## Song Sparrow Solar LLC Traffic Impact Study

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## Table of Contents

EXECUTIVE SUMMARY ..... I
1.0 INTRODUCTION ..... 1
2.0 DATA COLLECTION ..... 1
3.0 PROJECT TRIP GENERATION ..... 7
3.1 CONSTRUCTION ..... 7
3.1.1 CONSTRUCTION ANALYSIS ..... 8
3.2 OPERATION ..... 8
4.0 CONCLUSION ..... 9
LIST OF TABLES
Table 1: Level of Service Criteria for Two-Lane Highways ..... 6
Table 2: 2023 Peak Hour Two-Lane Highway Analysis ..... 6
Table 3 - Motor Vehicle Generalized Service Volume Table using AADT for Rural Roadways ..... 7
Table 4: Construction Year (2027) Peak Two-Lane Highway Analysis ..... 8
LIST OF FIGURES
Figure 1: Project Location ..... 2
Figure 2: KYTC Count Stations ..... 3
Figure 3: Population Projections ..... 4
LIST OF APPENDICES
APPENDIX A ..... A. 1
APPENDIX B ..... B. 2

## Executive Summary

Song Sparrow Solar LLC is proposing to construct and operate the Song Sparrow Solar Project (Project) located near the intersection of Gage and Davis Roads approximately 4 miles south of Kevil in Ballard County, Kentucky. The petitioner proposes to utilize the existing land to establish a solar facility on the site. The development will have access points along several routes around the facility. Analyses of the 2022 existing conditions (based on most recent counts provided by the Kentucky Transportation Cabinet, KYTC) and the 2026 construction year were performed. The traffic impact study (TIS) evaluated the operating conditions for the AM and PM peak hours at the roadway segments below:

- Station 004256: KY 286 (Wickliffe Road) from MP 0.000 to MP 14.343
- Station 073800: KY 286 (Wickliffe Road) from MP 0.000 to MP 2.226
- Station 004304: KY 358 (Hinkleville Road) from MP 0.000 to MP 7.984
- Station 004307: KY 473 (Gage Road) from MP 0.000 to MP 7.577
- Station 004274: KY 2532 (Kevil-Lovelaceville Road) from MP 0.000 to MP 5.268
- Station 004281: CR 1127 (Mosstown / Buchanan Road) from MP 0.000 to MP 0.186
- Station 004308: CR 1138 (Robey Road) from MP 0.297 to MP 0.497

Based on the results of the analysis, the following conclusions were developed:

- During construction, all highway segments are anticipated to continue to operate at acceptable level of service (LOS) standards during the peak hours. Therefore, the construction for this project will not adversely affect traffic operations on any of the roadways in and around the project area.
- After construction is complete, the site will be managed with negligible added traffic demand. During the operational phase of the project, the surrounding roadway network will continue to operate at an acceptable LOS during the peak hours.


## SONG SPARROW SOLAR LLC TRAFFIC IMPACT STUDY

INTRODUCTION

### 1.0 INTRODUCTION

The purpose of this study is to estimate the traffic impacts of the proposed Song Sparrow Solar Project ("Song Sparrow Solar" or the "Project") which is located near the intersection of Gage and Davis Roads approximately 4 miles south of Kevil six miles north of Gage in Ballard County, Kentucky. The Project site can be generally described as south of Robey Road, north of KY 286, east of KY 358 and west of KY 2532. The proposed Project site is shown in Figure 1.

The proposed solar Project will be situated on approximately 655 acres and will consist of solar photovoltaic panels and their associated racking systems, inverters, collection system, transmission line, project substation and other project equipment. Arrays of photovoltaic modules will be mounted on single access trackers arranged in rows. Power conversion systems will be distributed throughout the Project area, comprised of inverters, Project substation, and utility switching station. The equipment will connect via underground electrical wiring to a Project substation and switchyard proposing to interconnect to the existing Grahamville-to-Wickliffe 161 kV transmission line located in the southwest corner of the Project area north of Mosstown Road. The Project will have access points around the site with construction vehicle deliveries. A construction year of 2026 was evaluated as part of the study.

### 2.0 DATA COLLECTION

Annual Average Daily Traffic (AADT) traffic counts were obtained from the Kentucky Transportation Cabinet (KYTC) to establish the existing traffic conditions. Figure 2 shows the locations of the primary / adjacent count stations used in this analysis. The summarized count data for each of these stations (plus additional stations outside the immediate area) is included in Appendix A for the following count stations:

- Station 004256: KY 286 (Wickliffe Road) from MP 0.000 to MP 14.343
- Station 073800: KY 286 (Wickliffe Road) from MP 0.000 to MP 2.226
- Station 004304: KY 358 (Hinkleville Road) from MP 0.000 to MP 7.984
- Station 004307: KY 473 (Gage Road) from MP 0.000 to MP 7.577
- Station 004274: KY 2532 (Kevil-Lovelaceville Road) from MP 0.000 to MP 5.268
- Station 004281: CR 1127 (Buchanan Road) from MP 0.000 to MP 0.186
- Station 004308: CR 1138 (Robey Road) from MP 0.297 to MP 0.497

DATA COLLECTION


Figure 1: Project Location

## SONG SPARROW SOLAR LLC TRAFFIC IMPACT STUDY

DATA COLLECTION


Figure 2: KYTC Count Stations

## SONG SPARROW SOLAR LLC TRAFFIC IMPACT STUDY

## DATA COLLECTION

Ballard County population projections have continued to decline slightly since 2010, as shown in Figure 3. Therefore, a growth rate was not applied to the traffic count data as it is presumed that volumes have not increased.


Figure 3: Population Projections

KY 286 located directly south of the Project site is classified as a two-lane rural minor arterial with daily traffic volume ranging from 3,020 vehicles per day (VPD) in Ballard County to 3,840 VPD in McCracken County with a posted speed limit of 55 mph . KY 358 is a two-lane rural minor collector with a posted speed limit of 55 mph and daily traffic of 265 VPD . KY 473 is a two-lane rural minor collector with a posted speed limit of 55 mph and daily traffic of 190 VPD. To the east of the Project site, KY 2532 is a two-lane rural minor collector with a posted speed limit of 55 mph and daily traffic of 265 VPD. A map of speed limits is shown in Figure 4. Note that speed limits for Robey Road and Buchanan Road were not included in KYTC GIS database, but the posted speed limit for both routes is 35 mph .

DATA COLLECTION


55 Miles per Hour
35 Miles per Hour
Speed Limit not provided

Sources: Kentucky Division of Geographic
Information (DGI), Kentucky Transportation Cabinet

## SONG SPARROW SOLAR LLC TRAFFIC IMPACT STUDY

## DATA COLLECTION

Two-lane analyses were used to evaluate the roadways based on methods described in the Highway Capacity Manual (HCM) and implemented within the Highway Capacity Software (HCS 2023). The results can be found in Appendix B. The analyses were used to estimate capacity and Level of Service (LOS) for given traffic and geometric conditions. LOS provides a measure of the quality of traffic flow provided by a roadway facility, expressed in terms of letter grades with LOS A representing the highest quality traffic flow and minimal delay, and LOS F representing poor traffic operations and significant delay. For rural areas, LOS C or better is generally considered to be desirable. In urban areas, LOS D or better is generally considered desirable.

The two-lane highways method utilizes follower density (followers/mile) as the service measure for LOS, as shown in Table 1.

Table 1: Level of Service Criteria for Two-Lane Highways

| LOS | Density (followers $/ \mathrm{mi}$ ) <br> Speed Limit $\geq 50 \mathrm{mph}$ | Density (followers $/ \mathrm{mi}$ ) <br> Speed Limit $<50 \mathrm{mph}$ |
| :---: | :---: | :---: |
| A | $\leq 2$ | $\leq 2.5$ |
| B | $>2-4$ | $>2.5-5.5$ |
| C | $>4-8$ | $>5-10$ |
| D | $>8-12$ | $>10-15$ |
| E | $>12$ | $>15$ |
| F | Demand exceeds capacity | Demand exceeds capacity |

The results of the existing (2023) year peak hour traffic analyses for two-lane roads are summarized in
Table 2. The results indicate that all existing year project-adjacent two-lane roadways currently operate at acceptable LOS during the peak hour.

Table 2: 2023 Peak Hour Two-Lane Highway Analysis

| Route | Segment Description | 2023 Peak Hour |  |
| :---: | :---: | :---: | :---: |
|  |  | $\begin{gathered} \text { Density } \\ \text { (followers/mi/ln) } \end{gathered}$ | LOS |
| KY 286 | from KY 802 (MP 7.6) to KY 1367 (MP 13.6) | 0.6 | A |
|  | from KY 1367 (MP 13.6) to County Line (MP 14.3) | 1.1 | A |
| KY 358 | from KY 286 (MP 0.0) to US 60 (MP 8.0) | 0.0 | A |
| KY 473 | from KY 286 (MP 0.0) to US 60 (MP 7.6) | 0.0 | A |
| KY 2532 | from KY 286 (MP 0.0) to US 60 (MP 5.3) | 0.0 | A |
| CR 1127 | west of KY 473 from MP 0.0 to MP 0.2 | 0.0 | A |
| CR 1138 | east of KY 473 from MP 0.3 to MP 0.5 | 0.0 | A |

## SONG SPARROW SOLAR LLC TRAFFIC IMPACT STUDY

PROJECT TRIP GENERATION

Motor Vehicle Highway Generalized Service Volume Tables were also used to evaluate the roadways based on methods described in the 2023 Multimodal Quality/Level of Service Handbook (Q/LOS Handbook. The handbook is intended to be used by engineers, planners, and decision-makers to evaluate roadway users' quality/level of service (Q/LOS) at generalized planning levels. The Generalized Service Volume Tables are the primary tools for conducting generalized planning and are based on the Highway Capacity Manual (HCM) Sixth Edition and roadway, traffic, control characteristics and multimodal data. The Service Volume Table using Annual Average Daily Traffic (AADT) for Rural Roadways was used for this evaluation and are shown in Table 3. Rural roadways are in areas with a population less than 5,000 and not immediately adjacent to core urbanized, urbanized, or transitioning areas. Rural refers to sparsely settled lands that may include agricultural land mixed with grassland, woodland, or wetlands.

The results indicate that all existing Project-adjacent two-lane roadways have an AADT below, and in most cases far below, 4,600 vehicles per day and, therefore, operate at LOS A. Note that the AADT volumes which are less than the threshold volume for LOS B would be LOS A. As shown in Figure 2, the highest count volume count station is KY 286 at the Ballard / McCracken County line at 3,840 VPD.

Table 3 - Motor Vehicle Generalized Service Volume Table using AADT for Rural Roadways

| Roadway Type | Level of Service (LOS) |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | B | C | D | E |
| Two-Lane | 4,600 | 8,200 | 14,000 | 28,500 |
| Four-Lane | 32,000 | 45,800 | 55,700 | 63,900 |
| Six-Lane | 48,000 | 68,300 | 83,700 | 95,900 |

Source: 2023 Multimodal Quality/Level of Service Handbook, Appendix B

### 3.0 PROJECT TRIP GENERATION

### 3.1 CONSTRUCTION

The trip generation analysis for the construction of the Project would generally be based on the number of workers and the associated construction and delivery truck trips expected during the construction of the Project. Construction workers will consist of laborers, equipment operators, electricians, supervisory personnel, support personnel, and construction management personnel. It is envisioned that workers will

## SONG SPARROW SOLAR LLC TRAFFIC IMPACT STUDY

PROJECT TRIP GENERATION
arrive/depart from passenger vehicles and trucks daily during the AM (6:00 - 9:00 AM) and PM (3:00 7:00 PM) peak hours. Equipment deliveries will occur on trailers, flatbeds, or other large vehicles at various times during the day. While specific details concerning construction duration and intensity are not currently known, this study has employed a sensitivity analysis to demonstrate likely construction traffic levels will not have a significant, adverse effect on peak hour traffic operations. For this analysis, existing peak hour traffic volumes on roadways were increased by 50 percent which is far greater than would be anticipated for the actual construction of the Project.

### 3.1.1 CONSTRUCTION ANALYSIS

The 2026 construction year analysis assumed no changes to the existing roadway network and increases in traffic demand discussed above. The results of the construction year peak hour two-lane analysis are summarized in Table 4. Complete output reports are included in Appendix B. The results indicate that all analyzed roadway segments are anticipated to continue to operate at an acceptable LOS during construction for both peak hours.

Table 4: Construction Year (2026) Peak Two-Lane Highway Analysis

| Route | Segment Description | Year Peak Hour |  |
| :---: | :---: | :---: | :---: |
|  |  | Density (followers/mi/ln) | LOS |
| KY 286 | from KY 802 (MP 7.6) to KY 1367 (MP 13.6) | 1.2 | A |
|  | from KY 1367 (MP 13.6) to County Line (MP 14.3) | 2.1 | B |
| KY 358 | from KY 286 (MP 0.0) to US 60 (MP 8.0) | 0.0 | A |
| KY 473 | from KY 286 (MP 0.0) to US 60 (MP 7.6) | 0.0 | A |
| KY 2532 | from KY 286 (MP 0.0) to US 60 (MP 5.3) | 0.0 | A |
| CR 1127 | west of KY 473 from MP 0.0 to MP 0.2 | 0.0 | A |
| CR 1138 | east of KY 473 from MP 0.3 to MP 0.5 | 0.0 | A |

Similarly, the results indicate that all but one of the construction volumes (existing volumes increased by 50 percent) Project-adjacent two-lane roadways still have an AADT below 4,600 vehicles per day and would, therefore, still operate at LOS A. Increasing the highest count volume count station is KY 286 at the Ballard / McCracken County line by 50 percent would increase the volume to 5,760 VPD which would operate at LOS B.

### 3.2 OPERATION

Once operational, the facility will be managed and monitored by a small number of employees. The facility will have up to three full-time employees on site for 40 hours per week for site inspections and

## SONG SPARROW SOLAR LLC TRAFFIC IMPACT STUDY

CONCLUSION
repair. Operations workers are expected to commute to and from the project site individually during the peak AM and PM hours. Work can also be conducted at night up to thirty days a year. This additional volume of daily traffic is considered negligible, and the operational phase of the project will have no measurable impact on the traffic and/or transportation infrastructure.

### 4.0 CONCLUSION

As demonstrated in the traffic analysis, the construction period will not produce significant operational changes to existing roadways. All roadways within the Project area will continue to operate at an acceptable LOS (likely LOS A) during peak construction traffic. Although no significant adverse traffic impacts are expected during project construction or operation, using mitigation measures such as ridesharing between construction workers, using appropriate traffic controls, or allowing flexible working hours outside of peak hours could be implemented to minimize any potential for delays during the AM and PM peak hours.

## SONG SPARROW SOLAR PROJECT

Appendix A

## Appendix A

TRAFFIC COUNT DATA

Historical Traffic Volume Summary

Station Details:

| Sta ID: | 004256 |
| :--- | :--- |
| Sta Type: | Classification |
| Map: | Maplt |
| District: | 1 |
| County: | Ballard |
| Route: | $004-$-KY-0286 -000 |
| Route Desc: | WICKLIFFE RD |


| Begin MP: | 7.6140 |
| :--- | :--- |
| Begin Desc: | KY 802 |
| End Mp: | 13.5630 |
| End Desc: | KY 1367 |
| Impact Year: |  |
| Year Added: |  |

Newest Count:

| AADT: | 3011 |
| :--- | :--- |
| Year: | 2017 |
| \% Single: | 6.1530 |
| \% Combo: | 7.4950 |
| K Factor: | 8.50 |
| D Factor: | 53 |

## Definitions:

Sta. ID - Three digit county number + station number
MP - milepoint
Impact Year - year of significant change to traffic pattern within station segment
AADT - Annual Average Daily Traffic - the annualized average 24-hour volume of vehicles on a segment of roadway
\% Single - single unit truck volume as a percentage of the AADT
\% Combo - combination truck volume as a percentage of the AADT
K Factor - peak hour volume as a percentage of the AADT
D Factor - percentage of peak hour volume flowing in the peak direction

| Year | AADT | Year | AADT | Year | AADT |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2023 |  | 2013 |  | 2003 |  |
| 2022 |  | 2012 |  | 2002 | 2630 |
| 2021 |  | 2011 | 2040 | 2001 |  |
| 2020 |  | 2010 |  | 2000 | 2480 |
| 2019 |  | 2009 |  | 1999 |  |
| 2018 |  | 2008 | 2390 | 1998 |  |
| 2017 | 3011 | 2007 |  | 1997 |  |
| 2016 |  | 2006 |  | 1996 |  |
| 2015 |  | 2005 |  | 1995 |  |
| 2014 | 2427 | 2004 |  | 1994 | 2410 |



Historical Traffic Volume Summary
Station Details:

| Begin MP: | 0 |
| :--- | :--- |
| Begin Desc: | BALLARD COUNTY LINE |
| End Mp: | 2.2260 |
| End Desc: | US 62 |
| Impact Year: |  |
| Year Added: |  |

Newest Count:

| Sta ID: | 073800 |
| :--- | :--- |
| Sta Type: | Full Coverage |
| Map: | Maplt |
| District: | 1 |
| County: | McCracken |
| Route: | $073-$ KY-0286 -000 |
| Roun |  |


| AADT: | 3836 |
| :--- | :--- |
| Year: | 2020 |
| \% Single: | 6.1530 |
| \% Combo: | 7.4950 |
| K Factor: | 8.70 |
| D Factor: | 56 |

Route Desc: KY-286

## Definitions:

Sta. ID - Three digit county number + station number
MP - milepoint
Impact Year - year of significant change to traffic pattern within station segment
AADT - Annual Average Daily Traffic - the annualized average 24 -hour volume of vehicles on a segment of roadway
\% Single - single unit truck volume as a percentage of the AADT
\% Combo - combination truck volume as a percentage of the AADT
K Factor - peak hour volume as a percentage of the AADT
D Factor - percentage of peak hour volume flowing in the peak direction

| Year | AADT | Year | AADT | Year | AADT |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2023 |  | 2013 |  | 2003 |  |
| 2022 |  | 2012 |  | 2002 | 2930 |
| 2021 |  | 2011 | 2920 | 2001 |  |
| 2020 | 3836 | 2010 |  | 2000 |  |
| 2019 |  | 2009 |  | 1999 | 3060 |
| 2018 |  | 2008 | 3040 | 1998 |  |
| 2017 | 3292 | 2007 |  | 1997 |  |
| 2016 |  | 2006 |  | 1996 |  |
| 2015 |  | 2005 | 3100 | 1995 |  |
| 2014 | 3224 | 2004 |  | 1994 | 2650 |



Historical Traffic Volume Summary

| Station Details: |
| :--- |
| Sta ID: |
| Sta Type: |
| Full Coverage |
| Map: |
| District: |
| Maplt |
| County: |
| Route: |


| Begin MP: | 0 |
| :--- | :--- |
| Begin Desc: | KY 286 (WICKLIFFE ROAD) |
| End Mp: | 7.9840 |
| End Desc: | US 60 JUNCTION |
| Impact Year: |  |
| Year Added: |  |

Newest Count:

| AADT: | 263 |
| :--- | :--- |
| Year: | 2020 |
| \% Single: |  |
| \% Combo: |  |
| K Factor: | 12.10 |
| D Factor: | 56 |

Route Desc: HINKLEVILLE RD+BLUEGRASS DR

## Definitions:

Sta. ID - Three digit county number + station number MP - milepoint
Impact Year - year of significant change to traffic pattern within station segment
AADT - Annual Average Daily Traffic - the annualized average 24 -hour volume of vehicles on a segment of roadway
\% Single - single unit truck volume as a percentage of the AADT
\% Combo - combination truck volume as a percentage of the AADT
K Factor - peak hour volume as a percentage of the AADT
D Factor - percentage of peak hour volume flowing in the peak direction

| Year | AADT | Year | AADT | Year | AADT |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2023 |  | 2013 |  | 2003 |  |
| 2022 |  | 2012 |  | 2002 | 332 |
| 2021 |  | 2011 | 326 | 2001 |  |
| 2020 | 263 | 2010 |  | 2000 |  |
| 2019 |  | 2009 |  | 1999 |  |
| 2018 |  | 2008 | 303 | 1998 | 120 |
| 2017 | 274 | 2007 |  | 1997 |  |
| 2016 |  | 2006 |  | 1996 |  |
| 2015 |  | 2005 |  | 1995 |  |
| 2014 | 262 | 2004 |  | 1994 |  |



Historical Traffic Volume Summary
Station Details:

| Sta ID: | 004307 |
| :--- | :--- |
| Sta Type: | Full Coverage |
| Map: | Maplt |
| District: | 1 |
| County: | Ballard |
| Route: | $004-$-KY-0473 -000 |
| Route Desc: | GAGE RD |


| Begin MP: | 0 |
| :--- | :--- |
| Begin Desc: | KY 286 |
| End Mp: | 7.5770 |
| End Desc: | US 60 (KENTUCKY AVE) JUNCTION |
| Impact Year: |  |
| Year Added: |  |

Newest Count:

| AADT: | 189 |
| :--- | :--- |
| Year: | 2020 |
| \% Single: |  |
| \% Combo: |  |
| K Factor: | 12.20 |
| D Factor: | 56 |

## Definitions:

Sta. ID - Three digit county number + station number
MP - milepoint
Impact Year - year of significant change to traffic pattern within station segment
AADT - Annual Average Daily Traffic - the annualized average 24 -hour volume of vehicles on a segment of roadway
\% Single - single unit truck volume as a percentage of the AADT
\% Combo - combination truck volume as a percentage of the AADT
K Factor - peak hour volume as a percentage of the AADT
D Factor - percentage of peak hour volume flowing in the peak direction

| Year | AADT | Year | AADT | Year | AADT |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2023 |  | 2013 |  | 2003 |  |
| 2022 |  | 2012 |  | 2002 | 226 |
| 2021 |  | 2011 | 176 | 2001 |  |
| 2020 | 189 | 2010 |  | 2000 |  |
| 2019 |  | 2009 |  | 1999 |  |
| 2018 |  | 2008 | 157 | 1998 | 224 |
| 2017 | 77 | 2007 |  | 1997 |  |
| 2016 |  | 2006 |  | 1996 |  |
| 2015 |  | 2005 | 126 | 1995 |  |
| 2014 | 153 | 2004 |  | 1994 |  |



Historical Traffic Volume Summary
Station Details:

| Begin MP: | 0 |
| :--- | :--- |
| Begin Desc: | KY 286 |
| End Mp: | 5.2680 |
| End Desc: | EXIT BALLARD CO-MCCRACKEN CO |
| Impact Year: |  |
| Year Added: |  |

Newest Count:

| AADT: | 265 |
| :--- | :--- |
| Year: | 2022 |
| \% Single: |  |
| \% Combo: |  |
| K Factor: | 13.60 |
| D Factor: | 58 |

Route Desc: COUNTY LINE RD

## Definitions:

Sta. ID - Three digit county number + station number
MP - milepoint
Impact Year - year of significant change to traffic pattern within station segment
AADT - Annual Average Daily Traffic - the annualized average 24 -hour volume of vehicles on a segment of roadway
\% Single - single unit truck volume as a percentage of the AADT
\% Combo - combination truck volume as a percentage of the AADT
K Factor - peak hour volume as a percentage of the AADT
D Factor - percentage of peak hour volume flowing in the peak direction

| Year | AADT | Year | AADT | Year | AADT |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2023 |  | 2013 | 288 | 2003 |  |
| 2022 | 265 | 2012 |  | 2002 | 354 |
| 2021 |  | 2011 |  | 2001 |  |
| 2020 |  | 2010 | 355 | 2000 |  |
| 2019 | 362 | 2009 |  | 1999 |  |
| 2018 |  | 2008 |  | 1998 | 146 |
| 2017 |  | 2007 | 300 | 1997 |  |
| 2016 | 349 | 2006 |  | 1996 |  |
| 2015 |  | 2005 |  | 1995 |  |
| 2014 |  | 2004 | 413 | 1994 |  |



Historical Traffic Volume Summary
Station Details:

| Sta ID: | 004281 |
| :--- | :--- |
| Sta Type: | Local Road Bridge |
| Map: | Maplt |
| District: | 1 |
| County: | Ballard |
| Route: | $004-C R-1127$-000 |
| Route Desc: | MOSSTOWN RD |


| Begin MP: | 0 |
| :--- | :--- |
| Begin Desc: |  |
| End Mp: | 0.1860 |
| End Desc: |  |
| Impact Year: |  |
| Year Added: |  |

Newest Count:

| AADT: | 100 |
| :--- | :--- |
| Year: | 2010 |
| \% Single: |  |
| \% Combo: |  |
| K Factor: |  |
| D Factor: |  |

## Definitions:

Sta. ID - Three digit county number + station number
MP - milepoint
Impact Year - year of significant change to traffic pattern within station segment
AADT - Annual Average Daily Traffic - the annualized average 24 -hour volume of vehicles on a segment of roadway
\% Single - single unit truck volume as a percentage of the AADT
\% Combo - combination truck volume as a percentage of the AADT
K Factor - peak hour volume as a percentage of the AADT
D Factor - percentage of peak hour volume flowing in the peak direction

| Year | AADT | Year | AADT | Year | AADT |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2023 |  | 2013 |  | 2003 |  |
| 2022 |  | 2012 |  | 2002 |  |
| 2021 |  | 2011 |  | 2001 |  |
| 2020 |  | 2010 | 100 | 2000 |  |
| 2019 |  | 2009 |  | 1999 |  |
| 2018 |  | 2008 |  | 1998 |  |
| 2017 |  | 2007 |  | 1997 |  |
| 2016 |  | 2006 |  | 1996 |  |
| 2015 |  | 2005 |  | 1995 |  |
| 2014 |  | 2004 |  | 1994 |  |



Historical Traffic Volume Summary
Station Details:

| Sta ID: | 004308 |
| :--- | :--- |
| Sta Type: | Local Road Bridge |
| Map: | Maplt |
| District: | 1 |
| County: | Ballard |
| Route: | $004-C R-1138-000$ |
| Route Desc: | ROBEY RD |


|  |  | Newest Count: |  |
| :---: | :---: | :---: | :---: |
| Begin MP: | 0.2970 | AADT: | 77 |
| Begin Desc: |  | Year: | 2010 |
| End Mp: | 0.4970 | \% Single: |  |
| End Desc: |  | \% Combo: |  |
| Impact Year: |  | K Factor: |  |
| Year Added: |  | D Factor: |  |

## Definitions:

Sta. ID - Three digit county number + station number
MP - milepoint
Impact Year - year of significant change to traffic pattern within station segment
AADT - Annual Average Daily Traffic - the annualized average 24 -hour volume of vehicles on a segment of roadway
\% Single - single unit truck volume as a percentage of the AADT
\% Combo - combination truck volume as a percentage of the AADT
K Factor - peak hour volume as a percentage of the AADT
D Factor - percentage of peak hour volume flowing in the peak direction

| Year | AADT | Year | AADT | Year | AADT |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2023 |  | 2013 |  | 2003 |  |
| 2022 |  | 2012 |  | 2002 |  |
| 2021 |  | 2011 |  | 2001 |  |
| 2020 |  | 2010 | 77 | 2000 |  |
| 2019 |  | 2009 |  | 1999 |  |
| 2018 |  | 2008 |  | 1998 |  |
| 2017 |  | 2007 |  | 1997 |  |
| 2016 |  | 2006 |  | 1996 |  |
| 2015 |  | 2005 |  | 1995 |  |
| 2014 |  | 2004 |  | 1994 |  |



## SONG SPARROW SOLAR PROJECT

Appendix B

## Appendix B

HIGHWAY CAPACITY SOFTWARE (HCS 2023) FILES
2023
2026 (CONSTRUCTION YEAR)

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2023 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 286 (MP 7.614-13.563) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 31411 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 2 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 144 | Opposing Demand Flow Rate, veh/h | 128 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 7.50 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.08 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 56.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 3.35095 | Speed Power Coefficient $(\mathrm{p})$ | 0.56282 |
| PF Slope Coefficient (m) | -1.21377 | PF Power Coefficient $(\mathrm{p})$ | 0.77734 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 0.6 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 31411 | - | - | 55.4 |

## Vehicle Results

| Average Speed, mi/h | 55.4 | Percent Followers, \% | 23.6 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 6.45 | Follower Density (FD), followers $/ \mathrm{mi} / \mathrm{ln}$ | 0.6 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i / l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 202 | 0.04 | 0.6 | A |

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HCS

## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2023 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 286 (MP 13.563-14.343) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 11753 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 2 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 198 | Opposing Demand Flow Rate, veh/h | 155 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 7.50 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.12 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 56.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient (m) | 3.36301 | Speed Power Coefficient (p) | 0.55293 |
| PF Slope Coefficient (m) | -1.22274 | PF Power Coefficient (p) | 0.77472 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 1.1 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 11753 | - | - | 55.0 |

## Vehicle Results

| Average Speed, mi/h | 55.0 | Percent Followers, \% | 29.4 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 2.43 | Follower Density (FD), followers $/ \mathrm{mi} / \mathrm{ln}$ | 1.1 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i / l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 104 | 0.03 | 1.1 | A |

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## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2023 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 358 (MP 0.000-7.984) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 42156 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 19 | Opposing Demand Flow Rate, veh/h | 15 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.01 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 54.8 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 3.21149 | Speed Power Coefficient $(\mathrm{p})$ | 0.63372 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.15434 | PF Power Coefficient $(\mathrm{p})$ | 0.79102 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 42156 | - | - | 54.8 |

## Vehicle Results

| Average Speed, mi/h | 54.8 | Percent Followers, \% | 4.9 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 8.74 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i / l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 36 | 0.00 | 0.0 | A |

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## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2023 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 473 (MP 0.000-7.577) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 40007 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 14 | Opposing Demand Flow Rate, veh/h | 11 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.01 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 54.8 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient (m) | 3.20529 | Speed Power Coefficient (p) | 0.64000 |
| PF Slope Coefficient (m) | -1.14880 | PF Power Coefficient (p) | 0.79264 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 40007 | - | - | 54.8 |

## Vehicle Results

| Average Speed, mi/h | 54.8 | Percent Followers, \% | 3.8 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 8.30 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| $\mathbf{T}$ | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i} / \mathbf{l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 25 | 0.00 | 0.0 | A |

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## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2023 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 2532 (MP 0.000-5.268) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 27815 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 22 | Opposing Demand Flow Rate, veh/h | 16 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.01 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 54.8 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient (m) | 3.21289 | Speed Power Coefficient (p) | 0.63231 |
| PF Slope Coefficient (m) | -1.15559 | PF Power Coefficient (p) | 0.79066 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 27815 | - | - | 54.8 |

## Vehicle Results

| Average Speed, mi/h | 54.8 | Percent Followers, \% | 5.6 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 5.77 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i} / \mathbf{l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 28 | 0.00 | 0.0 | A |

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## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2023 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | CR 1127 (Mosstown Road) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 982 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 35 | Access Point Density, pts $/ \mathrm{mi}$ | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 5 | Opposing Demand Flow Rate, veh/h | 5 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.00 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 32.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 1.86433 | Speed Power Coefficient $(\mathrm{p})$ | 0.65149 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.17423 | PF Power Coefficient $(\mathrm{p})$ | 0.70924 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 982 | - | - | 32.0 |

## Vehicle Results

| Average Speed, mi/h | 32.0 | Percent Followers, \% | 2.6 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 0.35 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| $\mathbf{T}$ | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i / l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 0.00 | 0.0 | A |

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## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2023 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | CR 1138 (Robey Road) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 1056 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 35 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 3 | Opposing Demand Flow Rate, veh/h | 3 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.00 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 32.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 1.86017 | Speed Power Coefficient $(\mathrm{p})$ | 0.65585 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.17027 | PF Power Coefficient $(\mathrm{p})$ | 0.71031 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 1056 | - | - | 32.0 |

## Vehicle Results

| Average Speed, mi/h | 32.0 | Percent Followers, \% | 2.0 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 0.38 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i / l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 0.00 | 0.0 | A |

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## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2026 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 286 (MP 7.614-13.563) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 31411 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 2 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 216 | Opposing Demand Flow Rate, veh/h | 191 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 7.50 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.13 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 56.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 3.37727 | Speed Power Coefficient $(\mathrm{p})$ | 0.54167 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.23298 | PF Power Coefficient $(\mathrm{p})$ | 0.77170 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh $/ \mathrm{mi} / \mathrm{ln}$ | 1.2 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 31411 | - | - | 54.9 |

## Vehicle Results

| Average Speed, mi/h | 54.9 | Percent Followers, \% | 31.5 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 6.50 | Follower Density (FD), followers $/ \mathrm{mi} / \mathrm{ln}$ | 1.2 |
| Vehicle LOS | A |  |  |

## Facility Results

| $\mathbf{T}$ | VMT <br> veh-mi/AP | VHD <br> veh- $\mathbf{h} / \mathbf{p}$ | Follower Density, followers/ <br> $\mathbf{m i} / \mathbf{l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 302 | 0.10 | 1.2 | A |

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## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2026 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 286 (MP 13.563-14.343) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 11753 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 2 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 297 | Opposing Demand Flow Rate, veh/h | 233 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 7.50 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.17 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 56.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient (m) | 3.39206 | Speed Power Coefficient (p) | 0.53048 |
| PF Slope Coefficient (m) | -1.24317 | PF Power Coefficient (p) | 0.76863 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 2.1 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 11753 | - | - | 54.5 |

## Vehicle Results

| Average Speed, mi/h | 54.5 | Percent Followers, \% | 38.7 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 2.45 | Follower Density (FD), followers/mi/ln | 2.1 |
| Vehicle LOS | B |  |  |

## Facility Results

| $\mathbf{T}$ | VMT <br> veh-mi/AP | VHD <br> $\mathbf{v e h}-\mathbf{h} / \mathbf{p}$ | Follower Density, followers/ <br> $\mathbf{m i} / \mathbf{l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 155 | 0.07 | 2.1 | B |

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## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2026 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 358 (MP 0.000-7.984) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 42156 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 29 | Opposing Demand Flow Rate, veh/h | 22 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.02 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 54.8 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 3.22048 | Speed Power Coefficient $(\mathrm{p})$ | 0.62477 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.16229 | PF Power Coefficient $(\mathrm{p})$ | 0.78873 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh $/ \mathrm{mi} / \mathrm{ln}$ | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 42156 | - | - | 54.8 |

## Vehicle Results

| Average Speed, mi/h | 54.8 | Percent Followers, \% | 6.8 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 8.74 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i} / \mathbf{l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 54 | 0.00 | 0.0 | A |

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## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2026 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 473 (MP 0.000-7.577) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 40007 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 20 | Opposing Demand Flow Rate, veh/h | 16 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.01 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 54.8 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient (m) | 3.21289 | Speed Power Coefficient (p) | 0.63231 |
| PF Slope Coefficient (m) | -1.15559 | PF Power Coefficient (p) | 0.79066 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh/mi/ln | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 40007 | - | - | 54.8 |

## Vehicle Results

| Average Speed, mi/h | 54.8 | Percent Followers, \% | 5.1 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 8.30 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i / l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 36 | 0.00 | 0.0 | A |

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## Project Information

| Analyst | TT | Date | 8/15/2023 |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2026 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | KY 2532 (MP 0.000-5.268) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 27815 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 55 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 33 | Opposing Demand Flow Rate, veh/h | 24 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.02 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 54.8 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 3.22276 | Speed Power Coefficient $(\mathrm{p})$ | 0.62253 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.16428 | PF Power Coefficient $(\mathrm{p})$ | 0.78815 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh $/ \mathrm{mi} / \mathrm{ln}$ | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 27815 | - | - | 54.8 |

## Vehicle Results

| Average Speed, mi/h | 54.8 | Percent Followers, \% | 7.6 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 5.77 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh- $\mathbf{h} / \mathbf{p}$ | Follower Density, followers/ <br> $\mathbf{m i} / \mathbf{l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 41 | 0.00 | 0.0 | A |

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## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2026 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | CR 1127 (Mosstown Road) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 982 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 35 | Access Point Density, pts $/ \mathrm{mi}$ | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 7 | Opposing Demand Flow Rate, veh/h | 7 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.00 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 32.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 1.86943 | Speed Power Coefficient $(\mathrm{p})$ | 0.64619 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.17908 | PF Power Coefficient $(\mathrm{p})$ | 0.70794 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh $/ \mathrm{mi} / \mathrm{ln}$ | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 982 | - | - | 32.0 |

## Vehicle Results

| Average Speed, mi/h | 32.0 | Percent Followers, \% | 3.5 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 0.35 | Follower Density (FD), followers/mi/ln | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| $\mathbf{T}$ | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i} / \mathbf{l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 0.00 | 0.0 | A |

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## HCS Two-Lane Highway Report

## Project Information

| Analyst | TT | Date | $8 / 15 / 2023$ |
| :--- | :--- | :--- | :--- |
| Agency | Stantec Consulting | Analysis Year | 2026 |
| Jurisdiction |  | Time Analyzed |  |
| Project Description | CR 1138 (Robey Road) | Units | U.S. Customary |

## Segment 1

## Vehicle Inputs

| Segment Type | Passing Zone | Length, ft | 1056 |
| :--- | :--- | :--- | :--- |
| Lane Width, ft | 10 | Shoulder Width, ft | 0 |
| Speed Limit, mi/h | 35 | Access Point Density, pts/mi | 10.0 |

## Demand and Capacity

| Directional Demand Flow Rate, veh/h | 5 | Opposing Demand Flow Rate, veh/h | 5 |
| :--- | :--- | :--- | :--- |
| Peak Hour Factor | 0.94 | Total Trucks, \% | 0.00 |
| Segment Capacity, veh/h | 1700 | Demand/Capacity (D/C) | 0.00 |

## Intermediate Results

| Segment Vertical Class | 1 | Free-Flow Speed, mi/h | 32.0 |
| :--- | :--- | :--- | :--- |
| Speed Slope Coefficient $(\mathrm{m})$ | 1.86556 | Speed Power Coefficient $(\mathrm{p})$ | 0.65020 |
| PF Slope Coefficient $(\mathrm{m})$ | -1.17540 | PF Power Coefficient $(\mathrm{p})$ | 0.70892 |
| In Passing Lane Effective Length? | No | Total Segment Density, veh $/ \mathrm{mi} / \mathrm{ln}$ | 0.0 |
| \%Improvement to Percent Followers | 0.0 | \%lmprovement to Speed | 0.0 |

## Subsegment Data

| $\#$ | Segment Type | Length, ft | Radius, ft | Superelevation, \% | Average Speed, mi/h |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | Tangent | 1056 | - | - | 32.0 |

## Vehicle Results

| Average Speed, mi/h | 32.0 | Percent Followers, \% | 2.8 |
| :--- | :--- | :--- | :--- |
| Segment Travel Time, minutes | 0.38 | Follower Density (FD), followers $/ \mathrm{mi} / \mathrm{ln}$ | 0.0 |
| Vehicle LOS | A |  |  |

## Facility Results

| T | VMT <br> veh-mi/AP | VHD <br> veh-h/p | Follower Density, followers/ <br> $\mathbf{m i / l n}$ | LOS |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 0.00 | 0.0 | A |

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