated using vibration methods over the entire length and width of the caissons and the consolidation should be performed only after these fluids are removed and to the extent possible.

Pad & Pier / Single Mat Foundation

If the site has been prepared in accordance with the requirements noted in *Earthwork Recommendations – Equipment Mat,* the tower's foundation capacity can be determined using the soil's bearing capacity, passive pressure resistance, and a sliding friction factor.

Net Ultimate Bearing Capacity and Sliding Friction Factor

Depth[2] (ft)	Net Ultimate Bearing Capacity[1] (psf)	Sliding Friction Factor[1]
0.0 – 2.5	--	--
2.5 – 8.0	11,000	0.30
8.0 – 15.0	11,000	0.30

1. This value is a net ultimate value and an appropriate factor of safety or resistance factor should be used

Ultimate Passive Pressure and Friction Factor

Boring #	Depth (ft)	Ultimate Passive Pressure[1] (psf)[1]
B-1	0.0 – 2.0	0 – 400
	2.0 – 4.0	400 – 800
	4.0 – 8.0	800 – 1,600
	8.0 – 12.0	1,600 – 2,400
	12.0 – 20.0	2,400 – 4,000

1. Ultimate passive pressure can be interpolated for foundation depths with the depth ranges given

Anchor Blocks

Concrete gravity blocks achieve resistance against the applied loads through the weight of the concrete in the blocks, the weight of the soil above the blocks, skin friction between the block and surrounding soil, and passive pressures developed in the soil. For these calculations we recommend the following values be used:

Ultimate Passive Pressure and Friction Factor

Boring #	Depth (ft)	Ultimate Passive Pressure (psf)	Sliding Friction Factor
B-2 (Anchor)	0.0 – 2.0	0 – 400	0.30
	2.0 – 4.0	400 – 800	
	4.0 – 8.0	800 – 1,600	
	8.0 – 12.0	1,600 – 2,400	
	12.0 – 20.0	2,400 – 4,000	

Boring #	Depth (ft)	Ultimate Passive Pressure (psf)	Sliding Friction Factor
B-3 (Anchor)	0.0 – 2.0	0 – 400	0.30
	2.0 – 4.0	400 – 800	
	4.0 – 8.0	800 – 1,600	
	8.0 – 12.0	1,600 – 2,400	
	12.0 – 20.0	2,400 – 4,000	
B-4 (Anchor)	0.0 – 2.0	0 – 400	0.30
	2.0 – 4.0	400 – 800	
	4.0 – 8.0	800 – 1,600	
	8.0 – 12.0	1,600 – 2,400	
	12.0 – 20.0	2,400 – 4,000	

Seismic Parameters

The seismic design requirements for buildings and other structures are based on Seismic Design Category. Site Classification is required to determine the Seismic Design Category for a structure. The Site Classification is based on the upper 100 feet of the site profile defined by a weighted average value of either shear wave velocity, standard penetration resistance, or undrained shear strength in accordance with Section 20.4 of ASCE 7 and the International Building Code (IBC)

Seismic Site Classification

Item	Seismic Parameter
2018 International Building Code Seismic Site Classification	D[1]
Design Spectral Response Acceleration Parameters	S_{ds} = 0.203g S_{d1} = 0.151g

1. The IBC seismic site classification is based on the subsurface profile depth of 100 feet. The scope of work did not authorize exploration to a depth of 100 feet. A seismic Site Soil Classification of D should be used if insufficient details are known about the 100-foot soil profile.

Soil Resistivity

Laboratory soil resistivity test were conducted according to procedures designated in ASTM G187 and test results are presented in the following table. Soil resistivity values will vary with temperature and moisture content changes and may be different at other times.

Soil Resistivity Testing Results

Boring	Sample Depth (ft)	Resistivity (Ohm-cm)
B-1	1.5 – 3.0	44,000

LIMITATIONS OF REPORT

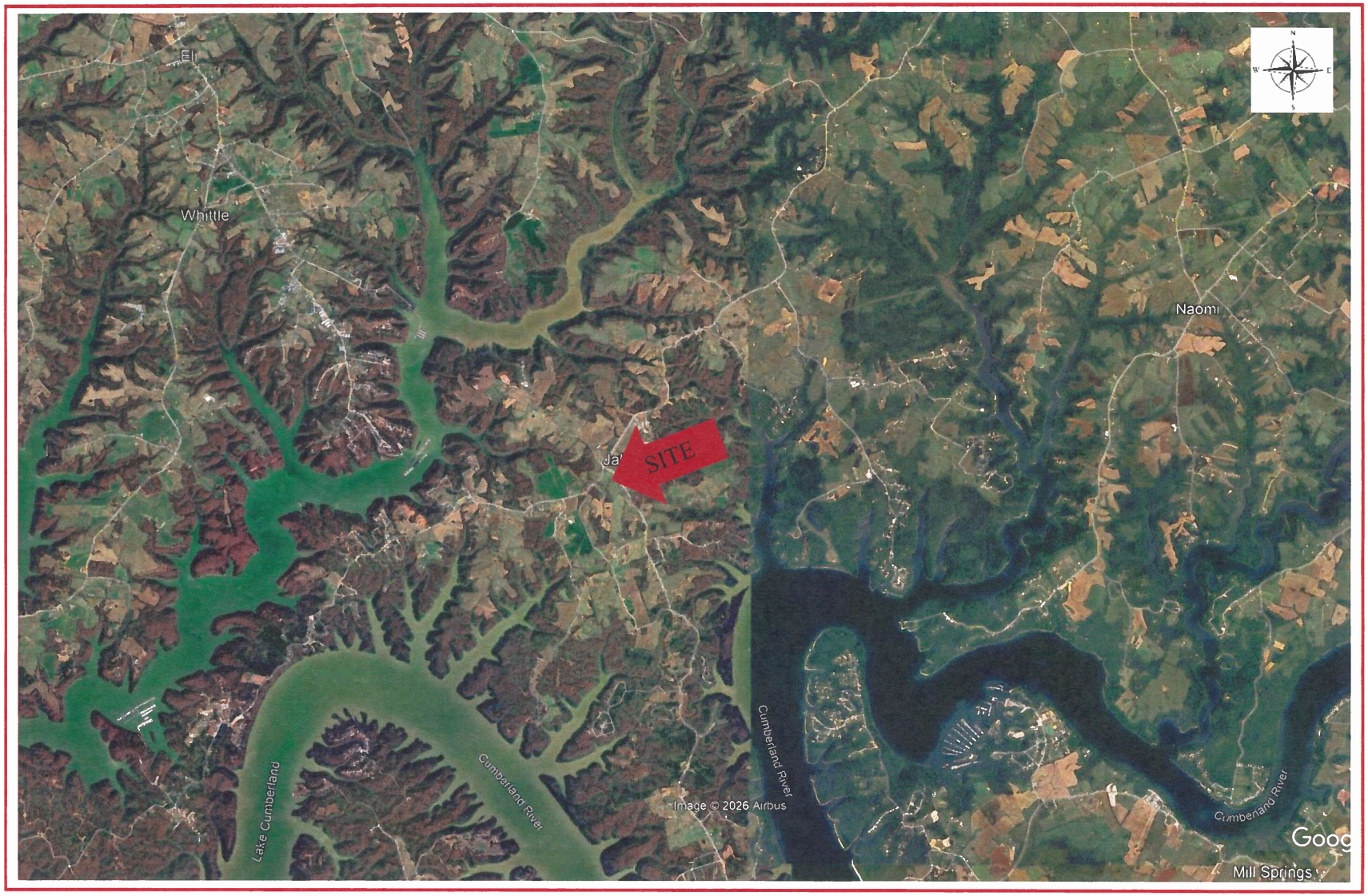
This report has been prepared in accordance with generally accepted geotechnical engineering practices for the specific application of this project. The conclusions in this report are based on the applicable standards of our practice in this geographic area at the time this report was prepared. No other warranty, expressed or implied, is made.

The analyses and conclusions submitted herein are based, in part, upon the data obtained from the subsurface exploration performed for this analysis. The soil and ground water conditions can vary across the site. Opinions and conclusions are subject to change if new or additional information is submitted for review.

APPENDIX A
LOCATION INFORMATION

SITE LOCATION PLAN
CK JABEZ (KY-4128)
Job Number: 25142527

ENGINEERED
TOWER SOLUTIONS



BORING LOCATION PLAN
CK JABEZ (KY-4128)
Job Number: 25142527

ENGINEERED
TOWER SOLUTIONS

SITE PHOTO
CK JABEZ (KY-4128)
Job Number: 25142527

ENGINEERED
TOWER SOLUTIONS

APPENDIX B
SOIL TEST BORING

BORING NUMBER B-1

ENGINEERED TOWER SOLUTIONS

CLIENT APC Towers
PROJECT NAME CK Jabez
PROJECT NUMBER 25142527
PROJECT LOCATION KY Highway 96, Nancy, KY 42602
DATE 1/6/2026
COORDINATES 36.98558, -84.891529
DRILLING METHOD Hollow Stem Auger (HSA) and Rock Coring
GROUND WATER LEVELS:
DRILLING EQUIPMENT CME 550
AT TIME OF DRILLING --- Not Encountered
LOGGED BY M. Nesbit
AT END OF DRILLING --- Not Encountered
NOTES
AFTER DRILLING --- Not Encountered

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)
0		**SANDY LEAN CLAY (CL),** brown and red, moist, medium stiff.	SS 1		3-2-3 (5)
		SANDY LEAN CLAY (CL), brown and red, moist, very stiff.	SS 2		6-10-14 (24)
5			SS 3		5-8-10 (18)
			SS 4		5-9-9 (18)
10			SS 5		4-6-10 (16)
		PARTIALLY WEATHERED ROCK (PWR), , Sampled as weathered limestone.	SS 6		50/1"
15		**LIMESTONE,** , Highly fractured and moderately to highly weathered limestone with thin interbeds of shale..	RC RC-1	77 (47)	
20		Bottom of Borehole at 20.5 feet.			

BORING NUMBER B-2

ENGINEERED TOWER SOLUTIONS

CLIENT APC Towers
PROJECT NAME CK Jabez
PROJECT NUMBER 25142527
PROJECT LOCATION KY Highway 96, Nancy, KY 42602
DATE 1/6/2026
COORDINATES 36.986044, -84.891687
DRILLING METHOD Hollow Stem Auger (HSA)
GROUND WATER LEVELS:
DRILLING EQUIPMENT CME 550
AT TIME OF DRILLING --- Not Encountered
LOGGED BY M. Nesbit
AT END OF DRILLING --- Not Encountered
NOTES
AFTER DRILLING --- Not Encountered

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)
0		**SANDY LEAN CLAY (CL),** brown and red, moist, medium stiff.	SS 1		3-3-3 (6)
			SS 2		2-3-2 (5)
		SANDY LEAN CLAY (CL), brown and red, moist, very stiff to hard.			
5			SS 3		4-7-15 (22)
			SS 4		9-22-25 (47)
		SANDY LEAN CLAY (CL), brown and red, moist, medium stiff.			
10			SS 5		4-4-3 (7)

Auger Refusal at 12.5 feet.

BORING NUMBER B-3

ENGINEERED TOWER SOLUTIONS

CLIENT APC Towers
PROJECT NAME CK Jabez
PROJECT NUMBER 25142527
PROJECT LOCATION KY Highway 96, Nancy, KY 42602
DATE 1/6/2026
COORDINATES 36.985458, -84.89095
DRILLING METHOD Hollow Stem Auger (HSA)
GROUND WATER LEVELS:
DRILLING EQUIPMENT CME 550
AT TIME OF DRILLING --- Not Encountered
LOGGED BY M. Nesbit
AT END OF DRILLING --- Not Encountered
NOTES
AFTER DRILLING --- Not Encountered

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)
0		**SANDY LEAN CLAY (CL),** brown and red, moist, stiff to very stiff.	SS 1		3-3-5 (8)
			SS 2		4-7-9 (16)
5			SS 3		3-4-6 (10)
			SS 4		5-7-12 (19)
		CLAYEY SAND (SC), gray, dry, very dense, with rock fragments.			
10			SS 5		7-28-30 (58)

Auger Refusal at 11.5 feet.

BORING NUMBER B-4

ENGINEERED TOWER SOLUTIONS

CLIENT APC Towers
PROJECT NAME CK Jabez
PROJECT NUMBER 25142527
PROJECT LOCATION KY Highway 96, Nancy, KY 42602
DATE 1/6/2026
COORDINATES 36.985239, -84.891951
DRILLING METHOD Hollow Stem Auger (HSA)
GROUND WATER LEVELS:
DRILLING EQUIPMENT CME 550
AT TIME OF DRILLING --- Not Encountered
LOGGED BY M. Nesbit
AT END OF DRILLING --- Not Encountered
NOTES
AFTER DRILLING --- Not Encountered

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)
0		**SANDY LEAN CLAY (CL),** brown and red, moist, stiff to very stiff.	SS 1		3-3-5 (8)
			SS 2		5-7-8 (15)
5			SS 3		7-10-11 (21)
			SS 4		6-7-13 (20)
		SANDY FAT CLAY (CH), brown and red, moist, stiff.			
10			SS 5		4-5-6 (11)

Auger Refusal at 11.3 feet.

STATE OF INDIANA)
)SS:
COUNTY OF MARION)

AFFIDAVIT OF CERTIFICATION
COMMONWEALTH OF KENTUCKY
PUBLIC SERVICE COMMISSION

I, Russell L. Brown, attorney for Cellco Partnership d/b/a Verizon Wireless do hereby certify that as the person supervising the preparation of this application and all statements and information contained herein are true and accurate to the best of my knowledge, information, and belief formed after a reasonable inquiry for all information within this application.

Russell L. Brown
Attorney for Cellco Partnership d/b/a Verizon Wireless

STATE OF INDIANA,
COUNTY OF MARION, SS:
Subscribed and sworn to before me this 30th day of March, 2026.

Notary Public
Printed Name of Notary: Megan N. Webb
My commission expires: May 24, 2028
My County of Residence: Marion
Commission #: 0634690