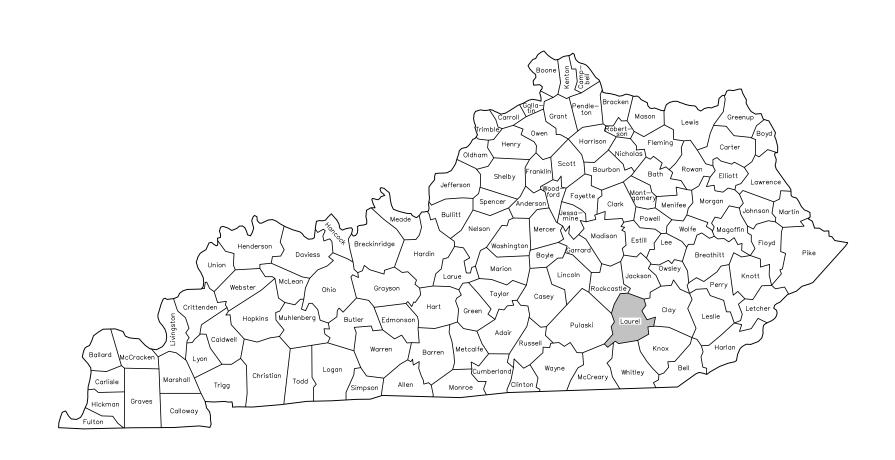
# EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS CONTRACT 2 LAUREL COUNTY, KENTUCKY



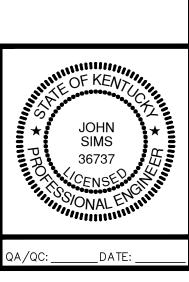


# **INDEX OF SHEETS**

<b>DESCRIPTION</b>	SHEET NO.
COVER SHEET	
GENERAL NOTES	1
TOPOGRAPHICAL LAYOUT	2
OLD SALEM ROAD (WEST)	3
MAPLESVILLE SCHOOL ROAD	4 - 7
OLD SALEM ROAD (EAST)	8 - 9
HODGE ROAD (ALT 1)	10
NORTH LAUREL HIGH SCHOOL WL (ALT 2)	11
<b>KY 472 PUMP STATION SITE PLAN</b>	12
<b>KY 472 PUMP STATION PLAN/SECTIONS</b>	13
PUMP STATION DETAILS	14
DIRECTIONAL BORE-LITTLE LAUREL RIVER	<b>B</b> 1
MISCELLANEOUS DETAILS	D1 - D3
ELECTRICAL PLANS	E1 - E4
STRUCTURAL PLANS	S1 - S5

**Prepared By:** 





- 3. The Contractor is solely responsible for determination of the existence and location of any and all other buried utilities in the vicinity of his work. Utilities shown on the Project Drawings are purported to be approximate only and not warranted to be complete nor accurately located. Additional buried utility lines, other than as shown on the Project Drawings, may exist in the vicinity of the Project Work. The Contractor shall contact local utilities and/or locating service at least 48 hours prior to commencing work on the Project and cause the buried utilities to be surface marked prior to excavation.
- 4. All excavation in the vicinity of any existing fiber optic telecommunication line(s) shall be conducted in the presence of an authorized representative of the owner of said fiber optic line(s) and in a manner (i.e. hand digging, hydro—excavation, etc.) specifically approved and supervised by the representative of the Owner of said fiber optic line(s).
- 5. All pipe trench under any public street or highway pavement or any residential or commercial driveway or entrance, including stone travel way surface, shall be backfilled fully with compacted No. 9 crushed stone and the surface repaired as shown on the detail sheets of these Project Drawings.
- 6. The Contractor shall be responsible for all traffic control measures necessary for the safe execution of his work, including but not limited to flaggers, traffic signage, barricades, construction fencing and nighttime warning lights. Traffic safety provisions shall be employed by the Contractor in accordance with the Standards of the applicable State and Local Public Highway Authorities.
- 7. The Contractor shall conduct his work such as to maintain at least one (1) emergency vehicle access route to every property in the Project area at all times during the construction period.
- 8. All excavation shall be considered unclassified excavation. No additional payment shall be due and payable to the Contractor for excavation and removal of rock or for the required dewatering of any trench or other excavation.
- 9. All work shall be provided in accordance with the construction authorization (permit) issued by the Kentucky Department for Environmental Protection, Division of Water. The Owner will secure said authorization and deliver a copy to the Contractor, to be maintained on—site during construction.
- 10. All work shall be provided in compliance with all applicable local, state, and national building and electrical codes.
- 11. All work shall be executed in compliance with the current workplace safety regulations of the U.S. Department of Labor, Occupational Safety and Health Administration (O.S.H.A.).
- 12. Any work within Kentucky State Highway right-of-way shall be provided in accordance with the Kentucky Transportation Cabinet/Department of Highways "Standard Specifications for Road and Bridge Construction", 2008 edition and as approved by the duly authorized KDOH authority on the site.
- 13. Existing utility lines may be cathodically protected. Any ductile iron pipe, fittings or appurtenances within 100' of said protected utility lines shall comply with ASTM A674 (polyethylene encasement), latest revision, and at no additional cost to the Owner.
- 14. The Contractor shall repair/replace any and all existing utility lines and equipment damaged by the Contractor's work, to the satisfaction of the damaged utility and at no additional cost to the Owner.
- 15. The Contractor shall protect all sewers, drainage culverts, and storm sewers in the vicinity of their work, and shall repair or replace all culverts damaged by their work at no additional cost to the Owner.
- 16. Prior to cutting existing driveways or limiting or restricting access, the Contractor shall notify the property owner/occupant at least 24 hours in advance and shall schedule their work such as to restrict access to any individual property not more than 6 hours in one (1)
- 17. Pipe crossing or installed under bituminous paved surfaces shall be open (saw) cut and repaired as specified, unless otherwise shown and noted on the Project Drawings to be bored and jacked.
- 18. If applicable, the Owner will secure authorization for construction encroachment within Kentucky State Highway right-of-way and deliver same to the Contractor. The Contractor shall not commence any work within Kentucky State Highway right—of—way until they have received written authorization from the Kentucky Department of Highways (KDOH) to do so, including all terms of the authorization.
- 19. The Contractor shall be responsible for securing any and all street/road-cut permits from City and County highway authorities as necessary to lawfully construct the Project within City/County public street/road right-of-way.
- 20. The Contractor shall be responsible for securing and posting any and all construction bonds required by the City and/or County Roadway Authorities, as applicable, for construction within the public right-of-way. The cost of said bond(s) shall be included in the Contractor's Bid.

# GENERAL NOTES (cont.)

- 21. The contractor shall obtain and pay for all grading, storm water, etc. permits, if required to complete the work. The contractor shall maintain compliance with all conditions, limitations and stipulations of all permits. The contractor shall not commence work, except mobilization. until he has obtained all required permits for said work. The contractor shall supply the owner with copies of all permits within 24 hours of receipt. A KPDES Storm Water Discharge Permit will be required for this project. The contractor shall fill out, sign and submit the Notice of Intent (NOI) and the Notice of Termination (NOT). The Notice to Proceed will not be issued until acknowledgement of submission of the NOI has been received from Division of
- 22. After the project is constructed and complete, it shall be the responsibility of the Contractor to provide the Engineer with one set of "As-Built" drawings before the final payment is released to the Contractor. The "As-Built" drawings shall include, at a minimum, the distance to each gate valve, fire hydrant, blowoff assembly, air release valve, leak detection meter, or any other appurtenance installed from two permanent features visible on the plan drawings, the distance between the pipeline and any permanent features visible on the plan drawings a minimum of every 300 feet. Each water meter shall be located by distances from two permanent features visible on the plan drawings.
- 23. The Contractor shall be responsible for coordinating all construction work with local utility companies and other concerned parties.
- 24. The Contractor shall have on hand at the job site 11 1/4°, 22 1/2°, 45° and 90° bends for use where necessary for proper installation.
- 25. Pipe joint deflection shall not exceed 2°. Bending of PVC pipe will not be allowed.
- 26. At some locations, the Contractor may be required to provide extra cover over line. Cost of extra cover is to be included in unit price bid for line installation and no separate payment will be made for such extra cover. All such locations are shown on the plans.
- 27. Connecting new lines to existing lines is subsidiary to the contract unless specifically itemized in the Bid Schedule. It includes fittings, sleeves, etc., but does not include gate valves, which are an extra pay item.
- 28. All fittings, thrust restraints and appurtenances to construct the pipelines as shown shall be
- 29. Ductile iron pipe shall be installed in accordance with Standard AWWA C150/ANSI A21.50 Laying Condition Type 3 unless otherwise noted.
- 30. It is the responsibility of the Contractor to comply with all regulations regarding the effect on the environment from the discharge of chlorinated water. See Technical Specifications for methods of sterilization and for disposing of heavily chlorinated water.
- 31. The time period for pressure testing in this project shall be 6 hours.

included in the unit cost for the pipe and are not separate pay items.

- 32. Tracer Tape and Tracer Wire shall be installed with the PVC pipe. See Technical Specifications and the miscellaneous details drawings.
- 33. During the process of tapping asbestos concrete mains, the contractor shall conform to OSHA regulations governing the handling of hazardous waste. Pieces of asbestos concrete resulting from the tap shall be double bagged, placed in a rigid container and disposed of in an approved landfill.
- 34. A mandrel/pig shall be used for the preliminary cleaning of each section of piping prior to desinfection for all waterlines 3—inches or greater prior to pressure testing and sterilization. This is not a separate bid item and shall be included in the unit price for pipe. See Technical Specifications.
- 35. The GENERAL CERTIFICATION NATIONWIDE PERMIT #58 UTILITY LINE BACKFILL AND BEDDING is contained in the Specifications. The Contractor shall read, understand and comply with the requirements and procedures. All crossings of streams that appear as a blue line on a USGS 7.5 minute topographical map shall be accomplished in accordance with: PERMIT #58, UTILITY LINE BACKFILL AND BEDDING. It is the intent of the plans to identify a stream crossing at each blue line stream. Small creek crossings, less than 15 feet measured from top of bank to top of bank, may be accomplished by trenching when the stream is in a no-flow condition. If the stream is in a flow condition, the crossing shall be accomplished by directional boring or other method that complies with the General Certification and is approved by the Engineer. Specific details for stream crossings are contained in the Miscellaneous Details. Bid items for specific stream crossings may be contained in the Bid Schedule with the type of crossing shown on the Plan Sheets. Payment shall be "Each" for directional bores of small stream crossings. All small stream crossings in the project shall be considered the same regardless of width (up to 15 L.F.) or depth. It is the responsibility of the Contractor to determine an average unit price that will be used for payment for each instance a blue line stream is crossed. Stream crossings may be added, for extended lines beyond those shown on the plans, at the same unit price providing the crossings are reasonably similar to those in the initial project. Stream crossings may be deleted, without effecting the unit price, if a line is deleted or shortened. Payment for specific bid item directional bored stream crossings shall be "Lump Sum".
- 36. Rough cleanup is included in the unit price for pipe installation and must be done before payment for pipe will be approved.
- 37. All water main fittings shall be ductile iron, mechanical joint compact fittings for water service complying with AWWA Standard C153. Unless otherwise specifically shown or noted, no PVC fitting, other than in-line repair couplings, will be accepted.
- 38. All water main fittings shall be anchored with poured concrete thrust blocks as shown in the miscellaneous details drawings. Wrap fittings in minimum 5-mil plastic (PVC) wrap prior to forming and pouring the block.
- 39. No water service shall be activated until the new work has been completed, sterilized, and tested in accordance with the Contract Documents and accepted in writing by the Owner.

# **ENVIRONMENTAL NOTES**

- 1. When crossing all streams, silt barriers, ie. straw bales or silt fences, shall be put in place to prevent sediment runoff into stream. Conventional stream crossings shall be accomplished during low flow periods. Stream banks shall be reseeded with native vegetation beneficial to wildlife immediately following completion of the stream crossing. Disturbed surfaces shall be restored to original contours and excess materials removed to a properly confined area.
- 2. If the removal of any trees greater than (6) inches in diameter at breast height is required. The tree removed shall be accomplished between October 15 and March 31.
- 3. Any excavation by the Contractor that uncovers a historical or archaeological artifact shall be immediately reported to the Owner and Engineer. Construction shall be temporarily halted pending the notification process and further directions after consultation with the State Historic Preservation Officer (SHPO).

# HIGHWAY DEPARTMENT NOTES

- 1. Underground utilities constructed inside state right of way shall be installed with a minimum depth of cover of 42 inches.
- 2. Underground utilities crossing any paved driveway inside state right of way shall be installed by boring unless written permission to open cut is obtained from the property owner.
- 3. Underground utilities shall not be installed in embankment fills or between edge of pavement and ditchline unless specifically noted on permitted plans.
- 4. Contact KYTC-DOH district office prior to beginning work.
- 5. All affected KYTC ditchlines shall remain free of excess silt or erosion and constructed to the normal typical section of the roadway with a minimum depth of 18 inches from the shoulder break point.

In compliance with the Kentucky Dig Law, the Contractor shall call (800) 752—6007

the Contractors responsibility to coordinate excavation with all Utility Owners.

(Kentucky811) toll free or dial 811 a minimum of two and no more than ten business days

prior to excavation for information of the location of existing underground utilities. It will be

6. All necessary steps shall be taken to prevent erosion or siltation of the public right-of-way, adjoining property and waterways.

JC OA DISTRI RTER RC F LAUREL WATER I SALEM ROAD/McWHOR SYSTEM IMPROVEME CONTRACT 2 H O

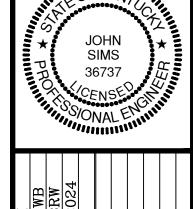
PROJECT NO. 2020052

EAST LA



PROJECT NO. 2020052

EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS



CHECKED BY: EWB
CHECKED BY: EWB
CHECKED BY: BRW
DATE: March 2024
SCALE: 1"=100'
REVISIONS

KENVIRONMENTAL BIOGINGERS



PROJECT NO.
2020052

SHEET NO.

DRAWN BY: PTH
CHECKED BY: EWB
CHECKED BY: EWB
CHECKED BY: EWB
DATE: March 2024
SCALE: 1"=100'
REVISIONS

KENVIRONS

Civil & Environmental Engineers



PROJECT NO. **2020052** 

SHEET NO.

CT 2

LER DISTRICT

CWHORTER ROAD

OVEMENTS EAST LAUREL OLD SALEM ROA



PROJECT NO. 2020052

EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS

DRAWN BY: PTH
CHECKED BY: EWB
CHECKED BY: BRW
DATE: March 2024
SCALE: 1"=100'
REVISIONS

LENVIRONS

vil & Environmental Engineers



PROJECT NO. 2020052

SHEET NO.

EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS

JOHN SIMS 36737

CHECKED BY: EWB
CHECKED BY: EWB
DATE: March 2024
SCALE: 1"=100'
REVISIONS

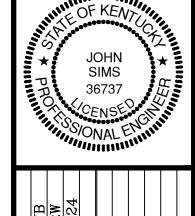
ENVIRONS & Environmental Engineers



PROJECT NO. **2020052** 

SHEET NO.

EAST LAUREI OLD SALEM ROA





PROJECT NO. 2020052

EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS

JOHN \* SIMS 36737 \*\*

\*\*CENSED CHARLES IN THE STATE OF KENTUCKY AND THE SIME STATE OF THE STATE

CHECKED BY: EWB
CHECKED BY: BRW
DATE: March 2024
SCALE: 1"=100'
REVISIONS

CHECKED
CHECKED
DATE: Mar
SCALE: 1"=
REVIS

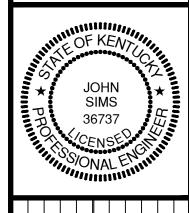
KENVIRON
Civil & Environmental Enginee



PROJECT NO. **2020052** 

SHEET NO.

EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS



CHECKED BY: JRP
CHECKED BY: EWB
CHECKED BY: BRW
DATE: March 2024
SCALE: 1"=100'
REVISIONS

KENVIRONS Civil & Environmental Engineers



PROJECT NO. **2020052** 

SHEET NO.

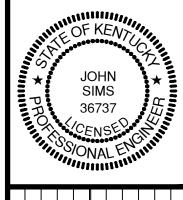
EAST OLD SA



PROJECT NO. 2020052

SHEET NO.

EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS CONTRACT 2



DRAWN BY:JKP
CHECKED BY: EWB
CHECKED BY: BRW
DATE: March 2024
SCALE: 1"=10'
REVISIONS

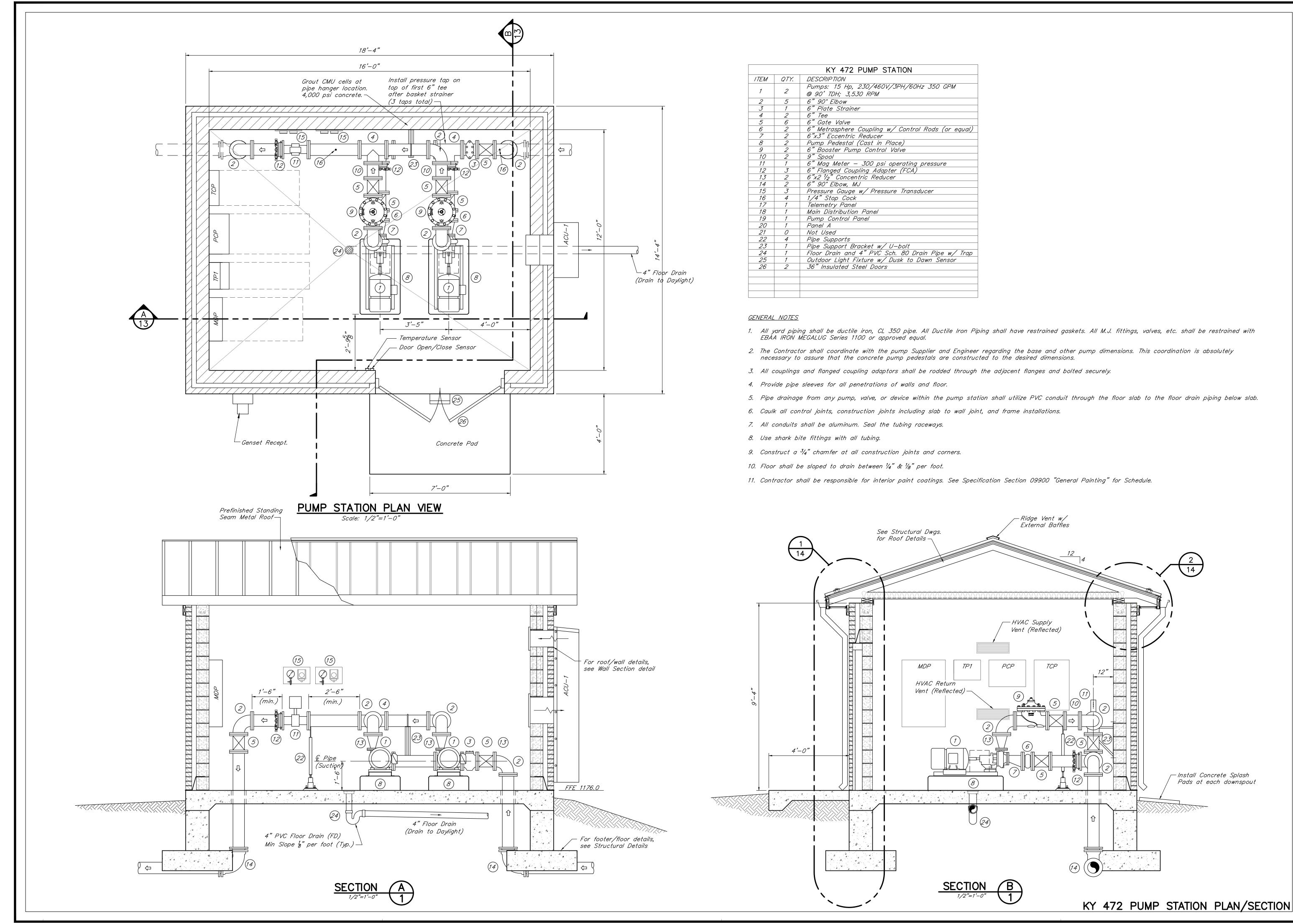
KENVIRONS

Sivil & Environmental Engineers



PROJECT NO. **2020052** 

SHEET NO.



PROJECT NO. 2020052

SHEET NO. 14



─Ball Valve (typ.)

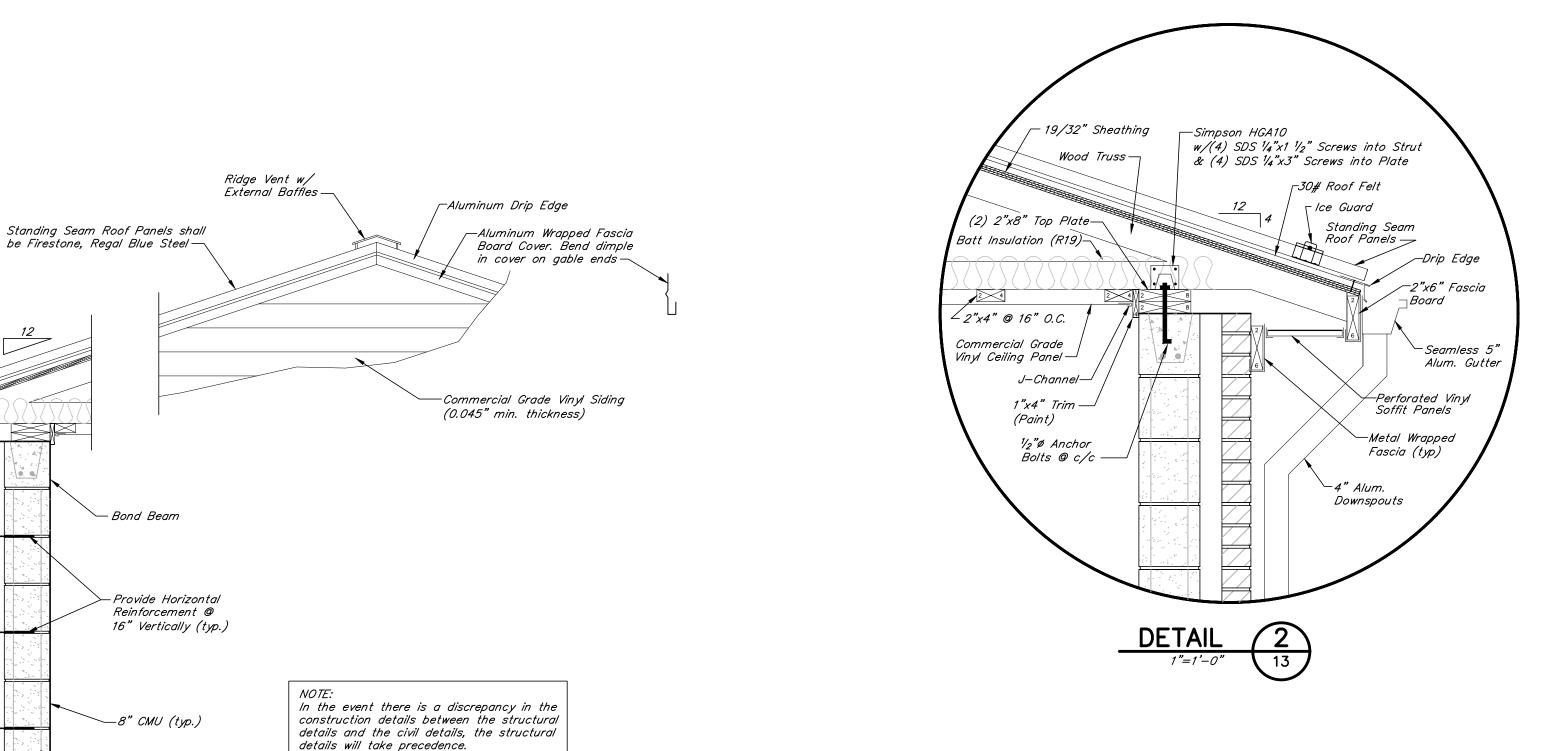
─\_To Floor Drain

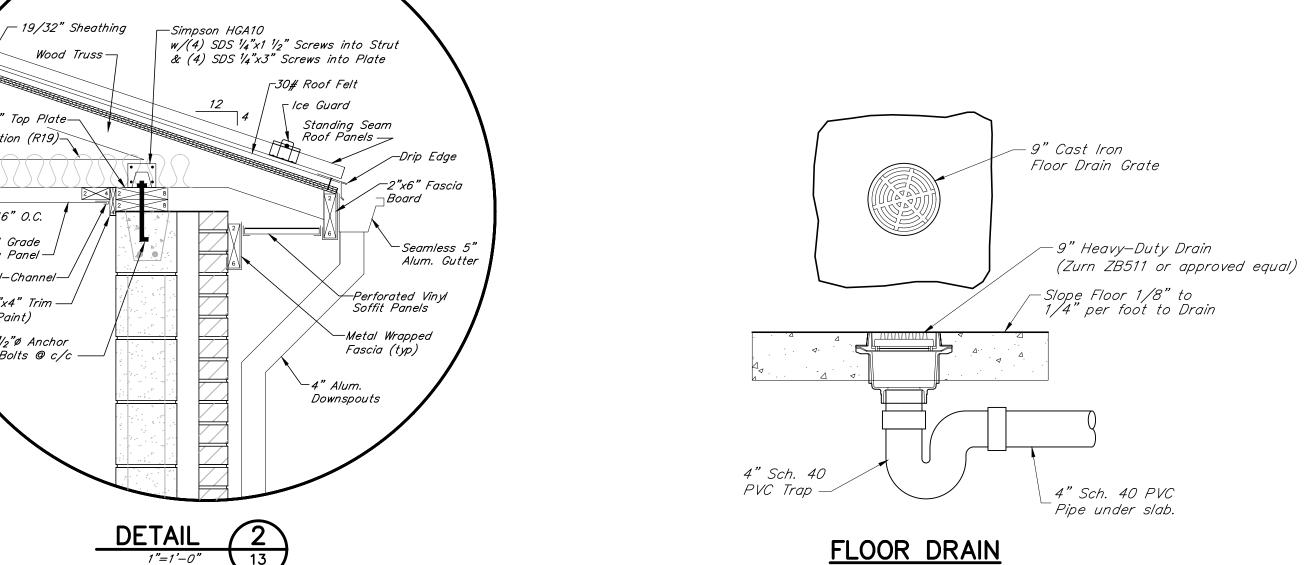
seal-offs.

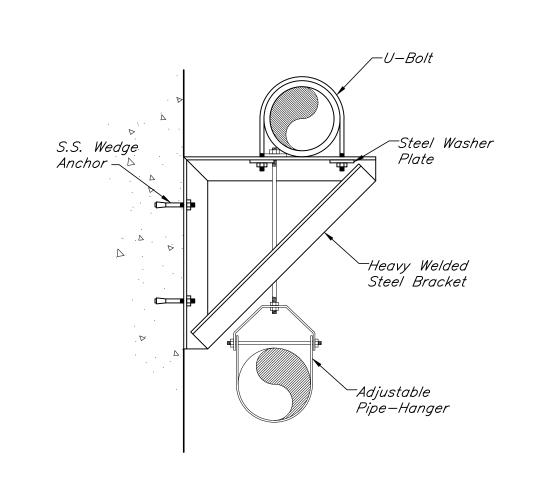
PRESSURE MONITORING PANELS

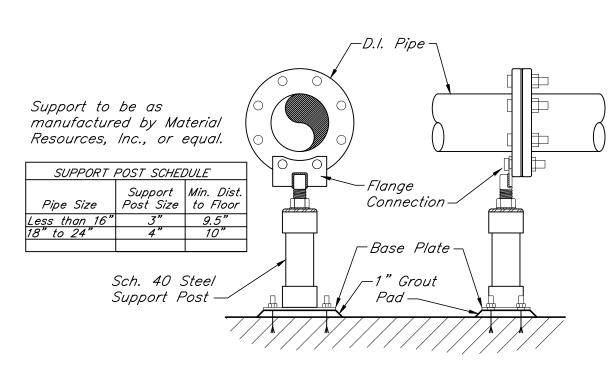
For long runs of tubing, 3/4" Aluminum conduit shall be used to run the

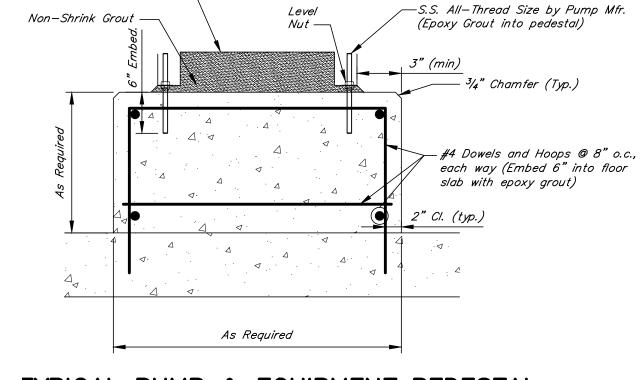
tubing below the floor slab. Seal ends with threaded











*⊏Equipment Base* 

PIPE BRACKET AND HANGER DETAIL

FLANGED PIPE SUPPORT

TYPICAL PUMP & EQUIPMENT PEDESTAL CONCRETE SUPPORT REINFORCEMENT

Aluminum

Mounting Panel

(Install 5' above FFE) —

4.5" Pressure Gauge

45-1279AS-02L, or equal -

1/4" Tubing with

Shark Bite fittings -

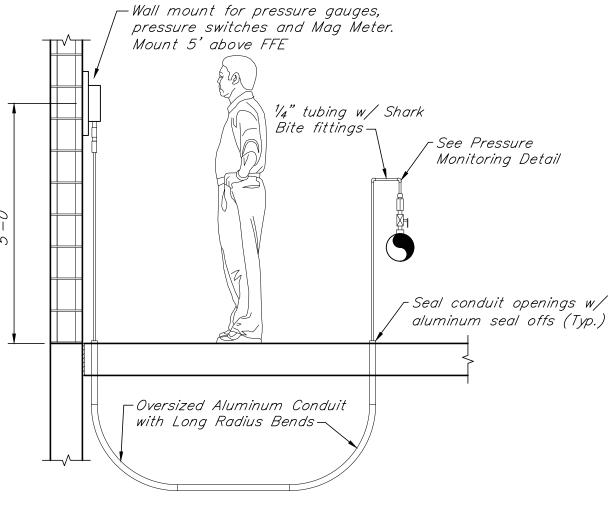
Pulsation Damper —

Tap location where indicated

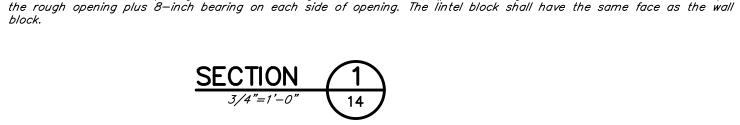
3/4" x 1/2" Red. Bushing

(Stainless Steel) -

Ashcroft Model



PRESSURE TAP CONNECTION



Wood trusses to be designed by the manufacturer. Trusses shall meet all applicable building codes and the standards of the Truss Plate Institute. Design criteria shall be as follows:

2. Provide lintels over all openings with lintel block grouted solid with 2-#5 Bars. Length of lintels shall be at least equal to

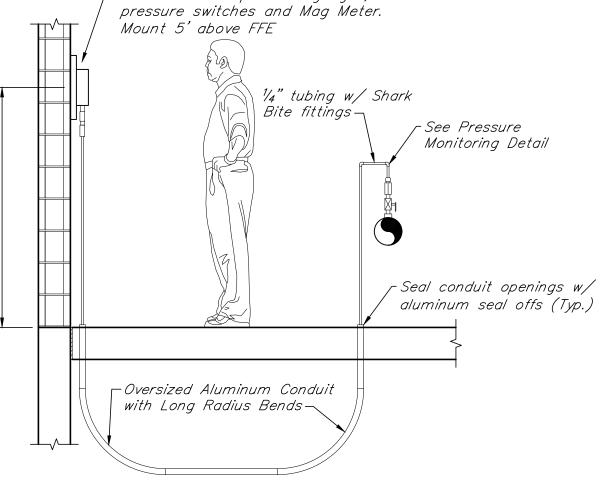
—CMU wall shall be filled with

foam installation in block core

—For Slab and Footing Details See Structural Drawings

—For Slab and Footing Details See Structural Drawings.

-No. 9 Crushed Stone Fill



5" Alum. Gutter & 4"

not shown for clarity)-

Downspouts. (Downspouts

4" Brick Veneer

shall be Belden, Modular Indian Red

Clear A, 09-30 or

approved equal —

1'-2"

0'-2"

Brick Tie (typ) (16" O.C. Vert, 32" O.C. Horz)—

Base Flashing—

Provide Concrete Splash Pad at each downspout—

Filter Fabric-

No. 9 Crushed Stone —

4" Sch. 40 PVC Foundation Drain—

NOTES:

1. WOOD TRUSSES

Span.... 13'-4" (Out to out of bearing)

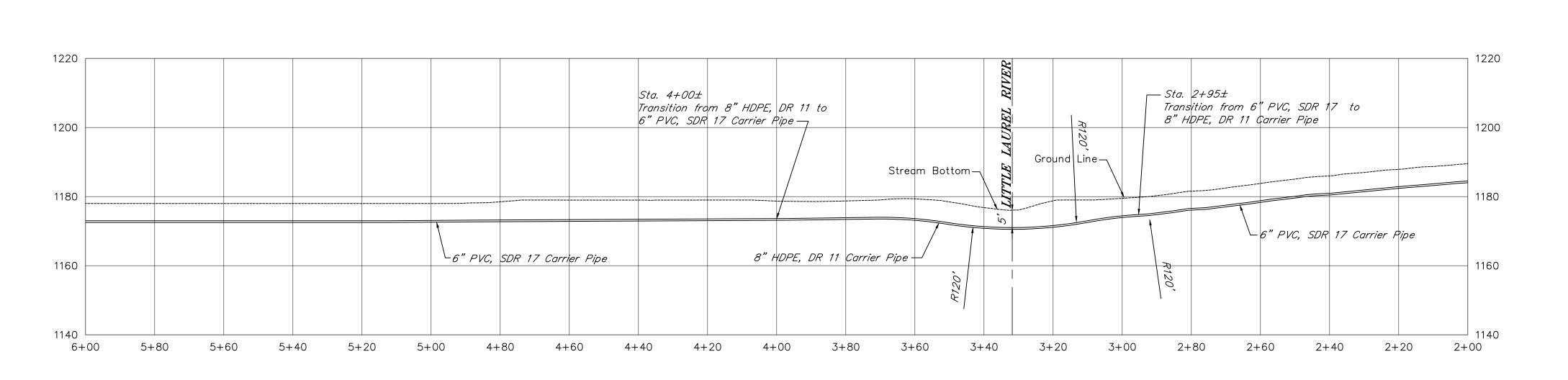
Max Deflection... L/240 (where L=span)

Spacing.... 24" o.c.

Top dead load... 15psf

Top live load... 30psf

Bottom dead load... 15psf



# LITTLE LAUREL RIVER DIRECTIONAL BORE

Scale: 1"=20'

LITTLE LAUREL RIVER DIRECTIONAL BORE-PLAN/PROFILE (ALTERNATE NO. 2)

EAST LAUREL WATER DISTRICT OLD SALEM ROAD/McWHORTER ROAD SYSTEM IMPROVEMENTS

PTH

3Y: EWB

3Y: EWB

3Y: EWB

3Y: EWB

3Y: EWB

YOUNG

Y

DRAWN BY: PTH
CHECKED BY: EWB
CHECKED BY: EWB
DATE: March 2024
SCALE: 1"=20'
REVISIONS

KENVIRONS

ivil & Environmental Engineers



PROJECT NO. **2020052** 

SHEET NO.

B1

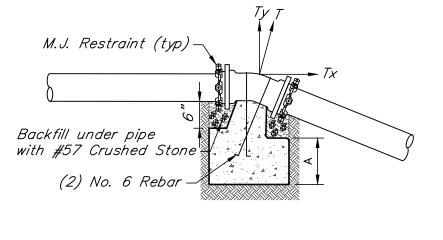
PROJECT NO. 2020052

SHEET NO.

MISCELLANEOUS DETAILS

5/8" x 3/4" Meter w/ Angle Ball Valve on Each Side of Pressure Regulator installation (when required)— Meter Setting --w/ Lid Provide wire length to extend 12" above top of meter box — Tracer Wire (typ) (Duct Tape to through Meter Tubing) setter into walls — #57 Aggregate Base (Min. Depth of 6") Valve with Valve Box Main Line Tap: Service Saddle & Corp Stop (each side of Valve) -Waterline ~ BRANCH/TEE ORIENTATION Underground Wire Connector. Direct Bury Wire Nuts equal Waterline – to DRYCONN Model 10999 Flow NOTES: 1. Leak detection Meters shall be installed where indicated on the Plans. 2. Gate Valves are a Separate Pay Item. Bid Item for Leak Detection Meters shall include the Main Line Taps, Piping, Meter Box, Setter, Ball Valves, and Meter in accordance with the Detail Shown on this drawing. 3. When installed for Creek Crossings, a second Gate Valve shall be installed on the water main a minimum of 500 feet from the Leak Detection Meter. STRAIGHT RUN ORIENTATION

# LEAK DETECTION METER



# NOTES:

1. Thrust restraint table is based on pipeline pressure of 200 psi and earthbearing capacity of 1500 psf. During construction, the specific soil type may be evaluated and concrete thrust block size revised at the discretion of the Engineer.

END CAP

- 2. On large diameter pipes where space limitations or construction difficulties render concrete thrust blocks not feasible or impractical, a joint restraint system may be used. This restrained joint system must be approved by the Engineer.
- 3. Concrete shall be 3000 psi minimum conforming to KYTC Specifications 601.
- 4. Accessibility to fittings and bolts must be maintained.
- 5. Wrap fittings in plastic prior to placing concrete.

SECTION A-A

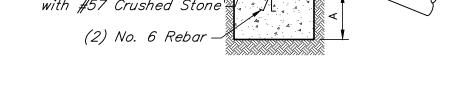
M.J. Restraint (typ)-

Service Line

Underground Wire Connector.
Direct Bury Wire Nuts equal
to DRYCONN Model 10999

		HOR	ZONTA	L THR	RUST E	BLOCK	SCHEE	ULE		
PIPE SIZE	90° (	90° BEND		45° BEND		BEND	11 1/4	* BEND	TEE/EN	ID CAP
(INCHES)	Α	В	Α	В	Α	В	Α	В	Α	В
3 & 4	3'-3"	1'-8"	2'-4"	1'-2"	1'-8"	1'-0"	1'-0"	1'-0"	2'-8"	1'-4"
6	4'-8"	2'-4"	3'-5"	1'-8"	2'-6"	1'-3"	1'-6"	1'-0"	3'-10"	2'-0"
8	6'-0"	3'-0"	4'-5"	2'-3"	3'-2"	1'-7"	2'-3"	1'-2"	5'-0"	2'-6"
10	7'-6"	3'-9"	5'-5"	2'-9"	3'-10"	2'-0"	2'-9"	1'-5"	6'-3"	3'-2"
12	8'-10"	4'-5"	6'-6"	3'-3"	4'-8"	2'-4"	3'-4"	1'-8"	7'-5"	3'-9"
14	10'-3"	5'-2"	7'-6"	3'-9"	5'-4"	2'-8"	3'-10"	2'-0"	8'-8"	4'-4"
16	11'-8"	5'-10"	8'-7"	4'-4"	6'-1"	3'-0"	4'-4"	2'-2"	9'-9"	4'-11"
18	13'-0"	6'-6"	9'-7"	4'-9"	6'-10"	3'-5"	4'-10"	2'-5"	11'-0"	5'-6"
20	14'-5"	7'-3"	10'-7"	5'-4"	7'-7"	3'-9"	5'-4"	2'-8"	12'-2"	6'-1"
24	17'-3"	8'-8"	12'-8"	6'-4"	9'-0"	4'-6"	6'-5"	3'-3"	14'-6"	7'-3"

HORIZONTAL THRUST BLOCK



- Meter Box

-Meter (Provided by owner, or transfer from existing

- #57 Aggregate Base

(Min. Depth of 6")

Service tubing pigtail, 24" long with CTS Pack

Joint coupling

equal to Legend model 313-215NL

- Provide wire length to extend 12" above

top of meter box

*– Pressure Regulator* (when required)

-Rod through

service line

Service Tap/Saddle

*—Tracer Wire* 

1. This detail reflects typical meters 1" and smaller with standard

2. Meter setting shall be placed inside property line as directed by

3. See Technical Specifications for more detail on meter box, lid,

METER SETTING

4. Service tubing pigtail to be incidental to Meter Setting.

Bearing Pressure

<u>PLAN</u>

Corp Stop

pressure regulators.

Undisturbed Soil

and yoke/setter requirements.

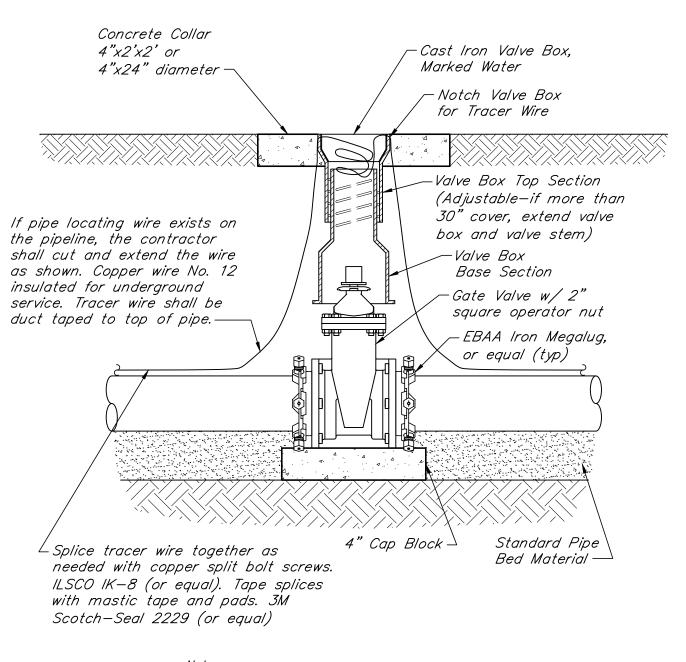
setter into walls

- 1. Thrust restraint table is based on pipeline pressure of 200 psi and earth bearing capacity of 1500 psf. During construction, the specific soil type may be evaluated and concrete thrust block size revised at the discretion of the Engineer.
- 2. On large diameter pipes where space limitations or constuction difficulties render concrete thrust blocks not feasible or impractical, a joint restraint system may 3. be used. This restrained joint system must be approved by the Engineer.
- 4. Concrete shall be 3000 psi minimum conforming to KYTC Specifications 601.
- 5. Accessibility to fittings and bolts must be maintained.
- 6. Wrap fittings in plastic prior to placing concrete.

V=Volume of gravity block in C.F. A=Bearing Area for T in S.F.

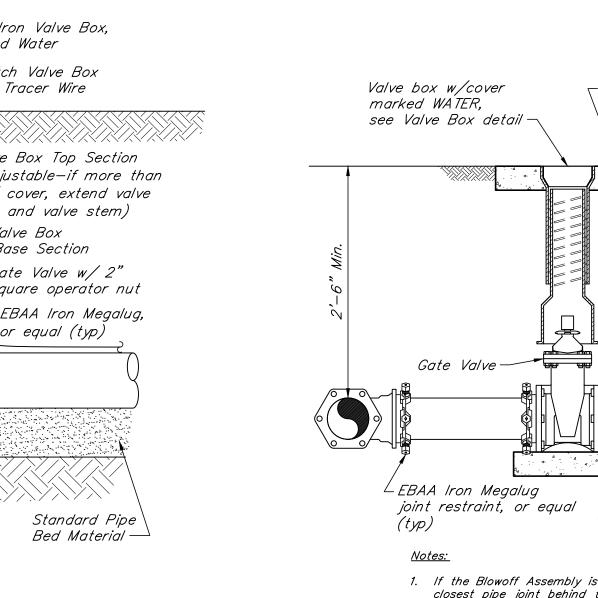
PIPE SIZE	90° 1	BEND	45° BEND 22 1/2° BEND 11 1/				11 1/4	· B
(INCHES)	٧	Α	٧	А	٧	Α	٧	
3 & 4	29	2	20	1	11	1	6	
6	64	5	46	2	25	1	13	
8	114	8	81	4	43	1	23	
10	174	12	123	5	66	2	35	
12	248	17	176	8	95	2	50	
14	337	23	238	10	128	3	67	
16	439	29	311	13	167	4	88	
18	555	37	393	16	211	5	111	
20	685	46	484	20	260	6	137	
24	985	66	696	29	374	8	197	

VERTICAL THRUST BLOCK



See Specifications for piping materials and piping joints.

VALVE BOX INSTALLATION



1. If the Blowoff Assembly is in-line, the assembly shall provide bell restraint at the closest pipe joint behind the valve. Engineer to verify the restrained length and number of split restraint bell harnesses to be used.

10' Min. or as

\_Shown on Plans\_

Pay Width

**HEAVY DUTY BITUMINOUS** 

LIGHT DUTY BITUMINOUS

1. The maximum allowable distance for dimension X shall be calculated as follows: X=pipe diameter+24"

4. Replace concrete or bituminous pavement with new pavement same thickness as existing pavement

PAVEMENT REPLACEMENT

 $\neg Valve box pad,$ 

see Valve Box detail

2. Mechanically tamped #57 crushed stone aggregate in layers not to exceed 6"

3. Concrete slab under bituminous surface to extend 12" on each side to trench

– Surface

┌ Base

Pavement-

-Class A

Stone

Concrete

Proposed

**CONCRETE PAVEMENT** 

CRUSHED STONE

5. Casing pipe is not required under private driveways

#57 Crushed

12"

└ Concrete

#57 Crushed in

- Sawed

Joints

¥57 Crushed

Stone

Casing Pipe

Proposed

-- Provide tracer wire;

able to extend 12"

above top of vault

—Cast Iron meter frame and cover

– 24"ø Plastic Vault

-Glued-on Sch. 80,

200 PSI, threaded

clean-out w/cap

Drill 1/4" Drain Hole

-See plans for

pipe size

Thrust Block,

see detail

-Cam lock hose coupling

-Casing Pipe

- 2. Mechanical Joint fittings may be restrained using  $\frac{3}{4}$ " allthread rods, duc-lugs, and HD nuts, in lieu of Megalug restraints.
- 3. All discharge shall be dechlorinated as directed in the technical specifications. BLOWOFF ASSEMBLY DETAIL

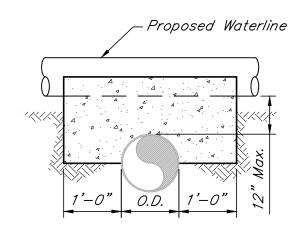
2020052 SHEET NO.

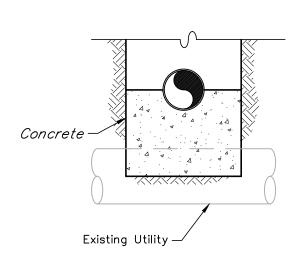
PROJECT NO.

Existing Utility — Concrete – Standard pipe bed material -Proposed Waterline

# SIDE VIEW (BELOW)

# END VIEW (BELOW)



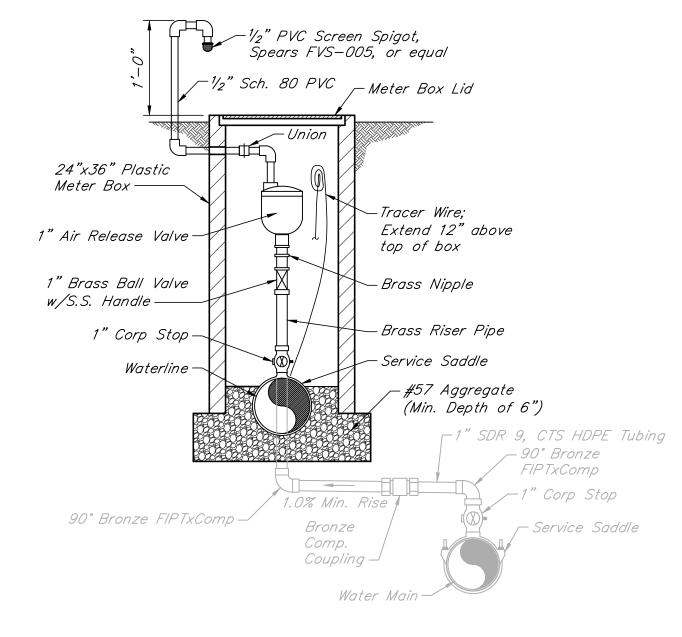


# SIDE VIEW (ABOVE)

END VIEW (ABOVE)

- 1. Concrete shall be used when clearance between Waterline and existing utility is 12" or less.
- 2. "Existing Utility" includes typical non—contaminating utilities, including but not limited to, water, natural gas, telecom, electrical, storm sewer. When crossing sanitary sewer or other potential contaminants, see detail "SANITARY SEWER CROSSING".

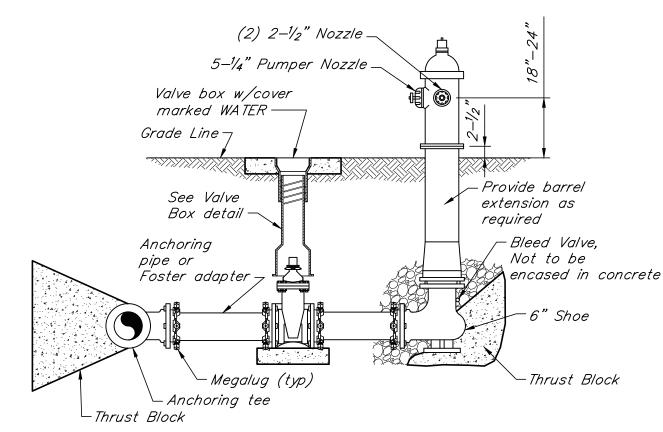
# UTILITY CROSSING



## NOTES:

- 1. All fittings shall be lead free, and NSF 61 certified.
- 2. When the Waterline is located in a road or ditchline, the Air Release Valve is to be located as directed by the Engineer, and connected by a <sup>3</sup>/<sub>4</sub>" Service line installed with a constant upgrade from the Waterline to ARV connection.

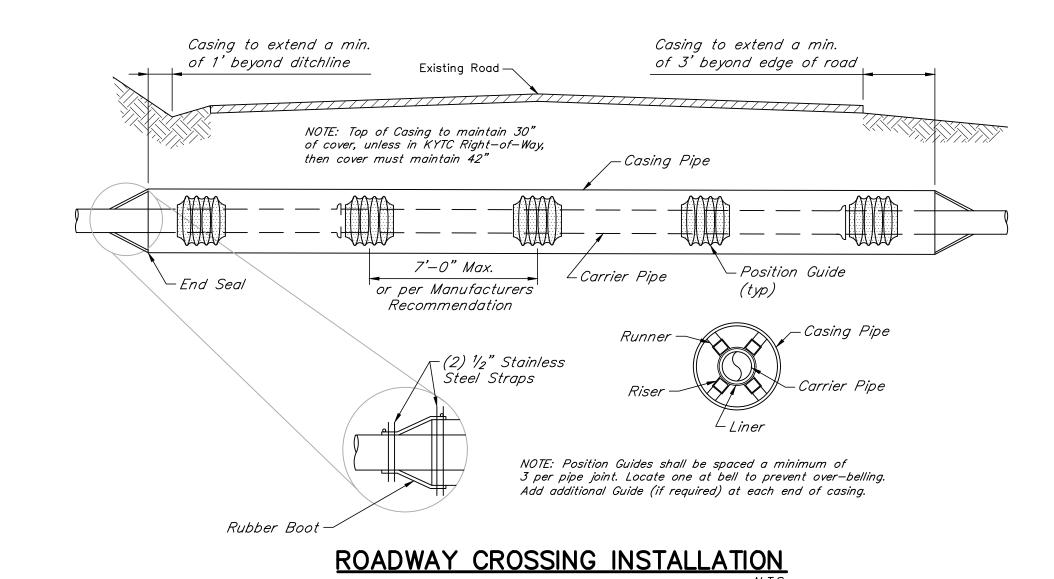
# **AUTOMATIC ARV INSTALLATION**

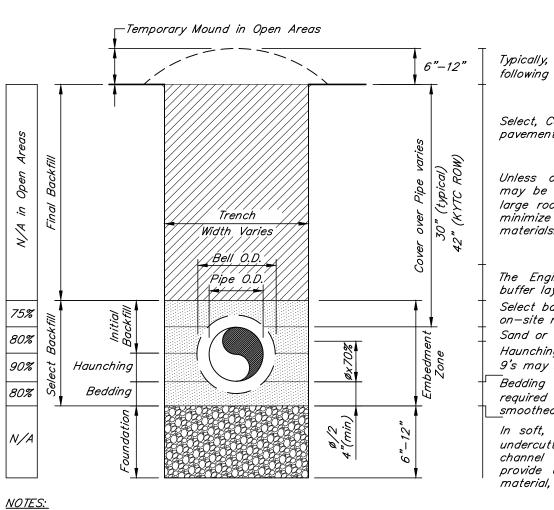


# <u>NOTES:</u>

- 1. Fire hydrant shall be rotated to reflect orientation as shown on plans, or as directed by Owner/Engineer
- Assembly includes branch pipe, fittings, gate valve and valve box, concrete base, thrust blocks, and M.J. restraints

# FIRE HYDRANT





Typically, open areas are final graded, dressed and seeded following two soaking rains. Excluding KYTC road ROW's.

Select, Compacted Backfill, or "Flowable Fill" if under future pavement sections. No Granular Material.

Unless otherwise specified, material excavated from trench may be used for final backfill provided it is relatively free of large rock (smaller than 8"), or mixed with sufficient dirt to minimize voids and settlement, and free of other unsuitable

The Engineer may require selective placement of an extra buffer layer for extremely rocky backfill to prevent migration Select backfill, lightly compacted (bucket shaping) using suitable on-site material, or dumped sand. Sand or very select material, hand tamped

Haunching to be carefully placed - Sand or sandy/clay soil. No. 9's may be required if weak foundation is encountered Bedding to be sand or approved equivalent, (No. 57's may be required if weak foundation encountered) hand placed and smoothed to uniform grade for support of pipe In soft, wet, muddy or otherwise yielding foundation conditions,

undercutting and replacement with No. 2 Stone and/or Class II channel lining, or equivalent, will be required. Objective is to provide a trench bottom free of large stones, clods, frozen material, etc. which is unyielding.

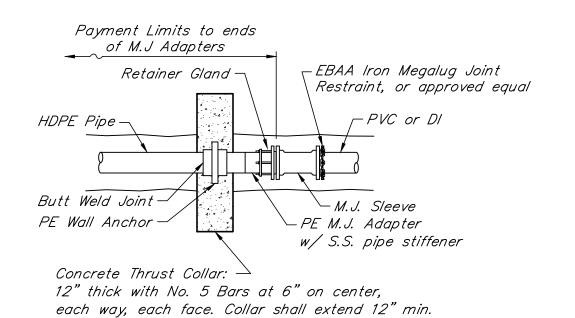
- 1. No rocks larger than 1-1/2" allowed in embedment zone.
- 2. Typical desired densities in open areas are depicted above in the boxes to the left of the figure. In other laying situations, more stringent selection, placement and compaction will be required.
- 3. Trench width should be no wider than necessary for adequate work room and to assure safe working conditions. Nominal outside diameter (O.D.) pipe plus 6" on each side is typically considered minimal.

TRENCH BACKFILL OPEN AREAS - PLASTIC PIPE

SHEET NO.

**D3** 

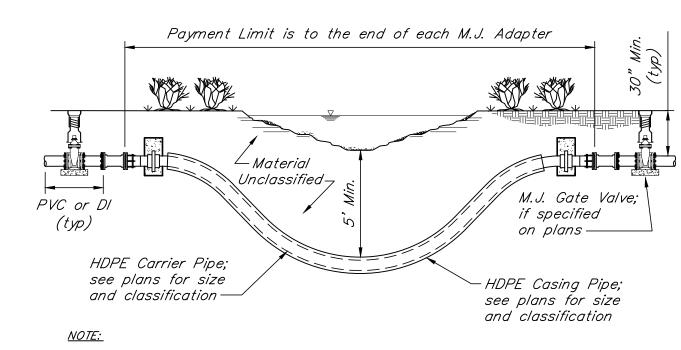
MISCELLANEOUS DETAILS



# END TREATMENT

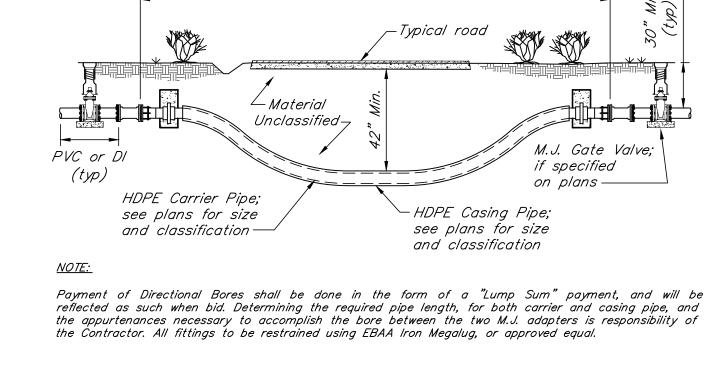
above and below Wall Anchor and 24" beyond

trench width (size will vary with pipe size)



Payment shall be "Lump Sum" for specific individual Bid Items for Directional Bores of large stream crossings and/or some classified small streams where the physical crossing characteristics differ significantly from the other small streams in the project. Determination of required length is responsibility of Contractor. When a creek crossing test meter is shown on the drawings and it is necessary to tap the HDPE pipe for the meter connection, the tapping saddle specifically manufactured for HDPE pipe shall be used.

# DIRECTIONAL BORE FOR STREAM CROSSINGS



Payment Limits to ends of M.J Adapters

HDPE Pipe -

Butt Weld Joint -

PE Wall Anchor-

Concrete Thrust Collar: ightharpoonup

12" thick with No. 5 Bars at 6" on center,

trench width (size will vary with pipe size)

each way, each face. Collar shall extend 12" min.

END TREATMENT

Payment Limit is to the end of each M.J. Adapter

above and below Wall Anchor and 24" beyond

Retainer Gland -

- EBAA Iron Megalug Joint

PVC or DI

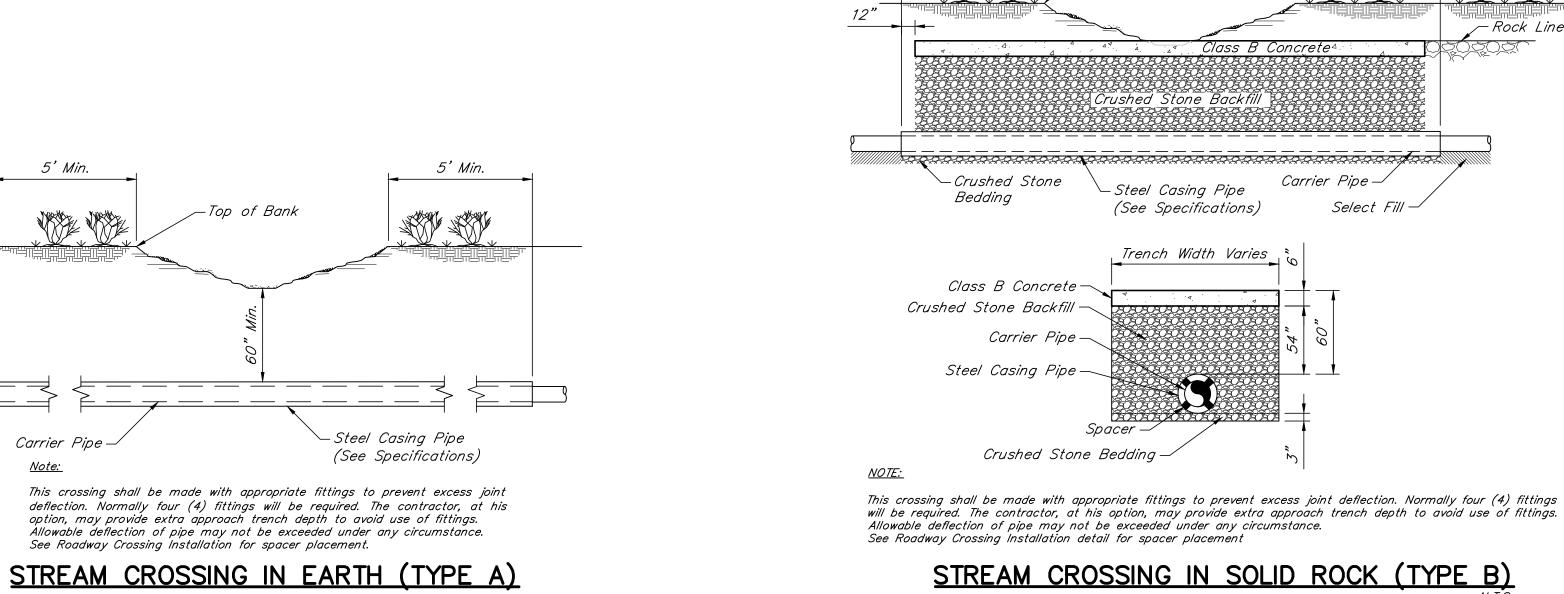
M.J. Sleeve

w/ S.S. pipe stiffener

*∖−PE M.J. Adapter* 

Restraint, or approved equal

# DIRECTIONAL BORE-WITH CASING



Allowable deflection of pipe may not be exceeded under any circumstance. See Roadway Crossing Installation for spacer placement.

DEVICE	HEIGHT AFF	REMARKS
RECEPTACLE - LOW	1'-4"	TO BOTTOM OF DEVICE BOX
IGHT SWITCH	4'-0"	TO BOTTOM OF DEVICE BOX
CONTROL STATIONS & PUSH-BUTTONS	4'-0"	TO BOTTOM OF DEVICE BOX
PANELBOARDS & CONTROL PANELS	6'-6"	TO TOP OF BOX
SAFETY SWITCH	4'-0"	TO TOP OF BOX
THERMOSTAT	4'-8"	TO BOTTOM OF DEVICE BOX
MERGENCY LIGHT FIXTURES	7'-4"	TO BOTTOM OF DEVICE BOX

LOCATION	CONDUCTORS	I/O TAG	TYPE	UNIT	CONTROL	MONITOR	TREND	HISTORIZE	TOTALIZE	AVERAGE	ALARM	REPORT	NOTES
KY-472 BPS	INTERNAL	POWER LOSS ALARM									X		
	2#14	DOOR OPEN ALARM	DI								Х		
	2#14	HEAT PUMP UNIT ALARM	DI								Х		
	2#14	PUMP 1 CALL-TO-RUN	DO		Х								
	2#14	PUMP 1 RUNNING STATUS	DI			Х		Х				Х	REPORT # STARTS & RUNTIMES
	2#14	PUMP 1 OVERTEMP	DI								X		
	2#14	PUMP1 OVERLOAD	DI								Х		
	2#14	PUMP 2 CALL-TO-RUN	DO		Х								
	2#14	PUMP 2 RUNNING STATUS	DI			Х		Х					REPORT # STARTS & RUNTIMES
	2#14	PUMP 2 OVERTEMP	DI								Х		
	2#14	PUMP 2 OVERLOAD	DI								Х		
	2#14	SUCTION PRESSURE ALARM	DI								Х		
	2#18 STIC	FLOWRATE	Al	GPM		Х	Х	Х					
	2#18 STIC	FLOW TOTAL PULSE	DI	GAL					Х			Х	REPORT DAILY & MONTHLY FLOW
	2#18 STIC	SUCTION PRESSURE	Al	PSIG		Х	Х	Х			Х		
	2#18 STIC	SUCTION PRESSURE PRE-STRAINER	Al	PSIG	_	Х	Х	Х			Х		
	2#18 STIC	DISCHARGE PRESSURE	Al	PSIG		Х	Х	Х			Х		

SCADA I-O TABLE

		WALL	MOUN	IT PAC	KAGED	HEA	T PUM	P SC	HEDULI					
		COOL	COOLING		SENSIBLE EER	FED	HEATING	COP	VOLTAGE /	OA		ELEC.		
TAG	MODEL	EAT DB/WB	OAT DB	COOLING COOLING	COOLING MBH	G ARI-390	© 5°F MBH	@ 5°F	PHASE	CFM	CFM	ESP	RPM	HEAT KW
HPU-472	W18H	80/67	95	17.5	13.1	11.3	7.0	1.61	230/1ø	20	600	0.1	A/R	4.0
NOTES:														

REFER TO HEAT PUMP SPECIFICATION FOR ADDITIONAL REQUIREMENTS

HEAT PUMP SHALL BE BARD OR APPROVED EQUAL PROVIDE MOTORIZED FRESH AIR DAMPER

. PROVIDE DIGITAL PROGRAMMABLE AUTO-CHANGEOVER THERMOSTAT 5. PROVIDE CUSTOM-COLOR - OWNER TO SELECT COLOR DURING SUBMITTAL REVIEW

AMPERE TRIP	\$3	SWITCH: 3=3 WAY; 4=4 WAY; K=KEY; WP=	Ţ	GROUND
AUTOMATIC TRANSFER SWITCH AMERICAN WIRE GAUGE	×h	WEATHERPROOF; M=MOTOR STARTER; PL=PILOT LT DUPLEX RECEPTACLE: WP = WEATHERPROOF; GFI =		
BARE COPPER		GROUND FAULT; NUMBER = MOUNTING HEIGHT	₩	CURRENT TRANSFORMER
CONDUIT (RACEWAY) AT	*	SINGLE RECEPTACLE	<del>                                     </del>	POTENTIAL TRANSFORMER
CIRCUIT BREAKER CLOSED CIRCUIT TELEVISION	ψ	208 or 240 VOLT RECEPTACLE	6 0	CIRCUIT BREAKER (GENERAL)
CIRCUIT		DUPLEX RECEPTACLE, FLUSH FLOORBOX MOUNTED	~~~~~	CIRCUIT BREAKER, THERMAL—MAGNETIC
CENTERLINE CEILING		SPECIAL PURPOSE RECEPTACLE OUTLET		
CONTROL PANEL	(T)	TUEDMOCTAT	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	CIRCUIT BREAKER, MAGNETIC-ONLY
CURRENT TRANSFORMER OR CONSTANT TORQUE  CONTROL		THERMOSTA T	-GF	GROUND FAULT PROTECTED CIRCUIT BREAKER
COPPER OR CONDENSING UNIT DELTA/WYE	M	MOTOR	<u> </u>	RELAY CONTACTS (NORMALLY OPENED)
DIRECT BURIAL	<b>(</b>	JUNCTION BOX — SMALL	<del> </del>	RELAY CONTACTS (NORMALLY CLOSED)
DOWN  DOUBLE POLE—SINGLE THROW	J	JUNCTION BOX — FLUSH—MOUNTED	ا گ	THERMAL OVERLOAD PROTECTION
EMPTY CONDUIT		SAFETY SWITCH — NONFUSED UNLESS NOTED	3	
EXHAUST FAN EQUIPMENT GROUND		OTHERWISE		<i>FUSE</i>
EQUIPMENT GROUND CONDUCTOR	452	MAGNETIC COMBINATION STARTER — THREE PHASE	Ĭ	DOT INDICATES A CONNECTION OF TWO WIRES
EXPANSION JOINT  ELEVATION	4	MAGNETIC COMBINATION STARTER — SINGLE PHASE		TERMINALS FOR CONNECTION OF REMOTE WIRING
ELECTRIC END-OF-LINE	⊲x	TELECOM OUTLET: $D = DATA$ ; $T = TELEPHONE$ ; $C$		RELAY/CONTACTOR COIL: C = CONTRACTOR; CR =
EMERGENCY		= CABLE; NUMBER = QTY OF CABLES & JACKS CONDUIT TURNED UP	H • • •	CONTROL RELAY; TR = TIMING RELAY; M = MOTOR HAND-OFF-AUTOMATIC SWITCH
ELECTRIC UNIT HEATER ELECTRIC WATER COOLER		CONDUIT TURNED DOWN	<del>**</del> • • • • • • • • • • • • • • • • • •	
ELECTRIC WALL HEATER/WATER HEATER		CONDUIT TURNED DOWN		
EXISTING FIRE ALARM	s	WALL MOUNTED SPEAKER OR ALARM HORN	<b> </b> ₹ <sup>x</sup>	FULL VOLTAGE NON—REVERSING MOTOR STARTER; X = NEMA SIZE
FIRE ALARM CONTROL PANEL		PANELBOARD (SURFACE MOUNTED)	X	PILOT LIGHT: R = RED; G = GREEN; A = AMBER; W = WHITE
FIBER OPTIC  FULL VOLTAGE, NON—REVERSING	<b>∤■</b> ■	PANELBOARD (FLUSH MOUNTED IN WALL)	X	PILOT LIGHT — PUSH—TO—TEST
GROUNDING ELECTRODE CONDUCTOR  GROUND FAULT CURRENT INTERRUPTING	Ð	HEATER-WALL MOUNTED	_	MOTOR
GROUND	<b>EF</b>	EXHAUST FAN/VENTILATOR		
HAND-OFF-AUTO SELECTOR SWITCH HORSEPOWER	<u>s</u>	SPEAKER GENERAL		FUSED DISCONNECT SWITCH
JUNCTION BOX	©	CLOCK	•	FLOAT SWITCH
KILOVOLT—AMPERES KILOWATT—HOUR			<del>-  </del>	TEMPERATURE SWITCH (THERMOSTAT)
THOUSAND CIRCULAR MILS	0	EXISTING POWER POLE		
LIGHTING FIXTURE (LUMINAIRE) LUMEN	•	NEW POWER POLE	•	PRESSURE SWITCH
LIGHTING LIGHTS	101-121 OR□-	LIGHTING POLE	0-0	LIMIT SWITCH
LIMIT SWITCH		PHOTO CELL	(CV) OD 0/1 0	SOLEMOID WALVE COV
LOW VOLTAGE MAIN CIRCUIT BREAKER		<i>MANHOLE</i>	SV OR o-//-o	SOLENOID VALVE COIL
MOTOR CIRCUIT PROTECTOR	MH)		— <u>ETM</u> —	ELAPSED TIME METER
MOTOR CONTROL CENTER MAIN DISTRIBUTION PANEL	РВ	PULLBOX	-alao o-	PUSHBUTTONS, N.C. & N.O. RESPECTIVELY
MANUFACTURER MANHOLE	$\qquad \qquad \Box \!$	MUSHROOM HEAD EMERGENCY SWITCH		SELECTOR SWITCH — TWO POSITION
MINIMUM		DUCT SMOKE DETECTOR	0 0	
MAIN LUGS ONLY MOUNTED	(i)	HEAT DETECTOR	$\hspace{1cm} \stackrel{\hspace{0.1cm} \circ}{\longrightarrow} \hspace{0.1cm}$	TIMER RELAY CONTACT: NORMALLY OPEN — TIMED OPEN UPON DEENERGIZATION
MEDIUM VOLTAGE			00	TIMER RELAY CONTACT: NORMALLY CLOSED -TIMED
NOT APPLICABLE NORMALLY CLOSED	(0)	SMOKE DETECTOR	V .	CLOSE UPON DEENERGIZATION
NATIONAL ELECTRICAL CODE NON LINEAR	P	FIRE ALARM MANUAL PULL STATION	$\sim$	TIMER RELAY CONTACT: NORMALLY OPEN — TIMED CLOSE UPON ENERGIZATION
NON LINEAR NORMALLY OPEN		FIRE ALARM HORN/STROBE	00	TIMER RELAY CONTACT: NORMALLY CLOSED — TIMED
NOT TO SCALE OVERHEAD		FIRE ALARM STROBE	\ Q 0	OPEN UPON ENERGIZATION TRANSFER SWITCH
OVERLOAD	ZAM	FIRE ALARM ZONE ADDRESSABLE MODULE	8	
POLE OVER TEMPERATURE		SPRINKLER SYSTEM FLOW SWITCH		GENERA TOR
PHASE		TAMPER SWITCH		EXTERNAL WIRING
PANEL POLY—VINYL CHLORIDE			-oÎo-	EMERGENCY STOP BUTTON
POWER RECEPTACLE	<b> </b>	MAGNETIC DOOR HOLDER		
SHEET	<b>∞</b>	KEYNOTE		
SOLID NEUTRAL SINGLE POLE	C	CALL SWITCH		
SURGE PROTECTION DEVICE	M	PASSIVE INFRARED MOTION DETECTOR		
STAINLESS STEEL STATION				
STANDARD	NEMA XX	ALL WORK IN THE ROOM/AREA SHALL CONFORM TO THE NEMA RATING INDICATED		

ELECTRICAL DIAGRAM SYMBOLS

TRANSFORMER

CAPACITOR

\*\*\*

ELECTRICAL PLAN SYMBOLS

TO THE NEMA RATING INDICATED

ELECTRICAL LINE UNDERGROUND

INSTRUMENTATION LINE UNDERGROUND

INSTRUMENTATION LINE OVERHEAD

TELEPHONE LINE UNDERGROUND

TELEPHONE LINE OVERHEAD

GROUND ROD

ELECTRICAL LINE OVERHEAD

EU

EO\_\_\_

10

10

TU

\_\_\_\_ TO\_\_\_

 $\odot$ 

EMERGENCY CIRCUIT

-

ELECTRICAL CIRCUIT: SHORT=PHASE CONDUCTOR; LONG = NEUTRAL, DASHED = EQUIPMENT GROUND

ELECTRICAL ABBREVIATIONS

*AMPERE* 

AFF

AFD

A TS

AWG

BC

CB

CCTV

CKT

C/L

CLG

CP

CTL

CU

DB

DN

EG

EGC

ΕJ

ELEC

*EOL* 

EUH

EWC

EWH

EΧ

FA

FO

FACP

FVNR

GEC

GND

HOA

KVA

KWH

L TG

MCB

MCP

MCC

MDP

MFR

MH

M/N

MV

NA

NEC

NTS

PNL

PVC

PWR

SHT

SPD

STA

SW

TB

TEL

TM

*TS* 

ΤV

UG

UH

WP

XFMR

TVSS

RECEP1

KCMIL

HP

GFCI OR GF

J OR JB

**EMERG** 

DPST

AMPERE FRAME

ABOVE FINISHED FLOOR

SWITCH

TERMINAL BOX

TAMPER SWITCH

UNDERGROUND

WEA THERPROOF

TRANSFORMER

UNIT HEATER

THERMAL MAGNETIC

VOLTAGE OR VOLTS

TELEPHONE

TELE VISION

SHIELDED TWISTED INSTRUMENT CABLE

TRANSIENT VOLTAGE SURGE SUPPRESSOR

ADJUSTABLE FREQUENCY DRIVE

LIGHT FIXTURE SCHEDULE											
TYPE	MANUFACTURER	CATALOG SERIES	LAMPS	VOLTAGE	MOUNTING	DESCRIPTION	SYMBOL				
LF-1	HOLOPHANE OR APPROVED EQUAL	EMS LED	6000 LM LED	120V	SURFACE	LINEAR FIXTURE, FIBERGLASS N4X WET LOCATION, CLEAR ACRYLIC DIFFUSER, 6KV/3KA SURGE, 5—YR WARRANTY, MEDIUM DISTRIBUTION, 4000K, 80 CRI					
LF-1E	HOLOPHANE OR APPROVED EQUAL	EMS LED	LM LED	120V	SURFACE	SAME AS LF-1 WITH 90 MINUTE EMERGENCY BATTERY					
LF-2	HOLOPHANE OR APPROVED EQUAL	HLWPC2	11700 LM LED	120V	WALL	WALLPACK, ALUMINUM POWDER-COAT, WET LOCATION IP-65, COLOR SELECTION BY OWNER, 10KV/10KA SURGE, FULL CUTOFF, 4000K, TYPE 3 MEDIUM DISTRIBUTION, 70 CRI, 5-YEAR WARRANTY	HX				
LF-3	HOLOPHANE OR APPROVED EQUAL	CZAFB	225 LM	120VAC	SURFACE	EMERGENCY EGRESS FIXTURE, WET LOCATION, ALUMINUM POWDER-COAT, DARK BRONZE, COLD RATED TO -22F, 5-YR WARRANTY, PHOTOCELL NORMALLY ON WITH BATTERY	Y				

ELECTRICAL SYMBOLS, ABBREVIATIONS, & SCHEDULES

KENTUCKY AUREL WATER J SYSTEM IMPROV CONTRACT 2 REL COUNTY, KEN UREL  $\Xi$ 

Bu Mult 5/05/2025 BENJAMIN L. MURPHY 25348

Y: JM BY: BLM BY: BRW 147 2022





PROJECT NO. 2020052

SHEET NO. E-1

KENTUCKY EI UR H

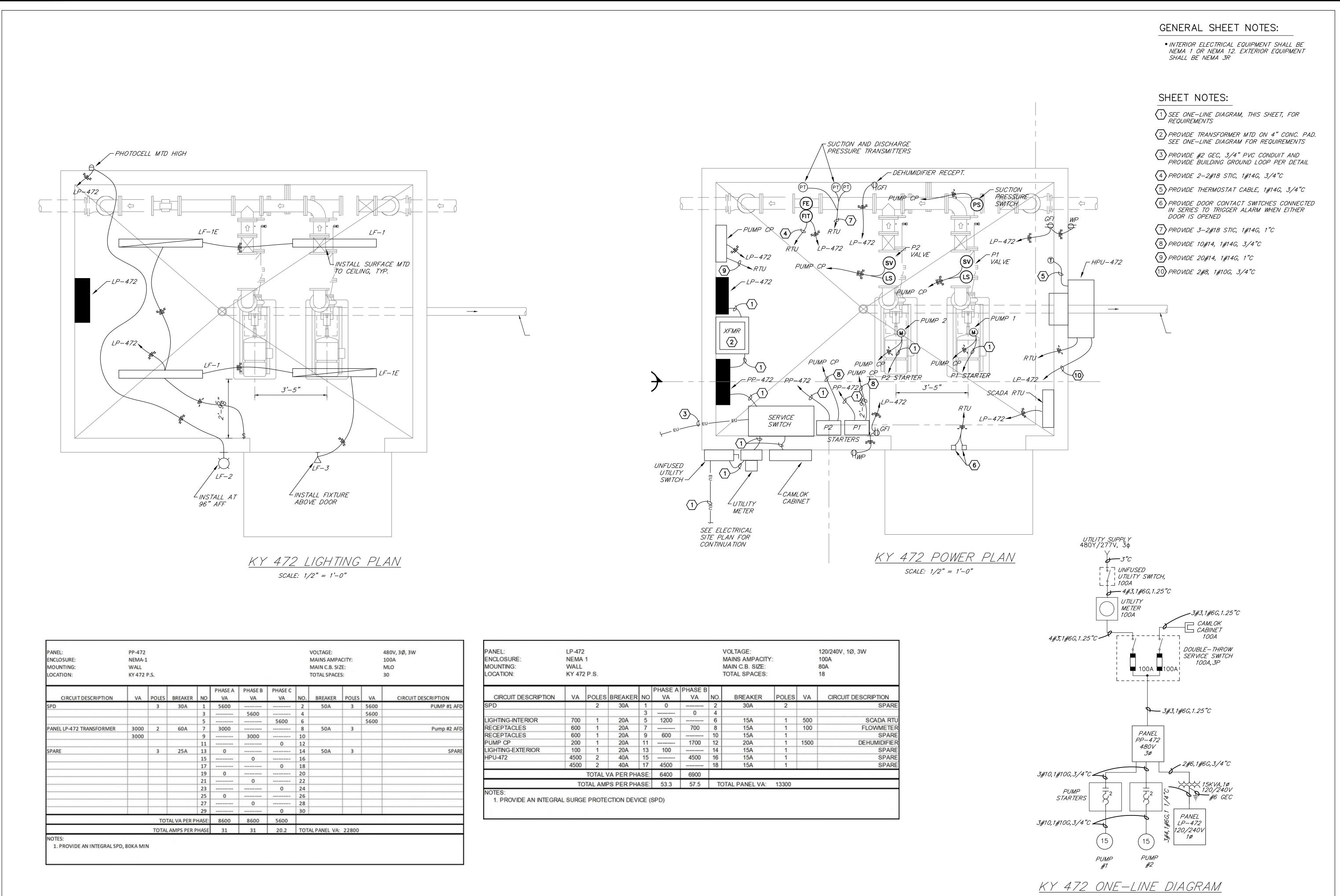
OF KENTIGE

BLM BRW

PROJECT NO. 2020052

SHEET NO.

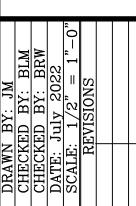
E-2



KY 472 PUMP STATION ELECTRICAL PLANS

EAST LAUREL WATER DISTRIC WATER SYSTEM IMPROVEMENTS CONTRACT 2 LAUREL COUNTY, KENTUCKY



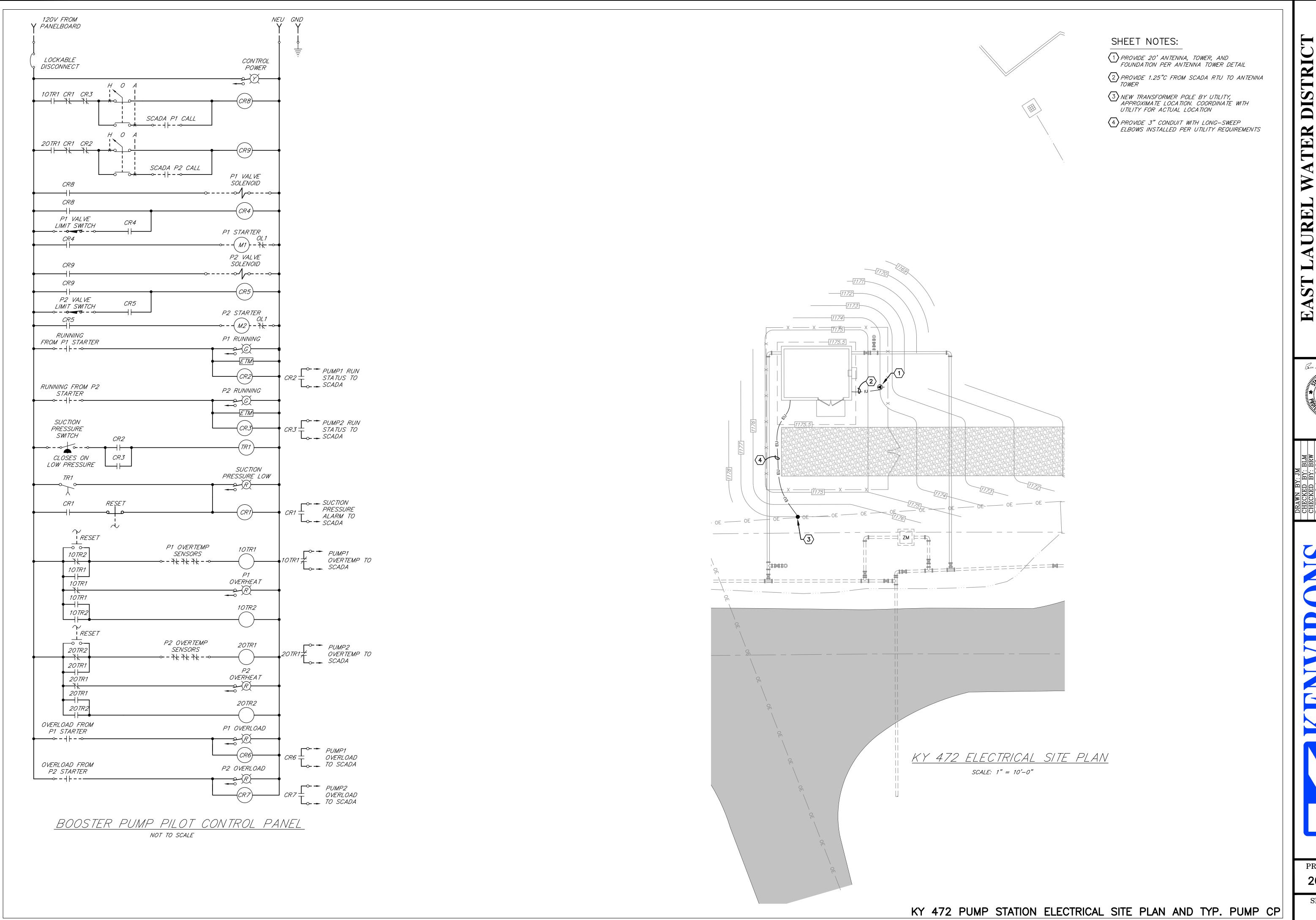






PROJECT NO. **2020052** 

SHEET NO. **E-3** 



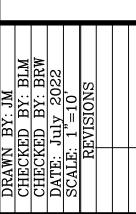
EAST LAUREL WATER DISTRIC WATER SYSTEM IMPROVEMENTS CONTRACT 2
LAUREL COUNTY, KENTUCKY

BENJAMIN L.

MURPHY

25348

SONAL







PROJECT NO. **2020052** 

SHEET NO.

E-4

### Seismic response factor Method of analysis Seismic coefficient

MATERIAL STRENGTHS USED IN DESIGN

Seismic design category

Basic structural system

Seismic importance factor

Seismic force resisting system

(for reference in calculations — see specifications or notes for actual material specifications) Concrete:

Class A (structural)(see specifications)	28	day f'c	=	4,500	psi
class b (non-struct)(see specifications)	28	day f'c	=	3,500	psi
Reinforcing bars (ASTM A615 OR A706 GRADE 60)		fy	=	60,000	psi
Welded wire fabric (ASTM A185)		fy	=	65,000	psl
Prestressing strand (ASTM A416 GRADE 270 LO LAX)		fu	=	270,000	psl
Deformed bar anchors (ASTM A496)		fy	=	80,000	psi
Structural steel sections W AND WT (ASTM A992)		fy	=	50,000	psi
Structural steel sections C, L, M, S, HP, MT and ST (ASTM A36	)	fy	=	36,000	psi
Structural steel plates bars, and rods u.n.o. (ASTM A36)		fy	=	36,000	psi
Structural steel sections HSS (ASTM A500 GRADE B)		fy	=	46,000	psi
Structural steel pipe (ASTM A53 GRADE B)		fy	=	35,000	psi
Structural bolts (ASTM A325)		fu	=	120,000	psi
Concrete masonry (VARIOUS)		f'm	=	1,500	psi
Soil allowable bearing pressure for foundations		qa	=	3,000	psf
Rock allowable bearing pressure		qa	=	30,000	psf

- 1. The requirements of these general notes apply unless otherwise noted on plans or in specifications.
- 2. All dimensions of existing conditions shall be verified prior to commencing work. Discrepancies between existing conditions or between the drawings and specifications shall be communicated to the structural engineer and architect.
- 3. This structure is designed to be stable and self—supporting only when fully completed. Stability of the structure during construction is the responsibility of the contractor. All necessary temporary bracing required to stabilize and support the structure during all construction phases shall be furnished and installed by the contractor. If required,
- temporary bracing shall be designed by a licensed engineer employed by the contractor. 4. Construction loads imposed on the structural framing shall not exceed the design capacity of the framing at the time such loads are imposed.
- 5. Non—structural elements of the building (architectural finishes, masonry veneer and associated ties, insulation, sheathing, ductwork, piping, etc.) are generally not shown on these structural drawings. Certain non—structural elements that are shown on the structural drawings are shown for reference only. Non—structural elements shall be constructed as shown on the architectural and trade drawings.
- 6. Any material ordered or work performed prior to the engineer's review and approval of the shop drawings is at the contractor's sole risk.

### FOUNDATIONS

Laurel

100 psf

20 psf

15 psf

le = 1.25

Cs = 0.22

R=2

Bearing Wall System

Intermediate Reinforced Masonry Shear Walls

Equivalent Lateral Force Procedure

- 1. The foundations have been designed based on assumed bearing capacities.
- 2. Foundation design is based on an allowable bearing capacity of 2,000 psf for native soil
- (undercut as may be required) and controlled fill and 6,000 psf for bedrock. 3. If required, a qualified testing company shall be engaged by the contractor to verify bearing capacities prior to installing foundations.
- 4. All footings shall be supported on undisturbed soil, engineered fill or competent bedrock
- where indicated. 5. Fill shall be compacted to 98% of optimum laboratory density in accordance with ASTM D
- 698 Standard Proctor Method in maximum 8" lifts unless noted otherwise.
- 6. All piers and spread footings are centered on column centerlines and all wall footings are centered under walls unless indicated otherwise.
- 7. Location of existing foundations, if any are shown on drawings, are approximate. exact condition shall be verified at time of construction.
- 8. The structural engineer shall be notified if soft, loose or lower bearing capacity soils or rock are encountered.
- 9. Existing underground utilities in greas of foundation construction shall be located prior to construction of foundations. appropriate measures shall be taken to avoid damage to existing utilities and to ensure adequate foundation bearing around utilities.
- 10. Foundations shall not be placed on mud or muck, soft or loose soil, in standing water or on frozen ground.
- 11. All non-cantilever walls shall be be adequately braced prior to backfill.
- 12. Cantilever retaining walls shall not be backfilled until the concrete has developed 100% of the required 28-day compressive strength for the class of concrete specified.

### CAST-IN-PLACE CONCRETE

- 1. All concrete construction shall be performed in accordance with aci 301-10, aci 318-11. ACI 117-10, ACI 308.1-11, and ACI SP-66, the ACI Detailing Manual-2004. Hot and cold weather concrete construction shall be performed in accordance with ACI 305 and ACI 306 as required. Shoring and reshoring of concrete structures shall be performed in accordance with ACI 347. Structural design and removal of concrete formwork, shores and reshores shall be the responsibility of the contractor.
- 2. Shop drawings showing the size, length, quantity, location and mark of all reinforcing bars, supports and accessories shall be submitted for approval prior to fabrication.
- 3. Mix designs and admixture product data shall be submitted for approval prior to ordering
- 4. Concrete properties shall be in accordance with the specifications.
- 5. Reinforcement and accessory properties shall be in accordance with the specifications. 6. Reinforcement compression splices shall be lapped 30 bar diameters of the larger bar.
- 7. Reinforcement tension splices shall be lapped in accordance with the following table:

<u>bar size</u>	3,000 psi conc. lap length	>=4,000 psi conc. lap length
#3	17"	15"
#4	23"	20"
<b>#</b> 5	28"	24"
#6	<b>34</b> "	29"
<b>#</b> 7	49"	43"
#8	56 <b>"</b>	49"
#9	69"	60"

add 30% for horizontal top bars with more than 12" of concrete below. add 50% for bar spacing less than two bar diameters. lap length adds are cumulative.

8. Concrete protection for reinforcement shall be in accordance with the following table: <u>clear cover over bars</u>

concrete cast against and permanently exposed to earth
concrete exposed to earth or weather
#6 through #18 bars
"5 · W74 B74 · · · ·

#6 through #18 bars	2
#5 bar, W31 or D31 wire and smaller	1 1/2"
concrete not exposed to weather or in contact with ground	
slabs, walls, and joists	
#14 and #18 bars	1 1/2"
#11 bar and smaller	3/4"

- 9. The typical details on these drawings contain additional general concrete construction notes and information.
- 10. All concrete shall be reinforced unless noted otherwise.
- 11. supports to adequately position reinforcing bars during construction shall be installed.
- walls, piers, and columns. 13. All reinforcing at wall and footing corners and intersections shall be continuous by the use of bent bars or corner bars unless indicated otherwise.

12. Foundation dowels of the same size and spacing as vertical steel shall be installed for all

- 14. Construction joints shall be positioned so as not to adversely affect the structural performance. Construction joint locations not indicated on the structural drawings shall
- be approved by the structural engineer. 15. Pipe sleeves and inserts shall be installed in concrete work at all penetrations.
- penetrations of beams, joists, columns or structural slabs not indicated on the structural drawings shall be approved by the structural engineer. 16. Only weldable reinforcing bars may be welded.
- 17. Admixtures containing chloride or other corrosive chemicals shall not be used in
- 18. Aggregates shall be free of deleterious or non—durable materials such as cherts.
- 19. reinforcing shall be adequately tied and supported to hold it in the correct position during construction.
- 20. Concrete shall be consolidated adequately during placement by mechanical vibration in accordance with published practices.
- 21. Unshored slab construction shall be finished level and have the minimum required thickness of concrete at the thinnest section. Beam camber shall be verified prior to placing unshored concrete slabs.
- 22. Plastic chairs shall be used in all concrete that will be exposed to view in the completed structure.
- 23. Exposed concrete corners shall be chamfered minimum  $\frac{3}{4}$ ".
- 24. Fill pockets around connections with concrete flush and smooth unless indicated
- 25. Concrete finishes shall be in accordance with the specifications.
- 26. Concrete slab—on—grade flatness and levelness shall be in accordance with the

### CONCRETE MASONRY

- 1. Concrete masonry walls shown on the structural drawings are structural walls. concrete masonry walls not shown on the structural drawings are partitions. Refer to architectural drawings for details of partitions unless indicated otherwise on the structural drawings.
- 2. Concrete masonry walls shown on structural drawings shall be constructed in accordance with ACI 530.1 "Specifications for Masonry Structures".
- 3. Installation drawings, product data and material certifications shall be submitted for approval. The submittals shall conform to the specifications.
- 4. Concrete masonry materials shall conform to the requirements of the specifications.
- 5. Minimum compressive strenath of concrete masonry (f'm) shall be 1.500 psi determined in
- accordance with the specifications.
- 6. Mortar cement shall be portland-lime cement. Masonry cement shall not be used.
- 7. The typical details on the drawings contain additional general masonry notes and details. 8. Bearing walls shall be anchored at intersections by galvanized steel straps 1 1/2" x 1/4" x 24" with 2" bend at 90 degrees each end. Install straps into grouted cores of c.m.u. at 24" maximum vertical spacing. do not install anchors at control joints or where non-bearing partitions abut
- bearing walls. 9. Corners of load bearing concrete masonry walls shall be laid in running bond.
- 10. Provide solid grouted concrete masonry around bearing ends of all beams and joists.
- 11. No openings for trades shall occur in concrete masonry walls within 16 inches of beam bearing
- 12. Pipe sleeves and inserts shall be installed in concrete work at all penetrations.
- 13. Embedded item locations shall be coordinated with the approved shop drawings of the trades. 14. Only weldable reinforcing bars may be welded.
- 15. Concrete masonry is supposed to absorb water from mortar and grout. do not place or grout wet concrete masonry units.
- 16. Webs of masonry units for piers, columns, pilasters, and the starter course shall be mortared. webs of masonry units shall also be mortared where required to confine grout.
- 17. Cells of masonry in piers, columns, pilasters and where otherwise indicated shall align. this may
- require the use of block styles other than stretchers (e.g. square-end block). 18. Spaces to be filled with grout shall be kept clean and free from protrusions of masonry or mortar.
- 19. All cells of below—grade concrete masonry units shall be grouted . 20. The maximum grout pour height for each specific type and size of concrete masonry unit shall not
- exceed the limits specified in ACI 530.1.
- 21. Masonry grouting shall conform to the specifications. 22. Vertical control joints are indicated on the civil or architectural drawings.
- 23. Vertical control joints shall be installed between all non-loadbearing partitions and bearing walls.
- 24. Spacing of control joints shall not exceed 24 feet unless noted otherwise. 25. Splice lap lengths for reinforcing shall be in accordance with the following table:
  - <u>bar size</u> <u>lap length</u> 18" 25" 31"
- 26. Do not embed any non-structural items in structural masonry without written permission from the structural engineer.

### STRUCTURAL STEEL

- 1. Detailing, fabrication, and erection of structural steel shall conform to the AISC "Specification for Structural Steel", (ANSI/AISC 360-10), AISC "Code of Standard Practice for Structural Steel Buildings and Bridges", AISC / RCSC "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts" and AWS D1.1 "Structural Welding
- 2. Shop drawings shall be submitted for approval prior to fabrication of structural steel. Shop drawings shall conform to requirements in the specifications.
- 3. Structural steel members shall conform to the following specifications:

member type	specification
wide flange	ASTM A 992
standard beam	ASTM A 36
channel	ASTM A 36
angle	ASTM A 36
plate	ASTM A 36
bar and rod	ASTM A 36
rectangular, square & round tube (hss)	ASTM A 500 Gr B
pipe	ASTM A 53 Gr B
threaded rod	ASTM A 36
anchor rod	ASTM F 1554 Gr 36
common bolts	ASTM A 307 Gr A
high strength bolts (twist off)	ASTM F 1582
high strength bolts (snug tight)	ASTM A 325
direct tension indicating washers	ASTM F 959
hardened washers	ASTM F 436
nuts	ASTM A 563
shear connectors (studs)	ASTM A 108
welding electrode	AWS D1.1 E70XX
, and the second	(except as otherwise

- 4. Grout shall conform to requirements in the specifications.
- 5. The typical details on the drawings contain additional general steel construction notes
- and details. 6. High—strength bolted connections shall be fully pretensioned unless noted as snug tight on the drawings.
- 7. Hardened washers shall be installed under all nuts for fully pretensioned bolts.
- 8. Hardened washers shall be installed over all oversized holes, standard slots and short slotted holes. plate washers  $\frac{5}{6}$ " thick shall be welded over large holes and long slots.
- 9. Bolted joints where relative movement is allowed shall have jam nuts to prevent
- 10. Structural steel surface preparation and finishes shall conform to the requirements in the specifications.

### PREFABRICATED WOOD TRUSS CONSTRUCTION

- 1. Truss design and manufacture shall conform to the current building code authorized edition of ANSI TPI-1, "National Design Standard for Metal-Plate Connected Wood Truss Construction."
- 2. Truss handling and erection shall conform to the latest edition of BCSI guides. See
- www.sbcindustry.com. 3. Truss layout and truss shop drawings shall be submitted for approval. These drawings shall include:
  - 3.7. a copy of the bcsi jobsite package, which are instructions for safe handling and
  - erection of wood trusses. 3.8. truss layout showing dimensioned location and shipping mark of each truss and
  - locations of all compression web and chord bracing. 3.7. truss configuration, including span, pitch and location of all member intersections.
  - 3.8. species, stress grade, and nominal size of lumber used. 3.9. design loads including point loads and reactions and load combinations used in
  - 3.10. printout of member axial and flexural stresses plus interaction of combined
  - stresses for the controlling load combination. 3.11. printout of truss deflections under service load combinations.
- 3.12. joint, splice, and truss to truss girder connection design and details. 4. Truss shop drawings, and calculations shall be sealed by a professional engineer licensed in the
- state of Kentucky. 5. Trusses shall be designed for a maximum vertical deflection of 1/480 of the span for 100% live
- load and 1/240 of the span for 100% total load. 6. Truss framing members shall be Southern Pine No. 2 or better.
- 7. All connections plates shall be hot—dipped galvanized according to ASTM A 153.
- 8. Trusses shall be spaced at 2'-0" o.c. maximum. Web arrangement shall be manufacturer's
- standard unless otherwise indicated. See all drawings for openings that may be required in trusses. 9. Permanent bracing for individual members of a wood truss shall be shown on the truss design drawings and shall be installed by the building contractor. Permanent bracing shall be installed as
- indicated on the truss manufacturer's drawings and instructions. 10. All bracing that terminates at or is interrupted by structural bearing walls shall be attached
- 11. Lateral brace splices shall be lapped at least two trusses.
- 12. Trusses delivered to the project in more than one piece and all multi-ply trusses shall be connected before installation or according to truss design drawings if indicated otherwise.
- 13. Concentrated loads from construction materials (e.g. roof sheathing bundles) shall not be placed on trusses until all required bracing has been installed and roof sheathing is permanently nailed in place. Trusses shall not be overloaded with construction materials.
- 14. Temporary bracing to prevent lateral movement during erection shall be installed according to the handling and installation guidelines.
- 15. Work points, overhangs and other dimensions not indicated on the structural drawings should be determined from the appropriate drawings. Conflicting dimensions shall be clarified in writing.

# ROOF AND WALL PLYWOOD SHEATHING

- 1. All sheathing shall be plywood (not OSB) manufactured in accordance with industry specification PS-1 and shall bear the stamp of either the American Plywood Association (APA) or Timberco inc.
- 2. All sheathing shall be exterior grade.
- 3. All roof and wall sheathing shall have veneer grade C—C or better.
- 4. Roof sheathing shall have tongue and groove edges and be either APA "Sturd—i—Floor" or TECO
- "Floor Span" with thickness and/or span rating as indicated on the drawings or as required. 5. Wall sheathing shall have plain square edges and be APA "Rated Sheathing" or TECO "Sheathing"
- Span" with thickness and/or span rating as indicated on the drawings or as required. 6. All edges of wall sheathing shall be blocked with a 2x wood member and nailed.
- 7. Minimum nailing for roof and wall plywood sheathing shall be 10d common nails at 12" o.c. in the panel interior and 6" o.c. at panel edges and boundaries.

### STRUCTURAL WOOD

- 1. All structural wood dimension lumber shall be Southern Pine No. 2 species stress grade and shall bear a stamp by the southern pine inspection bureau (SPIB) indicating this.
- 2. All structural composite lumber (LVLs) shall have the following allowable design stresses:

Fb =	2,750 psi	FcPERP =	750 psi
Fv =	285 psi	E =	2.0 Mpsi
Ft =	1,150 psi	Fc =	2,600 psi

- 3. Submit product data of structural composite lumber for approval prior to ordering. 4. Two-ply and three-ply LVLs shall be fastened together with two rows of Simpson SDS25312 screws
- or approved equal at 12 inches on center on each face. 5. All structural wood construction shall be in conformance with the AF&PA National Design
- Specification for Wood Construction (NDS). 6. All horizontal lumber members shall be fabricated and installed with natural camber (crown)

treated wood shall be hot dipped galvanized to ASTM A 153.

- 7. Nails shall be common wire nails unless noted otherwise. Nails exposed to weather or in preservative treated wood shall be hot dipped galvanized to ASTM A 153. Wood members shall be nailed as indicated in the wood nailing schedule of the International Building Code if not indicated
- 8. Bolts in wood members shall be ASTM A 307 with factory zinc coating. Holes in wood for bolts shall be  $\frac{1}{16}$  oversize. USS flat washers conforming to ASTM F 844 shall be used under bolt heads and nuts against wood. Bolts, nuts and washers exposed to weather or in preservative
- 9. Connectors indicated as "Simpson" on the drawings shall be manufactured by Simpson Strong—tie, Inc. or approved equal. These are also referred to as "framing connectors" or "framing anchors"

10. Simpson connectors shall be hot-dipped galvanized to ASTM A 123 where indicated or where

- exposed to weather. Simpson connectors shall be galvanized to ASTM A 653 G180 where in contact with preservative treated wood and not exposed to weather and shall be ASTM A 653 G90 otherwise or unless indicated otherwise.
- 11. Product data and a plan and schedule of Simpson connectors showing the model number, quantity, finish and type and number of fasteners for all connections shall be submitted for approval prior to ordering Simpson connectors.
- 12. Simpson anchors shall be installed in accordance with all of the manufacturer's instructions. 13. Preservative treated wood appropriate for the service shall be used where in direct contact with
- concrete or masonry or where exposed to weather. 14. Cutting structural lumber members other than as indicated on the structural drawings requires
- approval of the structural engineer. Notching of lumber will not be permitted. 15. Nominal 1x3 wood crossed bridging with beveled ends or Simpson TB36 steel joist bridging shall be installed at maximum 8'-0" spacing on all joists with a minimum of one row of bridging on all joists longer than 10 feet.
- 16. Structural wood members shall be protected from dirt, moisture, sunlight and damage during manufacture, fabrication, shipping, storage and construction.



 $\Xi$ **X** UR A

 $\Xi$ 

STR

0

HKFX



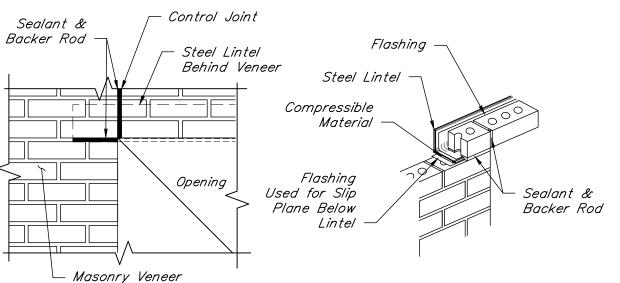
PROJECT NO. 2020052

S1

- 1. Special inspection is required according to section 1704 of the building code.
- 2. Special inspection on this project applies only to the following construction: 2.1. the pump station building
- All other structures shall be inspected according to these notes, but those inspections are not considered "special inspections" as required by the building code because these structures are not primarily for human occupancy and are not in the scope of the
- building code. The inspector shall keep special inspections and non—"special" inspections reports and tests separate and identifiable for record keeping purposes. 3. Special inspections shall be performed for the following work as required in the building
  - 3.1. Contractor's statement of responsibility in accordance with section 1704.4 3.1.1. Contractor shall submit a statement that:
  - 3.1.1.1. acknowledges the requirements stated in this statement of special
  - inspections. 3.1.1.2. acknowledges that control will be exercised over the quality of
  - construction to conform to the approved construction documents. 3.1.1.3. acknowledges that there are organizational procedures in place for exercising control of quality of the construction including:
  - 3.1.1.3.1. appointment of a person within the contractor's organization to exercise control quality of construction
  - 3.1.1.3.2. the persons within the contractor's organization to whom the quality control reports are distributed
  - 3.1.1.3.3. the method and frequency of reporting the quality control results within the contractor's organization.
  - 3.2. Fabricators in accordance with section 1704.2
  - 3.2.1. Submit report of inspector's approval of fabricator's gc plan or fabricator's nationally recognized qc certification.
  - 3.2.2. Submit fabricator's certificate of compliance stating that the work was performed in accordance with the approved construction documents. submitted at the completion of such work.
  - 3.3. Steel construction in accordance with section 1705.2
  - 3.3.1. Submit mill test reports and material certifications for all steel members, fasteners, bolts, nuts, washers, deck, and reinforcement steel for concrete and masonry.
  - 3.3.2. Submit report of inspection of marking and connection details for all members and connections. verify all steel members and steel deck are installed in the correct locations and are connected in accordance with the construction documents and approved erection drawings.
  - 3.3.3. Submit report of inspection of bolt tensioning for each applicable connection.
  - 3.3.4. Submit report of visual inspection of all field welds.
  - 3.4. Concrete construction in accordance with section 1705.3
  - 3.4.1. Submit material certifications of cement, aggregate, admixtures and reinforcement.
  - 3.4.2. Submit report of compressive strength, slump and air content test results. sample and test concrete at least once per day and once for every additional 100 cubic yards of concrete per day thereafter.
  - 3.4.3. Submit report of inspection of forms, reinforcement, and concrete delivery tickets prior to each placement of concrete.
  - 3.5.4. Submit report of inspection of installation of all wedge and chemical adhesive anchors in concrete.
  - 3.4. Masonry construction in accordance with section 1705.4
  - 3.4.1. Submit material certifications of cement, aggregate, admixtures and reinforcement.
  - 3.4.2. Submit report of test of mortar aggregate ratio and air content and observation of mortar proportioning. test once at beginning of project and once every 5,000 s.f. of wall thereafter.
  - 3.4.3. Submit report of placement of masonry, reinforcement and grout prior to and during each placement of grout.
  - 3.4.4. Submit report of installation of chemical adhesive anchorage in concrete at base of masonry walls. inspect installation of 10% of anchorage installations.
  - 3.5. Wood construction in accordance with section 1705.5
  - 3.5.1. See "Inspection of Fabricators" for inspection of prefabricated wood trusses. 3.5.2. Submit material certifications for wood members, sheathing and fasteners.
  - 3.5.3. Submit report of inspection of connection of roof roof trusses to structure.
  - 3.5.4. Submit report of inspection of all wood framing members and their connections. verify all wood framing members are the correct size and grade and are installed in the correct locations, and are connected in accordance with the construction documents.
  - 3.5.5. Submit report of inspection of nailing of roof sheathing to trusses and structure.
  - 3.6. Soils construction in accordance with section 1705.6
  - 3.6.1. Submit report that soil bearing capacity is adequate according to the
  - geotechnical report prior to each placement of foundation concrete. 3.6.2. Submit report of density and moisture content of controlled fill for each lift under building structure.
  - 3.7. Cast—in—place deep foundations in accordance with section 1705.8
  - 3.7.1. Submit report of continuous observation of all drilling operations including complete and accurate records for each drilled shaft.
  - 3.7.2. Submit report indicating the location, plumbness, diameter, length, concrete volume, embedment into bedrock, and adequate end-bearing strata capacity of each pier.
  - 3.7.3. For concrete, perform tests & inspections as required by the concrete special inspection requirements.
- 4. The type and extent of each test and inspection required for each type of work shall be as indicated in the specifications and/or the building code and the references incorporated therein.
- 5. Inspection reports shall include the:
  - 5.1. name, address, and telephone number of special inspector performing the inspection and making the report.
  - 5.2. dates and locations of samples and tests or inspections, date of report.
  - 5.3. record of temperature and weather conditions at time of sample taking and testing and inspecting
  - 5.4. description of the work, identification of products, specification section, tests, and inspection methods.
  - 5.5. photographs of the work inspected for that report
  - 5.6. complete test or inspection data.
- 6. Special inspection shall be performed by a qualified inspection and testing agency approved by the building official and the structural engineer.

SPECIAL INSPECTION & CONSTRUCTION INSPECTION & TESTING (CONTINUED)

- 7. Work requiring special inspection shall be inspected by the special inspector for conformance with the approved drawings and specifications. Inspection reports indicating the results of special inspections shall be promptly submitted to the contractor, the civil engineer, the structural engineer.
- 8. The special inspector shall observe activities, actions, and procedures performed before and during execution of the work to guard against defects and deficiencies and substantiate that proposed construction will comply with requirements.
- 9. All special inspections indicating non-conforming work shall be reported immediately to the contractor, the civil engineer and the structural engineer. Impending construction work that would impede economical correction of non-conforming work shall not proceed without written approval. The contractor shall maintain a discrepancy log on the site. log shall list each discrepancy documented by the special inspector, state the date of discovery and special inspector's report number, and room for the special inspector to sign and date when said discrepancy is corrected. Cost of additional retesting that are required due to non-conforming work may be charged to the contractor.
- 10. A final report certifying completion of all required special inspections and correction of any non-conforming work noted in the inspections shall be submitted by the special inspector at the completion of the project, or if not, detailing non-inspected and/or unresolved non-conformances.
- 11. The contractor shall notify the inspector when construction is ready to be inspected. contractor shall give timely and adequate notice to the special inspector.
- 12. The contractor shall provide the special inspector access to plans, shop drawings, and change orders at the jobsite.
- 13. The contractor shall retain at the jobsite all special inspection records submitted by the special inspector and provide these records for review by the engineer and building inspector upon request.



LOOSE LINTEL SCHEDULE (FOR MASONRY VENEER)

This schedule is for lintels over masonry openings not otherwise

	<u>-</u>	
<u>SPAN</u>	ANGLE SIZE	<u>PLAN M</u>
Up to 4'-0"	L3x 31/2 x5/16 LLV	AL-
Úp to 5'-6"	L4x 31/2 x5/16 LLV	AL-2
Up to 7'-6"	L5x 31/2 x5/16 LLV	AL-J
Up to 9'-6"	L6x 31/2 x5/16 LLV	AL-4

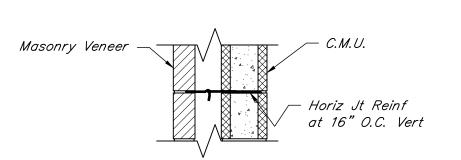
shown or noted on drawings.

- 1. Provide one angle for each 4" of masonry wall. 2. Angles exposed to weather shall be galvanized.
- 3. Minimum bearing length shall be 4" each end.
- TYPICAL MASONRY

VENEER LINTEL DETAIL Not to Scale

Ladder-Type Horiz Joint Reinf w/ Adjustable Pintle & Ring (Hook & Eye) Veneer Ties Joint Reinf: 3/16" Side Rods & 9GA Side Rods Ties: 3/16"Ø Adjustable All Hot-Dipped Galv to ASTM A 153

C.M.U. — Masonry Veneer Veneer Ties Spaced at 16" O.C. Each Way



TYPICAL HORIZ JT REINF & VENEER TIE DETAIL Not to Scale

### **EXPANSION ANCHORS**

- 1. Expansion anchors shall be one of the following products:
  - Kwik Bolt TZ by HILTI Trubolt+ by ITW Red Head
  - Strong-bolt by Simpson Strong-tie
- Other approved equal 2. All expansion anchors for the project shall be produced by the same manufacturer unless approved by the structural engineer.
- 3. Expansion anchor product data and a keyed plan showing the location, diameter, length,
- material and finish of each expansion anchor shall be submitted for approval.
- 4. The expansion anchor manufacturer's installation instructions shall be strictly followed, particularly with regard to drilling and cleaning out the hole.
- 5. If any of the following minimum distances are not indicated or available then verify the detail and field conditions with the structural engineer prior to installing: <u>anchor dia</u> <u>c to c distance</u> <u>edge distance</u> <u>embed distance</u> <u>mat'l thickness</u>

1/2" 3 1/2" 3 1/2" 5 1/2" 5/8" 3/4"

expansion anchor type, material or finish with the structural engineer prior to installing: cracked concrete or masonry near installation (see edge distance above) corrosive, chemical or abnormal temperature environment vibratory or fatigue loading of anchor

6. If any of the following conditions are indicated or present then verify acceptability of

impact or shock loading of anchor

continuous tension (e.g. hanging loads from ceilings)

### CHEMICAL ADHESIVE AND PROPRIETARY ADHESIVE ANCHORS

- 1. Chemical adhesives and proprietary adhesive anchors shall be produced by one of the following manufacturers:
  - HILTI, Inc.
  - ITW Red Head
  - Simpson Strong-tie Other approved equal
- 2. All chemical adhesives and proprietary adhesive anchors for the project shall be produced by the same manufacturer unless approved by the structural engineer.
- 3. Proprietary adhesive anchors shall be fastened with compatible chemical adhesive from the same manufacturer.
- 4. Chemical adhesive and proprietary adhesive anchor product data and a keyed plan showing the location, type of chemical adhesive and installation conditions of each adhesive anchor shall be submitted for approval. installation conditions are:

dry, damp or wet hole cored hole or hammer drilled hole standard (per manufacturer) or oversize hole horizontal, vertical or overhead surface

- temperature range of installation. 5. The chemical adhesive and proprietary adhesive anchor manufacturer's installation instructions shall be strictly followed, particularly with regard to drilling and cleaning out
- the hole and the installation conditions. 6. If any of the following minimum distances are not indicated or available then verify the detail and field conditions with the structural engineer prior to installing:
  - <u>c to c distance</u> <u>edge distance</u> <u>embed distance</u> <u>mat'l thickness</u> <u>anchor dia</u> 1/2" 3 1/2" 3 1/2" 5 1/2" 5/8"
- 7. If any of the following conditions are indicated or present then verify acceptability of chemical adhesive or proprietary adhesive anchor type, material or finish with the structural engineer prior to installing:

corrosive, chemical or abnormal temperature environment vibratory or fatigue loading of anchor

impact or shock loading of anchor

continuous tension (e.g. hanging loads from ceilings).

MATERIAL PATTERN LEGEND

3/4"



COMPETENT ROCK

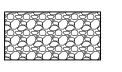
UNDISTURBED SOIL

ENGINEERED FILL

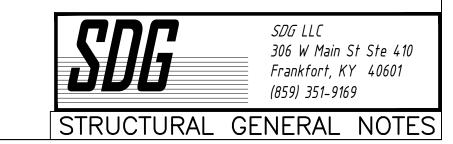


LEAN CONCRETE FLOWABLE FILL GROUT

CONCRETE



CRUSHED STONE DENSE GRADED AGGREGATE



H  $\Box$ UR  $\mathbf{E}$ 

DISTR EMENT

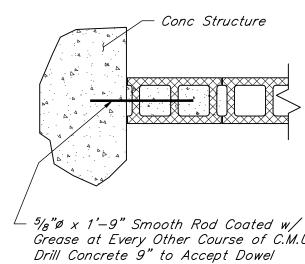
H





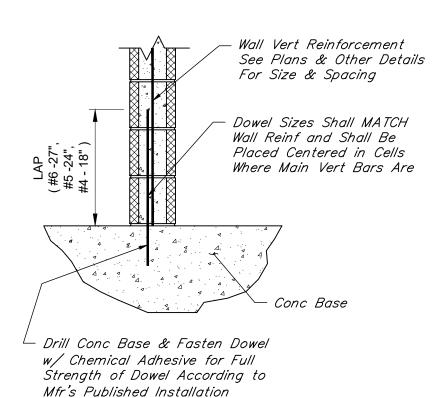
PROJECT NO. 2020052

# TYPICL CONCRETE Not to Scale



Grease at Every Other Course of C.M.U. Chip Webs of C.M.U. & Grout First Two Cells Adjacent to Wall Full Height

PLAN VIEW



SECTION VIEW

TYPICAL C.M.U. WALL DOWEL DETAIL Not to Scale

Cont Grout Fill

Cont Grout Stop

Non-Filled Cells

- Vertical Reinf Shall

Be Continuous thru

Screen Over

Bond Beam

Not to Scale

Reinforcing shall have 3/4" minimum

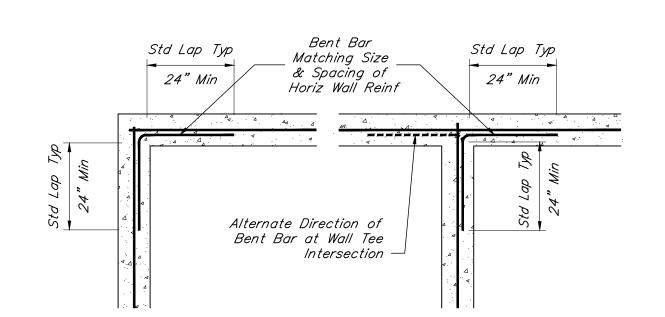
arout cover to all c.m.u. surfaces.

BOND BEAM DETAIL

TYPICAL C.M.U.

2. In lieu of using lintel block on the bottom of lintels which, requires shoring during construction, contractor may use prestressed, precast concrete lintels by "cast-crete" (www.castcrete.com) or approved equal. submit product data and a plan and schedule of lintel locations and sizes for





# See Plan & Schedules For See Civil & Site Dwgs Footing Size & Reinf for Size & Elev Lean Concrete ?" Compressible

Influence Zone Line

INFLUENCE ZONE

Lean Concrete Fill

Pipe Perpendicular

to Footing

See Detail Below

Provide concrete protection around utility line when line is within footing influence zone. See detail above for influence zone definition.

# TYPICAL UTILITY LINE BELOW FOOTING

Not to Scale

Filler All Around

Do Not Place Pipes Below Concrete

Pipes Parallel to

Located Outside

Influence Zone

Footing Shall Be

Footings

- 1. Where bar sizes differ, lap for larger size.
- 2. If bend radius creates problems fitting hairpins in wall, provide more smaller hairpins with equal total area to main bars.
- 3. Construction joints shall not occur within 5'-0" of a corner or tee unless indicated otherwise on the drawings.

# TYPICAL WALL INTERSECTION REINF

Not to Scale

Not to Scale

Main Bond Beam

See Other Details -

Bond Beam Course

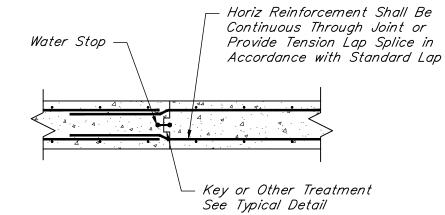
Reinforcement

Hairpin Bar Matching

Reinf Typ-

Size of Bond Beam Reinf

Lap w/ Main Bond Beam



NOTES:

1. Maximum Length of Wall Pour = 40'-0".

2. Minimum 48 Hours Between Adjacent Pours.

or Tee Intersection.

3. See Plans for Additional Joint Locations. 4. Submit Construction Joint Location Plan For Approval Prior to Construction. 5. Do Not Form Joints Within 5'-0" of a Corner

# TYPICAL WALL CONSTRUCTION JOINT

- Tooth intersecting walls together in running bond with min 6" overlap or use masonry strap every third course unless noted otherwise on plans.
- #5 vertical bar centered in grouted cell shall be installed at intersection.
- Horizontal joint reinforcement shall be lapped min 6" at wall intersection.

Cont Vert #5 Bar at Intersection

# TYPICAL MASY WALL INTERSECTION DETAIL

# Not to Scale

Additional Reinforcement

Preformed Rubber or P.V.C.

Control Joint Gasket (Cont)

1. See architectural drawings for control joint locations.

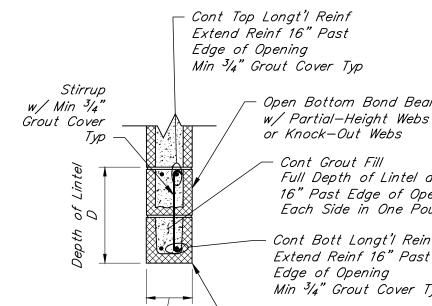
shall not exceed 24 feet.

2. Discontinue horizontal joint reinforcing at control joints.

3. Unless otherwise shown or noted, spacing of control joints

See Typ Details

Adjacent to Control Joint



Open Bottom Bond Beam w/ Partial-Height Webs Full Depth of Lintel and 16" Past Edge of Opening Each Side in One Pour Cont Bott Longt'l Reinf Extend Reinf 16" Past Min 3/4" Grout Cover Typ Lintel Block Over Openings and Open-Bottom Bond Beam for 16" Past Edge of Openings

Each Side

Foundation Wall

Install Additional

Reinf ea Side

of Opening to

Make Up for

Reinf Cut by Opening

Lap as Required

for Size & Elev

Either Provide a Keyway That Is One-Third the

Wall/Slab Thickness Wide By One-Sixth the

Remove Laitance and Roughen Joint to One Quarter Inch Average Amplitude and Apply

1. Joint May Be Wall-to-Wall or Slab-to-Slab.

3. See Other Details for More Joint Information

2. Waterstop May Be Required See Plans & Sections.

Not to Scale

Bonding Agent Prior to Placing Concrete

Cont Reinforcement

as Shown in Details

Provide Corner Bars

Open Bottom Bond

Partial Height Web or

Manufactured with

Knock-Out Web -

Where Required -

Beam Unit

Wall/Slab Thickness Deep

TYPICL CONSTRUCTION JOINT

CONCRETE PREPARATION

TYPICAL UTILITY LINE THRU FDN WALL

See Civil & Site Dwgs

Not to Scale

D.I. Pipe Sleeve

Cast in Wall

2" Polyethylene Foam

WALL-TO-WALI

WALL-TO-SLAB/FDN

Joint Filler -

at Utility Line —

MASONRY LINTEL SCHEDULE							
MARK	MAX OPENING SIZE	D	BOTTOM REINF	TOP REINF	STIRRUPS		
ML-1	5'-0"	8"	2~#5	None	None		
ML-2	8'-0"	16"	2~#5	None	None		
ML-3	11'-8"	24"	2~#5 (8" C.M.U.) 2~#6 (12" C.M.U.)	None	None		
ML-4	18'-0"	24"	2~#5 (8" C.M.U.) 2~#6 (12" C.M.U.)	2~#5	#3@8"		
V <i>OTES:</i>							

Instructions

- 1. Do not use this schedule if concentrated load is applied to the lintel at a height less than half the span above the lintel or if stack bond is specified.
- approval for this option.

### – Bond Beam at Roof & Floor Line See Sections for Size and Reinforcement - Min 2~ #5 at - Min One Bar Required Each Side 8" C.M.U. of Control Joint Typ — Vert Reinf at Typ Spacing Masonry Lintel Above Openings Typ See Sched Typ Dowel Bars Into Lintel Opening Opening , Bond Beam Below all Openings ∠ MIN 1~ #5 Bar TYP Bar Spacing Each Side of All Min 2~ #5 Bars Required Openings ——— See Plan Each Side of all Masonry Openings that Exceed 8" End of Run Control Joints 48" in Width — Full Height Where They Bond Beam Typ Occur at Sides of Openings Do Not Cross Reinf and

Run to Head of Door ---

- 1. Minimum vertical wall reinforcing shall be #5 @ 2'-0" 8. Use lintel block over openings and continue with unless noted otherwise.
- 2. Vertical wall reinforcing shall be continuous. 3. See typical detail for dowels required at base of walls.
- 4. Center reinforcing bars in grouted cells unless noted otherwise. 5. Use bar positioners at minimum 4'-0" spacing to
- support reinforcing bars. 6. Follow specified grouting procedures. 7. Clean mortar from edges of cells so grout can flow

smoothly and fill entire cell.

- open-bottom bond beam from edge of opening into wall so that vertical reinforcing at jamb can pass.
- 9. Control joints shall extend full height of wall and align from floor to floor. 10. Where a control joint occurs through a bond beam or lintel bearing, provide  $2 \sim 1/2$ " dowels across joint with grease on one side. Do not continue horizontal

reinforcing across control joint.

# TYPICAL C.M.U.

Backer Rod

& Sealant

# TYPICAL C.M.U.WALL REINFORCEMENT DETAILS

Not to Scale

# CONTROL JOINT DETAIL Not to Scale

TYPICAL C.M.U. LINTEL DETAIL Not to Scale

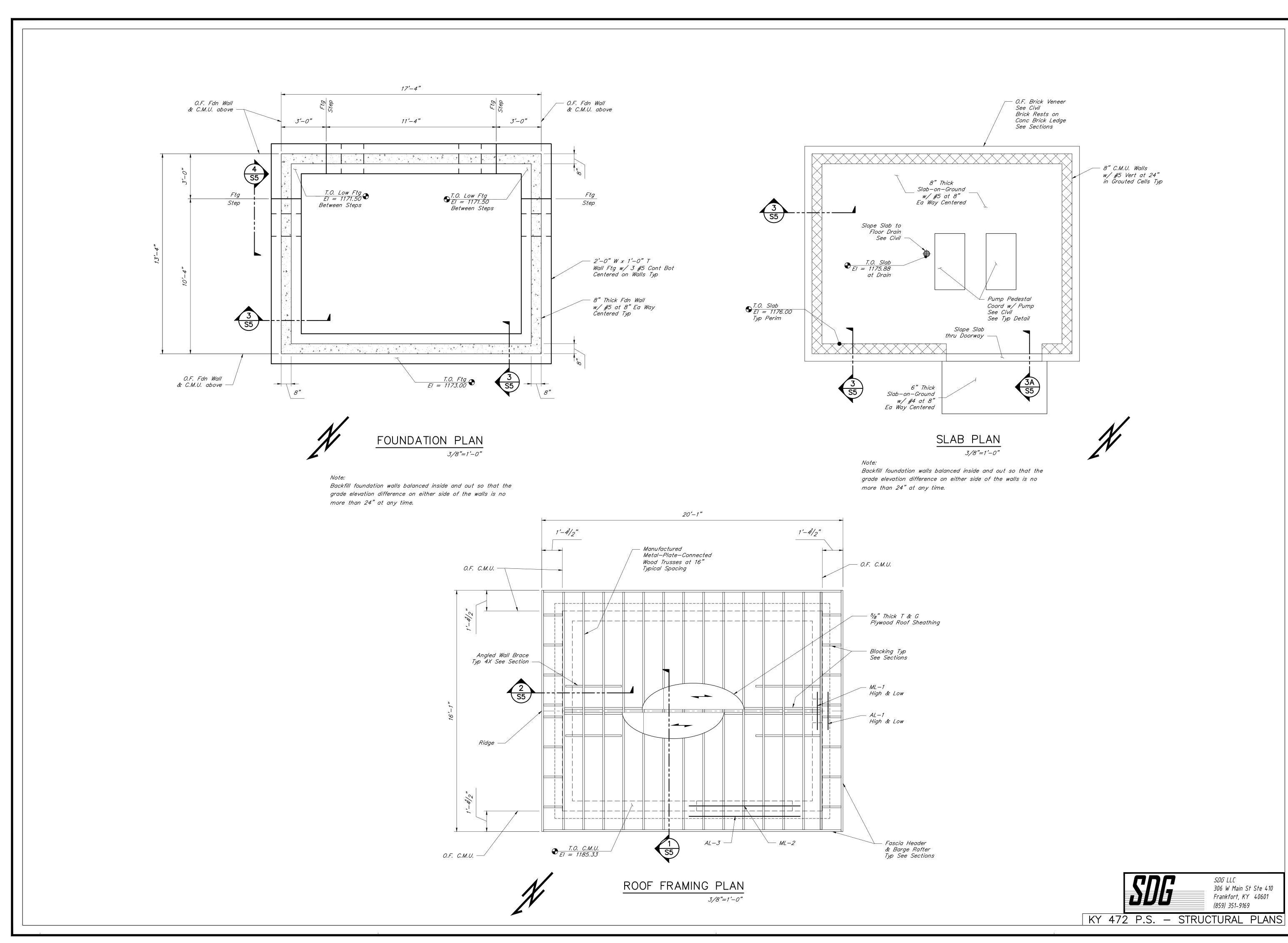
306 W Main St Ste 410 Frankfort, KY 40601 (859) 351-9169 STRUCTURAL TYPICAL DETAILS

RE EI UR  $\Xi$ 

TER

ngine **Environmental** 8

PROJECT NO. 2020052



EAST LAUREL WATER DISTRICT
WATER SYSTEM IMPROVEMENTS
CONTRACT 1
LAUREL COUNTY, KENTUCKY

RAWN BY: BWC
HECKED BY: MWC
ATE: July 2022
CALE:AS NOTED
REVISIONS

NVIRONS Environmental Engineers



PROJECT NO. 2020052

SHEET NO.

S4

EAST LAUREL WATER DISTRICT WATER SYSTEM IMPROVEMENTS CONTRACT 1
LAUREL COUNTY, KENTUCKY

DRAWN BY: BWC
CHECKED BY: MWC
DATE: July 2022
SCALE:AS NOTED
REVISIONS

VIVIRONS

Nironmental Engineers

KKEN Civil & Envir

PROJECT NO. 2020052

sheet no. S5

STRUCTURAL DETAILS

SDG LLC

306 W Main St Ste 410 Frankfort, KY 40601