# COMMONWEALTH OF KENTUCKY BEFORE THE PUBLIC SERVICE COMMISSION

In the Matter of:

THE APPLICATION OF	)	
CELLCO PARTNERSHIP D/B/A VERIZON WIRELESS	)	
AND HARMONI TOWERS, LLC FOR ISSUANCE	)	
OF A CERTIFICATE OF PUBLIC	) CASI	E NO. 2023-00133
CONVENIENCE AND NECESSITY TO CONSTRUCT	)	
A WIRELESS COMMUNICATIONS FACILITY	)	
IN THE COMMONWEALTH OF KENTUCKY	)	
IN THE COUNTY OF GRAVES	)	

SITE NAME: FANCY FARM

\* \* \* \* \* \* \*

# APPLICATION FOR CERTIFICATE OF PUBLIC CONVENIENCE AND NECESSITY FOR CONSTRUCTION OF A WIRELESS COMMUNICATIONS FACILITY

Cellco Partnership, d/b/a Verizon Wireless and VB BTS II, LLC d/b/a Vertical Bridge, LLC ("Co-Applicants"), by counsel, pursuant to (i) KRS §§278.020, 278.040, 278.650, 278.665, and other statutory authority, and the rules and regulations applicable thereto, and (ii) the Telecommunications Act of 1996, respectfully submits this Application requesting issuance of a Certificate of Public Convenience and Necessity ("CPCN") from the Kentucky Public Service Commission ("PSC") to construct, maintain, and operate a Wireless Communications Facility ("WCF") to serve the customers of the Co-Applicants with wireless communications services.

In support of this Application, Co-Applicants respectfully provide and state the following information:

- 1. The complete name and address of the Co-Applicants:
  - a. Cellco Partnership, d/b/a Verizon Wireless, having a local address of 2902 Ring Road, Elizabethtown, KY 42701.

b. VB BTS II, LLC d/b/a Vertical Bridge, LLC, having an address of 750 Park of Commerce Dr, Boca Raton, FL 33487.

# 2. Co-Applicants;

- a. Cellco Partnership, d/b/a Verizon Wireless is a Delaware general partnership, and a copy of the Statement of Good Standing from Delaware and Amended Certificate of Assumed Name is on file with the Secretary of State of Commonwealth of Kentucky is included as part of Exhibits A 1-2.
- b. VB BTS II, LLC d/b/a Vertical Bridge, LLC is a Delaware Limited Liability Company organized in the State of Delaware on December 2, 2015. We attest that VB BTS II, LLC d/b/a Vertical Bridge, LLC is in good standing with the State of Delaware and is also authorized to transact business in the Commonwealth of Kentucky. A copy of the Delaware Certificate of Formation and Certificate of Good Standing is included as part of **Exhibits A 3-4**. The Certificate of Authority is on file with the Secretary of State of Commonwealth of Kentucky and is included as part of **Exhibits A 5**.
- 3. Co-Applicants propose construction of an antenna tower for communications services, which is to be located in an area outside the jurisdiction of a planning commission, and Co-Applicants submit this application to the PSC for a certificate of public convenience and necessity pursuant to KRS §§ 278.020(1), 278.040, 278.650, 278.665, and other statutory authority.
- 4. The Co-Applicant, Cellco Partnership, d/b/a Verizon Wireless operates on frequencies licensed by the Federal Communications Commission ("FCC") pursuant to applicable FCC requirements. A copy of the Co-Applicant's FCC Application and Licenses

with Authorization to provide wireless services are attached to this Application as part of **Exhibit B**, and the facility will be constructed and operated in accordance with applicable FCC regulations.

- 5. The public convenience and necessity require the construction of the proposed WCF. The construction of the WCF will bring or improve the Co-Applicants' services to an area currently not served or not adequately served by the Co-Applicants by increasing coverage or capacity and thereby enhancing the public's access to innovative and competitive wireless communications services. A statement from Co-Applicant, Cellco Partnership, d/b/a Verizon Wireless's RF Design Engineer outlining said need is attached as **Exhibit R** along with Propagation Maps attached as **Exhibit Ra**. The WCF is an integral link in the Co-Applicant's network design that must be in place to provide adequate coverage to the service area.
- 6. To address the above-described service needs, Co-Applicants propose to construct a WCF on Kentucky Highway 80, Fancy Farm, KY 42039 (North Latitude: (36° 48' 09.61", West Longitude 88° 47' 55.21"), on a parcel of land located entirely within the county referenced in the caption of this application. The property on which the WCF will be located is owned by KM & K Farms, LLC pursuant to a Deed recorded at Deed Book 189, Page 85 in the office of the County Clerk. The proposed WCF will consist of a 290-foottall tower, with an approximately 5-foot-tall lightning arrestor attached at the top, for a total height of 295-feet. The WCF will also include concrete foundations and a shelter or cabinets to accommodate the placement of the Co-Applicant's radio electronics equipment and appurtenant equipment. The Co-Applicant's equipment cabinet or shelter will be approved for use in the Commonwealth of Kentucky by the relevant building inspector.

The WCF compound will be fenced, and all access gate(s) will be secured. A description of the manner in which the proposed WCF will be constructed is attached as **Exhibit C** and **Exhibit D**.

- 7. A list of utilities, corporations, or persons with whom the proposed WCF is likely to compete along with a map showing the proposed location as well as the identified like facilities is attached as **Exhibit E**.
- 8. The site development plan and a vertical profile sketch of the WCF signed and sealed by a professional engineer registered in Kentucky depicting the tower height, as well as a proposed configuration for the antennas of the Co-Applicant, Cellco Partnership, d/b/a Verizon Wireless has also been included as part of **Exhibit C**.
- 9. Foundation design plans signed and sealed by a professional engineer registered in Kentucky and a description of the standards according to which the tower was designed are included as part of **Exhibit D**.
- 10. Co-Applicants have considered the likely effects of the installation of the proposed WCF on nearby land uses and values and has concluded that there is no more suitable location reasonably available from which adequate services can be provided, and that there are no reasonably available opportunities to co-locate Co-Applicant's antennas on an existing structure. When suitable towers or structures exist, Co-Applicant's attempts to co-locate on existing structures such as communications towers or other structures capable of supporting Co-Applicant's facilities; however, no other suitable or available co-location site was found to be located in the vicinity of the site. A statement from Co-Applicant, Cellco Partnership, d/b/a Verizon Wireless's RF Design Engineer outlining exploration of co-location opportunities is attached as **Exhibit R**.

- 11. A copy of the Application for Federal Aviation Administration's ("FAA") and the FAA Determination of No Hazard to Air Navigation is attached as **Exhibit F**.
- 12. A copy of Application to the Kentucky Airport Zoning Commission ("KAZC") is attached as **Exhibit G**. The approval from KAZC will be submitted when received.
- 13. A geotechnical engineering report was performed by Power of Design Group, LLC, Louisville, KY, dated March 23, 2022, and is attached as **Exhibit H**. The name and address of the geotechnical engineering firm and the professional engineer registered in Kentucky who prepared the report are included as part of **Exhibit S**.
- 14. Clear directions to the proposed WCF site from the County seat are attached as **Exhibit I**. The name and telephone number of the preparer of **Exhibit I** are included as part of this exhibit.
- 15. Co-Applicants, pursuant to a written agreement, have acquired the right to use the WCF site and associated property rights. A copy of the agreement is attached as **Exhibit J**.
- 16. Personnel directly responsible for the design and construction of the proposed WCF are well qualified and experienced. The tower and foundation drawings for the proposed tower submitted as part of **Exhibit D** bear the signature and stamp of a professional engineer registered in the Commonwealth of Kentucky. All tower designs meet or exceed the minimum requirements of applicable laws and regulations. The identity and qualifications of each person directly responsible for design and construction of the proposed tower are contained in **Exhibit S**.

- 17. The Construction Manager for the proposed facility is Vince Caprino and the identity and qualifications of each person directly responsible for design and construction of the proposed tower are contained in **Exhibit S**.
- 18. As noted on the Survey attached as part of **Exhibit C**, the surveyor has determined that the tower site and access easement are not within any flood hazard area per Flood Hazard Boundary Map, Community Panel Number 21083C0125C, Dated December 3, 2009.
- 19. **Exhibit K** includes a map drawn to an appropriate scale that shows the location of the proposed tower and identifies every owner of real estate within 500 feet of the proposed tower (according to the records maintained by the County Property Valuation Administrator). This map and the associated Notice List is accompanied by a certificate signed and stamped by the registered surveyor that said information is from the PVA records, dated April 20, 2023. In addition, our office verified and updated the notification list with the Graves County **PVA** June 6. 2023 https://www.qpublic.net/ky/graves/index.html . Exhibit K also identifies every structure and every easement within 500 feet of the proposed tower or within 200 feet of the access road including intersection with the public street system.
- 20. Co-Applicants have sent certified notices to every person who, according to the records of the County Property Valuation Administrator, owns property which is within 500 feet of the proposed tower or contiguous to the site property, by certified mail, return receipt requested, of the proposed construction. Each notified property owner has been provided with a map of the location of the proposed construction, the PSC docket number for this application, the address of the PSC, and informed of his or her right to request

intervention. A list of the notified property owners and a copy of the form of the notice sent by certified mail to each landowner are attached as **Exhibit L** and **Exhibit M**, respectively. Thirteen (13) notices were sent to surrounding property owners on April 21, 2023; to date ten (10) notice green cards have been returned. After additional research, seventeen (17) notices were sent to the additional surrounding property owners on June 6, 2023, and Three (3) notices were resent to property owners on the original list that have not been returned, also on June 6, 2023. Copies of the mailed envelopes, returned green cards, are included in **Exhibit M**.

- 21. Co-Applicants have notified the applicable County Judge/Executive by certified mail, return receipt requested, of the proposed construction. This notice included the PSC docket number under which the application will be processed and informed the County Judge/Executive of his/her right to request intervention. A copy of this notice is attached as **Exhibit N**.
- 22. Notice signs meeting the requirements prescribed by 807 KAR 5:063, Section 1(2) that measure at least 2 feet in height and 4 feet in width and that contain all required language in letters of required height, have been posted, one in a visible location on the proposed site and one on the nearest public road. Such signs shall remain posted for at least two weeks after filing of the Application, and a copy of the posted text is attached as **Exhibit O**.
- 23. A legal notice advertisement regarding the location of the proposed facility has been published in a newspaper of general circulation in the county in which the WCF is proposed to be located. A copy of the newspaper legal notice advertisement is attached as **Exhibit P**.

- 24. The area of the proposed facility is in the unincorporated area of Graves County, Kentucky, part of the unincorporated town of Fancy Farm. The 0.5-mile search ring consists of the Highway 80 corridor, largely agricultural with a mix of residential and commercial / industrial properties. The terrain in this area is relatively moderate, rolling topography. There is no zoning or Plan Commission in Graves County. The general area where the proposed facility is to be located is a tilled field adjacent to a water tower. The nearest residential structure is 290 feet from the proposed tower site.
- 25. The process that was used by the Co-Applicant, Cellco Partnership, d/b/a Verizon Wireless radio frequency engineers in selecting the site for the proposed WCF was consistent with the general process used for selecting all other existing and proposed WCF facilities within the proposed network design area. Co-Applicant's radio frequency engineers have conducted studies and tests to develop a highly efficient network that is designed to handle voice and data traffic in the service area. The engineers determined an optimum area for the placement of the proposed facility in terms of elevation and location to provide the best quality service to customers in the service area. A radio frequency design search area prepared in reference to these radio frequency studies was considered by the Co-Applicant when searching for sites for its antennas that would provide the coverage deemed necessary by the Co-Applicant. A map of the area in which the tower is proposed to be located which is drawn to scale and clearly depicts the necessary search area within which the site should be located pursuant to radio frequency requirements is attached as Exhibit Q.
- 26. The tower must be located at the proposed location and proposed height to provide necessary service to wireless communications users in the subject area, as set out and

documented in the RF Design Engineers' Statement of Need and Propagation Maps

attached as Exhibit R and Ra. The proposed tower will expand and improve voice and

data service for Verizon Wireless customers.

27. Attached hereto as Exhibit T please find an Affidavit of Certification for all

information contained in this application.

28. All Exhibits to this Application are hereby incorporated by reference as if fully set out

as part of the Application.

29. All responses and requests associated with this Application may be directed to:

Russell L. Brown

Clark, Quinn, Moses, Scott & Grahn, LLP

320 North Meridian Street, Suite 1100

Indianapolis, IN 46204

Phone: (317) 637-1321

FAX: (317) 687-2344

Email: rbrown@clarkquinnlaw.com

Attorney for Cellco Partnership d/b/a Verizon Wireless

WHEREFORE, Co-Applicants respectfully request that the PSC accept the foregoing

Application for filing and having met the requirements of KRS §§278.020(1), 278.650, and 278

.665 and all applicable rules and regulations of the PSC, grant a Certificate of Public Convenience

and Necessity to construct and operate the WCF at the location set forth herein.

Respectfully submitted,

Russell L. Brown

Clark, Quinn, Moses, Scott & Grahn, LLP

320 North Meridian Street, Suite 1100

Indianapolis, IN 46204

Phone: (317) 637-1321 / FAX: (317) 687-2344

Email: rbrown@clarkquinnlaw.com

Attorney for Cellco Partnership d/b/a Verizon Wireless

# LIST OF EXHIBITS

A	Co-Applicant Entities
В	FCC Application and License Documentation
С	Site Development Plan: 500' Vicinity Map Legal Descriptions Flood Plain Certification Site Plan Vertical Tower Profile
D	Tower and Foundation Design
Е	Competing Utilities, Corporations, or Persons List And Map of Like Facilities in Vicinity
F	FAA Application and Determination of no Hazard
G	KAZC Application
Н	Geotechnical Report
I	Directions to WCF Site
J	Copy of Real Estate Agreement
K	500' Radius and Abutters Map with Surveyor Certification
L	Notification Listing
M	Copy of Property Owner Notification
N	Copy of County Judge Executive notice
O	Copy of Posted Notices
P	Copy of Newspaper Legal Notice Advertisement
Q	Copy of Radio Frequency Design Search Area
R	Copy of RF Design Engineer Statement of Need
Ra	Propagation Maps
S	List of Qualified Professionals
T	Affidavit of Certification



Page 1

I, JEFFREY W. BULLOCK, SECRETARY OF STATE OF THE STATE OF

DELAWARE, DO HEREBY CERTIFY "CELLCO PARTNERSHIP" IS DULY FORMED

UNDER THE LAWS OF THE STATE OF DELAWARE AND IS IN GOOD STANDING AND

HAS A LEGAL EXISTENCE SO FAR AS THE RECORDS OF THIS OFFICE SHOW, AS

OF THE TWENTY-SEVENTH DAY OF APRIL, A.D. 2023.

AND I DO HEREBY FURTHER CERTIFY THAT THE ANNUAL TAXES HAVE BEEN PAID TO DATE.

ELARY'S OFFICE OF THE PROPERTY OF THE PROPERTY

Authentication: 203227418

Date: 04-27-23



# Michael G. Adams Secretary of State

## **Certificate**

I, Michael G. Adams, Secretary of State for the Commonwealth of Kentucky, do hereby certify that the foregoing writing has been carefully compared by me with the original thereof, now in my official custody as Secretary of State and remaining on file in my office, and found to be a true and correct copy of

CERTIFICATE OF ASSUMED NAME OF VERIZON WIRELESS ADOPTED BY GENERAL PARTNERS OF CELLCO PARTNERSHIP FILED JUNE 21, 2006.

IN WITNESS WHEREOF, I have hereunto set my hand and affixed my Official Seal at Frankfort, Kentucky, this 10th day of May, 2023.

CONTRACTOR OF SHARE O

Michael G. Adams Secretary of State

Commonwealth of Kentucky

kdcoleman/0641227 - Certificate ID: 290787

# COMMONWEALTH OF KENTUCKY TREY GRAYSON SECRETARY OF STATE



0641227.07

cornish

Trey Grayson
Secretary of State
Received and Filed
06/21/2006 12:06:09 PM
Fee Receipt: \$20.00

## **CERTIFICATE OF ASSUMED NAME**

This certifies that the assumed name of	•		
Verizon Wireless			
Name under which the but	shoos will be conducted)		
has been adopted by See Addendum			
Red name - KR	13 365,015(1)		
which is the "real name" of YOU MUST CHECK ONE  a Domestic General Partnership	a Foreign General Partr	•	
a Domestic Registered Limited Liability Partnership	a Foreign Registered Li	mited Liabili	ty Partnership
a Domestic Limited Partnership	a Foreign Limited Partn	ership	
a Domestic Business Trust	a Foreign Business Tru	st	
a Domestic Corporation	a Foreign Corporation		
a Domestic Limited Liability Company	a Foreign Limited Liabi	lity Compan	у
a Joint Venture			
organized and existing in the state or country of		, and v	whose address is
One Verizon Way	Basking Ridge	NJ	07920
Street address, II ony	City	Stale	Zlp Code
The certificate of assumed name is executed by : NYNEX PCS Inc.			
- and Ochapher			
Jane A. Schapker-Assistant Secretary	Opedari	•	
Path or lagar reason and Tale	Print or type so ree and the		<del></del>
June 15, 2006	Deb	<del></del>	<del></del>

# Addendum

The full name of the Partnership is Cellco Partnership; a Delaware general partnership with its headquarters located One Verizon Way, Basking Ridge NJ 07920-1097.

·	
General Partners of Cellco Partnership	Address
Bell Atlantic Cellular Holdings, L.P.	One Verizon Way Basking Ridge, NJ 07920
NYNEX PCS Inc.	One Verizon Way Basking Ridge, NJ 07920
PCSCO Partnership	One Verizon Way Basking Ridge, NJ 07920
GTE Wireless Incorporated	One Verizon Way Basking Ridge, NJ 07920
GTE Wireless of Ohio Incorporated	One Verizon Way Basking Ridge, NJ 07920
PCS Nucleus, L.P.	2999 Oak Road, 7th Floor Walnut Creek, CA 94597
JV PartnerCo, LLC	2999 Oak Road, 7th Floor Walnut Creek, CA 94597

Page 1



I, JEFFREY W. BULLOCK, SECRETARY OF STATE OF THE STATE OF

DELAWARE, DO HEREBY CERTIFY THE ATTACHED IS A TRUE AND CORRECT

COPY OF THE CERTIFICATE OF FORMATION OF "VB BTS II, LLC", FILED

IN THIS OFFICE ON THE EIGHTH DAY OF JUNE, A.D. 2022, AT 1:01

O`CLOCK P.M.



Authentication: 203631822

Date: 06-08-22

6844426 8100 SR# 20222658754

# STATE OF DELAWARE CERTIFICATE OF FORMATION OF LIMITED LIABILITY COMPANY

The undersigned authorized person, desiring to form a limited liability company pursuant to the Limited Liability Company Act of the State of Delaware, hereby certifies as follows:

1.	The name of the lim	ited liability compa	my is		
		VB BTS II	I, LLC		
2.	The Registered Offi	ce of the limited lia 850 New Burton F		n the State of I	Delaware is (street),
	City of	Dover	the state of the s	19904	The
liabilit	ty company may be se	erved is COGENCY GLO	OBAL INC.		
		Ву:	/s/ D:	aniel Marinberg	
D	State of Delaware Secretary of State Division of Corporations belivered 01:01 PM 06/08/2022	2)		rized Person	
	FILED 01:01 PM 06/08/2022	Name:_	Dan	iel Marinberg	
SR 2022	22658754 - File Number 684442	5	Print o	or Type	



I, JEFFREY W. BULLOCK, SECRETARY OF STATE OF THE STATE OF

DELAWARE, DO HEREBY CERTIFY "VB BTS II, LLC" IS DULY FORMED UNDER

THE LAWS OF THE STATE OF DELAWARE AND IS IN GOOD STANDING AND HAS A

LEGAL EXISTENCE SO FAR AS THE RECORDS OF THIS OFFICE SHOW, AS OF

THE TWENTY-THIRD DAY OF JANUARY, A.D. 2023.

AND I DO HEREBY FURTHER CERTIFY THAT THE SAID "VB BTS II, LLC"
WAS FORMED ON THE EIGHTH DAY OF JUNE, A.D. 2022.

AND I DO HEREBY FURTHER CERTIFY THAT THE ANNUAL TAXES HAVE BEEN ASSESSED TO DATE.



Authentication: 202551773

Date: 01-23-23



## 202303080004

FAYETTE CO, KY FEE \$46.00 PRESENTED / LODGED: 03-08-2023 08:19:15 AM

RECORDED: 03-08-2023

SUSAN LAMB CLERK BY: HALLIE WOOSLEY DEPUTY CLERK

**BK: IB 428** PG: 690-690



# COMMONWEALTH OF KENTUCKY MICHAEL G. ADAMS, SECRETARY OF STATE

Michael G. Adams Kentucky Secretary of State Received and Filed: 3/7/2023 12:33 PM Fee Receipt: \$90.00

mmoore ADD

1265644.06

Division of Business Filings P.O. Box 718 Frankfort, KY 40802 (502) 564-3490 www.sos.ky.gov	Certificate (Foreign Bus	of Authority iness Entity)		FBE
Pursuant to the provisions of KRS 14/ and, for that purpose, submits the follo	A - 030 the undersigned hereby appli wing statements:	es for authority to transact bush	ness in Kentucky on	behalf of the entity named below
The entity is a: profit corpubusiness to limited party non-profit is:	ration monprofit X limited Sa itd cooper	corporation bility company ative association		ted liability company
2. The name of the entity is	profession	nai service corporation		
(The	name must be identical to the nam	VB BTS II, LLC	n of State )	·
1. The name of the entity to be used in	Kentucky is (if applicable):	e all tageta with the decisivi	A or smite'l	
	(Ciniu e	rovide if "real name" is unav	allable for use: oth	erwise, teave blank )
. The state or country under whose la i. The date of organization is	is nie ennth is diBsursed is		elaware	
	8/8/2022	and the period of duration is	A blank downth	'
The mailing address of the entity's p	incipal office is	tit is	are elsak, eurapen	s considered perpetual.)
750 Park of Comme	rce Drive, Suite 200	Soca Raton City	FL	33487
The street address of the entity's reg	islared office in Kentucky ic	City	State	Zip Code
828 Lane Allen f	Anad Suite 218	Lexington	КУ	40504
reet Address (No P.O. Box Number		City	State	Zip Code
d the name of the registered agent at	that office is	Cogency Gio	bal inc.	
The names and business addresses	of the entity's representatives (secret	EV. Officers and directors many	seem business or an	
Daniel Marinberg	750 Park of Commerce Dr. Ste 200			nerai pariners):
me	Street or P.O. Box	Gity Soca Raton	State FL	33487
me			arara	Zip Code
mp	Street or P.O. Box	City	State	Zip Code
ma	Street or P.O. Box	City		
			State	Zip Code
i a professional service corporation, a i treasurer are licensed in one or more lement of purposes of the corporation.	i the individual shareholders, not less slates or territories of the United Sta	than one half (1/2) of the direct les or District of Columbia to re	lors, and all of the of nder a professional	fficers other than the secretary service described in the
I certify that, as of the date of filing thi	s application, the above-named entity	validly exists under the laws of	the jurisdiction of its	s formation.
if a limited partnership, it elects to be	a limited liability limited partnership.	Check the box if applicable:	]	
if a limited liability company, check i	oox if manager-managed:			
This application will be effective upon				
sture of Authorized Representative	Adam B.	Ginder-Vice President & Associate Gene	and Counted	03/07/23
		Printed Namo & Titlo		Date
Cogency Glob	pal Inc.	Only to come on the section		
pe/Print Name of Registered Agent	Curs	ent to serve as the registered a	geni on behalf of the	Dusiness entity.
PSACO TO	FOR B	HOOD AS	STANTS	
			4/2/2/4/A	ECRETAY 3/76

ASR Application Search Exhibit B

# **Application A1218864**

**Application Detail** 

File Number A1218864 Constructed Registration 1324370 Dismantled

Number

NEPA EMI No

**Application Information** 

Status Granted Date Received 04/04/2023
Purpose Amendment Entered 04/04/2023

Mode Interactive

**Antenna Structure** 

Structure Type LTOWER - Lattice Tower

**Location** (in NAD83 Coordinates)

Lat/Long 36-48-09.6 N 088-47-54.2 W Address 10710 State Route 80 West -

16207023

City, State Fancy Farm , KY

Zip 42039 County GRAVES

Center of Position of Tower

AM Array in Array

**Heights (meters)** 

Elevation of Site Above Mean Sea Level Overall Height Above Ground (AGL)

131.3 90.0

Overall Height Above Mean Sea Level Overall Height Above Ground w/o Appurtenances

221.3 88.4

**Proposed Marking and/or Lighting** 

FAA Style E

**FAA Notification** 

FAA Study 2022-ASO-27278-OE FAA Issue Date 01/06/2023

**Owner & Contact Information** 

FRN 0003290673 Owner Entity General Partnership

Type

**Owner** 

Cellco Partnership P: (770)797-1070

Attention To: Network Regulatory F:

5055 North Point Pkwy E: Network.Regulatory@verizonwireless.com

Alpharetta, GA 30022

NP2NE Network Engineering

**Contact** 

Attention To: Network Regulatory P: (770)797-1070

5055 North Point Pkwy F:

NP2NE Network Engineering E: Network Regulatory@verizonwireless.com

Alpharetta, GA 30022

**Environmental Compliance** 

Does the applicant request a Waiver of the Is the applicant submitting an Environmental

Commission's rules for environmental notice? Assessment?

No No

Is another Federal Agency taking responsibility for

environmental review?

Yes

responsibility for environmental review

Basis for Certification

Reason for another Federal Agency taking

The FCC issued a Finding of No Significant Impact.

Does the applicant certify to No Significant

Environmental Effect pursuant to Section

Name of Federal Agency Local Notice Date

07/20/2022

National Notice Date

07/28/2022

Certification

Authorized Party Le Scanve, Christophe Title Authorized Representative

Receipt Date 04/04/2023

**Comments** 

**Comments** 

None

**History** 

**Event Date** 

04/04/2023 Amendment Received 04/04/2023 Application Granted

07/13/2022 New Application Received

**Trans Log** 

Date **Description Existing Value Requested Value** 

07/27/2022 Application: Identify the change type as Major or Minor Minor Major

07/27/2022 Environmental Compliance: National Notice Date 07/28/2022 09/19/2022

04/04/2023 Structure: The date the FAA determination was issued 01/06/2023

All Trans Log (10)

**Pleadings** 

**Pleading Type Filer Name** Description **Date Entered** 

None

**Automated Letters** 

Date **Description** 

None

**Attachments** 

**Description Date Entered Type** 

None

CLOSE WINDOW

#### REFERENCE COPY

This is not an official FCC license. It is a record of public information contained in the FCC's licensing database on the date that this reference copy was generated. In cases where FCC rules require the presentation, posting, or display of an FCC license, this document may not be used in place of an official FCC license.



# **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

## RADIO STATION AUTHORIZATION

LICENSEE: KENTUCKY RSA NO. 1 PARTNERSHIP

ATTN: LICENSING MANAGER KENTUCKY RSA NO. 1 PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> KNKQ306	<b>File Number</b> 0009611390
	Service Cellular
Market Numer CMA443	Channel Block B
	t Designator

FCC Registration Number (FRN): 0001836709

Market Name Kentucky 1 - Fulton

	<b>nt Date</b> 31-2021
--	---------------------------

#### **Site Information:**

Location Latitude Longitude Ground Elevation Structure Hgt to Tip Antenna Structure (meters) Registration No.

1 36-20-59.2 N 089-22-12.3 W 98.0

Address: 0.68 MILE SOUTH OF LASSITER CORNER & REEL FOOT LAKE

City: LASSITER CORNER County: LAKE State: TN Construction Deadline:

Antenna: 1

**Maximum Transmitting ERP in Watts: 135.800** 

Azimuth(from true north) 90 135 180 225 270 315 Antenna Height AAT (meters) 148.000 117.000 147.000 121.000 149.000 107.000 117.000 146.000 Transmitting ERP (watts) 133.300 103.500 36.500 4.500 1.500 3.900 38.800 109.600

### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

Location Latitude  2 36-45-58.0 N  Address: 416 Jimtown Road	<b>Longitude</b> 088-38-50.0 W		ound Eleva eters) 3.0	(	Structure Hgt (meters) 147.8	to Tip	Antenna St Registration 1043917	
	GRAVES State	: KY Co	nstruction	Deadli	ine:			
Antenna: 2  Maximum Transmitting ERP in Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	Watts: 140.820 0 124.300 91.200	<b>45</b> 120.000 87.100	<b>90</b> 100.800 85.110	135 92.100 85.110	<b>180</b> 88.300 89.130	<b>225</b> 103.100 87.100	<b>270</b> 108.600 89.130	<b>315</b> 100.800 89.130
Location Latitude	Longitude		ound Eleva		Structure Hgt	to Tip	Antenna St	
4 36-54-35.5 N Address: (Wickliffe) 353 CR	089-04-01.6 W 1307	(mo	eters) ).3		( <b>meters</b> ) 121.0		Registration 1030662	n No.
City: Bardwell County: CA	RLISLE State: 1	KY Con	struction I	Deadlin	e:			
Antenna: 4 Maximum Transmitting ERP in Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts) Antenna: 5 Maximum Transmitting ERP in Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	0 107.500 189.230 1 Watts: 140.820 0 107.500	45 98.100 48.640 45 98.100	90 119.800 1.690 90 119.800	135 96.700 0.930 135 96.700		225 133.300 0.930 225 133.300	270 130.900 1.810 270 130.900	315 130.400 52.120 315 130.400
Antenna: 6 Maximum Transmitting ERP in Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	0	<b>45</b> 98.100 0.350	368.980 90 119.800 1.230	174.580 135 96.700 35.330	180 86.900 112.440	0.930 <b>225</b> 133.300 35.270	0.930 <b>270</b> 130.900 1.000	0.930 <b>315</b> 130.400 0.350
<b>Location Latitude</b>	Longitude		ound Eleva eters)		Structure Hgt (meters)	to Tip	Antenna St Registration	
6 36-31-12.4 N	088-50-41.5 W	144	1.2		122.2		1030665	
Address: (Fulton) 550 Powell		7 Comet	motio D-	.ali				
City: Fulton County: HICK	MAN State: KY	Constr	ruction Dea	adiine:				
Antenna: 4 Maximum Transmitting ERP in Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts) Antenna: 5 Maximum Transmitting ERP in Azimuth(from true north)	0 128.200 110.570 1 Watts: 140.820 0	<b>45</b> 122.800 412.100	90 123.200 98.560	135 135.200 4.220	180 147.500 1.510	225 157,200 0.920	270 143.900 0.920	315 141.700 6.530
Antenna Height AAT (meters) Transmitting ERP (watts)	128.200 0.550	122.800 0.550	123.200 0.550	135.200 0.550		157.200 16.430	143.900 11.480	141.700 0.700

Location Latitude  6 36-31-12.4 N  Address: (Fulton) 550 Powel  City: Fulton County: HIC		( <b>n</b>	round Elev neters) 44.2 truction De		Structure Hgt (meters) 122.2	to Tip	Antenna St Registratio 1030665	
Antenna: 6 Maximum Transmitting ERP Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	0	<b>45</b> 122.800 5.650	<b>90</b> 123.200 2.230	135 135.20 0.920	180 00 147.500 1.320	<b>225</b> 157.200 5.450	<b>270</b> 143.900 78.640	<b>315</b> 141.700 402.820
Location Latitude 7 36-38-26.2 N	Longitude 088-16-00.1 W	(n	round Elev neters) 65.8	ation	Structure Hgt (meters) 90.8	to Tip	Antenna St Registratio 1030663	
Address: (Murray) 1431 Var City: Murray County: CA		KV C	onstruction	Doodli	no•			
City. Murray County: CA	LLOWAI States	KI C	onsti action	Deauli	110.			
Antenna: 4 Maximum Transmitting ERP i Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts) Antenna: 5	0	<b>45</b> 107.100 6.420	<b>90</b> 115.000 0.560	135 106.90 0.560	180 00 87.400 0.560	<b>225</b> 91.300 0.830	<b>270</b> 86.200 39.630	<b>315</b> 97.500 251.940
Maximum Transmitting ERP Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts) Antenna: 6	0 106.900 3.450	<b>45</b> 107.100 96.460	<b>90</b> 115.000 263.070	135 106.90 57.230		<b>225</b> 91.300 0.560	<b>270</b> 86.200 0.560	<b>315</b> 97.500 0.560
Azimum Transmitting ERP Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	0	<b>45</b> 107.100 0.370	<b>90</b> 115.000 0.370	135 106.90 12.730		225 91.300 104.340	<b>270</b> 86.200 9.310	<b>315</b> 97.500 0.370
Location Latitude	Longitude		round Elev neters)	ation	Structure Hgt (meters)	to Tip	Antenna St Registratio	
8 37-03-51.4 N	088-57-23.6 W	,	16.4		92.4		1030664	
Address: (La Center) 220 RI								
City: LA CENTER Count	ty: BALLARD St	ate: KY	Construct	ion Dea	adline:			
Antenna: 2 Maximum Transmitting ERP Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	0	<b>45</b> 78.400 71.430	<b>90</b> 71.900 167.460	135 66.000 63.670		<b>225</b> 67.000 0.640	270 87,700 0.330	<b>315</b> 96.100 0.330
Antenna: 3  Maximum Transmitting ERP Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	0	<b>45</b> 78.400 1.000	<b>90</b> 71.900 1.380	135 66.000 23.440		<b>225</b> 67.000 457.090	270 87.700 66.070	<b>315</b> 96.100 2.240

Location Latitude  8 37-03-51.4 N  Address: (La Center) 220 RIG	Longitude  088-57-23.6 W CHARDSON I N	(n	round Eleva neters) 16.4	tion	Structure Hgt (meters) 92.4	to Tip	Antenna St Registratio 1030664	
` '		tate: KY	Constructi	on De	adline:			
Antenna: 4 Maximum Transmitting ERP i Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	n Watts: 140.820 0 85.600 165.960	<b>45</b> 78.400 6.610	<b>90</b> 71.900 0.910	135 66.000 0.500	180 ) 65.300 0.500	<b>225</b> 67.000 0.890	<b>270</b> 87.700 45.710	<b>315</b> 96.100 223.870
Location Latitude  10 36-44-07.9 N  Address: 3975 State Route 2:	<b>Longitude</b> 088-58-29.2 W 206	(n	round Eleva neters) 31.9	tion	Structure Hgt (meters) 92.9	to Tip	Antenna St Registratio 1030723	
		e: KY	Construction	Dead	line:			
Antenna: 2 Maximum Transmitting ERP i Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	n Watts: 140.820 0 100.500 96.610	<b>45</b> 101.900 96.610	90 98.900 96.610	<b>135</b> 84.700 96.610		<b>225</b> 118.900 96.610	<b>270</b> 119.900 96.610	<b>315</b> 100.400 96.610
Location Latitude	Longitude		round Eleva neters)	tion	Structure Hgt (meters)	to Tip	Antenna St Registratio	
11 37-02-00.0 N	088-22-10.0 W	10	05.5		106.7		1040303	
Address: (Calvert City) 641 3 City: Calvert City County:	•	tate: KY	Constructi	ion De	eadline:			
Antenna: 2 Maximum Transmitting ERP i Azimuth(from true north)		45	90	135	180	225	270	315
Antenna Height AAT (meters) Transmitting ERP (watts) Antenna: 3	78.900 23.380	77.600 330.300	88.100 378.360	83.000 36.130	68.600	85.300 0.970	97.900 0.970	93.100 0.970
Maximum Transmitting ERP i Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts) Antenna: 4	<b>0</b> 78.900 0.970	<b>45</b> 77.600 0.970	<b>90</b> 88.100 0.970	135 83.000 14.730		<b>225</b> 85.300 357.480	<b>270</b> 97.900 49.940	<b>315</b> 93.100 1.230
Maximum Transmitting ERP i Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	n Watts: 140.820 0 78.900 63.740	<b>45</b> 77.600 2.060	<b>90</b> 88.100 0.660	135 83.000 0.660	180 68.600 0.660	<b>225</b> 85.300 4.020	<b>270</b> 97.900 107.530	<b>315</b> 93.100 274.970

Location Latitude 12 36-34-49.2 N	<b>Longitude</b> 088-31-45.2 W	(1	Fround Elemeters) 55.5	(1	Structure Hg meters) 91.4	t to Tip	Antenna St Registration 1202399	
Address: 12201 SR 97	CDAVEG GLA V	<b>V</b> C 4	41 D	111				
City: TriCity County:	GRAVES State: K	Y Const	ruction De	adline:				
Antenna: 2 Maximum Transmitting F	ERP in Watts: 140.820							
Azimuth(from true n Antenna Height AAT (me Fransmitting ERP (watts) Antenna: 3	ters) 75.100	<b>45</b> 73.400 4.680	<b>90</b> 74.100 67.610	135 70.100 91.200	<b>180</b> 102.600 13.180	225 100.900 0.450	<b>270</b> 74.700 0.250	<b>315</b> 81.300 0.200
Antenna: 3 Maximum Transmitting E Azimuth(from true n Antenna Height AAT (me Fransmitting ERP (watts) Antenna: 4	orth) <b>0</b> 75.100	45 73.400 0.200	<b>90</b> 74.100 0.200	135 70.100 0.350	<b>180</b> 102.600 18.200	<b>225</b> 100.900 89.130	<b>270</b> 74.700 66.070	<b>315</b> 81.300 2.630
Maximum Transmitting E Azimuth(from true n Antenna Height AAT (me Fransmitting ERP (watts)	orth) <b>0</b> 75.100	45 73.400 38.020	<b>90</b> 74.100 0.200	135 70.100 0.380	<b>180</b> 102.600 0.200	<b>225</b> 100.900 0.200	<b>270</b> 74.700 1.260	<b>315</b> 81.300 42.660
Location Latitude	Longitude		Fround Ele- neters)		Structure Hg meters)	t to Tip	Antenna St Registratio	
				`			O	
51-05- <del>4</del> 1.21 <b>\</b>	088-42-35.2 W	1	04.2	6	53.4		1200593	
Address: (Paducah West	t) 4415 Merredith Rd.				53.4	<b>11</b> 4	U	
Address: (Paducah West	t) 4415 Merredith Rd.	tate: KY			/	)14	U	
Antenna: 4	t) 4415 Merredith Rd. v: MCCRACKEN S				53.4	)14	U	
Address: (Paducah West City: Paducah County  Antenna: 4  Maximum Transmitting F  Azimuth(from true n  Antenna Height AAT (me  Fransmitting ERP (watts)	t) 4415 Merredith Rd. v: MCCRACKEN S  ERP in Watts: 140.820  oorth) 0 ters) 59.900				53.4	225 34.700 0.330	U	<b>315</b> 64.600 1.370
Address: (Paducah West City: Paducah County Antenna: 4 Maximum Transmitting F	t) 4415 Merredith Rd. v: MCCRACKEN S  ERP in Watts: 140.820 torth) 0 ters) 59.900 24.580  ERP in Watts: 140.820 torth) 0 ters) 59.900	45 55.900	90 65.200	135 50.700	33.4 line: 07-08-20	<b>225</b> 34.700	270 42.800	64.600

Call Sign: KNKQ306 **Print Date:** 08-31-2021 **File Number:** 0009611390

**Ground Elevation** Structure Hgt to Tip **Location Latitude** Longitude **Antenna Structure** (meters) (meters) Registration No. 15 36-46-54.2 N 088-03-28.1 W 199.0 126.5 1205551 Address: 14664 Canton Road City: Golden Pond **County: TRIGG** State: KY Construction Deadline: 05-19-2006 Antenna: 2 **Maximum Transmitting ERP in Watts:** 140.820 Azimuth(from true north)
Antenna Height AAT (meters) 90 135 180 225 270 315 45 165.000 178.000 183.900 160.400 174.500 170.600 167.000 177.000 Transmitting ERP (watts) 96.610 96.610 96.610 96.610 96.610 96.610 96.610 96.610 **Ground Elevation** Structure Hgt to Tip **Location Latitude** Longitude **Antenna Structure** (meters) (meters) Registration No. 16 089-10-30.9 W 1282534 36-34-03.0 N 109.4 91.4 Address: (Hickman site) Holley Street City: Hickman **County:** FULTON Construction Deadline: 05-28-2014 State: KY Antenna: 1 **Maximum Transmitting ERP in Watts:** 140.820 Azimuth(from true north) 45 90 135 180 225 270 315 Antenna Height AAT (meters) 105.500 102.800 107.900 96.700 89.300 75.700 68.400 107.300 Transmitting ERP (watts) 141.700 118.910 1.140 0.580 0.580 0.580 0.580 4.050 Antenna: 2 **Maximum Transmitting ERP in Watts:** 140.820 Azimuth(from true north) 90 180 225 270 315 45 135 Antenna Height AAT (meters) 105.500 102.800 96.700 89.300 75.700 68.400 107.900 107.300 Transmitting ERP (watts) 0.580 4.050 141.730 118.910 1.140 0.580 0.580 0.580 Antenna: 3 **Maximum Transmitting ERP in Watts: 140.820** Azimuth(from true north) 90 135 180 225 270 315 45 Antenna Height AAT (meters) 105.500 102.800 96.700 89.300 75.700 68.400 107.900 107.300 Transmitting ERP (watts) 45.610 0.460 0.460 0.460 0.460 0.460 7.710 24.600 **Ground Elevation Structure Hgt to Tip Location Latitude** Longitude **Antenna Structure** (meters) (meters) Registration No. 17 37-10-55.4 N 088-56-43.7 W 102.7 99.1 1252613 Address: (Monkey's Eyebrow) 4625 Odgen Colvin Circle **County:** BALLARD State: KY Construction Deadline: 10-24-2014 Antenna: 1 **Maximum Transmitting ERP in Watts: 140.820** Azimuth(from true north)
Antenna Height AAT (meters) 225 270 45 90 135 180 315 85.900 83.500 74.300 84.600 86.500 83.200 90.600 69.600 **Transmitting ERP (watts)** 7.080 125.890 478.630 112.200 4.570 1.580 1.000 1.000 Antenna: 2 **Maximum Transmitting ERP in Watts: 140.820** Azimuth(from true north)
Antenna Height AAT (meters) 225 270 45 90 135 180 315

83.200

1.000

84.600

64.570

74.300

446.680

86.500

2.820

85.900

1.000

Transmitting ERP (watts)

83.500

1.410

90.600

12.020

69.600

213.800

LocationLatitudeLongitudeGround Elevation (meters)Structure Hgt to Tip (meters)Antenna Structure Registration No.1737-10-55.4 N088-56-43.7 W102.799.11252613

Address: (Monkey's Eyebrow) 4625 Odgen Colvin Circle

City: Kevil County: BALLARD State: KY Construction Deadline: 10-24-2014

Antenna: 4

**Maximum Transmitting ERP in Watts:** 140.820 Azimuth(from true north)
Antenna Height AAT (meters) **0** 85.900 45 90 135 180 225 270 315 69.600 2.000 83.500 90.600 74.300 84.600 86.500 83.200 **Transmitting ERP (watts)** 2.000 2.000 398.110 549.540 2.000 2.000 4.900

**Control Points:** 

Control Pt. No. 3

Address: 500 W. Dove Rd.

City: Southlake County: TARRANT State: TX Telephone Number: (800)264-6620

Waivers/Conditions:

**NONE** 

#### REFERENCE COPY

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# **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

## RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

ATTN: REGULATORY CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> KNLH404	File Number
Radio	Service
CW - PCS	Broadband

FCC Registration Number (FRN): 0003290673

,			
<b>Grant Date</b> 04-24-2017	Effective Date 11-30-2017	Expiration Date 04-28-2027	Print Date
Market Number BTA339		nel Block	Sub-Market Designator
	<b>Market</b> Paducah-Murray		
<b>1st Build-out Date</b> 04-28-2002	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This authorization is subject to the condition that, in the event that systems using the same frequencies as granted herein are authorized in an adjacent foreign territory (Canada/United States), future coordination of any base station transmitters within 72 km (45 miles) of the United States/Canada border shall be required to eliminate any harmful interference to operations in the adjacent foreign territory and to ensure continuance of equal access to the frequencies by both countries.

## **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

This license may not authorize operation throughout the entire geographic area or spectrum identified on the hardcopy version. To view the specific geographic area and spectrum authorized by this license, refer to the Spectrum and Market Area information under the Market Tab of the license record in the Universal Licensing System (ULS). To view the license record, go to the ULS homepage at http://wireless.fcc.gov/uls/index.htm?job=home and select "License Search". Follow the instructions on how to search for license information.

Licensee Name: CELLCO PARTNERSHIP

Call Sign: KNLH404 File Number: Print Date:

700 MHz Relicensed Area Information:

Market Name Buildout Deadline Buildout Notification Status

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# **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

### RADIO STATION AUTHORIZATION

LICENSEE: ALLTEL CORPORATION

ATTN: REGULATORY ALLTEL CORPORATION 5055 NORTH POINT PKWY, NP2NE ENGINEERING ALPHARETTA, GA 30022

Call Sign WQBT313	File Number
Radio	Service
CW - PCS	Broadband

FCC Registration Number (FRN): 0002942159

•			
<b>Grant Date</b> 06-05-2015	Effective Date 05-07-2020	Expiration Date 06-23-2025	Print Date
Market Number MTA026		nel Block A	Sub-Market Designator
	<b>Market</b> Louisville-Lexin		
<b>1st Build-out Date</b> 10-23-2000	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This authorization is subject to the condition that, in the event that systems using the same frequencies as granted herein are authorized in an adjacent foreign territory (Canada/United States), future coordination of any base station transmitters within 72 km (45 miles) of the United States/Canada border shall be required to eliminate any harmful interference to operations in the adjacent foreign territory and to ensure continuance of equal access to the frequencies by both countries.

This authorization is subject to the condition that the remaining balance of the winning bid amount will be paid in accordance with Part 1 of the Commission's rules, 47 C.F.R. Part 1.

## **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

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Licensee Name: ALLTEL CORPORATION

Call Sign: WQBT313 File Number: Print Date:

This license is conditioned upon compliance with the provisions of Applications of AT&T Wireless Services, Inc. and Cingular Wireless Corporation For Consent to Transfer Control of Licenses and Authorizations, Memorandum Opinion and Order, FCC 04-255 (rel. Oct. 26, 2004).

Licensee Name: ALLTEL CORPORATION

Call Sign: WQBT313 File Number: Print Date:

700 MHz Relicensed Area Information:

Market Name Buildout Deadline Buildout Notification Status

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# **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

## RADIO STATION AUTHORIZATION

LICENSEE: ALLTEL CORPORATION

ATTN: REGULATORY ALLTEL CORPORATION 5055 NORTH POINT PKWY, NP2NE ENGINEERING ALPHARETTA, GA 30022

Call Sign WQBT318	File Number
Radio	<b>Service</b>
CW - PCS	Broadband

FCC Registration Number (FRN): 0002942159

•			
<b>Grant Date</b> 06-05-2015	Effective Date 05-07-2020	Expiration Date 06-23-2025	Print Date
Market Number MTA026		nel Block A	Sub-Market Designator
	<b>Market</b> Louisville-Lexin		
<b>1st Build-out Date</b> 06-23-2000	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This authorization is subject to the condition that, in the event that systems using the same frequencies as granted herein are authorized in an adjacent foreign territory (Canada/United States), future coordination of any base station transmitters within 72 km (45 miles) of the United States/Canada border shall be required to eliminate any harmful interference to operations in the adjacent foreign territory and to ensure continuance of equal access to the frequencies by both countries.

This authorization is subject to the condition that the remaining balance of the winning bid amount will be paid in accordance with Part 1 of the Commission's rules, 47 C.F.R. Part 1.

#### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

This license may not authorize operation throughout the entire geographic area or spectrum identified on the hardcopy version. To view the specific geographic area and spectrum authorized by this license, refer to the Spectrum and Market Area information under the Market Tab of the license record in the Universal Licensing System (ULS). To view the license record, go to the ULS homepage at http://wireless.fcc.gov/uls/index.htm?job=home and select "License Search". Follow the instructions on how to search for license information.

Licensee Name: ALLTEL CORPORATION

Call Sign: WQBT318 File Number: Print Date:

This license is conditioned upon compliance with the provisions of Applications of AT&T Wireless Services, Inc. and Cingular Wireless Corporation For Consent to Transfer Control of Licenses and Authorizations, Memorandum Opinion and Order, FCC 04-255 (rel. Oct. 26, 2004).

Licensee Name: ALLTEL CORPORATION

Call Sign: WQBT318 File Number: Print Date:

700 MHz Relicensed Area Information:

Market Name Buildout Deadline Buildout Notification Status

#### REFERENCE COPY

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# **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

## RADIO STATION AUTHORIZATION

LICENSEE: ALLTEL CORPORATION

ATTN: REGULATORY ALLTEL CORPORATION 5055 NORTH POINT PKWY, NP2NE ENGINEERING ALPHARETTA, GA 30022

Call Sign WQBT318	File Number
Radio	<b>Service</b>
CW - PCS	Broadband

FCC Registration Number (FRN): 0002942159

•			
<b>Grant Date</b> 06-05-2015	Effective Date 05-07-2020	Expiration Date 06-23-2025	Print Date
Market Number MTA026		nel Block A	Sub-Market Designator
	<b>Market</b> Louisville-Lexin		
<b>1st Build-out Date</b> 06-23-2000	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This authorization is subject to the condition that, in the event that systems using the same frequencies as granted herein are authorized in an adjacent foreign territory (Canada/United States), future coordination of any base station transmitters within 72 km (45 miles) of the United States/Canada border shall be required to eliminate any harmful interference to operations in the adjacent foreign territory and to ensure continuance of equal access to the frequencies by both countries.

This authorization is subject to the condition that the remaining balance of the winning bid amount will be paid in accordance with Part 1 of the Commission's rules, 47 C.F.R. Part 1.

#### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

This license may not authorize operation throughout the entire geographic area or spectrum identified on the hardcopy version. To view the specific geographic area and spectrum authorized by this license, refer to the Spectrum and Market Area information under the Market Tab of the license record in the Universal Licensing System (ULS). To view the license record, go to the ULS homepage at http://wireless.fcc.gov/uls/index.htm?job=home and select "License Search". Follow the instructions on how to search for license information.

Licensee Name: ALLTEL CORPORATION

Call Sign: WQBT318 File Number: Print Date:

This license is conditioned upon compliance with the provisions of Applications of AT&T Wireless Services, Inc. and Cingular Wireless Corporation For Consent to Transfer Control of Licenses and Authorizations, Memorandum Opinion and Order, FCC 04-255 (rel. Oct. 26, 2004).

Licensee Name: ALLTEL CORPORATION

Call Sign: WQBT318 File Number: Print Date:

700 MHz Relicensed Area Information:

This is not an official FCC license. It is a record of public information contained in the FCC's licensing database on the date that this reference copy was generated. In cases where FCC rules require the presentation, posting, or display of an FCC license, this document may not be used in place of an official FCC license.



## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WQGA718	<b>File Number</b> 0009793647	
Radio Service		
AW - AWS (1710-1755 MHz and		
2110-2155 MHz)		

FCC Registration Number (FRN): 0003290673

<b>Grant Date</b> 02-22-2022	Effective Date 02-22-2022	Expiration Date 11-29-2036	<b>Print Date</b> 02-23-2022
Market Number REA004		nel Block F	Sub-Market Designator 15
	<b>Marke</b> t Mississip		
1st Build-out Date	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This authorization is conditioned upon the licensee, prior to initiating operations from any base or fixed station, making reasonable efforts to coordinate frequency usage with known co-channel and adjacent channel incumbent federal users operating in the 1710-1755 MHz band whose facilities could be affected by the proposed operations. See, e.g., FCC and NTIA Coordination Procedures in the 1710-1755 MHz Band, Public Notice, FCC 06-50, WTB Docket No. 02-353, rel. April 20, 2006.

#### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

**Call Sign:** WQGA718 **File Number:** 0009793647 **Print Date:** 02-23-2022

700 MHz Relicensed Area Information:

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WQGA960	<b>File Number</b> 0009775572	
Radio Service		
AW - AWS (1710-1755 MHz and		
2110-2155 MHz)		

FCC Registration Number (FRN): 0003290673

<b>Grant Date</b> 01-03-2022	Effective Date 01-03-2022	Expiration Date 11-29-2036	<b>Print Date</b> 01-05-2022
<b>Market Number</b> BEA072	Channel Block B Sub-Market Designator 0		
Market Name Paducah, KY-IL			
1st Build-out Date	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This authorization is conditioned upon the licensee, prior to initiating operations from any base or fixed station, making reasonable efforts to coordinate frequency usage with known co-channel and adjacent channel incumbent federal users operating in the 1710-1755 MHz band whose facilities could be affected by the proposed operations. See, e.g., FCC and NTIA Coordination Procedures in the 1710-1755 MHz Band, Public Notice, FCC 06-50, WTB Docket No. 02-353, rel. April 20, 2006.

### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

700 MHz Relicensed Area Information:

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

ATTN: REGULATORY CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WQGD606	<b>File Number</b> 0009565676	
Radio Service		
AW - AWS (1710-1755 MHz and		
2110-2155 MHz)		

FCC Registration Number (FRN): 0003290673

<b>Grant Date</b> 12-16-2021	Effective Date 12-16-2021	Expiration Date 12-18-2036	<b>Print Date</b> 07-09-2022
Market Number BEA072			
Market Name Paducah, KY-IL			
1st Build-out Date	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This authorization is conditioned upon the licensee, prior to initiating operations from any base or fixed station, making reasonable efforts to coordinate frequency usage with known co-channel and adjacent channel incumbent federal users operating in the 1710-1755 MHz band whose facilities could be affected by the proposed operations. See, e.g., FCC and NTIA Coordination Procedures in the 1710-1755 MHz Band, Public Notice, FCC 06-50, WTB Docket No. 02-353, rel. April 20, 2006.

Special Condition for AU/name change (6/4/2016): Grant of the request to update licensee name is conditioned on it not reflecting an assignment or transfer of control (see Rule 1.948); if an assignment or transfer occurred without proper notification or FCC approval, the grant is void and the station is licensed under the prior name.

#### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

700 MHz Relicensed Area Information:

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

ATTN: REGULATORY CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WQJQ692	File Number
<b>Radio</b>	Service
WU - 700 MHz Up	per Band (Block C)

FCC Registration Number (FRN): 0003290673

,			
<b>Grant Date</b> 01-10-2020	Effective Date 02-11-2021	Expiration Date 06-13-2029	Print Date
Market Number REA004	Channel Block C Sub-Market Designator 0		
	<b>Market</b> Mississip		
1st Build-out Date 06-13-2013	<b>2nd Build-out Date</b> 06-13-2019	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

If the facilities authorized herein are used to provide broadcast operations, whether exclusively or in combination with other services, the licensee must seek renewal of the license either within eight years from the commencement of the broadcast service or within the term of the license had the broadcast service not been provided, whichever period is shorter in length. See 47 CFR §27.13(b).

This authorization is conditioned upon compliance with section 27.16 of the Commission's rules

#### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

Call Sign: WQJQ692 File Number: Print Date:

**700 MHz Relicensed Area Information:** 

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

ATTN: REGULATORY CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WREF214	File Number	
Radio Service		
UU - Upper Microwave Flexible Use		
Service		

FCC Registration Number (FRN): 0003290673

<b>Grant Date</b> 10-02-2019	Effective Date 10-02-2019	Expiration Date 10-02-2029	Print Date
Market Number C21083		el Block	Sub-Market Designator
Market Name GRAVES, KY			
1st Build-out Date	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

**NONE** 

### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

Call Sign: WREF214 File Number: Print Date:

700 MHz Relicensed Area Information:

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: STRAIGHT PATH SPECTRUM, LLC

ATTN: REGULATORY STRAIGHT PATH SPECTRUM, LLC 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WRHG984	File Number	
Radio Service		
UU - Upper Microwave Flexible Use		
Service		

FCC Registration Number (FRN): 0012576435

<b>Grant Date</b> 06-04-2020	<b>Effective Date</b> 06-04-2020	Expiration Date 06-04-2030	Print Date
Market Number PEA243 Channel Block M1 Sub-Market Designator 0			
Market Name Paducah, KY			
1st Build-out Date	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

**NONE** 

#### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

Licensee Name: STRAIGHT PATH SPECTRUM, LLC

Call Sign: WRHG984 File Number: Print Date:

700 MHz Relicensed Area Information:

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: STRAIGHT PATH SPECTRUM, LLC

ATTN: REGULATORY STRAIGHT PATH SPECTRUM, LLC 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WRHG994	File Number	
Radio Service		
UU - Upper Microwave Flexible Use		
Service		

FCC Registration Number (FRN): 0012576435

<b>Grant Date</b> 06-04-2020	Effective Date 06-04-2020	Expiration Date 06-04-2030	Print Date
Market Number PEA243			Sub-Market Designator ()
	Market Name Paducah, KY		
1st Build-out Date	2nd Build-out Date	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

**NONE** 

#### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

Licensee Name: STRAIGHT PATH SPECTRUM, LLC

Call Sign: WRHG994 File Number: Print Date:

700 MHz Relicensed Area Information:

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

ATTN: REGULATORY CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WRNG990	File Number
Radio	Service
PM - 3.7 G	Hz Service

FCC Registration Number (FRN): 0003290673

8			
<b>Grant Date</b> 07-23-2021	Effective Date 07-23-2021	Expiration Date 07-23-2036	Print Date
Market Number PEA243		el Block	Sub-Market Designator
Market Name Paducah, KY			
1st Build-out Date         2nd Build-out Date           07-23-2029         07-23-2033		3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This final license provides authorization during the full 15-year license term. Operation under this final license may begin on the earlier of (1) 12/5/2025 or (2) the date that the certification for accelerated relocation for this PEA is validated by the FCC pursuant to 47 CFR § 27.1412(g).

License is conditioned on compliance with all applicable FCC rules and regulations, including licensee making payments required by 47 C.F.R. §§ 27.1401- 27.1424 as described in FCC 20-22. See FCC 20-22, paras. 178-331.

### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

Call Sign: WRNG990 File Number: Print Date:

700 MHz Relicensed Area Information:

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## **Federal Communications Commission**

**Wireless Telecommunications Bureau** 

#### RADIO STATION AUTHORIZATION

LICENSEE: CELLCO PARTNERSHIP

ATTN: REGULATORY CELLCO PARTNERSHIP 5055 NORTH POINT PKWY, NP2NE NETWORK ENGINEERING ALPHARETTA, GA 30022

<b>Call Sign</b> WRNG985	File Number
<b>Radio</b>	Service
PM - 3.7 G	Hz Service

FCC Registration Number (FRN): 0003290673

<b>Grant Date</b> 07-23-2021	Effective Date 07-23-2021	Expiration Date 07-23-2036	Print Date
Market Number PEA243		Channel Block A1	
	<b>Market</b> Paduca		
<b>1st Build-out Date</b> 07-23-2029	<b>2nd Build-out Date</b> 07-23-2033	3rd Build-out Date	4th Build-out Date

#### Waivers/Conditions:

This final license provides authorization during the full 15-year license term. Operation under this final license may begin on the earlier of (1) 12/5/2025 or (2) the date that the certification for accelerated relocation for this PEA is validated by the FCC pursuant to 47 CFR § 27.1412(g).

License is conditioned on compliance with all applicable FCC rules and regulations, including licensee making payments required by 47 C.F.R. §§ 27.1401- 27.1424 as described in FCC 20-22. See FCC 20-22, paras. 178-331.

### **Conditions:**

Pursuant to §309(h) of the Communications Act of 1934, as amended, 47 U.S.C. §309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. § 310(d). This license is subject in terms to the right of use or control conferred by §706 of the Communications Act of 1934, as amended. See 47 U.S.C. §606.

Call Sign: WRNG985 File Number: Print Date:

700 MHz Relicensed Area Information:





# **FANCY FARM**

US-KY-5135

**KENTUCKY HIGHWAY 80** FANCY FARM, KY 42039 **GRAVES COUNTY** 

TENANT: KENTICKY RSA 1 PSHP d/b/a VERIZON "EV FANCY FARM'

ROM GRAVES COUNTY COURT CLERK, 101 E SOUTH ST #2, MAYFIELD, KY 42066: HEAD EAST ON E SOUTH ST TOWARD S 6TH ST (190 FT). TURN LEFT AT THE 1ST CROSS STREET ONTO S 6TH ST (361 FT). TURN LEFT ONTO E BROADWAY (2.4 MI). CONTINUE ONTO KY-80 W (7.8 MI). TURN RIGHT ONTO KY-339 N/KY-80 W (249 FT). TURN LEFT ONTO KY-80 W (0.4 MI). SITE WILL BE LOCATED ON RIGHT (NORTH) SIDE OF ROAD.

FROM EVANSVILLE MTSO: 800 RUSSELL ROAD CHANDLER. IN 47610: HEAD NORTH ON RUSSELL RD TOWARD GARDNER RD (0.2 MI). TURN LEFT ONTO GARDNER RD (1.6 MI). TURN LEFT ONTO IN-62 (4.2 MI). USE THE RIGHT LANE TAKE THE RAMP ONTO 1-69 S (0.4 MI). MERGE ONTO 1-69 S (7.8 MI). TAKE EXIT O FOR VETERANS MEM PKWY/US-41 TOWARD VINCENNES/HENDERSON KY (0.3 MI). KEEP LÉFT AT THE FORK, FOLLOW SIGNS FOR US-41 S AND MERG ONTO US-41 S (0.9 MI). MERGE ONTO US-41 S (6.1 MI). KEEP LEFT TO STAY ON US-41 S (4.3 MI). CONTINUE ONTO 1-69 (SIGNS FOR MADISONVILLE/FULTON) (41.8 MI). KEEP RIGHT TO STAY ON 1-69, FOLLOW SIGNS FOR PADUCAH (38.4 MI). TAKE EXIT 68B TO MERGE ONTO I-24 W/I-69 S TOWARD PADUCAH (16.2 MI). TAKE EXIT 25A ON THE LEFT FOR I-69 S TOWARD FULTON S (0.9 MI). CONTINUE ONTO I-69 (28.4 MI). TAKE EXIT 22 FOR KY-80 TOWARD MAYFIELD/FANCY FARM (0.2 MI). TURN RIGHT ONTO KY-80 W/W BROADWAY/FANCY FARM RD (8.3 MI). TURN RIGHT ONTO KY-339 N/KY-80 W (249 FT). TURN LEFT ONTO KY-80 W (0.4 MI). SITE WILL BE LOCATED ON RIGHT

## NEW 290'-0" SELF SUPPORT TOWER w/5' LIGHTNING ARRESTOR -TOTAL TOWER HEIGHT 295'-0"

## VERTICAL BRIDGE SITE

FANCY FARM SITE #: US-KY-5135

#### /ERIZON SITE

EV FANCY FARM FUZE ID: 16207023 LOCATION CODE: 495686

#### SITE ADDRESS

KENTUCKY HIGHWAY 80 FANCY FARM, KY 42039 GRAVES COUNTY E911 ADDRESS: TBD

#### CLIENT CONTACT

VERIZON 2902 RING ROAD FUZABETHTOWN KY 42701 CONTACT: JACKIE STRAIGHT PHONE: (290) 750-0023 E-MAIL: JACKIE.STRAIGHT@ VERIZONWIRELESS.COM

#### OWER OWNER

VICINITY MAP

VB BTS II, LLC 750 PARK OF COMMERCE DRIVE SUITE 200 BOCA RATON, FL 33487 CONTACT: GRETCHEN BLANTON MOBILE: (704) 472-0374 E-MAIL: GBLANTON@ VERTICALBRIDGE.COM

### PROPERTY OWNER

KM & K FARMS LLC COON RAPIDS, MN 55448 PHONE: (763) 248-2538 E-MAIL: KHAYDEN5191@ COMCAST NET

GRAVES COUNTY SHERIFF 101 E SOUTH ST #3 PHONE: (270) 247-4501

MILBURN VOLUNTEER FIRE DEPT. HIGHWAY 80 MILBURN, KY 42070 PHONE: (270) 694-3207

#### GENERAL INFORMATION

LATITUDE : 36.802670" N LONGITUDE : 88.798391" W 1983 (NAD83) 431'± AMSL FI EVATION . 1988 (NAVD88)

## PROPOSED LEASE AREA

100'-0" x 100'-0" (10,000 SF)

#### PROJECT TOTAL DISTURBED AREA

COMPOUND: (10,000 SF) = (0.23 ACRE (23.840 SF) = (0.55 ACRE

#### NOTE: ALL ITEMS WITHIN THESE CONSTRUCTION DOCUMENTS ARE BY TOWER OWNER'S GENERAL CONTRACTOR AND HIS SUB-CONTRACTORS UNI ESS NOTED AS (VZW GC) WHICH SHALL INCLUDE VERIZON

- INDICAL NUM:
  INSTALL (1) NEW "VERIZON ONLY" FIBER OPTIC CONDUIT WITH PULL TAPE FROM NEW "VERIZON ONLY" 24"
  \$36" HAND HOLE OUTSIDE COMPOUND AND STUB UP AT FUTURE FIBER REDESTAL LOCATION
  PERMANENT ELECTRIC POWER MUST BE AVAILABLE FOR VERIZON AT THE METER BASE PRIOR TO THE SITE
  BEING RELEASED AS TENANT READY.

ERIZON SCOPE (VZW GC):
 INSTALL A NEW 11'-6"x14'-9" PREFABRICATED CANOPY ON EXISTING CONCRETE PAD

- INSTALL NEW 30KW DIESEL GENERATOR ON EXISTING CONCRETE PAD INSTALL VZW ICE BRIDGE AND FOUNDATIONS INSTALL VZW ANTENNA MOUNTING SUPPORT STRUCTURE ON TOWER
- INSTALL VZW ANTENNAS, LINES, COAX, GPS ANTENNA AND RADIO EQUIPMENT INSTALL EXISTING SUBSURFACE GROUND LEADS TO VZW EQUIPMENT & FACILITIES INSTALL VZW ELECTRIC SERVICE COMDUCTORS FROM UTILITY H-FRAME TO VZW ILC ENCLOSURE
- INSTALL VZW GENERATOR CIRCUITS FROM VZW ILC & EQUIPMENT ENCLOSURES TO VZW GENERATOR INSTALL CIRCUITS FROM VZW ILC TO VZW EQUIPMENT ENCLOSURES INSTALL CIRCUITS FROM VZW ILC TO VZW EQUIPMENT ENCLOSURES INSTALL REW OUTDOOR OVP AND CABLING HERAME SUPPORT
- INSTALL (2) 1-1/4" & (1) 1" INNERDUCTS WITH PULL TAPES AND TRACER WIRE WITHIN OWNER INSTALLED "VERIZON ONLY" FIBER OPTIC CONDUITS

#### PROJECT DESCRIPTION



## WEST KENTUCKY RECC ADDRESS: 1218 W BROADWAY

## 2012 INTERNATIONAL ENERGY CODE (COMMERCIAL)

ACCESSIBILITY REQUIREMENTS: FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. HANDICAPPED ACCESS REQUIREMENTS ARE NOT REQUIRED IN ACCORDANCE WITH THE 2009 IBC BUILDING CODE

ALL WORK AND MATERIALS SHALL BE PERFORMED AND INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL

GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUCTED TO PERMIT

2018 KENTUCKY BUILDING CODE

#### APPLICABLE CODES

WORK NOT CONFORMING TO THESE CODES.

BUILDING CODE

STRUCTURAL CODE

MECHANICAL CODE

PLUMBING CODE

ELECTRICAL CODE

NERGY CODE

GAS CODE

FIRE/LIFE SAFFTY CODE

PREPARED BY: POWER OF DESIGN GROUP, LLC - (502) 437-5252

POWER OF DESIGN GROUP, LLC 11490 BLUEGRASS PARKWAY LOUISVILLE, KY 40299 PHONE: (502) 437-5252

11490 BLUEGRASS PARKWAY LOUISVILLE, KY 40299

PHONE: (270) 247-1321

### ARCHITECTURAL

TIA/EIA-222 - REVISION G (INCLUDES ADDENDUM #2)

2012 INTERNATIONAL MECHANICAL CODE (IMC 2012)

KENTUCKY STATE PLUMBING CODE (815 KAR CHAP, 20)

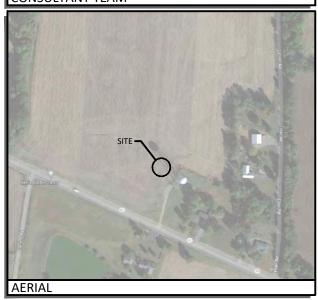
2017 NATIONAL ELECTRICAL CODE (NEC) - NFPA 70

2012 INTERNATIONAL FIRE CODE (2012 IEC)

2009 NATIONAL FUEL GAS CODE (NFPA 54)

POWER OF DESIGN GROUP, LLC

### **CONSULTANT TEAM**



DESCRIPTION PROJECT INFORMATION, SITE MAPS, SHEET INDEX

500' RADIUS & ABUTTERS MAP REVISION LOG

TOWER ELEVATION TE-1 TOWER ELEVATION

#### CIVIL C-1

SHEET NUMBER

B-1 TO B-1.1

B-2 TO B-2.1

OVERALL SITE PLAN W/AERIAL OVERLAY OVERALL SITE PLAN W/DISTANCES TO PROPERTY LINES C-1A TOWER DISTANCE TO RESIDENTIAL STRUCTURES C-1B DETAILED SITE PLAN DIMENSIONED SITE PLAN





## 04/25/2023



## ZONING **DRAWINGS**

REV.	DATE	DESCRIPTION
Α	9.28.22	ISSUED FOR REVIEW
0	11.15.22	ISSUED AS FINAL
1	4.25.23	500'R REVISION

## SITE INFORMATION:

### **FANCY FARM**

KENTUCKY HIGHWAY 80 FANCY FARM, KY 42039 **GRAVES COUNTY** 

VERTICAL BRIDGE SITE NUMBER US-KY-5135

**EV FANCY FARM** POD NUMBER

DRAWN BY

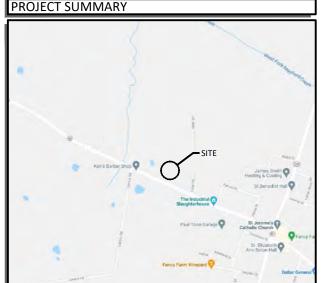
CHECKED BY: 09.28.22

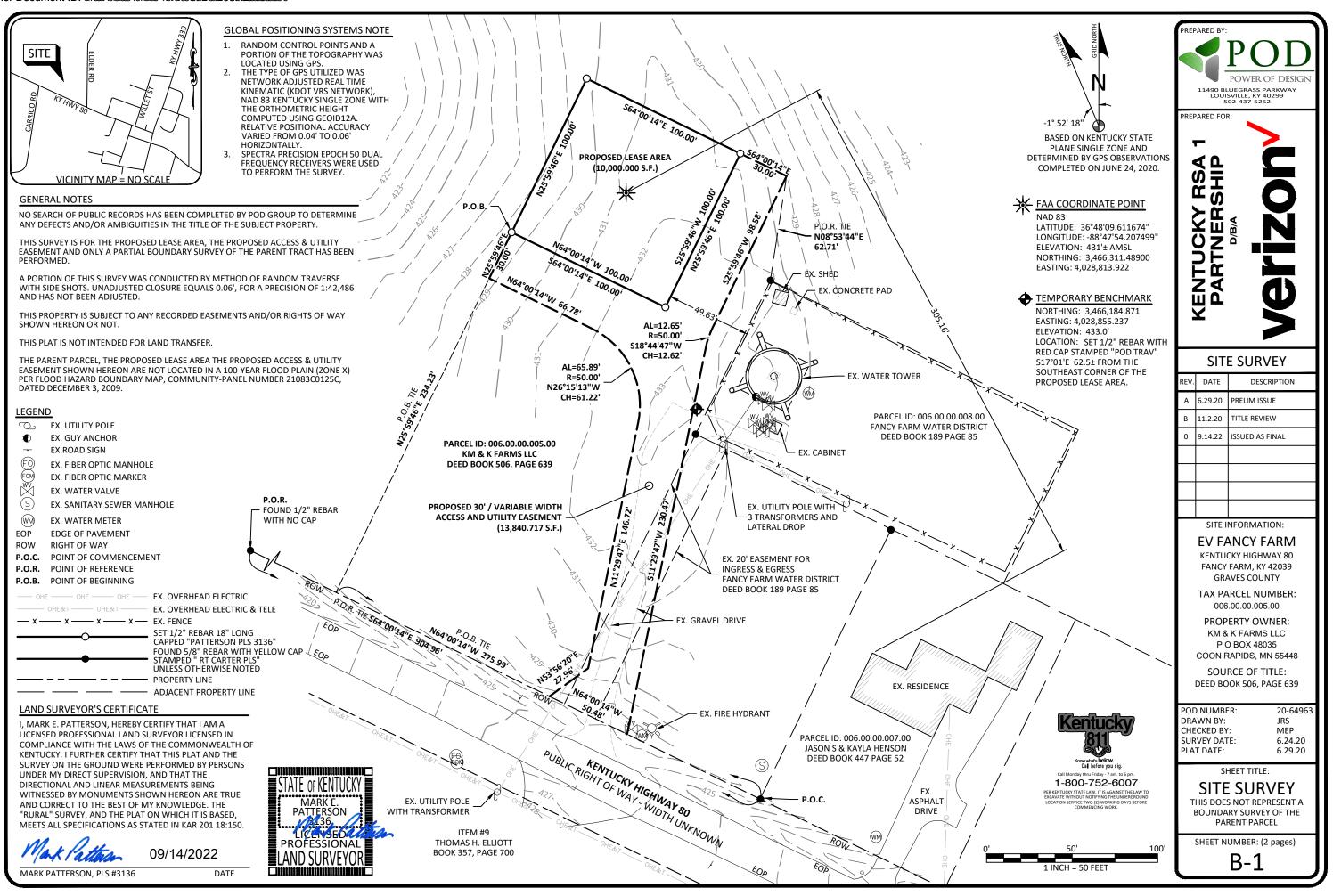
POD

SHEET TITLE:

**PROJECT** INFORMATION, SITE MAPS, SHEET INDEX

SHEET NUMBER:





#### TITLE OF COMMITMENT

THIS SURVEY DOES NOT CONSTITUTE A TITLE SEARCH BY POD GROUP, LLC. AND AS SUCH WE ARE NOT RESPONSIBLE FOR THE INVESTIGATION OR INDEPENDENT SEARCH FOR EASEMENTS OF RECORD, ENCUMBRANCES, RESTRICTIVE COVENANTS, OWNERSHIP TITLE EVIDENCE, UNRECORDED EASEMENTS, AUGMENTING EASEMENTS, IMPLIED OR PRESCRIPTIVE EASEMENTS, OR ANY OTHER FACTS THAT AN ACCURATE AND CURRENT TITLE SEARCH MAY DISCLOSE. INFORMATION REGARDING THESE MATTERS WERE GAINED FROM AMC SETTLEMENT SERVICES, LOAN NO.: VZW / EV FANCY FARM2, ORDER NO.: 50012406, DATED AUGUST 5, 2020 AND PER TITLE UPDATE REPORT ORDER NO. 37123186, DATED APRIL 27, 2022. THE FOLLOWING COMMENTS ARE IN REGARD TO SAID COMMITMENT AND THE NUMBERS IN THE COMMENTS CORRESPOND TO THE NUMBERING SYSTEM IN SAID POLICY.

#### SCHEDULE B SECTION II (EXCEPTIONS)

- 1. DEFECTS, LIENS, ENCUMBRANCES, ADVERSE CLAIMS OR OTHER MATTERS, IF ANY, CREATED, FIRST APPEARING IN THE PUBLIC RECORDS OR ATTACHING SUBSEQUENT TO THE EFFECTIVE DATE BUT PRIOR TO THE DATE THE PROPOSED INSURED ACQUIRES FOR VALUE OF RECORD THE ESTATE OR INTEREST OR MORTGAGE THEREON COVERED BY THIS COMMITMENT. (POWER OF DESIGN GROUP, LLC., DID NOT EXAMINE OR ADDRESS THIS ITEM.)
- 2. TAXES OR SPECIAL ASSESSMENTS WHICH ARE NOT SHOWN AS EXISTING LIENS BY THE PUBLIC RECORDS. (NOT A LAND SURVEYING MATTER, THEREFORE POWER OF DESIGN GROUP, LLC., DID NOT EXAMINE OR ADDRESS THIS ITEM.)
- 3. ANY ENCROACHMENT, ENCUMBRANCE, VIOLATION, VARIATION, OR ADVERSE CIRCUMSTANCE AFFECTING THE TITLE THAT WOULD BE DISCLOSED BY AN ACCURATE AND COMPLETE LAND SURVEY OF THE LAND. (NO ENCROACHMENTS WERE OBSERVED ON THE AREA OF THE PREMISES, OR EASEMENT. POWER OF DESIGN GROUP, LLC DID NOT PERFORM A BOUNDARY SURVEY OF THE PARENT PARCEL, THEREFORE, THIS SHOULD NOT BE CONSTRUED AS NO ENCROACHMENTS EXIST.)
- 4. RIGHTS OR CLAIMS OF PARTIES IN POSSESSION NOT SHOWN BY THE PUBLIC RECORDS. (RIGHTS OR CLAIMS ARE NOT A LAND SURVEYING MATTER, THEREFORE POWER OF DESIGN GROUP, LLC., DID NOT EXAMINE OR ADDRESS THIS ITEM.)
- 5. ANY LIEN OR RIGHT TO A LIEN, FOR SERVICES, LABOR, OR MATERIAL HERETOFORE OR HEREAFTER FURNISHED, IMPOSED BY LAW AND NOT SHOWN BY THE PUBLIC RECORDS. (NOT A LAND SURVEYING MATTER, THEREFORE POWER OF DESIGN GROUP, LLC., DID NOT EXAMINE OR ADDRESS THIS ITEM.)
- 6. EASEMENTS OR CLAIMS OF EASEMENTS, NOT SHOWN BY THE PUBLIC RECORDS. (POWER OF DESIGN GROUP, LLC., DID NOT EXAMINE OR ADDRESS THIS ITEM.)

#### TAXES

7. TAXES, OR SPECIAL ASSESSMENTS, IF ANY, NOT SHOWN AS EXISTING LIENS BY THE PUBLIC RECORDS.

PARCEL ID #: 006.00.00.005.00 COMMENTS: PARENT PARCEL

YEAR: TYPE: PERIOD: TAX AMOUNT: PENALTY: AMOUNT DUE: STATUS: DUE DATE: GOOD THRU DATE: 2019 CITY & COUNTY ANNUAL \$148.96 \$0.00 N/A PAID N/A N/A

PER TITLE UPDATE REPORT ORDER NO. 37123186, DATED APRIL 27, 2022

YEAR: TYPE: PERIOD: TAX AMOUNT: STATUS: ASSESSMENT: 2021 COUNTY ANNUAL \$150.53 PAID \$16,800.00

PARCEL ID #: 006.00.00.005.02

COMMENTS: LEASEHOLD PARCEL

YEAR: TYPE: PERIOD: TAX AMOUNT: PENALTY: AMOUNT DUE: STATUS: DUE DATE: GOOD THRU DATE: 2019 CITY & COUNTY ANNUAL \$168.44 \$0.00 N/A PAID N/A N/A

#### PER TITLE UPDATE REPORT ORDER NO. 37123186, DATED APRIL 27, 2022:

YEAR: TYPE: PERIOD: TAX AMOUNT: STATUS: ASSESSMENT: 2021 COUNTY ANNUAL \$170.24 PAID \$19,000.00

(NOT A LAND SURVEYING MATTER, THEREFORE POWER OF DESIGN GROUP, LLC., DID NOT EXAMINE OR ADDRESS THIS ITEM.)

#### MORTGAGE

NONE OF RECORD.

LIENS / JUDGEMENTS

NONE OF RECORD.

#### OTHER:

- 8. SUBJECT TO EASEMENT IN DEED FROM ALBERT EUGENE GOATLEY AND WIFE, MARY VERNON GOATLEY, TO DANNY TOON AND WIFE, NICOLE TOON DATED 8/26/1994 AND RECORDED 8/30/1994 IN BOOK 350, PAGE 477, GRAVES COUNTY RECORDS. (EASEMENT IN DEED AS RECORDED IN BOOK 350, PAGE 477 DOES NOT AFFECT THE PARENT PARCEL OR THE ACCESS & UTILITY EASEMENT.)
- 9. SUBJECT TO EASEMENT FOR POWER LINES AND WATER METERS IN DEED FROM JAMES M. ELLIOTT, TRUSTEE OF THE JAMES M. ELLIOTT DECLARATION OF TRUST TO THOMAS H. ELLIOTT DATED 10/9/1995 AND RECORDED 10/13/1995 IN BOOK 357, PAGE 700, GRAVES COUNTY RECORDS. (EASEMENT AS RECORDED IN BOOK 357, PAGE 700 DOES NOT AFFECT THE PARENT PARCEL OR THE ACCESS & UTILITY EASEMENT AND IS SHOWN HEREON.)
- 10. SUBJECT TO EASEMENT OF 20 FOOT WIDE STRIP OF LAND FORMERLY USED AS A ROADWAY REFERENCED IN DEED FROM MAIN STREET BANK & TRUST FKA BANKILLINOIS, SUCCESSOR TRUSTEE OF THE JAMES M. ELLIOTT DECLARATION OF TRUST UNDER AGREEMENT DATED 5/6/1994 TO KENT E. HAYDEN AND W. KEITH HAYDEN DATED 10/7/2005 AND RECORDED 11/14/2005 IN BOOK 427, PAGE 770, GRAVES COUNTY RECORDS. (NO COPY OF THE ACTUAL EASEMENT REFERENCED IN BOOK 80, PAGE 329, IS AVAILABLE) (THERE IS NOT A COPY OF OR A PLOTTABLE DESCRIPTION INCLUDED OF THE 20' EASEMENT AS REFERENCED IN BOOK 427, PAGE 770, THEREFORE POD CANNOT DETERMINE THE AFFECT ON THE PARENT PARCEL OR THE ACCESS & UTILITY EASEMENT.)
- 11. SUBJECT TO DEED OF EASEMENT FROM ALGENE GOATLEY AND WIFE, MARY VERNON GOATLEY TO FANCY FARM WATER & SEWER DISTRICT DATED 6/29/1995 AND RECORDED 6/30/1995 IN BOOK 356, PAGE 10, GRAVES COUNTY RECORDS. (EASEMENT AS RECORDED IN BOOK 356, PAGE 10 DOES NOT AFFECT THE PARENT PARCEL OR THE ACCESS & UTILITY EASEMENT.)
- 12. SUBJECT TO ANY MATTERS AS MAY BE SHOWN ON PLAT OF QUAIL HOLLOW SUBDIVISION (PHASE RECORDED 12/12/2016 IN PLAT BOOK F, PAGE 75, GRAVES COUNTY RECORDS. (THIS VOIDS THE EASEMENT SHOWN AS PARCEL 11, TRACT II IN VESTING DEED IN BOOK 506, PAGE 639) (MATTERS AS SHOWN IN PLAT IN BOOK 506, PAGE 639 DOES NOT AFFECT THE PARENT PARCEL OR THE ACCESS & UTILITY EASEMENT.)

PER TITLE UPDATE REPORT ORDER NO. 37123186, DATED APRIL 27, 2022, NO NEW DOCUMENTS OF RECORD WERE FOUND IN THE SEARCH PERIOD OF AUGUST 5, 2020 THROUGH APRIL 20, 2022.

#### **LEGAL DESCRIPTIONS**

#### PROPOSED LEASE AREA

THE FOLLOWING IS A DESCRIPTION OF THE PROPOSED LEASE AREA TO BE LEASED FROM THE PROPERTY CONVEYED TO KM & K FARMS LLC AS RECORDED IN DEED BOOK 506, PAGE 639 OF RECORD IN THE OFFICE OF THE CLERK OF GRAVES COUNTY, KENTUCKY, PARCEL ID: 006.00.005.00, WHICH IS MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEARING DATUM USED HEREIN IS BASED UPON KENTUCKY STATE PLANE COORDINATE SYSTEM, SINGLE ZONE, NAD 83, FROM A REAL TIME KINEMATIC GLOBAL POSITIONING SYSTEM OBSERVATION USING THE KENTUCKY TRANSPORTATION CABINET REAL TIME GPS NETWORK COMPLETED ON JUNE 24, 2020.

COMMENCING AT A FOUND 5/8" REBAR WITH A YELLOW CAP STAMPED "RT CARTER PLS" AT THE SOUTHWEST CORNER OF THE PARCEL CONVEYED TO JASON S & KAYLA HENSON AS DESCRIBED IN DEED BOOK 447, PAGE 52, PARCEL ID: 006.00.00.007.00 AND BEING IN THE NORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 80; FOR REFERENCE, SAID COMMENCEMENT POINT IS 564°00'14"E 904.96' FROM A FOUND 1/2" REBAR WITH NO CAP IN THE SOUTHWEST CORNER OF KM & K FARMS LLC AS RECORDED IN DEED BOOK 506, PAGE 639, PARCEL ID: 006.00.00.005.00 AND BEING IN THE NORTH RIGHT OF WAY LINE OF SAID KENTUCKY HIGHWAY 80, THENCE N64°00'14"W 275.99' TO A POINT ON THE NORTH RIGHT OF WAY LINE OF SAID KENTUCKY HIGHWAY 80 AND ALSO BEING THE SOUTH LINE OF SAID KM & K FARMS LLC; THENCE LEAVING SAID LINE N25°59'46"E 234.23' TO A SET 1/2" REBAR, 18" LONG, CAPPED "PATTERSON PLS 3136", HEREAFTER REFERRED TO AS A "SET IPC", AT THE SOUTHWEST CORNER OF THE PROPOSED LEASE AREA AND BEING THE TRUE POINT OF BEGINNING; THENCE N25°59'46"E 100.00' TO A SET 1PC; THENCE S64°00'14"E 100.00' TO A SET 1PC; THENCE S25°59'46"W 100.00' TO A SET 1PC; THENCE N64°00'14"W 100.00' TO THE POINT OF BEGINNING CONTAINING 10,000.000 SQUARE FEET AS PER SURVEY BY MARK E. PATTERSON, PLS #3136 DATED JUNE 24, 2020.

#### PROPOSED 30' / VARIABLE WIDTH ACCESS & UTILITY EASEMENT

THE FOLLOWING IS A DESCRIPTION OF THE PROPOSED 30' / VARIABLE WIDTH ACCESS AND UTILITY EASEMENT TO BE GRANTED FROM THE PROPERTY CONVEYED TO KM & K FARMS LLC AS RECORDED IN DEED BOOK 506, PAGE 639 OF RECORD IN THE OFFICE OF THE CLERK OF GRAVES COUNTY, KENTUCKY, PARCEL ID: 006.00.00.005.00, WHICH IS MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEARING DATUM USED HEREIN IS BASED UPON KENTUCKY STATE PLANE COORDINATE SYSTEM, SINGLE ZONE, NAD 83, FROM A REAL TIME KINEMATIC GLOBAL POSITIONING SYSTEM OBSERVATION USING THE KENTUCKY TRANSPORTATION CABINET REAL TIME GPS NETWORK COMPLETED ON JUNE 24, 2020.

COMMENCING AT A FOUND 5/8" REBAR WITH A YELLOW CAP STAMPED "RT CARTER PLS" AT THE SOUTHWEST CORNER OF THE PARCEL CONVEYED TO JASON S & KAYLA HENSON AS DESCRIBED IN DEED BOOK 447, PAGE 52, PARCEL ID: 006.00.00.007.00 AND BEING IN THE NORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 80; FOR REFERENCE, SAID COMMENCEMENT POINT IS S64°00'14"E 904.96' FROM A FOUND 1/2" REBAR WITH NO CAP IN THE SOUTHWEST CORNER OF KM & K FARMS LLC AS RECORDED IN DEED BOOK 506, PAGE 639, PARCEL ID: 006.00.00.005.00 AND BEING IN THE NORTH RIGHT OF WAY LINE OF SAID KENTUCKY HIGHWAY 80, THENCE N64°00'14"W 275.99' TO A POINT ON THE NORTH RIGHT OF WAY LINE OF SAID KENTUCKY HIGHWAY 80 AND ALSO BEING THE SOUTH LINE OF SAID KM & K FARMS LLC; THENCE LEAVING SAID LINE N25°59'46"E 234.23' TO A SET 1/2" REBAR, 18" LONG, CAPPED "PATTERSON PLS 3136", HEREAFTER REFERRED TO AS A "SET IPC", AT THE SOUTHWEST CORNER OF THE PROPOSED LEASE AREA AND BEING THE TRUE POINT OF BEGINNING; THENCE S64°00'14"E 100.00' TO A SET IPC; THENCE N25°59'46"E 100.00' TO A SET IPC; THENCE LEAVING SAID PROPOSED LEASE AREA S64°00'14"E 30.00' TO A POINT; FOR REFERENCE, SAID POINT IS N08°45'52"E 61.69' FROM A FOUND 5/8" REBAR WITH A YELLOW CAP STAMPED "RT CARTER PLS" AT THE NORTHWEST CORNER OF THE PARCEL CONVEYED TO FANCY FARM WATER DISTRICT AS RECORDED IN DEED BOOK 189 PAGE 85, PARCEL ID: 006.00.00.008.00; THENCE S25°59'46"W 98.58'; THENCE ALONG THE ARC OF A CURVE TO THE LEFT HAVING A RADIUS OF 50.00', ARC LENGTH OF 12.65', THE CHORD OF WHICH BEARS \$18°44'47"W 12.62'; THENCE \$11°29'47"W 230.47' TO THE NORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 80; THENCE ALONG SAID NORTH RIGHT OF WAY LINE N64\*00'14"W 50.48'; THENCE LEAVING SAID NORTH RIGHT OF WAY LINE N64\*00'14"W 50.48'; THENCE LEAVING SAID NORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 80; THENCE ALONG SAID NORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 80; THENCE ALONG SAID NORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 80; THENCE ALONG SAID NORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 80; THENCE ALONG SAID NORTH RIGHT OF WAY LINE OF KENTU

#### PARENT PARCEL, LEGAL DESCRIPTION, DEED BOOK 506, PAGE 639 (NOT FIELD SURVEYED)

ALL THAT PARCEL OF LAND IN THE IN THE COUNTY OF GRAVES AND COMMONWEALTH OF KENTUCKY AS MORE FULLY DESCRIBED IN DEED AND BOOK 506 PAGE 639 AND PARCEL # 006.00.00.005.00 AND 006.00.00.005.02, BEING KNOWN AND DESIGNATED AS:

DUE TO THE NUMEROUS PRIOR OUTSALES REFERENCED IN THE CURRENT VESTING DEED AND BACK DEEDS, A SURVEY IS REQUIRED IN ORDER TO DETERMINE WHAT THE CURRENT, CORRECT LEGAL DESCRIPTION OF THE SUBJECT PROPERTY IS.

#### PARCEL NUMBER: 006.00.00.005.00 AND 006.00.00.005.02

PATTERSON

LICENSED

PROFESSIONAL

BEING THE SAME PROPERTY ACQUIRED BY KM & K FARMS, LLC, A KENTUCKY LIMITED LIABILITY COMPANY BY DEED OF WILLIAM KEITH HAYDEN A/K/A W. KEITH HAYDEN, SINGLE, AND KENT E. HAYDEN A/K/A KENT HAYDEN AND MONICA HAYDEN, HUSBAND AND WIFE, DATED 12/12/2016 AND RECORDED 12/28/2016 IN BOOK / PAGE: 506 / 639

#### LAND SURVEYOR'S CERTIFICATE

I, MARK E. PATTERSON, HEREBY CERTIFY THAT I AM A LICENSED PROFESSIONAL LAND SURVEYOR LICENSED IN COMPLIANCE WITH THE LAWS OF THE COMMONWEALTH OF KENTUCKY. I FURTHER CERTIFY THAT THIS PLAT AND THE SURVEY ON THE GROUND WERE PERFORMED BY PERSONS UNDER MY DIRECT SUPERVISION, AND THAT THE DIRECTIONAL AND LINEAR MEASUREMENTS BEING WITNESSED BY MONUMENTS SHOWN HEREON ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE. THE "RURAL" SURVEY, AND THE PLAT ON WHICH IT IS BASED, MEETS ALL SPECIFICATIONS AS STATED IN KAR 201 18:150.



09/14/2022

6 0

PREPARED BY:

POD

POWER OF DESIGN

11490 BLUEGRASS PARKWAY

LOUISVILLE, KY 40299 502-437-5252

PREPARED FOR

KENTUCKY RSA PARTNERSHIP

## SITE SURVEY

REV	. DATE	DESCRIPTION
Α	6.29.20	PRELIM ISSUE
В	11.2.20	TITLE REVIEW
0	9.14.22	ISSUED AS FINAL

#### SITE INFORMATION:

## EV FANCY FARM

KENTUCKY HIGHWAY 80 FANCY FARM, KY 42039 GRAVES COUNTY

TAX PARCEL NUMBER: 006.00.00.005.00

PROPERTY OWNER: KM & K FARMS LLC P O BOX 48035 COON RAPIDS, MN 55448

SOURCE OF TITLE: DEED BOOK 506, PAGE 639

 POD NUMBER:
 20-64963

 DRAWN BY:
 JRS

 CHECKED BY:
 MEP

 SURVEY DATE:
 6.24.20

 PLAT DATE:
 6.29.20

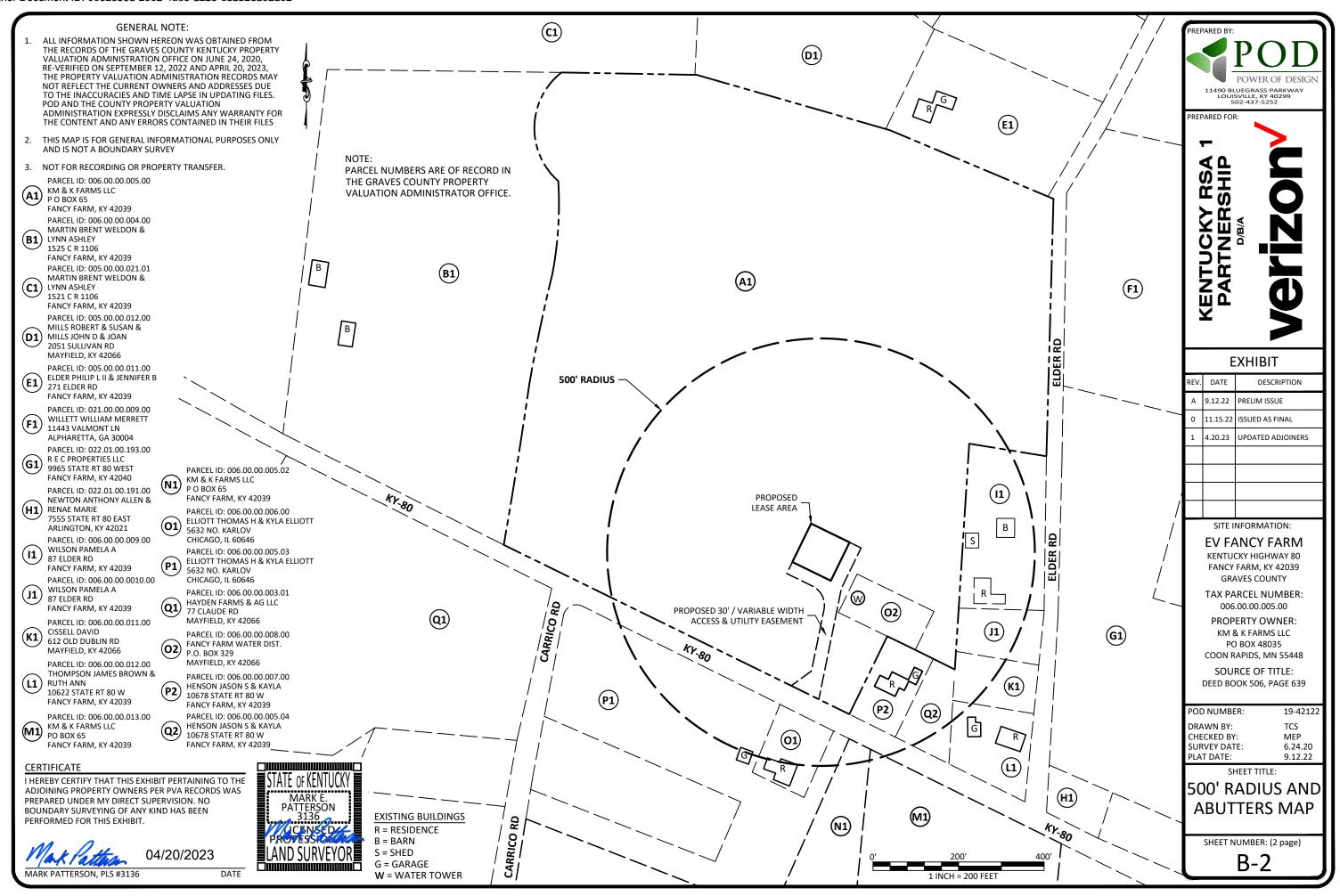
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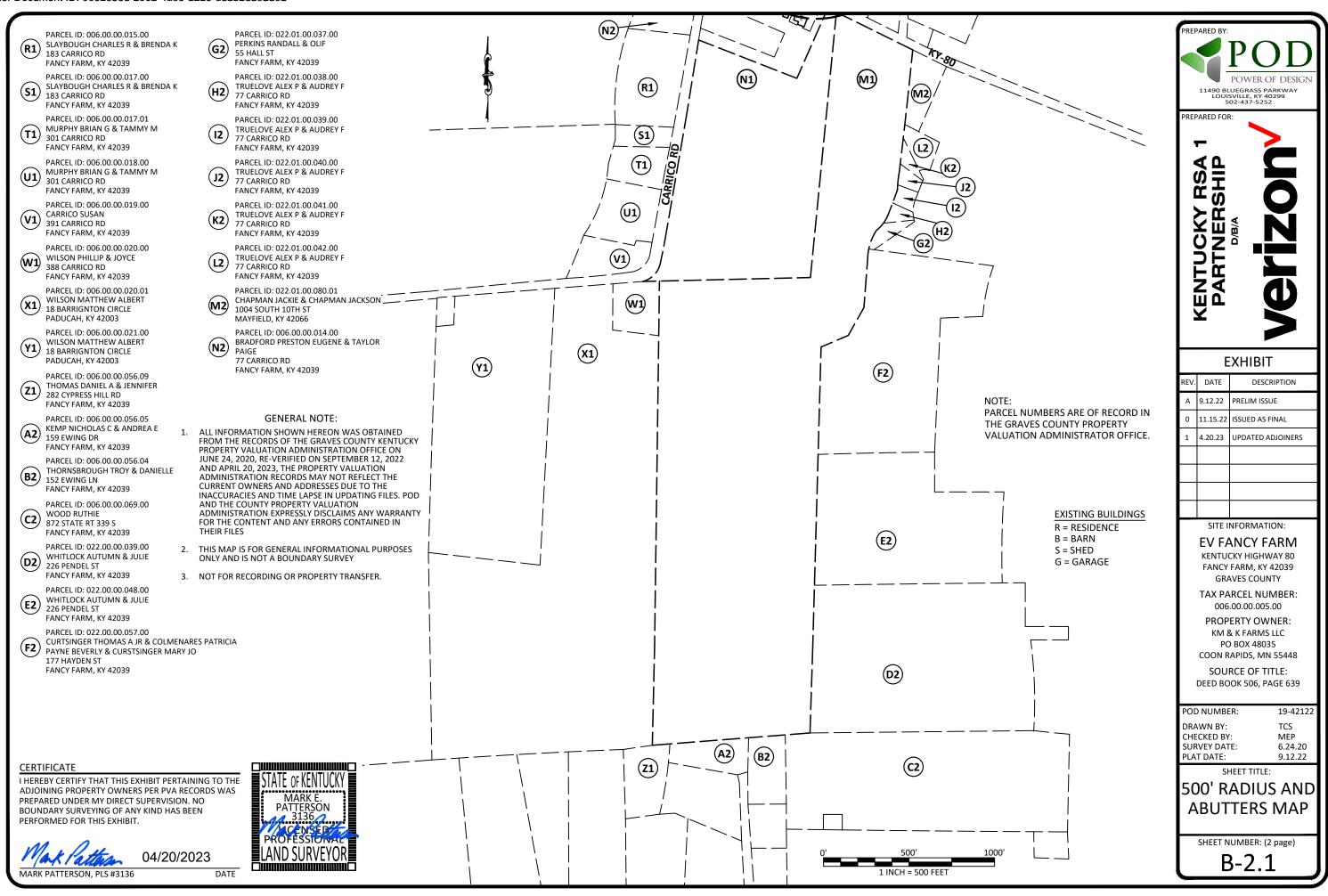
## SITE SURVEY

THIS DOES NOT REPRESENT A
BOUNDARY SURVEY OF THE
PARENT PARCEL

SHEET NUMBER: (2 pages)

B-1.1



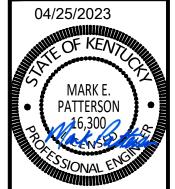


## **REVISION LOG**

REV *	MM/DD/YY	SHEET NUMBER	DESCRIPTION OF REVISION	
A	9/28/2022	ALL SHEETS	ISSUED FOR REVIEW	
0	11/15/2022	ALL SHEETS	ISSUED AS FINAL	
1	4/25/2023	B-2 & B-2.1	500'R REVISION	







EN PERMIT: 3594

## ZONING DRAWINGS

REV.	DATE	DESCRIPTION
Α	9.28.22	ISSUED FOR REVIEW
0	11.15.22	ISSUED AS FINAL
1	4.25.23	500'R REVISION

SITE INFORMATION:

**FANCY FARM** 

KENTUCKY HIGHWAY 80 FANCY FARM, KY 42039 GRAVES COUNTY

VERTICAL BRIDGE SITE NUMBER: US-KY-5135

VERIZON SITE NAME: EV FANCY FARM

POD NUMBER: 21-117201

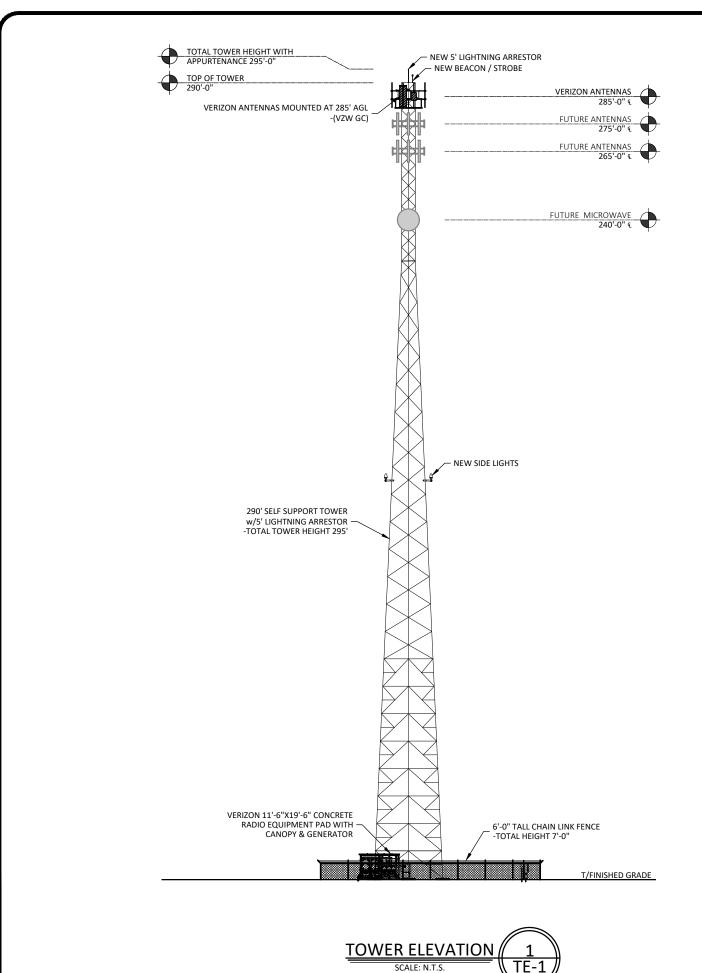
DRAWN BY: CHECKED BY: DATE: POD MEP 09.28.22

SHEET TITLE:

**REVISION LOG** 

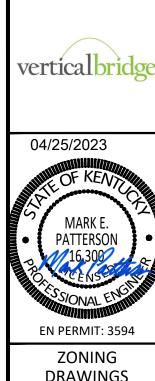
SHEET NUMBER:

R-1



### NOTE:

- 1. IT IS THE INSTALLING CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL ANTENNA INFORMATION AGAINST FINAL RADIO ENGINEERING PLAN PROVIDED BY KENTUCKY RSA 1 PSHP d/b/a VERIZON (VZW GC)
- 2. ALL TOWER LIGHTING SHALL BE INSTALLED AS REQUIRED BY THE FEDERAL AVIATION ADMINISTRATION AND RECOMMENDED BY THE USFWS INTERIM GUIDELINES (2000) FOR LIGHTING OF TOWERS OVER 200' IN HEIGHT.



POWER OF DESIGN

11490 BLUEGRASS PARKWAY LOUISVILLE, KY 40299 502-437-5252

# **DRAWINGS**

	REV.	DATE	DESCRIPTION
	Α	9.28.22	ISSUED FOR REVIEW
	0	11.15.22	ISSUED AS FINAL
	1	4.25.23	500'R REVISION
ı			

### SITE INFORMATION: **FANCY FARM**

KENTUCKY HIGHWAY 80 FANCY FARM, KY 42039

GRAVES COUNTY VERTICAL BRIDGE SITE NUMBER: US-KY-5135

> VERIZON SITE NAME: **EV FANCY FARM**

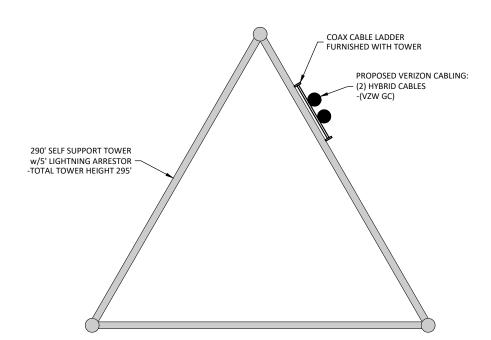
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DRAWN BY: POD CHECKED BY: 09.28.22

SHEET TITLE:

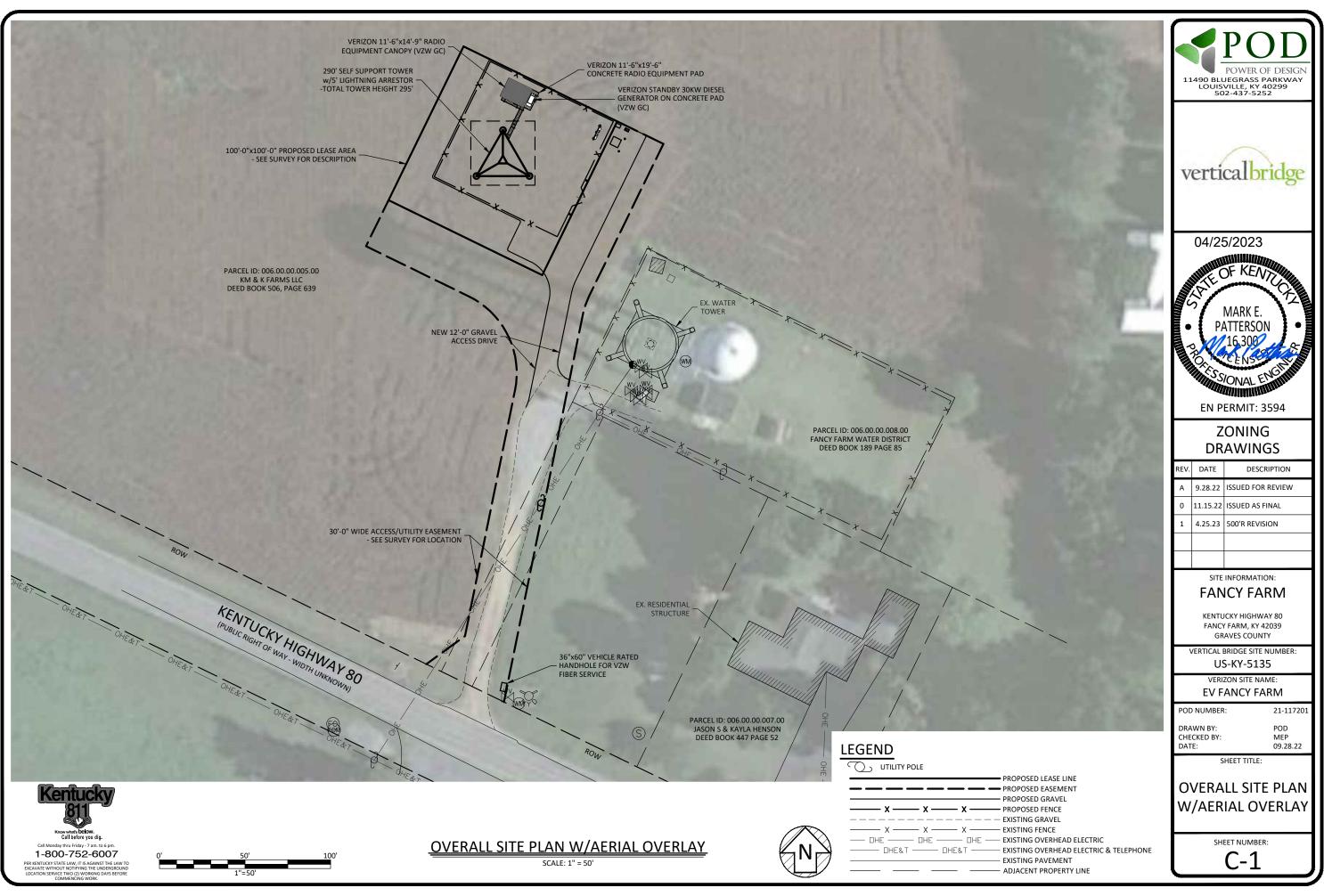
**TOWER ELEVATION** 

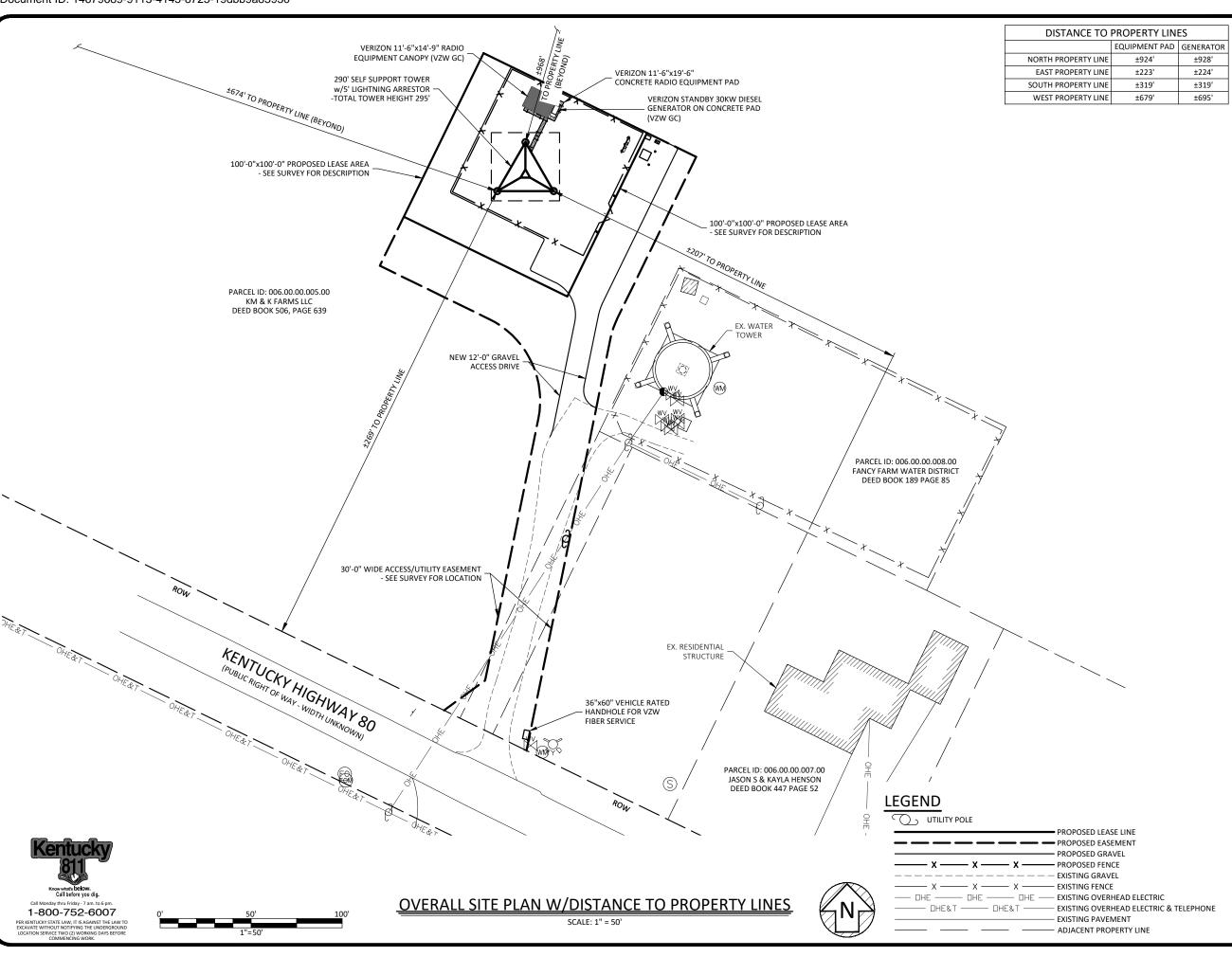
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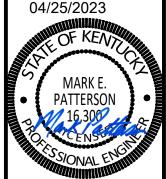
SCALE: N.T.S.











EN PERMIT: 3594

## ZONING DRAWINGS

REV.	DATE	DESCRIPTION
Α	9.28.22	ISSUED FOR REVIEW
0	11.15.22	ISSUED AS FINAL
1	4.25.23	500'R REVISION

## SITE INFORMATION:

## **FANCY FARM**

KENTUCKY HIGHWAY 80 FANCY FARM, KY 42039 GRAVES COUNTY

VERTICAL BRIDGE SITE NUMBER: US-KY-5135

VERIZON SITE NAME:

EV FANCY FARM

POD NUMBER: 21-11720 DRAWN BY: POD

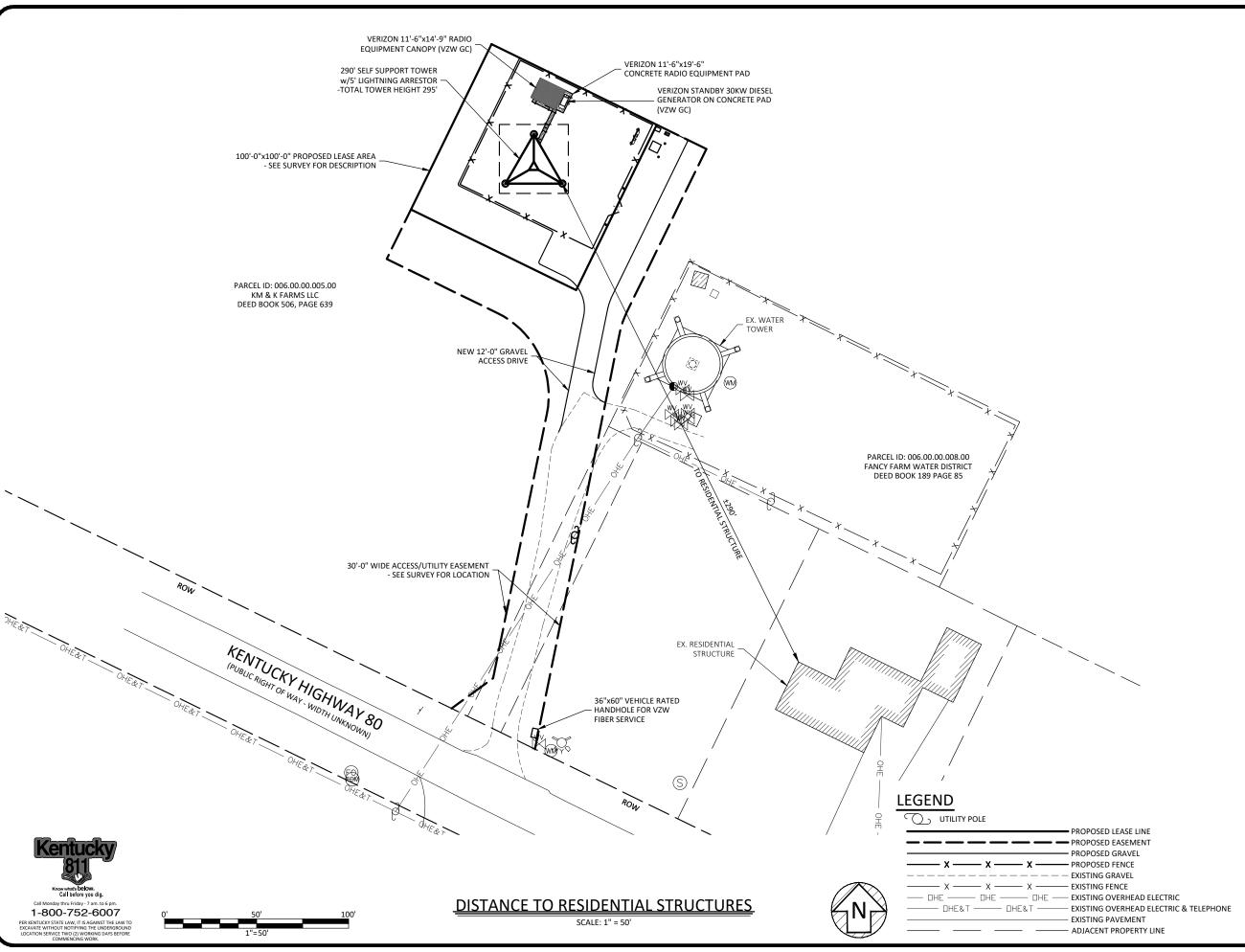
DRAWN BY: POD
CHECKED BY: MEP
DATE: 09.28.22

SHEET TITLE:

OVERALL SITE PLAN W/DISTANCE TO PROPERTY LINES

SHEET NUMBER:

C-1A







04/25/2023



EN PERMIT: 3594

## ZONING DRAWINGS

REV.	DATE	DESCRIPTION
Α	9.28.22	ISSUED FOR REVIEW
0	11.15.22	ISSUED AS FINAL
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## SITE INFORMATION:

**FANCY FARM** 

KENTUCKY HIGHWAY 80 FANCY FARM, KY 42039 GRAVES COUNTY

VERTICAL BRIDGE SITE NUMBER: US-KY-5135

VERIZON SITE NAME: EV FANCY FARM

POD NUMBER: 21-11720.

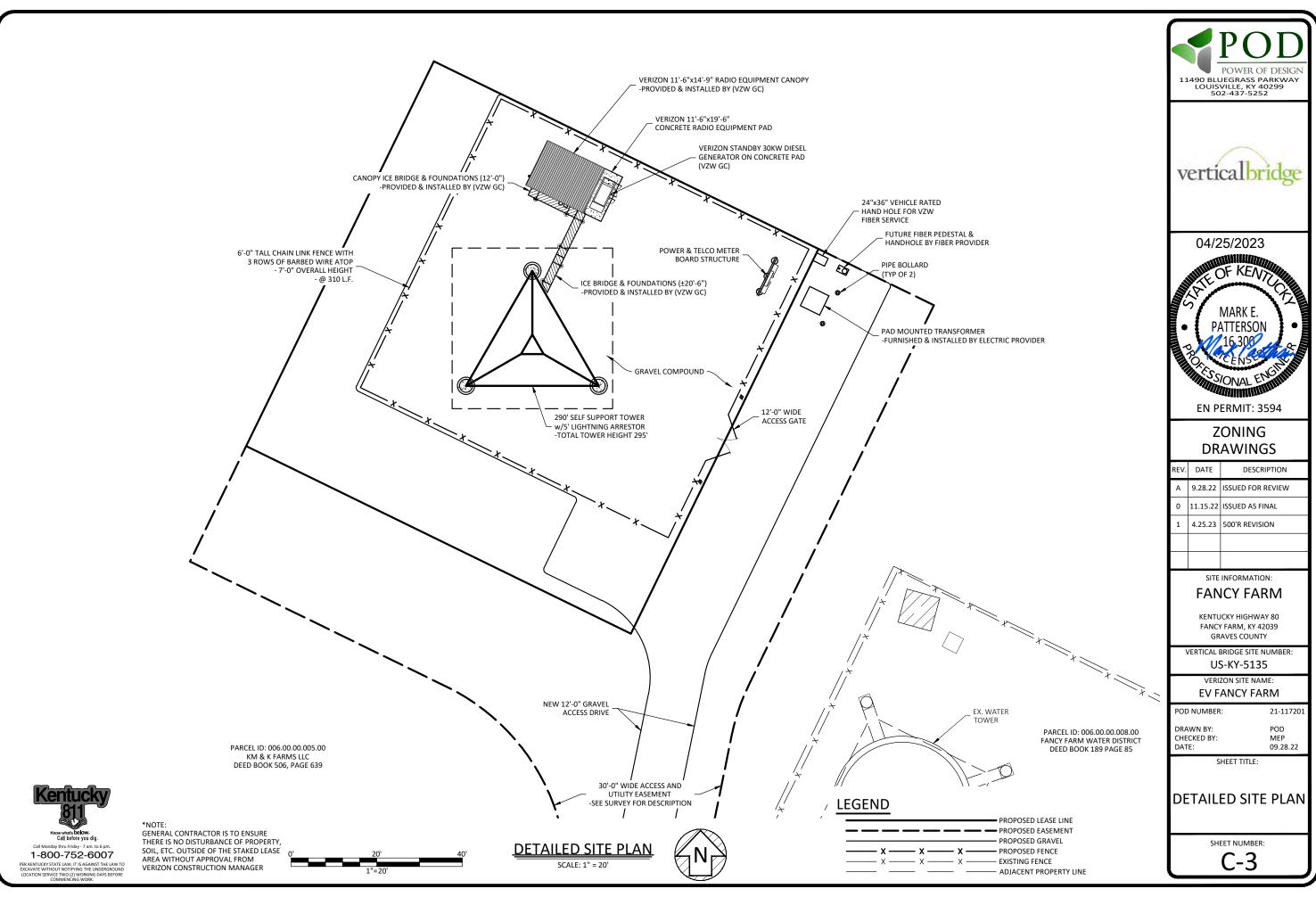
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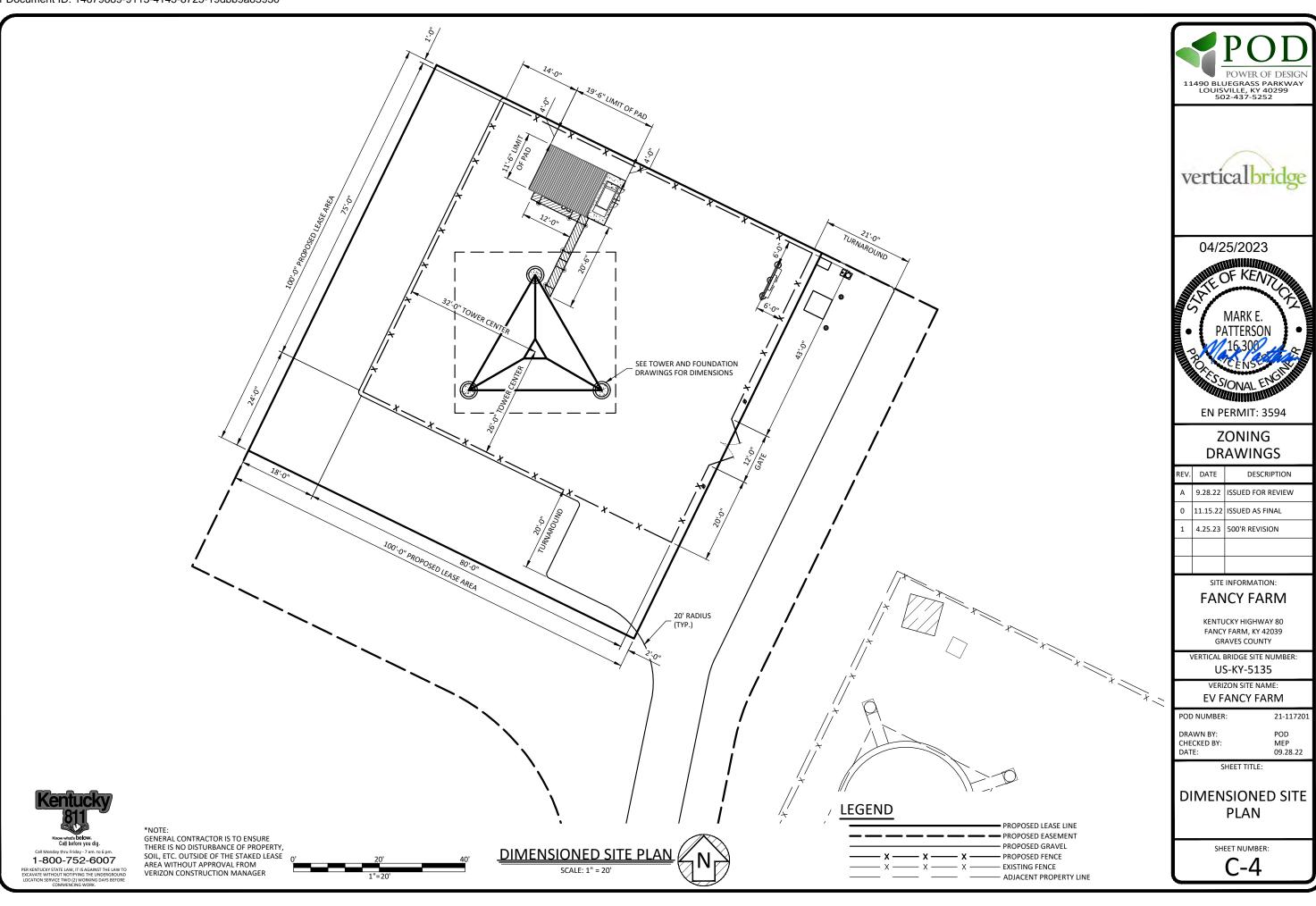
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DISTANCE TO RESIDENTIAL STRUCTURES

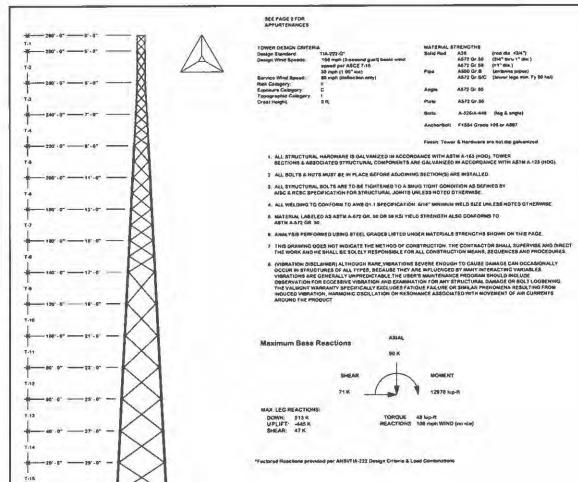
SHEET NUMBER:

C-1B





## **Exhibit D**



						TOWER CO	LUMN			
SECTION	ELEVATION	FACE WIOTH	PANELB	LEGRIZE	LEGSTYLE	GTY & DIA	Bracing Size	HORIZONAL BRACING SIZE	BRACHO BOLT QTY & DIA	BECTION
71	580, * 580.	\$.0"	2	2 50"	¥	4 x 2M*	18" x 2" x 2"	14" # 3" # 3"	1 x 3/4 *	854.3
12	280' - 280'	6.0	3	4.00"	V	6 x 3/4"	3/16" x 2-1/2" x 2-1/2"		1 a 364 "	1573.0
73	240' - 260'	7.0'	3	8.00"	v	0 x 3/4"	3/48" E 7" × 2"		1 4 344 *	1504.3
74	720' - 240'	p.0°	3	8.00"	٧	6 x 1"	3/18" x 2-1/2" x 2-1/2"		1 x 3/4 -	2273
TB	200.+550.	11.0	2	1.50"	1280FH	0 x 1"	3/16" x 2-1/2" x 2-1/2"		5 4 1 "	2578
Te	100, 1500.	13.0	2	1.75	1280FH	8 x 1 156"	184" x 2-1/2" x 2-1/2"		1×1-	3041.
17	160' - 180'	15.0"	8	1,73"	1280FH	8 x 1 1/4"	1M" = 2-1/2" = 2-1/2"		9 4 1 "	31413
TR	140' - 160'	170	2	1.76*	120DFH	0 4 1 964	3/18"× 3" × 3"		1117	3154
76	120' - 140'	19.0	1	2.00"	128DH3	12×1"	3rt6"x 2" = 3"		1 : 78"	0046
Thū	100'-120'	21.0		2.00"	1280N2	12 × 1"	3H4"x 3" x 3"		1 = 746 *	#188.
T 91	80'-100'	23.0	1	5.00.	1280H2	12 = 1"	3/44"x 3" x 3"		1 = 7/6 *	4238.0
712	60" - 80"	25.0"	1	2.25	120042	12 x 1"	3/16"× 3" x 3"		1 = 775	4828.
113	40"-00"	27.0	1	2.25"	128DH2	12 × 1"	14" = 3-1/2" = 3-1/2"		1 = 7/6 "	5701.
774	201.40	29.0"	1	2.25	128 DH2	12 = 1"	184" K 3-172" K 3-172"	1	1 × 746."	8817.
TVS	0'-20'	21.0	r	2.50	1280412	4 1 1 344"	164" 4 3-1/2" 1 3-1/2"		1 6.746"	6637



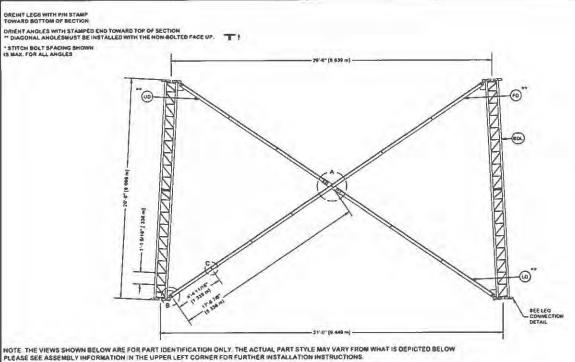
REV	DESCRIPTION	ON OF REVISIONS	CPD	BY   DATE	US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'	DESCRIPTION Tower View Page 1	valmont  1.677-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR	
MEA	OCOCINIT TIE	REVISION HISTOR		10/1 0//0	COPYRIGHT 2022			
SAN	SAN	JL	APPROVED BY	10/6/2022	PROPING TARY HOTE.  THE DATA AND TRO-MIGUES EDITAMED IN THIS GRAWING ARE PROPING TARY INFORBATION OF VALIDAY INFORESTICS AND OD-MIGRED A TRADE SECRET. ARY USE ON DISCLOSURE WITHOUT THE CONSENT OF VALIDAY WASHOOT INFORMATION.	565090	293706T	1 OF 17

DESIGNED APPURTENANCE LOADING	
TYPE	ELEVATION
(1) 848"X 16" LIGHTNING ROD	380 0000
(1) SEACON	580 0000
(1) 40,000 8Q.W (277.8 SQ.FT EPA)	288 9000
(1) 30,000 8Q.W. (206.3 8Q.FT EPA)	276 0000
(1) 50,000 BQ PL (206 3 SQ FT EPA)	288 0000
(2) 2-1/2" X T SCH. 40	740 0000
(1) 5" HP ()(240 DEG AZIMUTH)	240 0000
(1) SP1 R& (INCLUDES 4.8"X72" PIPE)	240 0000
(3) ÓB LIGHT	148.0000

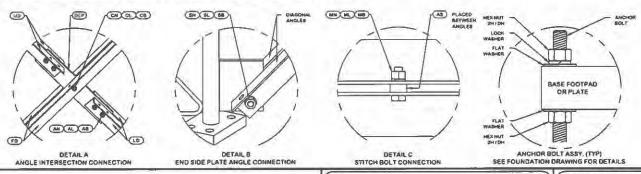


SITE DESCRIPTION US-KY-5135 FANCY FARM VB BTS II, LLC Tower View Page 2 U 31 X 290° 1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, CR **STRUCTURES** DESCRIPTION OF REVISIONS CPD BY DATE REV COPYRIGHT 2022 REVISION HISTORY PROPRIETARY NOTE.

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THE DATA AND TECHNICISES CONTAINED IN THE DRAWHOLDER OF DISCUSSION OF VALIDATE OF VALIDAT ENG. FILE NO. APPROVED BY DRAWN BY APPROVED BY SAN DESIGNED BY RELEASE DATE 10/6/2022 565090 293706T 2 OF 17



			PARTS LIST			
IVER QTY PART NO		PART NO	PART DESCRIPTION		UNIT WT	WET WIT
BOL	3	261171	#12 BASE SECTION - 2 1/2" LEG - 1/2" GRACE W(1)		1424 950	4274.860
VO		205817	U-31 UPPER ANGLE SINGLE BOLT FOR 20-0" LONG TA		97,370	594 229
LO		285818	U-31 COWER ANGLE - SINGLE BOLT FOR 20"0" LONG TA		104 820	627 72
FD		265616	U-31 LONG ANGLE - SINGLE BOLT FOR 20'-0" LONG TAP		208 820	1281 120
MC,	27	312123	EN" GALVANIZED LOCKWASHER (53-22230)		0.070	.0.846
Line .	27	312501	SIP"-11 HOT DIPPED GALVANIZED NUT		0 120	3.244
AS	27	237668	RING FILL SPACER SIS" THICK 1 040" HOLE		0.090	2.43
440	27	101896	88" 11 X 2 1M" A-328 BOLT 1 1M" THREAD		0.200	7,020
ABICE	15	101806	5/8" 11 X 2 1/4" A-325 BOLT 1 1/4" THREAD		0 200	3.900
AL CL	15	312123	ENT GALVANIZED LOCKWASHER (83-22238)	DE" GALVAMZED LOCKWASHER (53-22239)		9.300
ANION	15	312501	SW"-11 HOT DIPPED GALVANIZED NUT		0 129	9 800
OCF	3	211833	MID BRACE CONNECTION PLATE FOR 812 BID LEG ANGLES	MID BRACE CONNECTION PLATE FOR \$12 BID LEG ANGLES		01 FF6
SL	12	312193	7/8" GALVANIZED LOCKWASHER		0 050	0.804
SH:	12	212216	78F-8 HOT DIPPED GALVANIZED HUT		0 300	3.600
58	12	172276	172275 7/8"-9 X 2-1/2" A-325 BOLT WITH 1-1/2" THREAD		1 230	14 760
				Total WI	9837 97 (b [310	4 46 kg)



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US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290°

SECTION U-31.0 (0' - 20' ELEVATION)

1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

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21.314	197-11-1	100	1.00	10.0.00

**DESCRIPTION OF REVISIONS** 

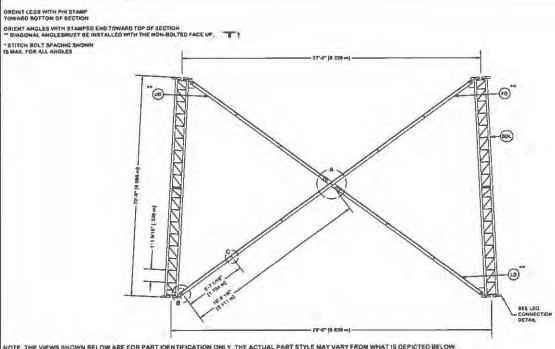
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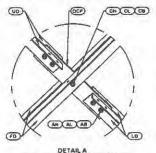
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3 OF 17

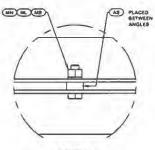


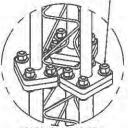
			PARTS LIST	0.00	
(TEM	OTY	FART NO.	PART DESCRIPTION	UNITWT	NET WT
GDL.	3	196960	#12 LEG SECTION 2-14" LEG 1/7" BRACE THE B	1190 520	3301 546
uo.	- 1	285799	U-26 UPPER ANGLE SMIGLE BOLT FOR 20-0"LONG TA	92.490	664 640
LO	- 4	265750	U-20 LOWER ANGLE - SINGLE BOLT FOR 20'-0" LONG TA	99.730	598.340
FD		285797	U-SWLONG ANGLE - SINGLE BOLT FOR 20'-8" LONG TAP	198.570	1191.420
RIE	24	312123	SIS" GALVANIZED LOCKWASHER (53-22230)	0 020	0 480
Ases	24	312501	58"-11 HOT OIPPED GALVANIZED NUT	0 120	2.640
AS	24	237850	RING FILL SPACER SIT THICK I DAY HOLE	0.000	2 160
MB	24	181495	M8"-11 X 2 1M" A-325 BOLT 1 1M" THREAD	V 260	8.240
AB/CB	75	461898	56"-11 X 2 14" A-325 BOLT # 14" THREAD	#.200	3.900
AL CL	10	312128	SAT GALVANIZED LOCKWASHER (53-22230)	0.020	0 300
ANICH	98	312601	SM"-11 HOT CHPPED GALVANIZED NUT	0 120	1.800
DCP	3	211838	MID SPLACE CONNECTION PLATE FOR \$12 BID LEG ANGLES	20 500	81 770
SL.	12	312193	78" GALVANIZED LOCKWASHER	ù 050	0.606
\$N	92	312216	78"-9 HOT DIPPED BALVANIZED HUT	4 300	3,400
88	12	172275	747-6 X 2-112" A-326 BOLT WITH 1-112" THREAD	1 230	14 760
LCS	38	222016	1" 4 X AJA" A J25 BOLT WITH 1-34" THREAD	1 369	48 600
LCF	38	312222	1" GALVANIZED FLAT WASHER (FASA)	0.140	5.040
LCL	3.0	312223	1" GALVANIZED LOCKWABNER	0.000	2,000
FCH	3.0	412804	1 4 HOT DIFFED GALVANIZED NUT	0.430	16 480
			Te	1017.07 lb (264	11.36 hg)

NOTE: THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW. PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.









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LEG TO LEG CONNECTION

DETAIL A ANGLE INTERSECTION CONNECTION

DETAIL B END SIDE PLATE ANGLE CONNECTION

DETAIL C STITCH BOLT CONNECTION

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**DESCRIPTION OF REVISIONS** CPD BY DATE US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290"

SECTION U-29.0 (20' - 40' ELEVATION)

1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

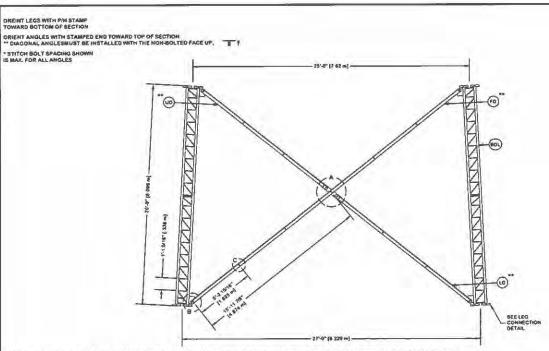
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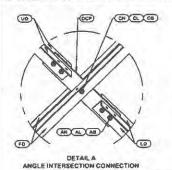
4 OF 17

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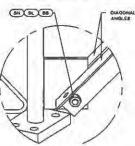


			PARTS LIST			
пен	gry	PART NO.	PART DESCRIPTION		UNIT WT	NET WT
BOL	3	196880	#12 LEG BECTION 2-14" LEG 1/2" BRACE 78"8	-	1100 520	3301 540
VØ-		285770	U-27 UPPER ANGLE SINGLE BOLT FOR 20 0" LONG TA		87.960	525.300
LD		286777	U-27 LOWER ANGLE - SINGLE BOLT FOR 26'0" LONG TA		95.000	670 000
FD	0	285776	U-27 LONG ANGLE - SINGLE BOLT FOR 20'0" LONG TAP	contacts.	189,900	1133 340
lat.	24	312123	SIR" GALVANIZED LOCKWASHER (53-22230)		0.020	0.490
NA .	24	312501	SW'-11 HOT DIPPED GALVANIZED NUT		8.120	2.680
AS	24	237650	RING FILL SPACER NOT THICK 1.040" HOLE		0.000	2.160
MIS	24	161806	SR"-11 X 2 14" A-325 BOLF 1 14" THREAD		6 260	6.240
ABICE.	1 15	101000	SR"-11 X 2 1M" A-325 BOLT 1 1M" THREAD		0 200	3,990
AL ICL	16	312123	SIE" GALVANIZED LOCKWASHER (\$3-22230)		0.029	0.300
AN/CH	16	312501	SAT 11 HOT DIPPED GALVANIZED HUT		0.179	1.890
DCP	3	211833	MID BRACE CONNECTION PLATE FOR \$12 BYO LEG ANGLES		20.500	61.770
8L	12	312193	7/8" GALVANIZEO LOCKWASHER		0.050	0.000
SM	.12	312216	FRE'4 HOT DIPPED GALVANIZED HUT		9.300	3.600
\$8	12	172278	78"-4 K 2-1/2" A-326 BOLT WITH 1-1/2" THREAD		1 230	14 740
LCB	36	222016	1" 8 X 4-3H" A-325 BOLT WITH 1-3H" THREAD		1.380	49 800
LCF	36	312222	1" GALVANIZED FLAT WASHER (F438)		0.140	5.040
LCL	36	312223	1" CALVANIZED LOCKWASHER		0 080	2.800
LCN	36	312504	1 - B HOT DIPPED GALVANIZED HUT		0 430	18 490
				Yotw Wt	6701 77 No [254	8.46 kg

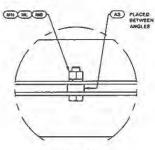
NOTE: THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW. PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.



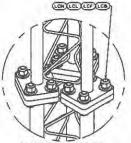




DETAIL B END SIDE PLATE ANGLE CONNECTION



DETAIL C STITCH BOLT CONNECTION



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)



SITE

US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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SECTION U-27.0 (40" - 60" ELEVATION)



1-877-457-4763 Plymouth, IN 1-800-547-2151 Salam, OR

**STRUCTURES** 

ENG. FILE NO.

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PAGE 5 OF 17

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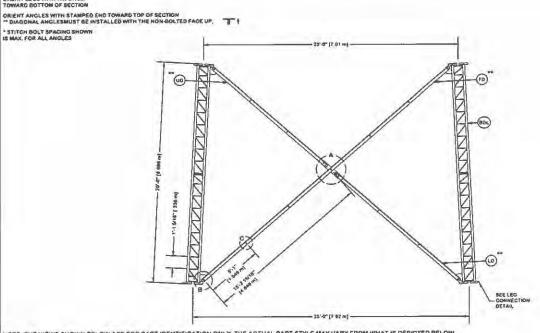
DESCRIPTION OF REVISIONS

REVISION HISTORY DESIGNED BY

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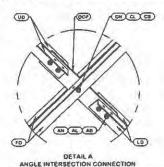
RELEASE DATE 10/6/2022

CPD BY DATE



			PARTS LIST			
ITEM	QTY	PART NO	PART DESCRIPTION		UNIT WT	HET WT
BDL	3	195960	#12 LEG SECTION 2-14" LEG -17" BRACE 74" B	Same.	1100 820	2301 580
VD	4	265757	U-25 UPPER ANGLE BINGLE BOLT FOR 20"4" LONG TA		52.010	317.480
LO		245754	U-28 LOWER ANGLE - BINGLE BOLT FOR 20-0" LONG TA		87.860	347 180
FO	6	285756	U-25 LONG ANGLE - SINGLE BOLT FOR 20'-0" LONG TAP		114 830	586 960
NAL.	24	312123	SIR" GALVANIZED LOCKWASHER (63-22250)		0 020	0 480
adde	24	312501	ME"-11 HOT SIPPED GALVANIZED NUT		0 120	2 880
AS	24	237658	RING FILL SPACER IN THICK 1.040" HOLE		0.090	2,180
ANCE	24	181806	SIGT 191 K 2 1M" A-328 BOLT 1 1M" THREAD		0.200	E.240
ABICB	15	161696	56"-11 X 2 14" A-325 BOLT 1 14" THREAD		0.260	3.900
AL I CL	16	317123	SRT GALVANIZED LOCKWASHER (53-22230)		0.020	9.300
AN CN	18	312501	SRT 11 HOT DIPPED GALVANIZED NUT		0 120	1,800
DCP	3	211833	MID BRACE CONNECTION PLATE FOR \$12 BYD LEG ANGLES		20.500	81,770
8L	12	312103	78" GALVANIZED "OCKWASHER		0.050	0.800
SN	12	312216	78" @ HOT DIPPED GALVANIZED NUT		# 300	3.600
58	12	172276	78"-0 X 3-112" A-326 BOLT WITH 1-1/2" THREAD		1 230	14 790
LCS	38	222016	1"-8 X 4-34" 4-325 BOLT WITH 1-34" THREAD		1,380	49.680
LCF	30	312222	1" GALVANIZED FLAT WASHER (# 436)		0 160	5.040
rcr	3.0	312223	1 GALVANIZED LOCKWASHER		0.000	2 880
LCN	38	312504	1"-8 NOT DIPPED GALVANIZED NUT		0.430	16 480
				Total Wil.	4826.73 lb (218	1 38 kg)

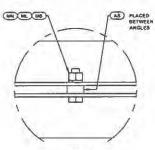
NOTE: THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICYED BELOW PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.



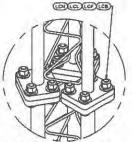
OREINT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION

\* STITCH BOLT SPACING SHOWN IS MAX. FOR ALL ANGLES





OETAIL C STITCH BOLT CONNECTION



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)



SITE US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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SECTION U-25.0 (60' - 80' ELEVATION)

1-877-467-4763 Phymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

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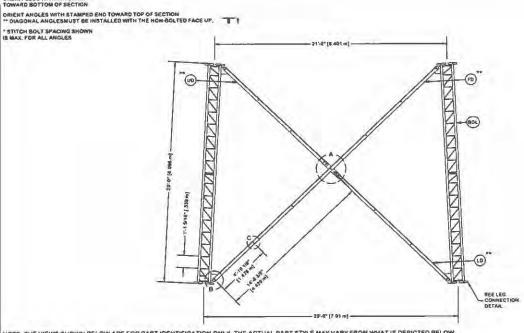
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6 OF 17

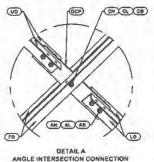
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			PARTS LIST			
IAEM	OTY.	PART NO	PART DESCRIPTION	70	UNIT WT	NET WIT
BOL	3	vacaja	#12 LEG SECTION - 2" LEG - 1/2" BRACE - 7/8" BOLT		926 920	2760 760
VD.		266733	U-23 UPPER ANDLE - SINGLE BOLT FOR 20"-0" LONG TA		49.920	299.620
ro		265732	U-23 LOWER ANGLE - SINGLE BOLT FOR 20"4" LONG TA		85.000	330 460
FD		266731	U-23 LONG ANGLE SINGLE BOLT FOR 20'-0"LONG TAP		100.040	454 340
Mil	24	312123	5-6" DALVANIZED LOCKWASHER (\$3-22230)		0.020	0.480
WH.	24	312501	BET-11 HOT DIPPED GALVANIZED NUT		0.126	2.880
AS	24	237658	RING FILL SPACER SIS" THICK 1.048" HOLE		0.090	2,160
int	24	181898	88"-11 X 2 14" A-328 BOLY 1 14" THREAD		6.289	0 240
ABICE	16	191895	58" 11 X 2 14" A-325 BOLT 1 14" THREAD		0.200	3.900
AL CL	16	312123	SHT GALVANIZED DCKWASHER (63-27239)		0 020	0.300
AN / CN	18	312501	54" (1 HOT DIPPED GALVANIZED NUT		0.120	1.800
DC#	3	211833	MID BRACE CONNECTION PLATE FOR #12 BIG LEG ANGLES		20 500	61 770
31_	12	312192	78" GALVANIZED LOCKWASHER		0 060	4.800
\$N	12	312216	THE G HOT DIPPED GALVAINZED HUT		0 300	3 600
28	12	172275	78"-9 X 2-1/2" A-326 BOLT WITH 1-1/2" THREAD		1 220	14 290
LCB	34	222016	1"-8 X 4-344" A-325 BOLT WITH 1-3H" THREAD		1 380	49,600
LCF	36	312222	1" GALVANIZED FLAT WASHER (F436)		0 140	B.D-00
LCL	34	312223	1 GALVANIZED LOCKWASHER		0 000	2.080
LCH	3.6	317504	1"4 NOT DIPPED GALVANIZED NUT		11.430	16.480
				Total Wit	4236.49 (b [192	(3.49 kg)

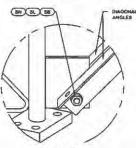
NOTE. THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.



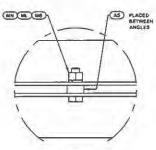
OREINT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION

\* STITCH BOLT SPACING SHOWN IS MAX. FOR ALL ANGLES

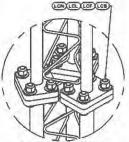




END SIDE PLATE ANGLE CONNECTION



DETAIL C STITCH BOLT CONNECTION



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)



US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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SECTION U-23.0 (80' - 100' ELEVATION)



1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

ENG. FILE NO.

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REVISION HISTORY APPROVED BY ORAWN BY SAN

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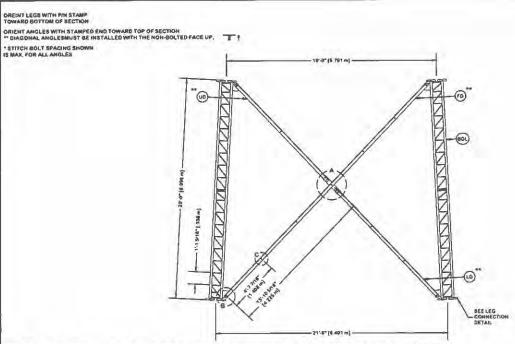
DESCRIPTION OF REVISIONS

APPROVED BY

RELEASE DATE 10/6/2022

CPO BY DATE

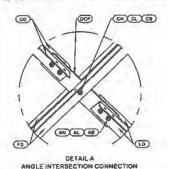
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			PARTS LIST			
ITEM	QTY	PART NO.	PART DESCRIPTION		TW TINU	NET WT
800	3	195639	HIZ LEG SECTION - 2" LEG - 172" BRACE - 78" BOLT		979 920	2790.740
UD	4	268709	U-21 WPPER ANGLE - SINGLE BOLT FOR 20:0" LONG TA		47 838	282 180
40	4	265798	U-21 LOWER ANGLE BUIGLE BOLF FOR 28'-0"LONG TA		82.476	314.820
FO		248707	U.21 LONG ANGLE SINGLE BOLT FOR 20'4 LONG TAP		103,560	521 360
MIL	21	312123	BIS" DALVANIZED LOCKWASHER (53-22230)		0 076	6.420
NAME .	21	312501	547-11 HOT DIPPED GALVANIZED NUT		0 120	2.620
AS	. 21	237658	HING FILL SPACER BAT THICK 1 048" HOLE		0.090	1.990
ME	21	161696	845"-11 X 2 1M" A-326 SOLT 1 1M" THREAD		0.290	6.480
ABICS	15	181898	BR"-11 X 2 14" A-326 BOLT 1 1M" THREAD		0.200	3.900
AL/CL	16	312123	SM* GALVANIZED LOCKWASHER (53-22230)		0 020	0 300
AN I ON	+6	312501	SET-11 HOT DIPPED GALVANIZED NUT		0.120	1,800
DCP	3	211833	MID BRACE CONNECTION PLATE FOR #12 BYD LEG ANGLES		20,500	61 770
SL	12	312105	78" GALVANIZED LOCKWASHER		0.050	0.600
SN	12	312218	78"-0 HOT GIPPED GALVANIZED NUT		9.300	3,600
80	12	172275	787-8 X 2-112 A-328 BOLT WITH 1-112 THREAD		1:230	14 700
LCB	34	222616	1"-8 X 4-3M" A-325 BOLT WITH 1-3M" THREAD		1 389	49 680
LCF	36	312322	1" GALVANIZED FLAT WASHER (F434)		D 140	\$.040
LCL	38	512223	1" GALVANIZED LOCKWASHER		0.000	2,980
LCN	36	312604	1"-8 HOT DIPPED GALVANIZED NUT		0.430	15.400
-				Torse Wit	4100.22 fb (186	(2.86 kg)

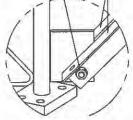
NOTE THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW. PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS

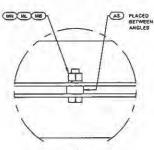
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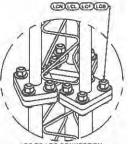


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DETAIL B END SIDE PLATE ANGLE CONNECTION

CPD BY DATE

DETAIL C STITCH BOLT CONNECTION

SITE

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LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)

US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290°

ENG. FILE NO.

SECTION U-21.0 (100' - 120' ELEVATION)

1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

DESCRIPTION OF REVISIONS REV REVISION HISTORY DRAWN BY

RELEASE DATE APPROVED BY DESIGNED BY APPROVED BY 10/6/2022

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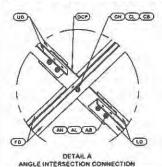
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DREINT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION ORIENT ANGLES WITH STAMPED ENG TOWARD TOP OF SECTION

"DIAGONAL ANGLESMUST BE INSTALLED WITH THE NON-BOLTED FACE UP. \* STITCH BOLT SPACING SHOWN IS MAX, FOR ALL ANGLES SEE LEG 19'-0" (\$.791 m) -

			PARTS LIST			
ITEM	QTY	PART NO	PART DESCRIPTION		UMIT WT	NET WT
BDL	3	196437	BIZLEG SECT -2" TO 1-34" TRAMS LEG 1/2" BRACE	1000	906 870	2720 810
VO		265682	U-19 UPPER ANGLE - SINGLE BOLT FOR 20'-9" LONG TA		44 280	295.840
LD		205481	U-IN LOWER ANGLE SINGLE BOLT FOR 29"-O" LONG TA		50 080	300.380
FO		265600	U-19 LONG ANGLE SHIGLE BOLT FOR 20"4" LONG TAP	10,000	98.380	800 200
MIL	21	312123	58" GALVANIZED LOCKWASHER (\$3-27230)		0 020	0 420
Sales	21	317501	SIP-11 HOT DIPPED GALVANIZED BUT		0 120	2.520
AS	21	237656	RING FILL SPACER SIG" THICK 1 049" HOLE		0.000	1.890
MB	21	161805	MET-11 K 2 1M A-325 BOLT 1 1M THREAD		0 260	S 460
AB/CIB	15	181886	587 11 K 2 14" A-325 BOLT 1 14" THREAD	-	9 260	3.900
AL CL	16	312123	BIST GALVAINZED LOCKWASHER (83-22230)		0.020	9.300
WH ) CH	15	312801	SAT-11 HOT CIPPED GALVANIZED NUT		0.120	1,800
DCF	- 3	211833	MID BRACE CONNECTION PLATE FOR \$12 BIO LEG ANGLES		20,590	81 270
8L	12	392193	78" GALVANIZED LOCKWASHER		0.950	0.600
SH	12	312215	TREA HOT OFFED GALVANIZED NUT		0.300	3 600
58	12	172276	78 4 2-1/2 A-326 BOLT WITH 1-1/2 THREAD		1 230	14.780
LCB	34	222510	THE READ A 326 BOLT WITH 1-3M THREAD		1 380	49 800
LGF	.34	312222	1 GALVANIZED FLAT WASHER (F436)		0 140	6.040
LCL	34	312223	17 GALVANIZED LOCKWASHER		8.080	2 860
LCN	34	312504	1 - H HOT DIPPED GALVANIZED NUT		0 430	16 400
				Total Wt	4046 P1 ID (183	7 33 kg)

NOTE: THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW. PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.

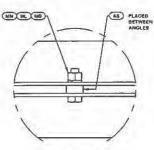


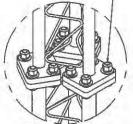
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DETAIL 8 END SIDE PLATE ANGLE CONNECTION

CPO BY DATE

DETAIL C STITCH BOLT CONNECTION

SITE

LEG TO LEG CONNECTION
(SIDE PLATES NOT SHOWN FOR CLARITY)

US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

SECTION U-19.0 (120' - 140' ELEVATION)

DESCRIPTION OF REVISIONS REVISION HISTORY

APPROVED BY APPROVED BY RELEASE DATE 10/6/2022 DESIGNED BY

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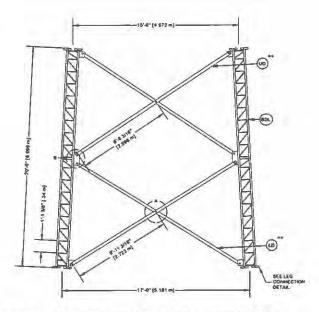
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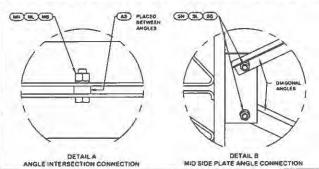
1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR **STRUCTURES** 

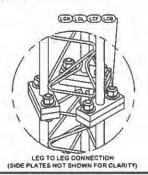
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ORIENT LEGS WITH PIN STAMP
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"DIAGONAL ANGLES MUST BE INSTALLED
WITH THE NON-BOLTED FACE UP, 7 IT
THIS MAY BE ON THE OPPOSITE SIDE OF THE
SIDE PLATE THAN WHAT IS SHOWN IN THE DETAIL.



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			PARTS LIST			
IVEM	DTY	PARTNO	PART DESCRIPTION		UNIT WT	HET WY
BOL	-3	195217	#12 LEG SECTION 1-34" LEG 1/2" BRACE 1" BOL		746.710	2246 13
ro		279264	U-18 UPPER DIAGONAL 3" 4 3" 4 3/16" ANGLE (A\$72		89.430	416 59
AS .		104291	RING FILL SPACER 1/2" THICK 1.049" HOLE		0.070	0.42
1,004	- 6	312902	SM" 16 HOT DIPPED GALVANIZED NUT		0.196	1,94
ML		312153	3H" GALVANIZED LOCKWASHER		0.036	0.18
MIS	6	186427	3M"-10 X 3" A-3257 BOLY WITH FULL THREAD		0 470	2.82
SL	24	312223	1" GALVANIZED LOCKWASHER		0.000	1.92
\$N	24	312304	1"-B HOT DIPPED GALVANIZED NUT	O. Ye	0.430	80 33
88	24	172266	1"-8 X 2-1H" A-325 BOLT WITH 1-3H" THREAD		0,840	20.10
UD		126620	U-18 LOWER DIAGONAL 3" x 3" x 3/16" ANGLE (A672		66 120	396 72
LCS	16	222022	1-14"-7 % \$-12" A-325 BOLT WITH 2" THREAD		2 530	48.64
LCF	18	312282	1-144" GALVANIZED FLAT WASHER (F438)		0 130	2.34
LCL	18	312263	1.14 GALVANIZED LOCKWASHER		0.150	2.70
LOH	18	312807	1.1M". THOT DIPPED GALVANIZED HU?		0 730	12 14
	-	-		Total Wit	3154.11 (5 [143	1.99 40)

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US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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SECTION U-17.0 (140' - 160' ELEVATION)

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PROPRIETARY NOTE
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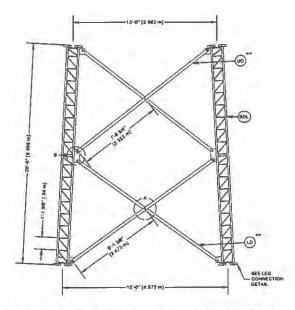
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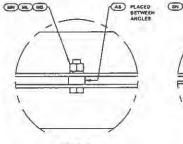
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ORIENT LEGS WITH PIN STAMP TOWARD SOTTOM OF SECTION ORIENT ANGLES WITH STAMPED END TOWARD TOP OF SECTION

"DAGONAL ANGLES MUST BE INSTALLED WITH THE NON-BOLTED FACE UP. THE THIS MAY BE ON THE OPPOSITE SIDE OF THE SIDE PLATE THAN WHAT IS SHOWN IN THE DETAIL.

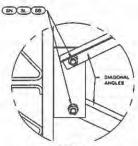


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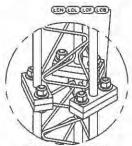


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DETAIL 8 MID SIDE PLATE ANGLE CONNECTION

CPD BY DATE



LEG TO LEG CONNECTION

ITEM	DIA	PART NO	PART DESCRIPTION		UNIT WT	NET WT
BOL	3	195217	#12 LEG SECTION - 1-34" LEG - 1/2" BRACE - 1" BOL		748.710	2240.130
LO	- 6	279250	U-18 UPPER DIAGONAL - 2 1/2" × 2 1/2" × 1M" ANGLE		88.430	410 580
AS		104291	RING FILL SPACER 1/2" THICK 1 948" HOLE		0 070	0 420
MM		312802	34"-18 HOT DIPPED GALVANIZED NUT		0 180	1 140
RAL.	6	312163	34 GALVANIZED LOCKWASHER		0.030	0.480
MS	- 4	160427	SHT-18 K 3" A-225T BOLT WITH FULL THREAD	1007	0.470	2.820
SL	24	312223	1" GALVANIZED LOCKWASHER	1" GALVAMZED LOCKWASHER		1.020
SN	24	212604	1-6 HOT DIPPED GALVANIZED NUT		0 430	19 320
35	24	172266	1"-8 X 2-114" A-325 BOLT WITH 1-34" THREAD		0 840	20 190
VO	4	279227	U-14 LOWER GIAGONAL - 2 1/2" = 2 1/2" = 1M" ANCLE		66.000	390.000
LCS	10	223022	1.44-TX 6-10" A-375 BOLT WITH T THREAD		3.530	45 540
LCF	18	312202	1-14" GALVANIZED FLAT WASHER (F436)		0.130	7.340
LCL	18	312283	1-1M GALVANIZED LOCKWASHER		0 150	2.700
LCH	16	312507	1.4M-7 HOT DIPPED GALVANIZED NUT		0.730	13 140
				Total WI	2141 29 10 [142	6.22 Ng)

PARTS LIST



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US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290"

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SITE

DESCRIPTION

SECTION U-15.0 (160' - 180' ELEVATION)

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1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR STRUCTURES

REVISION HISTORY CO

ORAWN BY APPROVED BY OSSIGNED BY APPROVED BY RELEASE DATE SAN SAN JL J\_L 10/6/2022

DESCRIPTION OF REVISIONS

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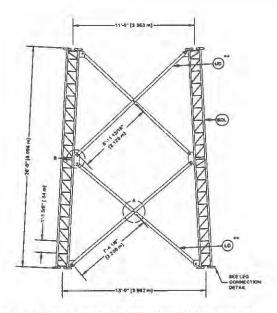
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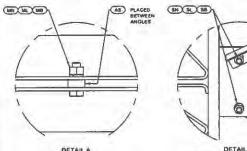
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PAGE 11 OF 17

ORIENT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION ORIENT ANGLES WITH STAMPED END TOWARD TOP OF SECTION TO DIAGONAL ANGLES MUST BE INSTALLED WITH THE MON-BOLTED FACE UP. 71
THIS MAY BE ON THE OPPOSITE SIDE OF THE SIDE PLATE THAN WHAT IS SHOWN IN THE DETAIL.



NOTE: THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.





DESCRIPTION OF REVISIONS

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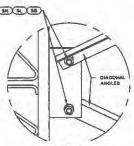
REVISION HISTORY

DESIGNED BY

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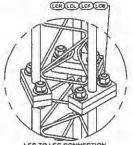
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DETAIL B MID SIDE PLATE ANGLE CONNECTION

APPROVED BY



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)

			PARTS LIST			
rrem	DIX	PART NO.	PART DESCRIPTION		UNIT WY	NET WT
BOL	3	195213	#12 LEG #ECT - 1-344" TO 1-1/2" TRANS LEG - 1/2" 8		730 890	2218 670
LO		279226	U-14 UPPER DIAGONAL - 2 1/2" x 2 1/2" x 1M" ANGLE		61,680	370.000
AS		104291	RING FILL SPACER 1/2" THICK 1.040" HOLE		0.070	0.429
MM		312602	3M"-10 HOT DIPPED GALVANIZED NUT		0.190	1,140
ML		312163	3M" GALVANIZEO LOCKWASHER		0.030	9,180
MS		190427	347-10 x 3" A-326T BOLT WITH FULL THREAD		0.470	2.820
SL	24	312223	1" GALVANIZEO LOCKWASHER		0.080	1.920
314	24	312504	1"-6 HOT DIPPED GALVANIZED NUT	_	0.430	16 320
98	24	172266	1"4 X 2-1M" A-325 BOLT WITH 1-3M" THREAD		0.940	20 180
un	4	278671	U-12 LOWER DIAGONAL 2 1/2" x 2 1/2" x 14" ANOLE		56.490	350 940
LCB	18	222022	1.44-7 % \$-1/2" A-326 BOLT WITH 2" THREAD	3	2 530	48 540
LGF	18	312282	1-144" GALVANIZED FLAT WASHER (F434)		0.130	2,340
LCL	18.	312263	1.1M* GALVANIZED LOCKWASHER		0.760	2.700
LON	16	312807	1.144-7 HOT DIPPED GALVANIZED MUT		0 730	13 140
				Total Wt	3041 37 % [136	0.61 kg)



DESCRIPTION

SECTION U-13.0 (180' - 200' ELEVATION)

1-877-467-4763 Phymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

CPD BY DATE COPYRIGHT 2022

RELEASE DATE

10/6/2022

SITE

PROPRIETARY NOTE.

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US-KY-5135 FANCY FARM

VB BTS II, LLC U 31 X 290'

ENG. FILE NO.

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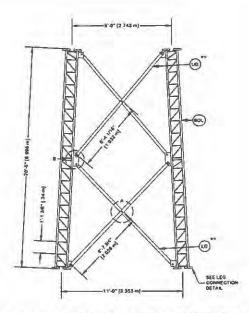
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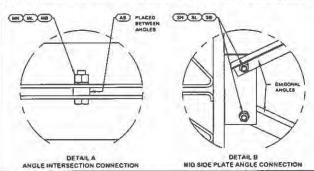
PAGE 12 OF 17

DRIENT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION ORIENT ANGLES WITH STAMPED END TOWARD TOP OF SECTION TO DIAGONAL ANGLES MUST BE INSTALLED WITH THE MON-BOLTED FACE UP.
THIS MAY BE ON THE OPPOSITE SIDE OF THE SIDE PLATE THAN WHAT IS SHOWN IN THE DETAIL.

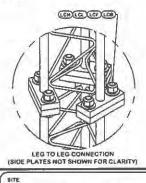
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DESCRIPTION OF REVISIONS



	-		PARTS LIST			
ITEM	DIX	PART NO	PART DESCRIPTION		UNIT WT	MET WT
BOL	3	194551	#12 LEG SECTION 1-1/2" LEG - 1/2" BRACE - 1"BOL		802 830	1808 480
LO		124801	U-12 UPPER DIAGONAL - 2 1/2" x 2 1/2" x 3/16" ANGL		42.250	253 500
AS	- 1 - 0	104291	RING FILL SPACER 172" THICK I SHE" HOLE		0.070	0 420
AMPA		312002	3M" 16 HOT DIPPED GALVANIZED NUT		0 190	1 140
ML	1 - 6	312185	3H" GALVANIZED LOCKWASHER		0 030	0,180
MS		180427	3M" 10 X 3" A-326T BOLT WITH FULL THREAD		0 470	2.020
8L	24	312223	1" GALVANIZED LOCKWASHER		6.080	1,920
SM .	24	312504	1"4 HOT DIFFED CALVANIZED HUT		9 430	10 320
86	24	172286	1-6 K 2-1H" A 325 BOLT WITH 1-3M" THREAD		0.940	20.160
UD.		126797	U-10 LOWER GIAGONAL 2 1/2" + 2 1/2" + 3/18" ANGL	- 35	40,670	240 420
LOB	18	222018	1-6 x 4-34" A-326 BOLT WITH 1-34" THREAD		1.280	24 840
LCF	18	312222	1- GALVANIZED FLAT WASHER (F438)	-	0 140	7 520
LCL	16	312223	1" GALVANIZED LOCKWASHER		9.080	9 440
LON	18	312804	1"4 HOT DIPPED GALVANIZED HUT		0.430	7.740
	_			Total Wil	2376 R1 No T107	9 68 601



US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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CPO BY DATE

SECTION U-11.0 (200' - 220' ELEVATION)

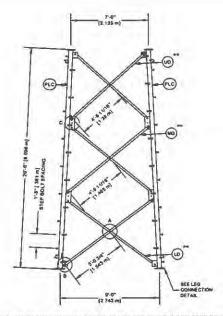
1-877-487-4763 Plymouth, IN 1-890-547-2151 Salem, OR

**STRUCTURES** 

REVISION HISTOR DWG NO. Proprietant note
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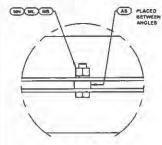
DRIENT LEGS WITH PIN STAMP TOWARD SOTTOM OF SECTION ORIENT ANGLES WITH STAMPED END TOWARD TOP OF SECTION

"DIADONAL ANGLES MUST BE INSTALLED WITH THE HON-BOLTED FACE UP. "] 1 THIS MAY BE ON THE OPPOSITE SIDE OF THE SIDE PLAYETHAN WHAT IS SHOWN IN THE DETAIL.



	1997		PARTS LIST			
ITEM	QTY	PART NO	PART DESCRIPTION		UNIT WT	NET WT
PLC	3	229377	PIPE LEG SECTION 20:0" (CLIMBING) 6" SCH. 40 V-SE		\$37.946	1613.820
519	45	226199	STEP BOLT ASSY SWE-11 K F WILDCK WASHER HEAVY		1.100	52.800
LO		284757	V-9 L QWER CLIPPED ANGLE - 2 1/2" + 2 1/2" + 3/16"		32.000	192.840
MB		227680	SHE'-11 X 2-1M" ASTET HOT DIPPED GALV BOLT (FULL		0 840	8.760
AS.		293154	RING FILL SPACER DIST THICK I GAS" HOLE		0 000	0 540
MM		312501	64T-11 HOT DIPPED GALVANIZED NUT		0 120	1 080
ML		312123	SAF GALVANIZED LOCKWASHER (53-22230)		0.020	0.180
3L	34	312183	3H" GALVANIZED - DCKWASHER		0 030	1 0400
SN.	36	312802	34" 10 HOT DIPPED GALVANIZED NUT		0 190	8.840
58	3-6	227526	3M 10 X 2-1M A-3261 BOLT WITH FULL THREAD		6 429	18 120
MO		284756	V-8 MIG ANGLE -2 1/2" x 2 1/2" x 3/14" ANGLE (AST		30 530	183 186
UD		204/16	A-9 OLLER WHOTE - 3 :15, = 5 1/2, = 31/4, WHOTE (V		28,900	179 400
LCS	100	172272	1"-8 X 4-1M" A-328 BOLT WITH 1-3M" THREAD		E 845	16 120
LCF	18	112222	1" GALVANIZED SLAT WASHER (F436)		0.140	2 520
LCL	18	312773	1 GALVANIZED LOCKWASHER		0.063	1.440
LCN	18	312604	1"4 HOT DIPPED GALVANIZED NUT		9.430	7 740
				Total Wt	2273 16 Ib (103	2 03 kg)

NOTE: THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY. THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW. PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.



DETAIL A ANGLE INTERSECTION CONNECTION

APPROVED BY

SAN

REV

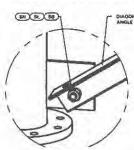
DRAWN BY

SAN

DESCRIPTION OF REVISIONS

REVISION HISTORY

DESIGNED BY



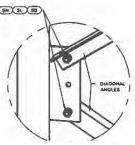
DETAIL B END SIDE PLATE ANGLE CONNECTION

APPROVED BY

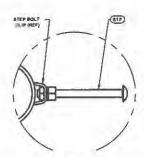
CPO BY DATE

RELEASE DATE

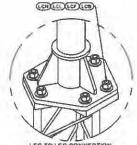
10/6/2022



DETAIL C MID SIDE PLATE ANGLE CONNECTION



STEP BOLT INSTALLATION



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)



SITE

US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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PROPRIETARY NOTE
THE BATA MAD TE OPHICAGE CONTAMED IN THE DRAWING ARE PROPRIETARY INFORMATION OF VALUDATE
THE BATA MAD TE OPHICAGE CONTAMES IN THAT BROWNERS AND VIETOR OF ONCO, OURSE INTHOUT THE CONSENT OF
VALUDATINE AND THE STREET A TRACE SECONT ANY VIETOR ONCO, OURSE INTHOUT THE CONSENT OF
VALUDATINE AND THE STREET A TRACE SECONT.

SECTION V-9.0 (220' - 240' ELEVATION)



1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR

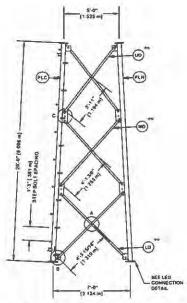
**STRUCTURES** 

ENG. FILE NO.

565090

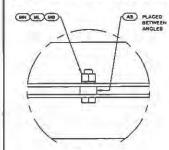
DWG. NO 293706T

ORIENT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION ORIENT ANGLES WITH STAMPED END TOWARD TOP OF SECTION "DIAGONAL ANGLES MUST BE INSTALLED WITH THE NON-BOLTED FACE UP. "] †
THIS MAY BE ON THE OPPOSITE SIDE OF THE SIDE PLATE THAN WHAT IS SHOWN IN THE DETAIL.

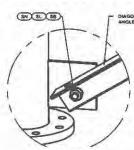


			PARTS LIST			
IVEN	QTY	PART NO.	PART DESCRIPTION		UHIT WT	NET WT
PLC	1	226200	PIPE LEG SECTION 20'4" (CLIMBING) 5" BCH. 40 V-SE		388.910	389 610
MUI	2	226201	MIPE LEG SECTION 20'-0" (NON-CLIMBING) IT SCH 40		386.250	772 400
STP	10.	226188	STEP BOLT ASSY ME'-11 X F WILDCK WASHER HEAVY		1 100	17 800
LO	- 8	284733	V-7 LOWER CLIPPED ANGLE - 2" x 2" x 3"16" ANGLE (A		21.810	129.000
148	1	227580	MET-11 X Z-1MT A376T HOT DIPPED GALV. BOLT (FULL		0 840	8.760
A\$		293154	RING FILL SPACER 38" THICK 1.048" HOLE		0 000	0 640
MAI		312501	5/8*-11 HOT DIPPED GALVANIZED NUT		0 120	1.080
ARL	9	312123	PM. CYTANYSED FOCKMYSHER (P2-55530)		0 020	9.100
8L	34	312163	3H" GALVANIZED LOCKWASHER		0 636	1.040
804	36	312502	34"-10 HOT DIPPED GALVANIZED HUT		0 180	4.840
88	34	227570	3MT-10 X 2-MT A-326T BOLT WITH FULL THREAD		0.420	15 130
MD		284732	V-7 MID ANGLE 2" = 2" + 3H 6" ANGLE JASTZ QR 60		20.440	122 640
UD	- 0	284731	V-7 UPPER ANGLE 2" 17" INT ANGLE (ASTE GR		19.370	116 229
LCS	24	227648	SM"-10 K 3-1/2" A-326T BOLT WITH FULL THREAD		0 640	12 860
LCF	24	312152	3M" GALVAMIZED FLAT WASHER (F430)		0.000	1.829
LGL	24	312153	34" GALVANIZED LOCKWASHER		0.030	0.720
LCH	24	312602	24" 18 HOT DIPPED GALVANIZED NUT		0.190	4 560
				Total Wt	1666 30 fb (725	60 40)

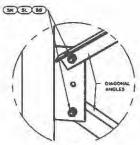
NOTE. THE VIEWS SHOWN BELOW ARE FOR PART IDENTIFICATION ONLY THE ACTUAL PART STYLE MAY VARY FROM WHAT IS DEPICTED BELOW PLEASE SEE ASSEMBLY INFORMATION IN THE UPPER LEFT CORNER FOR FURTHER INSTALLATION INSTRUCTIONS.



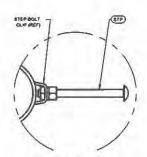




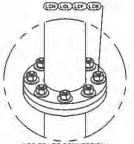
DETAIL B END SIDE PLATE ANGLE CONNECTION



DETAIL C MID SIDE PLATE ANGLE CONNECTION



STEP BOLT INSTALLATION



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)



SITE

US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290

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DESCRIPTION

SECTION V-7.0 (240' - 260' ELEVATION)



1-877-467-4763 Phymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

DESCRIPTION OF REVISIONS CPD BY DATE REVISION HISTORY

REV RELEASE DATE APPROVED BY DESIGNED BY APPROVED BY 10/6/2022 SAN SAN

PROPRIETARY MOTE
THE DATA AND TECHNOLOGIS CONTANIED IN THIS DRAINING ARE PROPRIETARY INFORMATION OF VALIDATE
THE DATA AND TECHNOLOGIS OF TRACE SECRET. ANY USE OF DISCLOSURE WITHOUT THE CONCENT OF
VALIDATE MODULITIES IS \$170°C To PROPRIETED.

ENG. FILE NO.

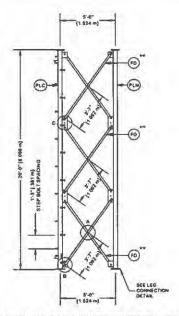
565090

293706T

DRIENT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION

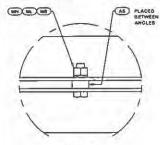
ORIENT ANGLES WITH STAMPED END TOWARD TOP OF SECTION

"DIAGONAL ANGLES MUST BE INSTALLED WITH THE NON-BOLTED FACE UP. "] †
THIS MAY BE ON THE OPPOSITE SIDE OF THE SIDE PLATE THAN WHAT IS SHOWN IN THE DETAIL.

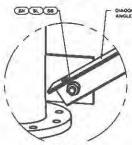


			PARTS LIST			
ITEM	OTY	PARTNO	PART DESCRIPTION		UNITWI	NETWY
PLC	1	226164	PIPE LEG SECTION 29'4" (CLIMBING) 4" BCH. 40 V-BE		302.000	302 000
PLN	2	226186	PIPE LEG SECTION 20"-0" (NON-CLIMBING) 4" 8CH. 40		284 870	869 340
STP	16	226199	STEP BOLT ASSY SIET-11 X 7" WI LOCK WASHER HEAVY		1 100	17 800
FO	18	286012	V-8 DIAGONAL ANGLE - 2 172" + 2 172" + 3198" ANGLE		24.300	437 400
tet.		312123	BIF GALVAINZED LOCKWASHER (85-22236)		8.020	0.180
AS.		116467	RING FILL SPACER IN-THICK I GHE DIA HOLE		0 250	2,280
688	9	227580	SIST-11 X 2-144" A325T HOT DIPPED GALV. BOLT (FULL		0 640	6.700
NIN	9	312501	SAT-11 HOT DIPPED GALVANIZED NUT		0.120	9.000
3L	34	312183	SH" GALVANIZED LOCKWASHER		0.030	1.000
8N	38	312602	3M"-10 HOT DIPPED GALVANIZED NUT		0.190	6,840
88	36	227579	3MT-16 X 3-1MT A-32ST BOLT WITH FULL THREAD		0 420	18 120
LCB	10	227800	34"-10 X 3-1/2" A-328T BOLT WITH FULL THREAD		0.540	1.720
LCF	18	312152	34" GALVANIZEO FLAT WASHER (F436)		0.060	1.440
LCL	18	312153	3M" GALVANIZED LOCKWASHER		0.030	0.540
LON	18	312802	2M-10 HOT SIPPED GALVANIZED NUT		0.190	3.420
				Yorkel Wit-	1373 88 lb [423	74 kg)

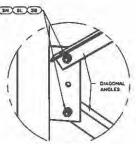
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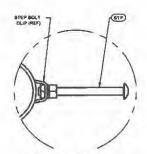
DETAIL A ANGLE INTERSECTION CONNECTION



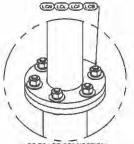
DETAIL B END SIDE PLATE ANGLE CONNECTION



DETAIL C MID SIDE PLATE ANGLE CONNECTION



STEP BOLT INSTALLATION



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)



SITE

US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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SECTION V-5.0 (260' - 280' ELEVATION)



1-877-487-4763 Plymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

ENG. FILE NO.

565090

DWG NO 293706T

16 OF 17

REV	DESCRIPTION OF REVISIONS	CPO	BY	DATE
	REVISION HISTORY			

APPROVED BY SAN SAN

DESIGNED BY

APPROVED BY

RELEASE DATE 10/6/2022

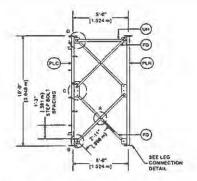
PROPRIETARY NOTE
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VALUEDIN THE GOTAL THE STREET, A TRICKING TECHNIQUES.

THE GOTAL THE GOTAL THE STREET, A TRICKING TECHNIQUES.

ORIENT LEGS WITH PIN STAMP TOWARD BOTTOM OF SECTION

INSTALL ANGLES WITH STAMPED END TOWARD TOP OF SECTION

"Diagonal angles must be installed with the non-bouted face up. To this may be on the opposite side of the side plate than what is shown in the detail.

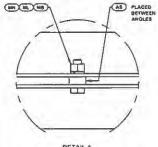


			PARTS LIST			
17EM	QTY	PART NO.	PART DESCRIPTION		UAIT WT	NET WT
PLC	- 1	226172	PIPE LEG BECTION 10'40" (CLIMBING) 2 1/2" BCH 40		108.330	196 330
PLN	2	226173	PIPE LEG BECTION 10'-0" (NON-CLIMBING) 2 1/2" SCH		106.650	213 300
210		226188	STEP BOLT ASSY SWITTE T WILDCK WASHER HEAVY		9 900	8 800
FD	12	286900	V-6 DIADONAL ANGLE - 2" x 2" x 1/8" ANGLE (AS72 GR		10.820	129 540
ML	- X	212123	SAP" GALVANIZED LOCKWASHER (\$3-22730)		0.020	0 120
A5	- 6	116467	RING FRE SPACER 14" THICK 1 B48" DIA HOLE		0 250	1.500
MB	- 6	227589	SIT-11 X 2-1H" A328T HOT DIPPED GALV BOLT (FULL		0.640	3.840
4000	4	312801	847-41 HOT DIPPED GALVANIZED NUT		0.129	0.720
\$L	24	312183	344" GALVANIZED LOCKWASHER		0.036	0.720
SN	24	212502	3H*46 HOT DIPPED GALVANIZED NUT		8.196	4.500
86	24	227676	3M"-10 X 2-1M" A-326T BOLT WITH FULL THREAD		0.420	10 000
UH	3	206974	V-5 HORIZONTAL ANGLE (TYPE 1) 2" ± 3" = 141" ANG		21,600	64 500
LCB	12	227868	34"-10 X 3-1/2" A-328T BOLT WITH FULL THREAD		9.540	8,480
LOF	12	312152	BA" GALVANIZED FLAT WASHER (F438)		0 000	0.940
rcr	12	312153	3M* GALVANIZED LOCKWASHER		0.030	0.340
LCN	12	312502	34"-10 HOT DIPPED GALVANIZED NUT		D 100	2.286
-				Total Wt	454 26 to 1252	fit to)

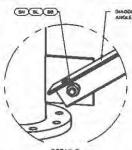
ANDREW

ROSS

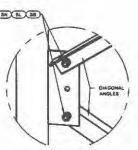
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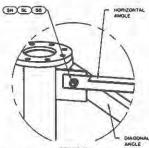




OFFAIL B END SIDE PLATE ANGLE CONNECTION

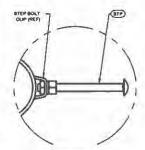


DETAIL C MID SIDE PLATE ANGLE CONNECTION

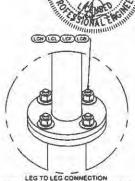


DETAIL D

UPPER HORIZONTAL ANGLE CONNECTION



STEP BOLT INSTALLATION



LEG TO LEG CONNECTION (SIDE PLATES NOT SHOWN FOR CLARITY)

SITE

US-KY-5135 FANCY FARM VB BTS II, LLC U 31 X 290'

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SECTION V-5.0 (280' - 290' ELEVATION)



1-877-467-4763 Plymouth, IN 1-800-547-2151 Salem, OR

**STRUCTURES** 

ENG. FILE NO.

565090

293706T

PAGE

REV DRAWN BY SAN

APPROVED BY SAN

DESIGNED BY

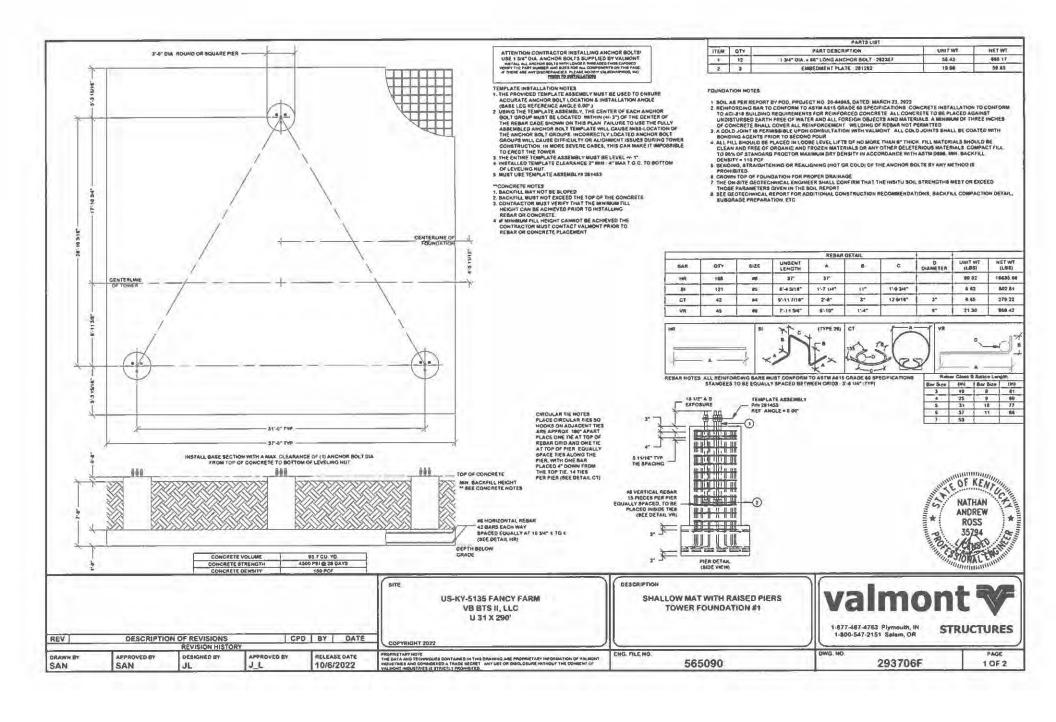
DESCRIPTION OF REVISIONS

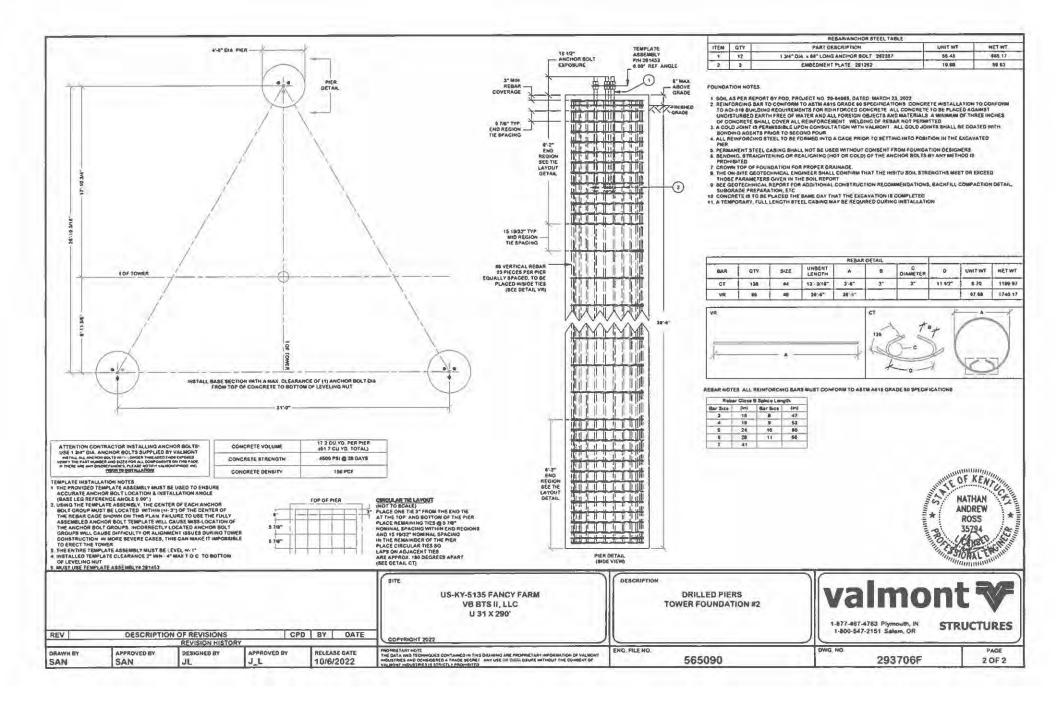
REVISION HISTORY

APPROVED BY

CPD BY OATE

RELEASE DATE 10/6/2022





### **UNIT BASE FOUNDATION SUMMARY**

## VB BTS II, LLC US-KY-5135 Fancy Farm

V- 31.0 A- 565090 290

V 4.5

undation Dimen	The same of the same of	-
rau wout, w.	37.50	ft
Depth, D:	7.00	ft
Ext. above grade, E:	0.50	ft
Pier diameter, d <sub>i</sub> :	3.00	ft
Pad thickness, T:	1.75	ft
Depth neglected, N:	7.00	ft
Volume, Vo:	95.66	cy

pad rebar qty., mp:	42	bers*
size, s p:	8	
pier vertical qty, m_c:	15	verticals/pier
size, <b>s</b> _c:	8	2.5' cage
pler tie qty., m_t	14	ties/pier
size, S ;	4	default hool

<sup>\*</sup>Reber to be equally speced, both ways, top & bottom, for a

Soil Information Per: POD, Project No. 20-64965, Dated: March 23, 2022

Soil unit weight, Y:	110	pcf
Ultimate Bearing, B <sub>c</sub>	7.000	ksf
Cohesion, Ca.	1,000	kel
Frietten angle: F	0.0	degree:
Ult. Passive P., Pp:	0.396	pcf
Base sliding, µ.	0.30	
Seismic Design Cat.:	D	
Water at:	none	ft

Anchor Steel Sele	ction	
Part Number, P/N:	262357	Dta = 1.75 Length = 60°

Material Properties	Material Properties					
Steel tensile str. Fy.	60000	psi				
Conc. Comp str. F'c.	4500	psi				
Conc. Density, δ:	150	pcf				
Clear cover, cc	3.00	in				

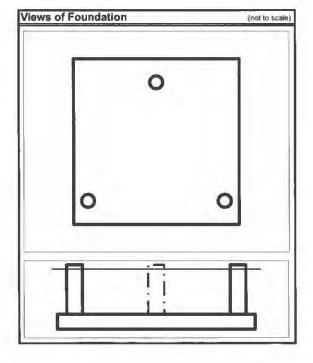
Backfill Compaction	วก	
Lift thickness:	8	In
Compaction:	95	%
Standard Proctor:	ASTM	D698

# Tower design conforms to the following:

- \* International Building Code (IBC)
- ANSI TIA-222-G
- \*Building Code Requirements for Reinforced Concrete (ACI 318-14)

Note: The centroid of the tower is offset from the centroid of the foundation

oundation Loadi	ng				
		strass rati	o: 98,6%	mark up.	1.4%
Shear (Per Leg), Si:	47.00	kips	x 1.01 =	47.66	kips
Shear (total), 5:	71.00	kips	x 1.01 =	71.99	kips
Moment, M:	12978.00	ft-kips	x 1.01 =	13159.69	ft-kips
Compression/Leg, C:	513.00	kips	x 1.01 =	520.18	kips
Uplift/Leg, U:	445.00	kips	x 1.01 =	451.23	kips
Tower Weight, W.:	90.00	kips	=	90.00	kips



# NATHAN ANDREW ROSS 35794 Digitally signed by Nathan A Ross Date: 2022-10-06 09:31-07:00

# Additional Notes:

- \* No foundation modifications listed.
- \* See attached "Foundation Notes" for further information.

# **FOUNDATION NOTES**

- 1 THE ON-SITE GEOTECHNICAL ENGINEER SHALL CONFIRM THAT THE INSITU SOIL STRENGTHS MEET OR EXCEED THOSE PARAMETERS GIVEN IN THE SOIL REPORT.
- 2 SEE GEOTECHNICAL REPORT FOR ADDITIONAL CONSTRUCTION RECOMMENDATIONS, BACKFILL COMPACTION DETAIL, SUBGRADE PREPARATION, ETC.

# UNIT BASE FOUNDATION (DL - 1.2) V- 31.0 A- 565090

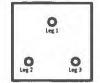
# VB BTS II, LLC US-KY-5135 Fancy Farm

290

V 4.7

Reactions stress ratio 98.6% mark up: 1,4% x 1.01 × 47.66 kips Shear (Per Leg), S; 47.00 kips Shear (total), S. 71.00 kips 71.99 kips x 1.01 = Moment, M: 12978.00 ft-kips × 1.01 \* 13159.69 ft-kips Compression / leg, C: 513.00 kips 520.18 kips x 1.01 = x 1.01 = 451.23 kips Uplat/leg, U. 445.00 kips

Soil per: POD, Project No. 20-64965, Dated: March 23, 2022



Uplift / log, U. 445.00 Tower weight, W <sub>2</sub> : 90.00				18		23
TOWN WASHE WY GO.OU	Kips 2 80.00 K	ps		Ung 2		O (et)
0.042 30.05						
Physical Parameters:		in the second second	440		150	
Concrete volume:	V = T * W2 + 3 * (di2 / 4 * 1	r)*(D+E-T)	V =	95.7	су	
Concrete weight.	W <sub>c</sub> = V * 8	***	W <sub>c</sub> =	387.4	kips	
Soil weight	W <sub>a</sub> = (D - T) * (Wz - 3 * (d)z	/4-π))-γ	W <sub>1</sub> =	799.9	kips	
Total weight	P = Wc + Ws + Wl		P=	1277.29	kips	
Passive Pressure:						
Pp coefficient:	$K_p = TAN(45 + \phi/2)^2$		K <sub>p</sub> =	1.000		
	Ppn = Kp * y * N + 2 * Co * 1	2017	P <sub>on</sub> =	2.770	ksf	
	$P_{pt} = Kp * y * (D - T) + 2 * C$		P <sub>pt</sub> =	2,578	ksf	
	Ppb = Kp * y * D + 2 * Co * 1		P <sub>pb</sub> =	2.770	ksf	
	P <sub>ptop</sub> = IF(N < (D - T), Ppt, Pp	n)	P <sub>picp</sub> =	2.8	ksf	
	Pp' = (Pptop + Ppb) / 2		Pp' =	2.770	ksf	
Shear area.	T <sub>pp</sub> = 0		T <sub>pp</sub> =	0.0	ft	
	App = Tpp * W		A <sub>pp</sub> =	0.00	ft <sup>2</sup>	
Shear Capacity:	$S_{achie} = (Pp' * App + \mu * P) * \varphi$		S <sub>ached</sub> =	287.391	kips	
φr ≈ 0.75	Check	S <sub>actual</sub> = 287,39 kips	>= S=	71.99	kips	OK
Overturning Moment Resistance	and Too.					
W1 of soil wedge:	W = D * (D * TAN(q)) /2*	W · v	W <sub>mv</sub> =	0.0	kips	
Dist. from leg to edge:	O = (W - 0.866 ° W) / 2	- 1	0=	5.327	ft	
Additional offset of WL	O. = W/2 - (1/3 ° 0.866 °	w+0)	0,=	4.474	ft	
Resisting moments	M <sub>ref</sub> = P * W / 2 - Wl * Oa		M <sub>red</sub> =	23546.57	ft-kips	
	M <sub>m</sub> = Pp' ' App ' (D - N) / 3		M <sub>ro</sub> =	0.00	ft-kips	
	Mrow = Wsw * (W + D * TAN(		M <sub>raw</sub> =	0.00	ft-kips	
Total resisting:	M <sub>rt</sub> = (Mrwt + Mrp + Mrsw)	11	M <sub>r1</sub> =	17659.92	ft-kips	
φr = 0.75	ing - (mar , mp , mon)	Ψ'	11/41	11000,02	пчоро	
Total overturning	Mo = M + S * (D + E)		M <sub>o</sub> =	13699.65	ft-kips	
	Check	M <sub>n</sub> = 17659.92 ft-kips	>= M <sub>o</sub> =	13699.65	ft-kips	OK
Bearing Resistance due to Press	sure Distribution					
Area of met	area = W²		area =	1406.3	R2	
Section modulus	SM = W3 / 6		SM =	8789.1	R <sup>3</sup>	
Fectored total weight	P' = (W(/1.2 + Wc + Ws)	*1.2	P'=	1514.8	kip	
Pressure exerted:	P <sub>soa</sub> = P' / area + Mo / SM		P <sub>mer</sub> =	2.636	ksf	
	Pneg = P' / area - Mo / SM		P <sub>min</sub> =	-0.482	ksf	
Note: The	stress resultant is NOT within the	kern. Bearing area has been ad	-			
Load accentricity:	e <sub>c</sub> = Mo / P'		e <sub>0</sub> =	9.04	A	
in Parallel Direction	Pad = 2 * P' / (3 * W * (W / 2	- ec))	P <sub>ed</sub> =	2.8	ksf	
in Diagonal Direction	Pad_day see Diegonal Bearing Sheet (		Pad day =	3.7	ksf	
Adj. applied pressure:	q <sub>a</sub> = iF(Pneg >= 0, Ppos, F		q. =	2.775	ksf	_
Overburden Pressure: (fectored)	q <sub>obe</sub> = NA- Gross Bearing Pr	20 Y	Q <sub>obe</sub> =	0.000	ksf	
gy = 0.75		qa - q <sub>obp</sub> = 2.775 ksf	<= B <sub>c</sub> * φr =	5.250	ksf	OK
Concrete Shear Strength: Une way beam action at d , from tower						
Effective depth:	d <sub>c</sub> = T -cc -db_p/2		d <sub>e</sub> =	17.500	in	
Distance from edge of pad to per face	d' = 0 - di / 2		ď =	3.827	n	
Chillance from edge of ped to do	d" = d' - dc		d* =	2.389	ft	
Bearing Pressure Slop	q, = qa / Weff		q,=	0.095	kcf	
Required shear:		/2 )]"d""W - [1.2"(D - T)"Y"d""W]	V <sub>a1</sub> =	174.87	klps	
Available shear	V <sub>c1</sub> = 0s * 2 * \( \( \frac{1}{2} \) \( \frac{1} \) \( \frac{1} \) \( \frac{1}{2} \) \( \frac{1}{2} \		V <sub>c1</sub> =	792.41	kips	
[ACI 22 5 5 1] φs = 0.76 [ACI		-	▼c1 <sup>-</sup>	192.41	nipo	
	Check	V <sub>c1</sub> = 792.41 kips	>= V <sub>n1</sub> =	174.87	kips	OK

Eq. S	Quare Column (ACI 8.10.1.3 &	d add to d						dan-	21.00	in	
	22.6.4.1.2) Mat effective width in bearing	d <sub>eq</sub> =di / 2 ° vt W <sub>eff</sub> = Min (W,		11				deq =	31.90 29.12	in R	
-		The second second second second	The second second						1,00	R	
Hane o	I long side to short side of Pier  Length	$b_1 = dc/2 + \epsilon$	lane or round					β = b <sub>1</sub> =	63.70	in	
	Vidin:	b <sub>2</sub> = (dc + de	STATE OF THE PARTY OF	THE RESERVE				b <sub>2</sub> =	88.62	in	
Critical	Perimeter:	b <sub>0</sub> = b1 + b2	g . se - on to	0) WIJI Z				b <sub>o</sub> =	152.32	in	
Section			/ . / / . / /	2. 1 62 0	of a l			C=	13.320	ft	
	Centrold  Eccentricity:	e <sub>c</sub> = (deq + d	b1/2)/(b1°	00+02	uc)		_		13.320		
	Poler MOI	J <sub>c</sub> = [(dc * b1	And the second second	* dens / 12	) + (h1 * /	le * (h1 /2 - r	1A21 + (h1		1.063E+06		
Moment					7 (0)	o inita	1) 2] . (0)				
Fraction ansierred	flexure: eccentricity	Y <sub>1</sub> = 1 / (1 + 2	13°4 (b1/b	2))				٧,=	0.64		
by by	of shear	$y_v = 1 - yf$						Y <sub>v</sub> =	0.36		
Bearing I	Pressure Slope.	q <sub>e</sub> = qa / Wef	f					q <sub>s</sub> =	0.095	kcf	
verage Be	saring Pressure:	Qapt = ((Weff -	01) 1 qs + qa)	12				Qapi=	2.522	ksf	
Shear F	orce at Section:	V <sub>n pw</sub> 0						V <sub>n_pier</sub> =	421.326	kips	
	Slab Moment	Mec = SI * (D -	T + E) + Vn_p	ier * e				M <sub>ac</sub> =	673.66	ft-kips	
	Required shear gs = 0.75	ACI 21 Z II = (Vn. pier	/ b0 * dc) + (v	v * Msc* o	/ Jc)				194.62	psi	
		C(22.6.5.2) = gs * MII	Name and Address of the Owner, where the Parket of the Owner, where the Parket of the Owner, where the Owner, which is the Owner, where the Owner, which is the Owner, which		THE RESERVE OF THE PARTY OF THE	2+(as*dc/bo))	*A* \(Fc))		201.246	psi	
			Check		201.25		>=	V <sub>n2</sub> =	194.62	psi	OK
Mor	ment transfered:	W. saanna		-						***************************************	
2.2	(Pier 1)	M <sub>n1</sub> = yf * Msc	F 6 T 4 8 8 10 2	CAT DA		News	2.1	M <sub>nt</sub> =	311.643	ft-kips	
E Mectr	ve Beam Width:	W <sub>eff1</sub> = deq + 1.		5 · ι , (W	- w 511	(ton) - aed )	2)	W <sub>eff1</sub> =	7.909	ft in <sup>2</sup>	-
		A <sub>M_P1</sub> ' = Mn1 / (0		undf41				Adpi =	3.957		
-	ALL STREET	$a_{p1} = Ast_p1'$ $A_{st_p,st_p} = Mn1/(F$	the second second second second	-				a <sub>p1</sub> =	0.674	in in <sup>2</sup>	
	Required steel:			2))				A <sub>60,p,611</sub> =	3.632	in <sup>2</sup>	
	nel in entire mat: ment transfered:	Aup_mi = Asi_p_si	I THENY ( VV I					A <sub>sl_p_ste1</sub> =	17.219	***	
	(Pier 2 or 3)	M <sub>n2</sub> = yf * Msc	(Controlling t	Case: Con	ner.)			M <sub>n2</sub> =	430.392	ft-kips	
Effective	ve Beam Width	w <sub>err2</sub> = deq + 1.	5 * T + MIN( 1	5 * T . (W	- w/ - dec	1)/2)		W <sub>eff2</sub> =	7.204	ft	
		A N DZ' = Mn2/(0	9 * Fy * dc)					A <sub>et,02</sub> ' =	5.465	in <sup>2</sup>	
		a <sub>p2</sub> = Asl_p2	Fy/(β ' F'c'	weff2)				a <sub>p2</sub> =	1.022	in	
	Required steet:	A <sub>N_P_M2</sub> = Mn2 / (F	y * (dc - ap2 /	2))				A	5.067	in	
equired ste	set in entre mat:	An_p_st = Ast_p_st	2 * W / weff2					A <sub>82_P_8862</sub> =	26.373	in <sup>a</sup>	
								Pier Con	trolling Case		Pier 2 Corn
wo way b	neam action at d , / 2 from low										
	inforcement Dia	di <sub>1</sub> =di -2*cc-2	"db_1 - 1"db_	C				di <sub>y</sub> =	28.000	in	
Eq. S	Equare Column (ACI 8.10.1.3 & 22.6.4.1.2)	d <sub>eq T</sub> =dprebar /	2 * √π					d <sub>eq T</sub> =	24.81	in	
Critical	Section Length:	b1 T = deq T +	dc					b <sub>i T</sub> .	42.314	in	
Critical Se	ction Perimeter:	b <sub>o_T</sub> = 4 * (deq	+ dc)					b <sub>e T</sub> .	169.26	in	
	Polar MOI	Je T = (b1_T^3	and the latest and th	T*d*3/	6)+(dc * 8	1 T . b2 TA	2/2)		921710.251	in <sup>4</sup>	
Shear F	Force at Section:	V <sub>n.pres_T</sub> = U	and the second second					V <sub>n_per_T</sub> =	451.23	kips	
	Required shear as = 0.75	Contract Value of the last of	T/b1 T*de	1 + (vv * h	Asc* c T/	Jc T)			182.533	psi	
		C/ 22.6.5.2] ' = qs * MI					)*λ*√(Fc))		201.25	psi	
	11.510.00 1.05.00		Check		201.25		>2	V <sub>nt2</sub> =	182.53	psi	OK
olumn	Compression Capacity:		4								
	ression reaction:	Pc = qc * 0.8	5 ° F'c * (di² / 4	( *π)				P <sub>c</sub> =	2530.7	kips	
	pc = 0.65 [ACI 21.2 2.2]		Project								
			Check	P. =	2530.69	kips	>=	C=	520.18	kips	OK
ler Rain	nforcement;										
	s-sectional area:	A = di2 * 11/	4					A <sub>o</sub> =	1017.88	in	
2030	se of steel (pier):	A <sub>N c</sub> =Ag * 0.01						A <sub>st_c</sub> =	10.18	in	
	[ACI 10.6.1.1] & [ACI 10.3.1.2]							M_C			
	Caga circle:	d <sub>o</sub> = di - 2 ° c	c - db_c - 2 *	db_t				d <sub>e</sub> ≖	28.00	in	
	Reber:	s_c = 8				d <sub>b_c</sub> =	1	in			
		m_c = 15				A <sub>0_c</sub> =	0.79	in <sup>2</sup>		- (1)	
		A <sub>a_c</sub> = Ab_c * n	1_0					A <sub>s,c</sub> =	11.85	in <sup>2</sup>	
			Check	As c =	11.85	in <sup>2</sup>	>=	A <sub>t,s</sub> =	10.18	in²	OK
	Actual moment:	M <sub>max</sub> = (D - T +	E)*S/2					M <sub>rmin</sub> =	206.98	ft-kips	
	noment capacity:	M <sub>aton</sub> per Maxmo	rent ids (see altec	hed)				M <sub>atom</sub> =	212.41	ft-kips	
	Annorth Capacity.			-				6.4	-	-	
	tonion a vapatory.		Check	Malow =	212.41	ft-kips	>=	M <sub>ross</sub> =	206.98	ft-kips	OK
	Bar separation:	B <sub>e_c</sub> = (do * π)		M <sub>allow</sub> =	212.41	ft-kips	>¤	M <sub>max</sub> =	206.98 4.86	ft-kips in	OK

Reinforcement location: [ACI 25.4.2.4]	Ψ <sub>L</sub> <sub>c</sub> = if the space	e under the r	ebar > 12 in	, use 1.3	, else use	1.0	Ψ_=	1.3		
Epoxy coating	Ψ <sub>a c</sub> = if epoxy-c	pated bars are	e not used.	use 1.0:	but if eoox	v-coaled	ψ <sub>α,ε</sub> =	1.0	_	
[ACI 25 4 2 4]	1.00	sed, then if B					70			
Max term: (ACI 25.4.2.4)	Ψ <sub>1</sub> Ψ <sub>1_s</sub> = the produc	ct of wt & we,	need not be	taken la	rger than	1.7	ψιψος =	1,3		
Reinforcement size: [ACI 25.4.2.4]	$\psi_{i\_0}$ = if the bar :	size is 6 or les	s, then use	0.8, else	use 1.0		ψ, =	1		
Light weight concrete: [ACI 25,4,2,4]	λ <sub>c</sub> = if lightwier	ght concrete is	s used, 0.75	, else us	e 1.0		λ = =	1.0	J	
Specing/cover- [ACI 25.4 2.4]	C <sub>c</sub> the smalle	r of: half the l	bar spacing	or the co	ncrete ede	e distace	c_ =	2.93	in	
Transverse bars: [ACI 25.4.2.3]	k <sub>w_c</sub> = 0 in (p	er simplificati	an)				k <sub>e_c</sub> =	0	in	
Max term [ACI 25.4.2.3]	c_s' = MIN( 2.5	5. (c_c + ktr_c	) / db_c)				c_c' =	2.500	1	
Excess reinforcement: [ACI 25.4.10.1]	R <sub>c</sub> = 1		(axt	cess reinfo	ndber Inemed	tion is not used)	R_# =	1.00		
Development (tensile) (ACI 25.4.2.2)	La' = (3 / 40) *	(Fy / 1 _c - √(F	.ci) . (Athe-	c " ws_c	R_c / c_c)	, qp_c	L <sub>d</sub> '_, =	34.88	in.	
Minimum length: [ACI 25,4.2,1]	L <sub>d_min</sub> = 12 inche	35					L <sub>d_min</sub> =	12.0	in	
Development length	Late = MAX( Le	min, Ldt' a	)				Lac=	34.88	in	
Confining Reinforcement [ACI 25.4.9.3]	$\psi_{c,c} = 1$	7					$\psi_{t,s} =$	1.00		
Development (comp.). [ACI 26.4.9.2]	L <sub>00</sub> '_0 = Fy 'ψι_	c * db_c * R_	c/(50 * λ_c	* v(F'c)			Loc'c =	17.89	ln	
	L <sub>de</sub> " = 0.0003	db_c Fy t	pr_c * R_c				Lac** =	18.00	in	
Development length:	Lac = MAX( 8.	Ldc'_c, Ldc"	c)				Lac =	18.00	in	
Length available in pier:	L,c = D - T + 1	E - cc					L <sub>vc</sub> =	66.0	in	
		Check	L <sub>ic</sub> =	66.0	in	>=	Lac =	34.9	in	OK
		Check	L <sub>vc</sub> =	86.0	in	200	Lace	18.0	in	OK
Length available in pad	Lp = T - cc						L <sub>vp</sub> =	18.0	in	
		Check	L <sub>10</sub> =	18.0	in	>=	Lanc =	34.9	in	HOOK
		Check	L <sub>sp</sub> =	18.0	in	>=	Lac.o =	18.0	in	OK
artical Rebar Hook Ending:										
Ser size & clear cover: [ACI 25.4.3.2]	Ψ <sub>L</sub> h = if the bar :	size <= 11 an	d side cc >=	2.5", us	e 0.7, else	use 1.0	Ψ(,, =	0.7		
Epoxy coating: [ACI 25.4.3.1]	Ψ <sub>e_h</sub> = if epoxy-c	oated bars ar	e used, use	1.2, else	use 1.0		Ψ•_h =	1.0		
Light weight concrete: [ACI 25.4.3.1]	A <sub>h</sub> if lightwie	ght concrete i	s used, 0.75	i, else us	e 1.0		A <sub>jh</sub> =	1.0		
Confining Reinforcement. [ACI 25.4 3.2]	Ψ <sub>r_h</sub> = 1						$\psi_{\tau,j_0} =$	1.00		
Development (hook): [ACI 25.4 3.1]	Lan' = (Fy * wt	_h	r_h * R_c/	(50 · A_F	• \(F'c)))	db_c	Lan' =	12.5	in	
Minimum length: [ACI 25.4.3.1]	L <sub>dh_min</sub> the larger	of: 8 * db or t	3 in				L <sub>dh_min</sub> ×	8.0	in	
Development length:	Lah = MAX( L	-	The state of the s				Lan=	12.5	in	
		Check	L <sub>vp</sub> =	18.0	in	>=	L <sub>oh</sub> =	12.5	in	ОК
Hook fail length:	La_tel 12 " db be	yond the ben	d radius				Ln_w=	16.0	in	
Length available in pad	Lh.pad = (W - W	- di) / 2					Lh_past =	21	in	
		Check	Lh pag =	21.0	in	>=	Langual =	16.0	in	ОК

des per Anchor B	ismic zone is  ive seismic zo max1, Bs_t) / Bs_t_max  Check	ones, else _max2, Bs	18*	Bs_t_n	d <sub>b,1</sub> = A <sub>b,1</sub> = max4)	0.5	Signed = z = in in²  Ball maxi = Ball maxi = Ball maxi = Ball maxi = max	1 8 12 9 12 8 10.6 10.6	in in in in	C
1 = 4 1 = 14 1 = 14  Ameri = 8 * db_c  Ameri = 24 * db_t  Ameri = 12* in acti  Ameri = 12* in acti  Ameri = (0 - T + E  Ameri = 262357  Ameri = 1.75  Ameri = 267257  Ameri =	ive seismic zo L_max1, Bs_t_ ) / Bs_t_max Check in	nes, else _max2, Bs + 2	18° _t_max3,	Bs_t_n	max4)	0.2 >=	in $\ln^2$ $B_{a,t,max} = B_{a,t,max} = B_{a,t,max} = B_{a,t,max} = B_{a,t,max} = m_{a,t,max} = m_{a,t,t,max} = m_{a,t,t,max} = m_{a,t,t,t,max} = m_{a,t,t,t,t,t,t,t,t,t,t,t,t,t,t,t,t,t,t,t$	8 12 9 12 8 10.6	in in	r i
1 = 14  ment = 8 * db_c  ment = 24 * db_t  ment = 12* in acti  mos = MIN( Bs_ min = (O - T + E  1.75    mathrid = 262357  mathrid = 1.75    mathrid = 267257  mathrid = 267257	L_max1, Bs_t_ )/Bs_t_max Check	_max2, Bs + 2	t_max3,	Bs_t_r	max4)	0.2 >=	$in^2$ $B_{a,t,max} = B_{a,t,max} = B_{a,t,max} = B_{a,t,max} = B_{a,t,max} = B_{a,t,max} = M_{a,t,max} = M_{a,t,$	12 9 12 8 10.6	in in	ris control
ment = 8 * db_c  = 24 * db_t  = di / 4  ment = 12* in acti  max = MIN( Bs_ min = (D - T + E  = 262357  = 1.75  per Anchor B:	L_max1, Bs_t_ )/Bs_t_max Check	_max2, Bs + 2	t_max3,	Bs_t_r	max4)	>=	$B_{s\_t\_max} = B_{s\_t\_max} = B_{s\_t\_max} = B_{s\_t\_max} = m_{t\_max} = m_{t\_max$	12 9 12 8 10.6	in in	ris control
maid = 24 * db_t  maid = di / 4  maid = 12* in acti  main = (D - T + E  [ ]  main = 262357  main = 1.75  main = 267357  main = 267357	L_max1, Bs_t_ )/Bs_t_max Check	_max2, Bs + 2	t_max3,				$B_{s\_t\_max} = B_{s\_t\_max} = B_{s\_t\_max} = B_{s\_t\_max} = m_{t\_max} = m_{t\_max$	12 9 12 8 10.6	in in	Pi
med = di / 4  med = 12" in acti  mes = MIN( Bs_ min = (D - T + E  = 262357 = 1.75  per Anchor B:	L_max1, Bs_t_ )/Bs_t_max Check	_max2, Bs + 2	t_max3,				$B_{a,l,max} =$ $B_{a,l,max} =$ $B_{a,l,max} =$ $m_{a,l,max} =$	9 12 8 10.6	in in	
main = 12" in acti  main = MIN( Bs_ min = (D - T + E  (D - T + E  1.75   min = 1.75    main = 262357  main = 1.75    main = 262357	L_max1, Bs_t_ )/Bs_t_max Check	_max2, Bs + 2	t_max3,				B <sub>e_1 max</sub> =  B <sub>e_1 max</sub> =  m <sub>s_min</sub> =	12 8 10.6	in	
min = (D - T + E min = (D - T + E 262357 = 1.75	L_max1, Bs_t_ )/Bs_t_max Check	_max2, Bs + 2	t_max3,				B <sub>e 1 max</sub> =	8 10.6		Die
(D - T + E	) / Bs_t_max Check	+2					m <sub>1,min</sub> ≖	10.6	in	Ce
= 262357 = 1.75	Check in		14.0		L_=					C
= 1.75	in				=		Cine			
= 1.75	***				L =	57				
= 1.75	***				=					
des per Anchor B	***				E. =	66 55.50	in io			
		41)				20.00	L <sub>ton</sub> =	45.63	in	
man David Africand M	olts (see attached						Loss, ren	34.88	in	
inel borunano	Check	L <sub>des</sub> =	45.63	in		>=	L <sub>dea_max</sub> =	34.88	in	01
mes =D+E - pc	-						E <sub>se rese</sub> =	85	in	-
	Check	E. =	55.50	in		-CH		85.00	in	Ol
@ =D + F.T						-	-	Loan total		
2-2-1		+ 6 in	>=		E <sub>m</sub> =	55.50	in or	≪=	72 in	OI
min per ancstsel	ds (see attached	)					d <sub>o mm</sub> =	25.25	in	
	Check	d <sub>0</sub> =	28.00	in	-	>=	d <sub>o min</sub> =	25.25	in	0)
1111				111	1					
							B=	37.5	ft	
								0.000		
L1_2 =W							B <sub>11,2</sub> =	37,5	ft	
The second second							S <sub>11.2</sub> =		1767	
L1_2 =(W - W) /	2						21_2"	3.25	ft	
L1_2 =(W - w) / : R1_2 =SL1_2 + w\	2						S <sub>R1_2</sub> =	3.25	R	
	Moment (A.*	Wps)	1000							(ft*kips)
RI_2 = S <sub>LI_2</sub> + w\		wps)	1000	,		_	S <sub>R1_2</sub> =		ħ	[ft*kips]
RI_2 = S <sub>LI_2</sub> + w\		W(ps)				<u></u>	S <sub>R1_2</sub> =		ħ	[ft*kips]
RI_2 = S <sub>LI_2</sub> + w\		%(ps)		/		_	S <sub>R1_2</sub> =		ħ	[ft*kips]
R() = S() 1 + wh			500	5.00	0 10	0.00 11	S <sub>R1_2</sub> =	34.25	ħ	
R() = S() 1 + wh	Moment (%		500	5.00	00 10	9.00 11	S <sub>R1.2</sub> = Direction 2	34.25	Moment	
R1, = S1, 2 + WA  11  00 25.00 3  m1, = M <sub>mext_5</sub>	Moment (%		500	5.00	0 10	9.00 19	Sa1_2= Direction 2  5.00 20.00  M_ma1_1=	25.00	Moment	
R <sub>1,2</sub> = S <sub>1,2</sub> + wA 1	Moment (%		500	5.00	0 10	9.00 15	Sa1.2= Direction 2	25.00 : 844.77	Moment  Moment  1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
Relative School   1	Moment (h*	40.00	-500 -500 -1000 -0.00	5.00	0 10	.00 11	Sa1_2= Direction 2  5.00 20.00  M <sub>max1_2</sub> = M <sub>max1_2</sub> =	25.00 : 844.77 784.43	Moment  Moment  Associated as the second sec	
	Q = D + E - T	D + E - T + cc  Check 72  The per ancisted at (see affactive Check  Check  A  A  A  A  A  A  A  A  A  A  A  A  A	Check E <sub>so</sub> =  @ = D + E - T + cc Check 72 + 6 in  pres per ancisser its (see siturched) Check d <sub>o</sub> =  W <sub>1</sub> W <sub>1</sub> W <sub>1</sub> W <sub>1</sub> W <sub>1</sub> W <sub>1</sub> W <sub>2</sub> W <sub>3</sub> W <sub>4</sub>	Check E <sub>m</sub> = 55.50  © = D + E - T + cc  Check 72 + 6 in >=  The control of the co	Check E <sub>m</sub> = 55.50 in  (a) = D + E - T + cc    Check 72 + 6 in   >=     Dren per anosterious (see attached)   Check   d <sub>0</sub> = 28.00 in   Check   d <sub>0</sub> = 28.00 in   Check   d <sub>0</sub> = 28.00 in   Check   d <sub>0</sub> = 28.00 in	Check E <sub>m</sub> = 55.50 in	Check E <sub>m</sub> = 55.50 in <=   @ = D + E - T + cc  Check 72 + 6 in >= E <sub>m</sub> = 55.50  pren per anosterio de (see effected)  Check d <sub>0</sub> = 28.00 in >=   "1  "1  "1  "1  "1  "1  "1  "1  "1  "	Check   E <sub>ss</sub> = 55.50 in   <=   E <sub>ss_m</sub> =   E <sub>ss_m</sub> =       @ = D + E - T + cc	Check   E <sub>ss</sub> = 55.50 in   <u e<sub=""  ="">ss,max = 85.00                                    </u>	Check E <sub>m</sub> = 55.50 in

d Reinforcement:										
	b = #(Fc <=	4000, 0.86, IF(Fc	= 8000, 0.65	0.85 - (Fc	4000) * 0.05))		b =	0.825		
Effective width:	W <sub>e</sub> = W						W. =	37,500	ft	
	A <sub>et_p</sub> ' = Mn / (0	1.9 * Fy * dc)					Aug =	29.260	in <sup>2</sup>	
	ap = Asl_p	* Fy / (B * Fc *	We)				a <sub>p</sub> =	1.05	in	
Required steel:	Aup = Mn/(F	y * (dc - ap / 2)	) * (W / We	9)			Aug at "	27,149	in <sup>2</sup>	
Shrinkage:	ran = IF(Fy	= 60000, 0.00°	18, 0.002)				r <sub>sh</sub> =	0.0018		
	And put = psh " \	N°T/2					A <sub>st p sh</sub> =	8.505	in	
	An - MAX(A	st p st. Ast p	sh. Ast p	ste1, Ast	p ste2)		A <sub>st p</sub> =	27.149	in <sup>2</sup>	
Reber	5 = 8	Equally spaced			d <sub>0 0</sub> =	1	in			
10000		battom, both die			A 0 =		in <sup>2</sup>			
	A <sub>n p</sub> = Ab p		V122/1/20		n'h	-	A, p =	33.18	in²	
	مريدا وروب	Check	Aw	33.18	in	>=	A <sub>N p</sub> =	27.15	in <sup>2</sup>	OK
S	B - IN 2	*cc - db_p) / (r	- 10				B <sub>s p</sub> =	9.80	in	UIN
Bar separation	Dep - 144 - 2	Check	17	>=	8, p =	9.80	in	>=	4"	DK
		Check	- 11		U <sub>s.p</sub> ~	9.00	IB .	-	-	Un
d Development Length:										
Reinforcement tocation:	Ψ <sub>Lo</sub> = if the spa	ace under the re	abar > 12 ii	n, use 1.3	, else use 1.	0	= بψ	1.3		
[AC125.42.4]										
Epoxy coeting	Ψ <sub>n,p</sub> = if epoxy-	coated bars are	not used,	use 1.0:	out if epoxy-	coated	Ψ <sub>0_0</sub> =	1.0		
[ACI 25 4.2.4]	bars are	used, then if B	s < 6 * db c	or oc < 3 '	db. use 1.5,	else 1.2				
Max ferm: [ACI 25.4 2.4]	Ψ <sub>t</sub> ψ <sub>e,p</sub> = the prod	uct of wt & we.	need not b	e taken la	rger than 1.7	7	ΨιΨ•_0 =	1,3		
Reinforcement size: [ACI 25 4.2.4]	$\psi_{i,j} = \text{if the ba}$	r size is 6 or les	s, then use	0.8, else	use 1.0		ψ,_p =	1		
Light weight concrete: (ACI 25.4.2.4)	الله = if lightwi	eght concrete is	used, 0.7	5, else us	e 1.0		λ_p =	1.0		
Spacing/bover. (ACI 25.4.2.4)	c_, the small	ller of: half the I	par spacing	or the co	ncrete edge	distace	a رو	3.50	in	
Transverse bars: [ACI 25.4.2.3]	k <sub>trup</sub> = 0 in	(per simplificati	on)				k <sub>u_p</sub> =	0	in	
Max term: (AC) 25.4 2.31	c_p' = MIN( 2	2.5, (c_p + ktr_p	)/db_p)				c^, =	2.500		
Excess reinforcement: [ACI 25.4 10.1]	R <sub>p</sub> = 1		(0)	roess reinfor	cement réductio	n is not use	d) R_p =	1.00		
Development (tensile). (ACI 25 4.2.2)	L <sub>d</sub> = (3 / 40	) * (Fy / λ_p * √	(F'c)) * ψtų	e_ρ • ψs	p°R_p°db	_p/c_p'	_u L <sub>op</sub> '=	34.9	in	
Minimum length:	L <sub>d min</sub> = 12 inc	hes					Le min s	12.0	in	
[ACI 25 4.2.1]	-0_nan - 12 110							74.10	7.4	
Development length.	L <sub>do</sub> = MAX(	Ld min, Ldp')					L <sub>do</sub> =	34.9	in	
Length available in ped:	Load = (W/2						Load =	36.0	in	
	pan (177. m	Check		36.00	in	>=	L <sub>to</sub> =	34.88	in	OK

VB BTS II, LLC US-KY-5135 Fanc	y Farm				V- 31.0 A- 565090	290			¥ 4.7
Reactions	stress ratio 98.6	% mark up	1.4%						
Shear (Per Leg), S <sub>i</sub> :	47.00 kips x 1 fl		os	Soil per: POD, P	roject No. 20-649	965,			
Shear (total). S:	71.00 kips x 1.0			Dated:	March 23, 2022				
	2978.00 ft-kips x 1.0	1 = 13159.69 ft-	kips					0	
Compression / leg. C.	513.00 kips x 1.0	1 = 520.18 kij	ps					Leg 1	
Uplift / leg. U	445.00 kips x 1.0	1 = 451.23 kij	ps						
Tower weight, W <sub>I</sub> :	90.00 kips	* 90.00 ki	ps				Logi		O leg3
hysical Parameters:									
Concrete volume:	V = T * 1	V2 + 3 * (di2 / 4 * π	) * (D + E -	T)		V =	95.7	су	
Concrete weight.	Wc = V .					W <sub>c</sub> =	387.4	kips	
Soil weight	W <sub>e</sub> = (D -	T) * (W* - 3 * (di² /	/4°π))°γ			W, =	799.9	kips	
Total weight:	P = Wc	Ws + Wt				P=	1277.29	kips	
assive Pressure:							4 444		
Pp coefficient:	107,01400	(45 + \$ 12) <sup>3</sup>	lare-s			K <sub>p</sub> =	1.000	10.7	
		Y "N+2"Co"V	Control of the local division in the local d			P <sub>pn</sub> ≠	2.770	ksf	
		Y* (D-T) + 2 * C				P <sub>00</sub> =	2.578	ksf ksf	
	The second secon	y "D+2"Co" ν <(D-T), Ppt, Ppt				P <sub>ptop</sub> =	2.770	ksf	
		op + Ppb) / 2	m)			Pp'=	2.770	ksf	
Sheer area:	$T_{pp} = 0$	op + Ppo) / 2				Tpp =	0.0	ft	
GE/2010 18 001	A <sub>pp</sub> = Tpp	•w				App =	0.00	ft <sup>2</sup>	
Sheer Cepecity: gr = 0.75		* App + µ * P) * φι				Sactual #	287.391	kips	
		Check	Sacked =	287.39 klps	>=	S =	71.99	klps	0
Overturning Moment Res		D - T - 11-11 / G - 1	A1 *			W <sub>ew</sub> =	0.0	libra	
Wt of soll wedge.		D*TAN(φ)) / 2* \	ik. A			O =	5.327	kips	
Dist. from leg to edge: Additional offset of VA:		0.866 ° w) / 2 2 - (1 / 3 ° 0.866 ° ·	W+0)			0.=	4,474	ft	
Resisting moments		V / 2 - Wt * Oa	n + O)			M <sub>ret</sub> =	23546.57	ft-kips	
Troubling Troubline		App * (D - N) / 3				M <sub>rp</sub> =	0.00	ft-kips	
		" (W + D - TAN(	0)/3)			M <sub>rms</sub> =	0.00	ft-kips	
Total resisting: or = 0.75		rt + Mrp + Mrsw) *				M <sub>st</sub> =	17659.92	ft-kips	
Total overturning:	M <sub>o</sub> = M +	S*(D+E)				M <sub>o</sub> =	13699.65	ft-kips	
JA 160 040		Check	M <sub>r1</sub> = 1	17659.92 ft-kips	>=	M <sub>o</sub> =	13699.65	ft-kips	0
Rearing Resistance due	to Pressure Distribution								
Area of met.	area = W²					area =	1406.3	ft <sup>2</sup>	
Section modulus:	SM = W <sup>3</sup>					SM =	8789.1	ft <sup>3</sup>	
Fectored total weight:		/ 1.2 + Wc + Ws) '	0.9			P' =	1136.1	lúp	
Pressure exerted.		area + Mo / SM			-	P <sub>mex</sub> =	2.367	ksf	
- 2.1		area - Mo / SM		I Tomas I Toma		P <sub>rein</sub> ≖	-0.751	ksf	
	te: The stress resultant i		kem, Bear	ing area has bee	in adjusted belo		12.00		
Load accentricity:	e <sub>c</sub> = Mo		llan			0 <sub>6</sub> =	12.06	ft	
Parallel Direction		2/(3.M.(M/5				P	3.018	ksf ksf	
n Diagonal Direction		gonal Bearing Sheet (a neg >= 0, Ppos, P	Control of the Contro			Pade dag =	4.072		
Adj. applied pressure:		THE RESERVE TO SERVE				Q <sub>a</sub> =	0.000	ksf ksf	
Overburden Pressure: (fec	torouj Hopp - NA-	Gross Bearing Pro	le - Goop =	3.018 ksf	Cm Cm	Q <sub>obp</sub> = B <sub>c</sub> ° φr =	5.250	ksf	0
Concrete Shear Strength One way beam action at d; fr		O. NOCK	HI NOOD	U.010 Kai		-c wi-	0.2.00	ner	. 0
Effective depth:		oc-dbp/2				d <sub>c</sub> =	17.500	in	
Distance from edge of pad to	d' '= O -	-				d" =	3.827	ft	
pier face	u -0.	-V1 -							
Distance from edge of ped to	d" = d' -	to				d" =	2.369	R	

V<sub>c1</sub> = 792.41 kips

 $V_{n1} = [(qa - d^{+} \circ qs) + (d^{+} \circ qs/2)] \circ d^{-}W - [0.9 \circ (D - T) \circ \gamma^{+} d^{-}W]$ 

Bearing Pressure Slop

Required shear:

Available shear: [ACI 22 5 5 1] qs = 0.75 [ACI 21 2 1]

q, = qa / Weff

Vc1 = qs . 2 . A . V(F'c) . W . dc

Check

 $q_a =$ 

Vc1 =

V<sub>n1</sub> =

0.1504

792.41

206.13 kips

206.13 kips

kcf

kips

OK

Eq S	quara Column (ACI 8.10.1.3 & 22.6.4.1.2)	d <sub>m</sub> =di/2 * √r						deg =	31.90	in	
		W <sub>eff</sub> = Min (W,		11					20.073402	12.7	
	Met effective width in bearing	The second second	NAME OF TAXABLE PARTY.	Market Land			_	W <sub>eff</sub> =		n	
Ratio of	long side to short side of Pier Langth:	b <sub>1</sub> = dc / 2 + ε	uare or round					β ≃ b₁ =	1,00 63,70	in	
	Width:	b <sub>2</sub> = (dc + de		THE RESERVE				b <sub>2</sub> =	88.62	in	
Critical	Parimeter	b <sub>o</sub> = b1 + b2	1 . 11 - 00140	1 41116				b <sub>o</sub> =	152.32	in	
Section	Centrold		b1/2)/(b1*	do 1 h2 5	day.			C=	13.320	ft	
	Eccentricity'	e <sub>c</sub> = (deq + d		0C + 0Z	aci				11.3818909		
	Polar MOI	Je = [(de * b1		· Mma 2 / 15	01 + /h1 * .	le * /h1 /2 - /	W21 + (h1		1.063E+06		
Moment			7,000		7 10.	(	, e1 . (n)				
Fraction	flexure eccentricity	Yt = 17 (1 + 2	/3°√(61/6	2))				γ <sub>1</sub> =	0.64		
ansferred by	of shear;	y. = 1 - yf						γ <sub>v</sub> =	0.36		
Bearing F	Pressure Slope	q <sub>a</sub> = qa / Wef						Q. =	0.150	kcf	
varage Be	ering Pressure:	q <sub>apt</sub> = ((Weff - I	o1) * qs + qa)	12				Qapi =	2.619	ksf	
Shear Fr	orce at Section	V <sub>n_pier</sub> = C - qa.pl	*(b1 * b2)					V <sub>n_prer</sub> =	417.495	kips	
	Stab Moment:	Mac = SI * (D -	T + E) + Vn_p	ler * a				Mec =	670.02	ft-kips	
	Required shear gs = 0.75	ACI 21 2 1] = (Vn_pier	/ b0 * dc) + (v	v " Msc"	: / Jc)				192 99	psi	
	Available shear: [A	C/22.6.5.2/ = \$8 * MI	√(4°λ°√(Fc),	(2+(4/B))*.	1* V(Fc) . (	2+(as*dc/bo))	*A*√(Fc))		201.246	psi	
			Check	V <sub>c2</sub> =	201.25	psi	>=	V10 =	192,99	psi	OK
Mon	nent transfered: (Pler 1)	Mel = vf * Msc	700000000000000000000000000000000000000					M <sub>n1</sub> =	306,779	ft-kips	
F. Martin	re Beam Width	$W_{eff1} = deq + 1.3$	5 * T + MIN/ 1	5 T //	- w) * SII	(60) - den 1	21	W <sub>eff</sub> ) =	7.909	ft.	
a.sreruit		A <sub>et o1</sub> ' = Mn1 / (0.	A CONTRACTOR OF THE PARTY OF TH		, m, Oli	-tool - ned );	-,	A <sub>nt p)</sub> '=	3.896	in <sup>2</sup>	
		a <sub>p1</sub> = Ast p1'		weff1)				a <sub>pt</sub> =	0.663	in	
	Required steel:	Ante at = Mn1/(F						A <sub>plus att</sub> =	3.574	ln <sup>2</sup>	
	sel in entire met.	A <sub>st_p_ste1</sub> = Ast_p_st		-1/			-	A <sub>st p stol</sub> =	16.946	ini	
	nent transfered:			4.50	2.2						
	(Pier 2 or 3)	Maz = yf " Msc	7		200			M <sub>n2</sub> =	428.070	ft-klps	
Effective	re Beam Width:	W <sub>eff2</sub> = deq + 1.	A STATE OF THE PARTY OF THE PAR	.5 ° T , (W	/ - w\ - dec	1)/2)		Weft2 =	7.204	ft	
		A <sub>M,D2</sub> ' = Mn2 / (0.						Aa, p2 =	5.436	ln <sup>2</sup>	
		a <sub>p2</sub> = Ast_p2' '	and the same of th	T. Williams				a <sub>p2</sub> =	1.016	in . 2	
	Required steet:	Ast.p. 102 = Mn2/(F		2))				AND WZ	5.039	in <sup>2</sup>	
Required sta	eel in entire met:	Aut.p_ese2 = Ast_p_s	2 * W / weff2					A <sub>st_p_ste2</sub> =	26.226	311	
		- (+0) 00 0 0 1 (4-10						Cor	ntrolling Case		Pier 2: Come
	earn action at d / 2 from lowe		0.16 4 4 9.46					all	00.000	da.	
	inforcement Dia quare Column (ACI 8 10.1.3 &	di <sub>T</sub> =di -2*cc-2	_db_t - 1_db_	5				di <sub>T</sub> =	28.000	in	
	22.6.4.1.2)	d <sub>eq_T</sub> =dprebar /	2 * Vm					dea_T =	24.81	in	
Critical	Section Length:	b <sub>1_T</sub> = deq_T +	de					b1_T+	42.314	in	
Critical Sec	ction Perimeter:	b <sub>o_T</sub> = 4 * (deq	+ dc)					b <sub>0_7</sub> =	169.26	in	
	Polar MOI	J. T = (b1_T^3	* dc / 6)+ (b1	T°d^3/	6)+(dc * t	1_T b2_T	2 / 2)	J <sub>c,T</sub> =	921710.251	in*	
Shear Fe	orce at Section:	V <sub>n_per_T</sub> ≖ U						V <sub>n_tem_T</sub> =	451.23	kips	
	Required sheer; qs = 0.75	ACI 21 2.1] = (Vn_pier	T/b1_T * do	) + (yv * #	Asc' C_T /	Jc_T)			182.533	psi	
	Available shear [A	C122.6.5.2] "= \ps " MI	N( 4" 1" V(Fc) .	(2+(4/B))°	λ°√(Fc) . I	2+(as*dc/bo)	)*\^*\(Fc))		201.25	psi	
		-	Check	Vu =	201.25	psl	>=	Vn(2 =	182.53	psi	OK
olumn (	Compression Capacity:	16									
Compri	ession reaction:	P <sub>c</sub> = φc * 0.8!	Fc (dr /	িন)				P <sub>c</sub> =	2530.7	kips	
	gc * 0.65 [ACI 21.2 2.2]		Check	D =	2530.69	bine	>=	C=	520.18	kips	OK
		3	- Uneca	re-	2000.09	NUS			520.10	NUS	Un
ler Rein	forcement:										
Gress	-sectional area:	A <sub>g</sub> = di <sup>2</sup> * π / ·	1					A <sub>q</sub> =	1017.88	in <sup>2</sup>	
Min. are-	e of steel (pier):	A <sub>st</sub> = Ag * 0.01						A <sub>st_c</sub> =	10.18	in	
	[ACT 10 6.1.1] & [ACT 10.3.1.2]										
	Cage circle:	d, = d - 2 ° o	c - db_c - 2 *	ib_t				d <sub>o</sub> =	28.00	in	
	Rebar-	s_= 8				d <sub>0.0</sub> =	1	in			
		m_s = 15				A <sub>b_0</sub> =	0.79	in <sup>2</sup>			
		A <sub>0_c</sub> = Ab_c * n	_c					A <sub>e_c</sub> =	11.85	in <sup>2</sup>	
			Check	As c =	11.85	in²	>=	Ast,c =	10.18	in²	OK
- 3	Actual moment:	M <sub>max</sub> = (D - T +	E)*\$/2					M <sub>max</sub> =	206.98	ft-kips	
Play m	pment depecity:	M <sub>alkov</sub> per Mexmor	nnt.xis (see atlac	hed)				Matter =	212.41	ft-kips	
			Check	M <sub>attow</sub> =	212.41	ft-kips	>=	M <sub>max</sub> =	206.98	ft-kips	OK
	Bar separation	$B_{s,c} = (do * \pi)$	m_c - db_c					Bage =	4.86	in	

Reinforcement location:	Ψ <sub>C</sub> = if the space	a under the r	ebar > 12 in	use 1.3	, else use	1.0	Ψι_c =	1.3		
[ACI 25.4.2.4] Epoxy coeting:	Ψ <sub>e c</sub> = if epoxy-co	ated bars an	e not used. 1	ise 1.0:	but if epoxy	-coaled	ψ <sub>0,z</sub> =	1.0	_	
[ACI 25.4.2.4]		sed, then if B								
Mex term: (AC) 25.4.2.4]	ψ <sub>1</sub> ψ <sub>e_c</sub> = the produc	Control of the Contro			the Section of the Section 2	The second secon	Ψ,Ψα_ς 22	1.3	4	
Reinforcement size:	$\psi_{s_{\underline{c}}} = \text{ if the bar s}$	ize is 6 or les	ss, then use	0.8, else	use 1.0		ψ <sub>1_0</sub> =	1		
[ACI 25.4.2.4] Light weight concrete:	λ <sub>c</sub> = if lightwleg	hi concrete i	s used 0.75	Alse us	e 1.0		λ =	1.0		
[ACI 25.4.2.4]	n's - n agranag		3 0000, 0.70	, 0.00 00			-	1.0		
Specing/cover: [ACI 25.4.2.4]	c_c the smalle	r of: half the	bar spacing	or the co	ncrete edg	e distace	c_c =	2.93	in	
Transverse bars: [ACI 25.4.2.3]	k <sub>w_c</sub> = 0 in (p	er simplificati	lon)				K <sub>b</sub> _c ≡	0	in	
Mex term: [ACI 25.4.2.3]	c_c' = MIN( 2.5	, (c_c + ktr_	c) / db_c)				c, =	2.500		
Excess reinforcement: [ACI 25.4.10.1]	R <sub>c</sub> = 1		(exc	ese reinfor	cement reduc	tion is not used)	R_c=	1.00		
Development (tensile): [ACI 25.4.2.2]	L <sub>di_c</sub> = (3 / 40) *	Fy / A_c * √(F	.c)) , (fighe c	"ms_c.	R_c/c_c) *	db_c	- ولها	34.88	in	
Minimum length: [ACI 25.4.2.1]	L <sub>d_min</sub> = 12 Inche	S					Ld_min =	12.0	ln	
Development length:	Lat c = MAX( Lo	_min, Ldt'_c	)				Lac=	34.88	in	
Confining Reinforcement. [ACI 25.4.9.3]	$\psi_{r_c} = 1$	2 27 2 25					ψ, ,, =	1.00		
Development (comp.): [ACI 25.4.9.2]	Lde's = Fy 'wr_c	*db_c* R	c/(50° A_c	' √(F'c)	)		Lec's =	17.89	ln	
	L <sub>dc</sub> " <sub>c</sub> = 0.0003 *	db_c * Fy *	ψr_c*R_c				Lac" =	18.00	in	
Development length:	Loc = MAX(8,	Ldc' c, Ldc"	(c)				Lee ==	18.00	in	
Length available in pier:	L <sub>vc</sub> = D - T + I	77	Τ-6*				L <sub>vc</sub> =	66.0	in	
		Check	L <sub>ic</sub> =	66.0	in	>=	Lac =	34.9	in	OK
		Check	L <sub>vc</sub> ≡	68.0	in	>=	Lac =	18.0	in	OK
Length available in pad:	L.p = T - cc						L <sub>vp</sub> =	18.0	ln.	
		Check	L <sub>vp</sub> =	18.0	in	>a	Lage =	34.9	in	ноок
		Check	L <sub>sp</sub> =	18.0	in	>=	Late =	18.0	in	ОК
ertical Rebar Hook Ending;				- Transition		***				
Bar size & clear cover: [ACI 25.4.3.2]	Ψω = if the ber s	lze <= 11 an	d side cc >=	2.5", us	e 0.7, else	use 1.0	ψ <sub>1</sub> η =	0.7		
Epaxy coating: [ACt 25.4.3.1]	ψ <sub>e h</sub> = if epoxy-co	ated bars ar	e used, use	1.2, else	use 1.0		ψ <sub>e_h</sub> =	1.0		
Light weight concrete: [ACI 25.4.3.1]	λ <sub>n</sub> if lightwieg	ht concrete i	s used, 0.75	, else us	e 1.0		λ_h =	1.0		
Confining Reinforcement: [ACI 25.4.3.2]	$\psi_{r,h} = 1$						$\psi_{r\_h} =$	1.00		
Development (hock). [ACI 25.4.3.1]	Lan' = (Fy * wt	h. me_p. d	pr_h * R_c /	(50 ° λ_l	* v(F'c))) *	db_c	L <sub>dh</sub> ' =	12.5	in	
Minimum length:	Laturin the larger	of: 8 * db or	6 in				Los min =	8.0	in	
[ACI 25.4.3.1]										
Development length	Lan = MAX( Lo	lh_min, Ldh'	)				L <sub>oh</sub> =	12.5	in	
		Check	L <sub>sp</sub> =	18.0	in	>=	L <sub>oh</sub> ×	12.5	in	OK
Hook tail length:	Lh tai 12 * db be	yond the ber	nd radius				Lington 22	16.0	In	
Length available in pad	Lh_pad = (W - W'	di) / 2					Lh_pag =	21	in	
		Check	Lh_per =	21.0	In	>=	Lon tol 2	16.0	in	OK

Ties: Afinimum size:	S_tmin =IF	(s c <= 11	0, 3, 4)						\$ 1,000	3		
	-70m; -11								200			
[ACI 25.7.2.2]		of the nate	min sees to 1	nee there	2 ple= 4 5				z=	1		
z factor:		o if the sels	mic zone is I	ซรร เกสก	∠ erse 1.U			0.5		1		
Tie perameters:	s <sub>.1</sub> = 4 m <sub>.1</sub> = 14						6]= 6]=	0.5	in in <sup>d</sup>			
Allowable tie specing.		15.0										
per vertical rebar [ACI 25.7.2.1] & [ACI 18.4.3.3]									Ba_Umax1 =	8	in	
per tio size (ACI 25.7.2.1) & (ACI 18.4.3.3	B <sub>s_t_max</sub> = 2	24 * db_t							B <sub>e_t_med</sub> =	12	in-	
per pier diameter [ACI 25.7.2.1] & [ACI 18.4.3.3	B <sub>a_l_med</sub> ≡ e	di / 4							B <sub>s_Cmax3</sub> =	9	in	
per seismic zone [ACI 25.7 2.1] & [ACI 18.4 3.3		12" in active	e seismic zor	nes, else	18"				Ball_mart =	12	in	
		MIN( Bs t	max1, Bs_t_	max2. Bs	t max3.	Bs t m	nax4)		B <sub>s,t,rue</sub> =	8	in	
		Committee Committee of the	/Bs_t_max						m , =	10.6		
	Charles A	port.	Check	m j =	14.0			>=	m <sub>_t,min</sub> =	10.6		OH
hor Steel:						_						
A/S perameters	P = 2	62357				1		66	in			
	100	1.75 in	i					55.50	in			
evelopment evailable	-		ts (see attached	43			- All	ON UT	L <sub>m</sub> =	45.63	in	
			is (see attached is (see attached							34.88	in	
tequired development	-das_min pe				4E 02	in	-	>=	Loss min =			pa a
	E -	- 1	Check	L <sub>das</sub> sr	45.63	in		28	L <sub>dat_res</sub> =	34.88	in	OF
To bottom rebar grid:	E <sub>at_max</sub> =D			-		12			E <sub>ss_mex</sub> =	85	in	
		- Luc	Check	E <sub>m</sub> =	55.50	ii)		<0	E <sub>as_max</sub> =	85.00	ln	OF
To top reber grid.	rebar @ = 0	700	The second second						rebar @	72.00	in	
		_		+ 6 in	>=	E	M 2	55.50	in or	< m	72 in	OF
			s (see attached)	)					do_min =	25.25	in	
Min. cage dia:	do_mn pe	-	Check	d <sub>o</sub> =	28.00	in P <sub>2</sub>	<b>e</b> <sub>2</sub>	>= 	d <sub>o_max</sub> =	25.25	in 20.07	-
Alin. cage dia:  Reactions:		-			28.00		• <sub>2</sub>	<b>&gt;=</b>		ngth in bearin	g. 20.07	
					28.00	Î	e <sub>2</sub>	<b>&gt;=</b>	Effective les	ngth in bearin	g. 20.07	ft
Reactions:	^^	<b>+ + + +</b>			28.00	Î			Effective len	ngth in bearin	g 20.07 g 17.43	ft
Reactions:  Total Beem Length	A, A,	↓ ↓ ↓ v			28.00	Î			Effective lang	ngth in bearingth not bearing	g 20.07 g 17.43	ft
Reactions:	B <sub>12,1</sub> = V S <sub>12,1</sub> = C	↓ ↓ ↓ ↓ v			28.00	Î			Effective len	ngth in bearin	g 20.07 g 17.43 ft	ft
Reactions:  Total Beem Length	A, A,	↓ ↓ ↓ ↓ v			28.00	Î		**	Effective lang	ngth in bearingth not bearing	g 20.07 g 17.43	ft
Reactions:  Total Beam Length resion of Left Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = V S <sub>12.1</sub> = V	V V V V V V V V V V V V V V V V V V V			28.00	Î			Effective length of the state o	37.5 5.327 32.17	g 20.07 g 17.43 ft ft ft	ft
Reactions:  Total Beam Length reabon of Left Support:	B <sub>12.1</sub> = V S <sub>12.1</sub> = V S <sub>12.2</sub> = V B <sub>12.2</sub> = V	v v o v o v			28.00	Î			Effective lang  Effective lang  BL2.1=  SL2.1=	37.5 5.327 32.17	g 20.07 g 17.43 ft	ft
Reactions:  Total Beem Length realism of Right Support tion of Right Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>R2.1</sub> = V B <sub>12.2</sub> = V S <sub>12.3</sub> = (1)	V V V V V V V V V V V V V V V V V V V			28.00	Î			Effective length of the state o	37.5 5.327 32.17	g 20.07 g 17.43 ft ft ft	ft
Reactions:  Total Beem Length realison of Left Support tion of Right Support office Geometry Input (Option 2) Total Beam Length	B <sub>12.1</sub> = V S <sub>12.1</sub> = V S <sub>12.2</sub> = V B <sub>12.2</sub> = V	V V V V V V V V V V V V V V V V V V V			28.00	Î			Effective lenger	37.5 5.327 32.17	g 20.07 g 17.43 ft	ft
Total Beem Length realism of Right Support tion of Right Support Total Beam Length receion of Left Support total Beam Length receion of Left Support tion of Right Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>R2.1</sub> = V B <sub>12.2</sub> = V S <sub>12.3</sub> = (1)	V V V V V V V V V V V V V V V V V V V		do=	28.00	Î			Effective length in the second	37.5 5.327 32.17 37.5 3.25	g 20.07 g 17.43 ft ft	ft ft
Total Beem Length restion of Left Support tion of Right Support Total Geametry Input (Option 2) Total Geam Length restion of Left Support tion of Right Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>82.1</sub> = V S <sub>82.2</sub> = V S <sub>82.2</sub> = (S <sub>82.2</sub> = S	V V V V V V V V V V V V V V V V V V V	Check	do=	11	Î			Effective length of the state o	37.5 5.327 32.17 37.5 3.25	g 20.07 g 17.43 ft ft ft ft ft	ft ft
Total Beem Length resison of Left Support tion of Right Support Total Beam Length resison of Left Support total Beam Length resison of Left Support toon of Right Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>82.1</sub> = V S <sub>82.2</sub> = V S <sub>82.2</sub> = (S <sub>82.2</sub> = S	V V V V V V V V V V V V V V V V V V V	Check	do=	2000	Î			Effective length of the state o	37.5 5.327 32.17 37.5 3.25	g 20.07 g 17.43 ft ft ft ft ft	ft ft
Total Beam Length resison of Left Support tion of Right Support tions of Beam Length receising from the Support total Beam Length scelon of Left Support tion of Right Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>82.1</sub> = V S <sub>82.2</sub> = V S <sub>82.2</sub> = (S <sub>82.2</sub> = S	V V V V V V V V V V V V V V V V V V V	Check	do=	2000 1500 1000	Î			Effective length of the state o	37.5 5.327 32.17 37.5 3.25	g 20.07 g 17.43 ft ft ft ft ft	ft ft
Total Beam Length resiston of Left Support tion of Right Support tion of Right Support Total Beam Length receion of Left Support tion of Right Support tion of Right Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>82.1</sub> = V S <sub>82.2</sub> = V S <sub>82.2</sub> = (S <sub>82.2</sub> = S	V V V V V V V V V V V V V V V V V V V	Check	do=	2000	Î			Effective length of the state o	37.5 5.327 32.17 37.5 3.25	g 20.07 g 17.43 ft ft ft ft ft	ft ft
Total Beam Length cebon of Left Support tion of Right Support tion of Right Support total Beam Length scession of Left Support tion of Right Support tion of Right Support	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>82.1</sub> = V S <sub>82.2</sub> = V S <sub>82.2</sub> = (S <sub>82.2</sub> = S	V V V V V V V V V V V V V V V V V V V	Check	do=	2000 1500 1000	Î			Effective length of the state o	37.5 5.327 32.17 37.5 3.25	g 20.07 g 17.43 ft ft ft ft ft	ft ft
Total Beem Length resistant of Right Support total Beam Length received of Right Support.  Olids Geometry Input (Option 2)  Total Beam Length received of Left Support.  bon of Right Support.	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>82.1</sub> = V S <sub>82.2</sub> = V S <sub>82.2</sub> = (S <sub>82.2</sub> = S	V V V V V V V V V V V V V V V V V V V	Check **	do=	2000 1500 1000 500 0	Î			Effective length of the state o	37.5 5.327 32.17 37.5 3.25	g 20.07 g 17.43 ft ft ft ft ft	ft ft
Total Beam Length cebon of Left Support tion of Right Support tion of Right Support total Beam Length scession of Left Support tion of Right Support tion of Right Support	B <sub>12.1</sub> = W S <sub>12.1</sub> = O S <sub>82.1</sub> = W B <sub>12.2</sub> = W S <sub>12.2</sub> = S Direction 1	V V V V V V V V V V V V V V V V V V V	Check **	do=	2000 1500 1000 500	Î			Effective length of the state o	37.5 5.327 32.17 37.5 3.25 34.25	g 20.07 g 17.43 ft ft ft ft ft	ft ft
Total Beem Length Incebon of Left Support Both of Right Support Total Beem Length Incebon of Right Support Both of Right Support	B <sub>12.1</sub> = W S <sub>12.1</sub> = C S <sub>82.1</sub> = V S <sub>82.2</sub> = W S <sub>82.2</sub> = S Direction 1	V V V V V V V V V V V V V V V V V V V	-Moment (ft*)	d <sub>o</sub> =	2000 1500 1000 500 0	,A.,			Effective for A	37.5 5.327 32.17 37.5 3.25 34.25	g 20.07 g 17.43 ft	ft ft
Total Beem Length Incebon of Left Support Both of Right Support Total Beem Length Incebon of Right Support Both of Right Support	B <sub>12.1</sub> = W S <sub>12.1</sub> = O S <sub>82.1</sub> = W B <sub>12.2</sub> = W S <sub>12.2</sub> = S Direction 1	V V V V V V V V V V V V V V V V V V V	-Moment (ft*)	d <sub>o</sub> =	2000 1500 1000 500 0	,A.,			Effective form	37.5 5.327 32.17 37.5 3.25 34.25	\$ 20.07 g 17.43 ft ft ft ft ft ft	ft ft
Total Beem Length resident of Left Support tion of Right Support tion of Right Support total Beem Length resident of Left Support tion of Right Support ti	B <sub>12.1</sub> = V S <sub>12.1</sub> = O S <sub>R2.1</sub> = V B <sub>12.2</sub> = V S <sub>12.2</sub> = (V S <sub>R2.2</sub> = S Direction 1	V V V V V V V V V V V V V V V V V V V	-Moment (ft*)	d <sub>o</sub> =	2000 1500 1000 500 0	,A.,			Effective length of the state o	37.5 5.327 32.17 37.5 3.25 34.25	g 20.07 g 17.43 ft	ft ft
Total Beem Length resison of Left Support tion of Right Support total Beam Length resison of Left Support total Beam Length. scation of Left Support tion of Right Support tion of Right Support tion to the S	B <sub>12.1</sub> = W S <sub>12.1</sub> = C S <sub>12.2</sub> = C S <sub>12.2</sub> = C S <sub>12.2</sub> = C S <sub>12.2</sub> = S Direction 1	V V V V V V V V V V V V V V V V V V V	-Moment (ft*)	d <sub>o</sub> =	2000 1500 1000 500 0	,A.,			Effective length of the state o	37.5 5.327 32.17 37.5 3.25 34.25	g 20.07 g 17.43 ft	ft ft
Total Beam Length resison of Left Support tion of Right Support tion of Right Support total Beam Length receison of Left Support tion of Right Support tio	B <sub>12.1</sub> = V S <sub>12.1</sub> = C S <sub>R2.1</sub> = V B <sub>12.2</sub> = V S <sub>12.3</sub> = (1) S <sub>R2.2</sub> = S Direction 1	V V V V V V V V V V V V V V V V V V V	-Moment (ft*)	d <sub>o</sub> =	2000 1500 1000 500 0 -500 9.00	,A.,			Effective length of the state o	37.5 5.327 32.17 37.5 3.25 3.4.25 25.00	g 20.07 g 17.43 ft	ft ft

Pad Reinforcement:										
	b = IF(Fc ca	4000, 0.85, IF(F'c	>= 8000, 0.65	0.85 - (Fc - 4	1000) * 0.05))		b=	0.825		
Effective width:	We = W						W <sub>0</sub> =	37 500	ft	
	A <sub>al.p</sub> ' = Mn / (0.	9 * Fy * dc)					A <sub>H_0</sub> ' =	34.904	in <sup>2</sup>	
	a <sub>p</sub> = Ast_p' '	Fy/(B · Fc ·	We)				a <sub>p</sub> =	1.25	in	
Required steet:	Ascos = Mn / (F	y * (dc - ap / 2)	) * (W / We	9).			Ada at =	32.580	in <sup>2</sup>	
Shrinkaga		60000, 0.00					feb =	0.0018		
	A <sub>et o eh</sub> = psh * V	and the second second					An pan=	8.505	in <sup>2</sup>	
	A <sub>M p</sub> = MAX(A		sh Ast o	ste1 Ast r	ste2)		A <sub>st o</sub> =	32 580	in <sup>2</sup>	
Reber	\$ = 8	Equally spaced		_5(01,115(_)	d <sub>b p</sub> =	1	in	02.000		
Neoes					A <sub>b,p</sub> =	0.79	in <sup>2</sup>			
		bottom, both di	rechons.		Ob_p **	0.13		22.40	in²	
	A _ Ab_p *	particular designation of the last of the		00.10	n²	-	A <sub>8,p</sub> =	33.18	in <sup>2</sup>	
	S	Check	A <sub>s,p</sub> ≃	00.10	11	>=	Aug =	32,58		OK
Bar separation:	B (W - 2 '						B. 0 =	9.80	in	
		Check	17	>#	B <sub>ap</sub> ≠	9.80	in	>=	4°	OK
Pad Development Length:										
Reinforcement location.	Ψ <sub>Lp</sub> = if the spa	ce under the re	ebar > 12 ir	, use 1.3, e	Ise use 1.0		Ψι, =	1.3		
[ACI 25.4 2.4]										
Epoxy coating	ψ <sub>e,p</sub> = if epoxy-c						Ψ=_0=	1.0		
[ACI 25.4.2.4]	The second secon	used, then if B			STATE OF THE OWNER, WHEN	else 1.2				
Max term:	ψιψω <sub>D</sub> = the produ	ict of wt & we,	need not b	e taken larg	er than 1,7		ΨιΨο_p =	1.3		
[AC) 25.4 2.4]										
Reinforcement size:	ψω = if the bar	size is 6 or les	s, then use	0.8, else u	se 1.0		ψ <sub>a_p</sub> =	- 1		
[ACI 25.4 2.4]	4 (40.10.3	1.0					4 1		_	
Light weight concrete: [ACI 25.4 2.4]	$\lambda_p = \text{if lightwise}$	ght concrete is	used, 0.75	o, else use 1	0.1		= مر	1.0		
Specing/cover	c o = the small	ne of half the h	ar rancina	or the cons	rate adap d	intana	c <sub>o</sub> =	3.50	in	
[ACI 25.4.2.4]	o_p - the attibili	di Ot. Hell Did L	ar spacing	OI BIG CONG	icio dago o	is is the	-0_0	0.00		
Transverse bars	k <sub>v o</sub> = 0 in (	per simplification	on)				K <sub>e o</sub> =	0	in	
(ACI 25.4 2.3)	1		***							
Mex term	c,' = MIN( 2	5. (c p + ktr p	) / db p)				c ,' =	2.500		
(ACI 25.4.2.3)										
Excess reinforcement:	R <sub>6</sub> = 1		fex	ess reinforcen	nent reduction	is not used	Re=	1.00		
[ACT 25.4 10.1]			,,,,,,				, 3			
Development (tensila)	L <sub>d</sub> = (3 / 40)	*(Fy/A_p * \	(F'c)) * ψtų	e_p * ws_p	* R_p * db_	p/c_p'_	u L <sub>dp</sub> '=	34.9	in	
[ACr 25.4 2 2]								10.2		
Minimum length:	La_min = 12 inch	es					Ld_mm =	12.0	in	
JACI 25.4.2 1J										
Development length	Lop = MAX( L	d_min, Ldp' )					L <sub>dp</sub> =	34.9	in	
Longth evailable in pad:	Lped = (W / 2	-W12)-cc					Lped #	36.0	in	
		Check	Load =	36.00		>=	L <sub>do</sub> =	34.88	in	OK

# UNIT BASE FOUNDATION DIAGONAL BEARING CHECK

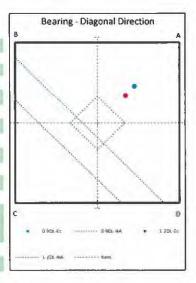
VB BTS II, LLC US-KY-5135 Fancy Farm

V- 31.0

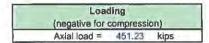
290

A- 565090

		Load Case - DL 1.2	Load Case - DL 0.9	
Moment of Inertia of Mat	MOI	164794.92	164794.92	ft <sup>4</sup>
Total Factored Weight	P'	1514.75	1136.06	kips
Load Eccentricity	е	9.04	12.06	ft.
Bearing at Corner A	B <sub>c a</sub>	3.28	3.01	ksf
Bearing at Corner B	Beb	1.08	0.81	ksf
Bearing at Corner C	Всс		-1.40	ksf
Bearing at Corner D	B <sub>c d</sub>		0.81	ksf
Initial Location of Neutral Axis from C	NA c ini		16,80	ft
Calculated Location of Neutral Axis from C	NA c cal	17.10	24.09	ft
MOI for Effective Bearing Area	MOI		117004.22	ft⁴
Distance to Point Load from NA	L <sub>p</sub>	18.46	14.49	ft
Effective Length in Bearing along AB & AD	Well	37.50	37.50	ft
Total Vol. Difference	Voltot	1514.75 -0.0002 ok	1136.06 0.0000 ok	kips kips
Adjusted Bearing at A	B <sub>c_s_sd</sub>	3.6505	4.0723	ksf
Adjusted Bearing at B & D	B <sub>c_td_adj</sub>	The second secon	0.34	ksf
Maximum Diagonal Bearing Pressure	B <sub>c_dis_max</sub>	3.6505	4.0723	ksf
Bearing Available	B <sub>c</sub> * φr		5.2500	ksf
Check		OK	OK	



# THIS SPREADSHEET IS SET UP FOR A MAXIMUM OF 56 BARS. MAXIMUM FACTORED MOMENT OF A CIRCULAR SECTION



Found	lation	
Concrete		
Pier diameter =	3.00	ft
Pier area =	1017.9	in^2
Reinforcement		
Clear cover =	3.00	in
Cage diameter =	2.33	ft
Bar size =	8	
Bar diameter =	1.000	in
Bar area =	0.785	in^2
Number of bars =	15	

Material Strength	8	
Concrete compressive strength =	4500	psi
Reinforcement yield strength =	60000	psi
Modulus of elasticity =	29000	ksi
Reinforcement yield strain =	0.00207	
Limiting compressive strain =	0.003	

(per ACI 10.3.5 - OK)

Seismic	
SDC =	D
Are hooks required?	yes

#### Minimum Area of Steel

Required area of steel = 10.18 in/2
Actual area of steel = 11.78 in/2
Bar spacing = 5.28 in

#### **Axial Loading**

Load factor = 1.00

Reduction factor = 0.65575 (per ACI 9.3.1 & 2) 0.6557471

Factored axial load = 451.23 kips

#### Neutral Axis

Distance from extreme edge to neutral axis = 3.85 iii

Equivalent compression zone factor = 0.825 (per ACI 10.2.7.3)

OK

Distance from extreme edge to

Equivalent compression zone factor = 3.18 in Distance from centroid to neutral axis = 14.15 in

#### Compression Zone

Area of steel in compression zone = 0.00 in^2

Angle from centroid of pier to intersection of

equivalent compression zone and edge of pier = 34.58 deg

Area of concrete in compression = 44.13 in^2 44.130982

Force in concrete = 0.85 \* Fc \* (Acc - steel in comp zone) = 168.80 kips (per ACI 10.3.6.2)

Total reinforcement forces = -620.03 kips

Force in concrete = -168.80 kips

Force in concrete - - 100.00 kips

Sum of the forces in concrete = 0.00 kips OK

#### **Maximum Moment**

First moment of the concrete area in compression about the centoid = 710 62 in<sup>3</sup>
Distance between centroid of concrete in compression and centroid of pier = 16.10 in

Moment of concrete in compression = 2718.12 in-kips

Total reinforcement moment = 1168.94 in-kips

Nominal moment strength of column = 3887.06 in-kips
Factored moment strength of column = 2548.93 in-kips 212.41 ft-kips

Maximum allowable moment of the pier = 212.41 ft-kips

# Individual Bars

Bar	Angle from first bar	Distance to centroid	Distance to neutral axis	Distance to equivalent comp. zone	Strain	Area of steel in compressi on	Axial force	Moment
#	(deg)	(in)	(in)	(in)		(in^2)	(kips)	(in-kips)
1	0.00	0.00	-14.15	-14.82	-0.01101	0.00	-47.12	0.00
2	24.00	5.69	-8.45	-9.13	-0.00658	0.00	-47.12	-268.34
3	48.00	10.40	-3.74	-4.42	-0.00291	0.00	-47.12	-490.28
4	72.00	13.31	-0.83	-1.51	-0.00065	0.00	-14.74	-196.29
5	96.00	13.92	-0.22	-0.90	-0.00017	0.00	-3.95	-55.03
6	120.00	12.12	-2.02	-2.70	-0.00157	0.00	-35.85	-434.65
7	144.00	8.23	-5.92	-6.59	-0.00461	0.00	-47.12	-387.78
8	168.00	2.91	-11.24	-11.91	-0.00875	0.00	-47.12	-137.17
9	192.00	-2.91	-17.06	-17.73	-0.01328	0.00	-47.12	137.17
10	216.00	-8.23	-22.38	-23.05	-0.01742	0.00	-47.12	387.78
11	240.00	-12.12	-26.27	-26.95	-0.02045	0.00	-47.12	571.35
12	264.00	-13.92	-28.07	-28.74	-0.02185	0.00	-47.12	656.12
13	288.00	-13.31	-27.46	-28.14	-0.02138	0.00	-47.12	627.44
14	312.00	-10.40	-24.55	-25.22	-0.01911	0.00	-47.12	490.28
15	336.00	-5.69	-19.84	-20.51	-0.01545	0.00	-47.12	268.34

Foundation:	Pier diameter =	3.0	ft	Cover between side of pier and cage =	3 00 in.
	Cage diameter =	2.33	ft	Cover between top of pier and cage =	3.00 in.
	Rebar size =	8		Compressive strength of concrete =	4500 psi
	Number of bars =	15		Rebar yield strength =	60000 psi
	Clear spacing =	4.86	in.	1100.550.550.550.550.550	211121313
	Are there hooks?	n			
	Check Compression?	n			
Anchor Steel:	Part number:	262357			
	Embedment length =	55.5	in.		
	Bolt Diameter =	1.75			
Anchor Plate:	Part number:	281262			
	Plate width =	19.25	in.		
Required develops	nent length (compression) =	999.00	in.		
Required dev	elopment length (tension) =	34.88	in.		
Ava	ilable development length =	45.625	in.		
		OK			

Foundation:	Pier diameter =	3.0	ft	Cover between side of pier and cage =	3.00 in.
	Cage diameter =	2.333333	ft	Minimum cover between A/S and cage =	3.00 in.
Anchor Steel:	Part number:	262357		Angle of anchor steel in foundation =	0 degrees
	Embedment length =	55.5	in.		
Anchor Plate:	Part number:	281262			
	Largest plate width =	19.25	in.		
	Bolt Diameter =	1.75	in.		
	Minimum cage diameter =	25.25	in.		
	Actual cage diameter =	28	in.		
		OK			

# **SELF-SUPPORT TOWER FOUNDATION DESIGN SUMMARY**

# VB BTS II, LLC **US-KY-5135 Fancy Farm**

V- 31 A- 565090 290

V 24

ier Dimensions		
Pler diameter, d <sub>i</sub> :	4.00	ft
Depth, D:	36.5	ft
Ext. above grade, E:	0.50	ft
Bell diameter, b <sub>d</sub> :	none	ft
Volume, V <sub>a</sub> .	17.22	cy / leg

Rebar	m_e	23	verticals
	size, s_c:	8	equally space in 35' ceo
Ties	size, S_i:	4	default hook
	m_t:	46	tie qty

**Anchor Bolts** 

Uplift/Leg. U:

Soil Information Per: POD, Project No. 20-64965, Dated: March 23, 2022

te Parameters		
Ultimate Bearing, Bc:	16.500	ksf
Ultimate Pp:	0.846	kcf
Uit. Skin Friction, SF:	1.325	ksf
Seismic Design Cat.:	D	
Depth neglected, N:	4.00	ft
Neglect bottom, N <sub>b</sub> :	none	ft

# Additional Notes:

\* No foundation modifications listed.

451.23 kips

\* See attached "Foundation Notes" for further information.

aterial Properties					
Steel tensile str. Fy:	60000	psi			
Conc. Comp. str, F'c	4500	psi			
Conc Density, 5:	150.0	pcf			
Clear cover, cc:	3.00	lin			

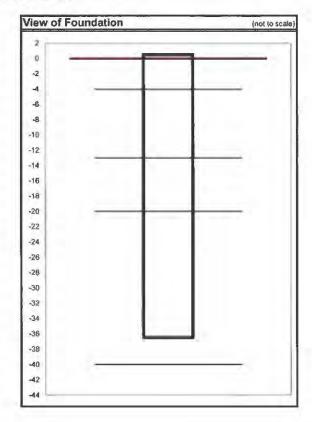
#### Tower design conforms to the following:

- International Building Code (IBC)
- \* ANSI TIA-222-G
- \* Building Code Requirements for Reinforced Concrete (ACI 318-

P/N: 262357	66*	long. 1 75° diame	ter .			
Foundation Loading	g					-, ,, ,,,,,
Max Corner Reacti	ons	stress rat	0. 95.5%		mark up	1.4%
Shear/Leg, 5:	47.00	kips	x 1.014	2	47.66	kips
MomenVLeg, M:	0.00	ft-kips	x 1.014	=	0.00	ft-kips
Compression/Leg, C:	513.00	kips	x 1.014	=	520.18	kips

445.00 kips

x 1.014 =





# **FOUNDATION NOTES**

- THE ON-SITE GEOTECHNICAL ENGINEER SHALL CONFIRM THAT THE INSITU SOIL STRENGTHS MEET OR EXCEED THOSE PARAMETERS GIVEN IN THE SOIL REPORT.
- 2 SEE GEOTECHNICAL REPORT FOR ADDITIONAL CONSTRUCTION RECOMMENDATIONS, BACKFILL COMPACTION DETAIL, SUBGRADE PREPARATION, ETC.
- 3 CONCRETE IS TO BE PLACED THE SAME DAY THAT THE EXCAVATION IS COMPLETED.
  4 A TEMPORARY, FULL LENGTH STEEL CASING MAY BE REQUIRED DURING INSTALLATION.

# SST DRILLED PIER FOUNDATION

# VB BTS II, LLC US-KY-5135 Fancy Farm

V- 31.0 290 A- 565090

Design Summary					
Pier diameter:	4.00	R			
Design depth:	36.5	ft			
Output value	47 00				

Pier diameter:	4.00	R
Design depth:	36.5	ft
Concrete volume:	17.22	cu.yd. sach
Use #4 circular ties		

Min. concrete compressive strength to be 4500 pai. Use anchor bolt p/n 135816

Maximum Loading						
Max, Uplift, U max	451.23	kips/leg				
Max. Comp., C max:	602.83	kips/leg				
Max. Shear, S mex:	47.66	kips/leg				

Soil per: POD, Project No. 20-64965, Dated: March 23, 2022

...

Ultimate bearing: Ultimate S F (uplift): Ultimate S F (comp.): 18.500 ksf 1.325 ksf 1.325 ksf

ayer	From	To	Cont. layer	Pier			100	Y								
V	100		length	diameter	Cohesion	Phi	Unit weight of soil	Overburden pressure	Average overburden pressure		Factored Incton force	weight	Upim reessi.	Factored skin Inction	friction force	Factored bearing capacity
N	(ft)	(ft)	(ft)	(ft)	(ksf)	(deg)	(pcl)	(ksf)	(ksf)	(ksf)	(kips)	(kips)	(kips)	(kal)	(kips)	(ksf)
1	0.00	4.00	4.00	4.00	0.500	0.000	110.0	0.440	0.220	0.000	0.00	6.36	8.36	0.000	0.00	-
2	4.00	13.00	9.00	4.00	1.500	0.000	120.0	1.520	0.980	0.300	33.93	12.72	46.65	0.300	33.93	- 2
3	13.00	20.00	7.00	4.00	2.000	0.000	120.0	2.360	1,940	0.375	32.99	9.90	42.86	0.375	32.99	
4	20.00	36.50	16.50	4.00	0.000	32.000	90.0	3.845	3.103	1.875	388.77	23.33	412.10	1.875	388.77	12.38
	Lateral	pressure o	oefficient =	0.6							Total Upark Cap	secuty (hips) =	508.00	Total friction	capecity (lops) =	455.69
					•								OK		сериону (кари) =	155.51
									Warment Aust	ige Skin Friction	up#/i	1 325	lost	Total Comp	Capacity (kips) =	81120

Reinforcement Design:

Concrete Clear Cover (in) = 3.00

# of bars	Bar size	Area per bar (sq.in.)	Clear spacing (in.)	Bar area (sq.in.)	Steel required (sq.in.)	Ultimate Lateral Resist (kcf) *	Maimum length (ft) **
23	8	0.79	4.74	18.17	18.10	0.848	12.70

\*see Passive (attached)
\*see Broms method (attached)
\*\*\*see Maximum Factored Moment of a
Circular Section (attached).

Minimum area of steel is OK Minimum pier length is OK Rebar spacing is OK

Moment Check (fi-k) 261.06

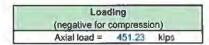
**Equivalent Weighted Average Cohesion** 

Layer	From (ft)	To (ft)	Layer Length (ft)	Neglect?	Cohesion (ksf)	Weighted Cohesion (ksf)
1	0.00	4.00	0.00	У	0.500	0.00
2	4.00	13.00	9.00	n	1.500	13.50
3	13.00	20.00	7.00	n	2.000	14.00
4	20.00	36.50	16.50	n	0.000	0.00
5	36.50	36.50	0.00	n	0.000	0.00
6	36.50	36.50	0.00	n	0.000	0.00
7	36.50	36.50	0.00	n	0.000	0.00
8	36.50	36.50	0.00	n	0.000	0.00
9	36.50	36.50	0.00	n	0.000	0.00
10	36.50	36.50	0.00	n	0.000	0.00
11	36.50	36.50	0.00	n	0.000	0.00
12	36.50	36.50	0.00	n	0.000	0.00
13	36.50	36.50	0.00	n	0.000	0.00
14	36.50	36.50	0.00	n	0.000	0.00
15	36.50	36.50	0.00	n	0.000	0.00
16	36.50	36.50	0.00	n	0.000	0.00
17	36.50	36.50	0.00	n	0.000	0.00
18	36.50	36.50	0.00	n	0.000	0.00
19	36.50	36.50	0.00	n	0.000	0.00
20	36.50	36.50	0.00	n	0.000	0.00
Bell	36.50	36.50	0.00	n	0.000	0.00
		Total =	32.50	1	Total =	27.50

Weighted Average Equivalent Cohesion =	0.85	(ksf)
--	------	-------

Diameter of pier, di:	4.00	ft			S/leg	M/leg
Extension above grade, E:	0.50	ft			(kips)	(k-ft)
Neglect at ground surface, N:	4.00	ft			7.45.37	
Ultimate Passive Pressure, Po	0.846	kcf		LC	47.66	0
Reduction Factor, f.	0.75					
Nominal Passive Pressure (Pp*f), Ppa-	0.635	kcf				
# of pier dia Pp acts over, Nd.	3.00					
Solved Brom's $G_a = \sqrt{((S * (E + N + F / 2) + 1))}$	M) / ((N	<sub>d</sub> / 3) * 2.25 *	P <sub>pa</sub> * d <sub>i</sub> ))			
Brom's $G_a = \sqrt{((S * (E + N + F / 2) + I))}$ Equation LC	M) / ((N	<sub>d</sub> / 3) * 2.25 *	P <sub>pa</sub> * d <sub>i</sub> ))			
Brom's $G_a = \sqrt{((S * (E + N + F / 2) + I))}$ Equation LC for $G_a$ (ft) 6.64	M) / ((N	<sub>d</sub> / 3) * 2.25 *	P <sub>pa</sub> * d <sub>i</sub> ))			
Brom's $G_a = \sqrt{((S * (E + N + F / 2) + I))}$ Equation LC for $G_a$ (ft) 6.64  Minimum $L = E + N + F + G_a$ length of LC	M) / ((N	<sub>d</sub> / 3) * 2.25 *	P <sub>pa</sub> * d <sub>i</sub> ))			
Brom's $G_a = \sqrt{((S * (E + N + F / 2) + I))}$ Equation LC for $G_a$ (ft) 6.64  Minimum $L = E + N + F + G_a$ length of LC	M) / ((N	<sub>d</sub> / 3) * 2.25 *	P <sub>pa</sub> * d <sub>i</sub> ))			
Brom's $G_a = \sqrt{((S * (E + N + F / 2) + I))}$ Equation LC for $G_a$ (ft) 6.64  Minimum $L = E + N + F + G_a$ length of LC	M) / ((N	<sub>d</sub> / 3) * 2.25 *	P <sub>pa</sub> * d <sub>i</sub> ))			
Brom's $G_a = \sqrt{((S * (E + N + F / 2) + I))}$ Equation  For $G_a$ (ft) $LC$ Minimum $L = E + N + F + G_a$ length of or incomplete to the content of th	12.70	ft				
Brom's $G_a = \sqrt{((S * (E + N + F / 2) + I))}$ Equation for $G_a$ (ft)  Minimum length of pier, L (ft)  Minimum length req'd, L:	12.70	ft				

## THIS SPREADSHEET IS SET UP FOR A MAXIMUM OF 56 BARS. MAXIMUM FACTORED MOMENT OF A CIRCULAR SECTION



Found	lation	
oncrete		
Pier diameter =	4.00	ft
Pier area =	1809.6	in^2
einforcement		
Clear cover =	3.00	in
Cage diameter =	3.33	ft
Bar size =	8	
Bar diameter =	1.000	in
Bar area =	0.785	in^2
Number of bars =	23	

		hs	Material Strength
	psi	ă.	oncrete compressive strength =
11	psi		Reinforcement yield strength =
	ksi		Modulus of elasticity =
(per		0	Reinforcement yield strain =
			Limiting compressive strain =

Saismic SDC=

D

yes

(per ACI 10.3.5 - OK)

Bar size =	8		Are hooks required?
Bar diameter =	1.000	in	
Bar area =	0.785	in^2	

#### Minimum Area of Steel

Required area of steel = 18.10 in^2
Actual area of steel = 18.06 in^2 No Good IIII
Bar spacing = 4.74 in

#### **Axial Loading**

Load factor = 1.00 Reduction factor = 0.65575 (per ACI 9.3.1 & 2)

Factored axial load = 451.23 kips

#### **Neutral Axis**

Distance from extreme edge to neutral axis = 5.93 In

Equivalent compression zone factor = 0.825 (per ACI 10.2.7.3)

Equivalent compression zone factor = 4.90 In

Distance from centroid to neutral axis = 18.07 in

#### Compression Zone

Area of steel in compression zone = 1.57 in^2 Angle from centroid of pier to intersection of equivalent compression zone and edge of pier = deg Area of concrete in compression = 96.95 in^2 Force in concrete = 0.85 ° fc ° (Acc - steel in comp zone) = 364.83 kips (per ACI 10.3.6.2) Total reinforcement forces = -816.06 kips Factored axial load = 451.23 kips Force in concrete = -364.83 kips

#### **Maximum Moment**

First moment of the concrete area in compression about the centoid = 2043.81 in^3
Distance between centroid of concrete in compression and centroid of pier = 21.08 in

kips

Moment of concrete in compression = 7690.92 in-kips
Total reinforcement moment = 5023.33 in-kips
Nominal moment strength of column = 12714.25 in-kips

Factored moment strength of column = 8337.33 in-kips 694.78 ft-kips

Sum of the forces in concrete =

Maximum allowable moment of the pier = 694.78 ft-kips

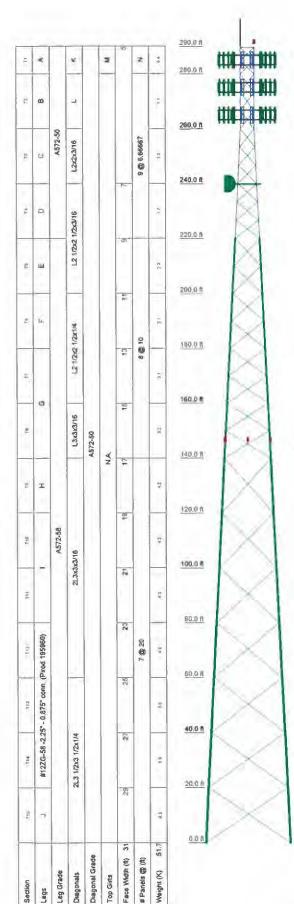
OK.

#### Individual Bars

Bar	Angle from first bar	Distance to centroid	Distance to neutral axis	Distance to equivalent comp. zone	Strain	Area of steel in compressi on	Axial force	Momen
#	(deg)	(in)	(in)	(in)		(ln^2)	(kips)	(in-kips)
1	0.00	0.00	-18.07	-19.10	-0.00913	0.00	-47.12	0.00
2	15.65	5.40	-12.67	-13.71	-0.0064	0.00	-47.12	-254.28
3	31.30	10.39	-7.67	-8.71	-0.00388	0.00	-47.12	-489.70
4	46.96	14.62	-3.45	-4.49	-0.00174	0.00	-39.71	-580.42
5	62.61	17.76	-0.31	-1.35	-0.00016	0.00	-3.54	-62.94
6	78.26	19.58	1.52	0.48	0.00077	0.79	17.46	341.84
7	93.91	19.95	1.89	0.85	0.00095	0.79	21.74	433.72
8	109.57	18.85	0.78	-0.26	0.00039	0.00	8.98	169.18
9	125.22	16.34	-1.73	-2.76	-0.00087	0.00	-19.87	-324.74
10	140.87	12.62	-5.44	-6.48	-0.00275	0.00	-47.12	-594.79
11	156.52	7.97	-10.10	-11.14	-0.0051	0.00	-47.12	-375.48
12	172.17	2.72	-15.34	-16.38	-0.00776	0.00	-47.12	-128.33
13	187.83	-2.72	-20.79	-21.83	-0.01051	0.00	-47.12	128.33
14	203.48	-7.97	-26.03	-27.07	-0.01316	0.00	-47.12	375.48
15	219.13	-12.62	-30.69	-31.73	-0.01551	0.00	-47.12	594.79
16	234.78	-16.34	-34.40	-35.44	-0.01739	0.00	-47.12	769.98
17	250.43	-18.85	-36.91	-37.95	-0.01866	0.00	-47.12	888.06
18	266.09	-19.95	-38.02	-39.06	-0.01922	0.00	-47.12	940.28
19	281.74	-19.58	-37.65	-38.69	-0.01903	0.00	-47.12	922.77
20	297.39	-17.76	-35.82	-36.86	-0.01811	0.00	-47.12	836.81
21	313.04	-14.62	-32.68	-33.72	-0.01652	0.00	-47.12	688.80
22	328.70	-10.39	-28.46	-29.50	-0.01439	0.00	-47.12	489.70
23	344.35	-5.40	-23.46	-24.50	-0.01186	0.00	-47.12	254.28

Foundation:	Pier diameter =	4.0	ft	Cover between side of pier and cage =	3.00 in.
	Cage diameter =	3.5	ft	Cover between top of pier and cage =	3.00 in.
	Rebar size =	8		Compressive strength of concrete =	4500 psi
	Number of bars =	23		Rebar yield strength =	60000 psi
	Clear spacing =	4.74	in.		(2.000.00)
	Are there hooks?	n			
	Check Compression?	n			
Anchor Steel:	Part number:	262357			
	Embedment length =	55.5	in.		
	Bolt Diameter =	1.75			
Anchor Plate:	Part number:	281262			
	Plate width =	19.25	In.		
Required developm	ent length (compression) =	999.00	in.		
Required devi	elopment length (tension) =	26.83	in.		
10 Tyrate 20 Co., 10 Co.			in.		
Avai	lable development length =	38.625	in.		
		OK			

Foundation:	Pier diameter =	4.0	ft	Cover between side of pier and cage =	3.0	00 in.
	Cage diameter =	3.5	ft	Minimum cover between A/S and cage =	150	00 in.
Anchor Steel:	Part number:	262357		Angle of anchor steel in foundation =	0	degrees
	Embedment length =	55.5	in.			
Anchor Plate:	Part number:	281262				
	Largest plate width =	19.25	in.			
	Bolt Diameter =	1.75	in.			
	Minimum cage diameter =	25.25	in.			
	Actual cage diameter =	42	in.			
		OK				



#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
5/8" x 10" lightning rod	290	2-1/2" x 7" Sch. 40	240
Beacon	290	2-1/2" x 7" Sch. 40	240
40,000 sq.in. (277.8 sq.ft. EPA)	285	6" HP	240
30,000 sq.in, (208,3 sq.ft, EPA)	275	OB light	145
30,000 sq.in. (208.3 sq.ft. EPA)	265	OB light	145
SP1 R5 (Includes 4.5"x72" Pipe)	240	OB light	145

SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	P- 2.50" - 0.75" conn. 10" -C-(Pirod 226172)	Н	#12ZG-58 -2.00" - 0.875" connTR3-(Pirod
В	B P- 4.00*- 0.75" conn20" -C-Trans-6B-48-(Pirod 226184)		195637)
			#12ZG-58 -2.00" - 0.875" conn. (Pirod 195639)
C	P= 8.00"- 0.75" connTrans-20' -C-(Pirod 226200)	J	#12ZG-58 BASE - 2.50" - 0.875" connTR4-(Pirod
D	D P- 6.00"- 0.75" connHBD-Trans-20"-C-(Pirod 229377)		281171)
			L2x2x1/8
E	#12ZG-58 - 1.50" - 1.00" conn. (Pirod 194651)	L	L2 1/2x2 1/2x3/16
F	#12ZG-58 - 1.75" - 1.00" connTR1-(Pirod 195213)	M	L3x3x1/4
G	#12ZG-58 - 1.75" - 1 (00" conn. (Pirod 195217)	N	2 @ 4.79167

**MATERIAL STRENGTH** 

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A572-58	58 ksi	75 ksi

#### **TOWER DESIGN NOTES**

1. Tower is located in Graves County, Kentucky.

- Tower designed for Exposure C to the TIA-222-G Standard.
   Tower designed for a 106 mph basic wind in accordance with the TIA-222-G Standard.
- Tower is also designed for a 30 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
- Deflections are based upon a 60 mph wind. Tower Risk Category II.
- 6.
- Topographic Category 1 with Crest Height of 0.00 ft
- A KA Factor of 1.00 has been applied to the anchor tenant EPA loading (for shielding).
   A KA factor of 0.82 has been applied to EPA loading for other 2 carriers (for shielding).
   TOWER RATING: 98.6%

ALL REACTIONS ARE FACTORED

MAX. CORNER REACTIONS AT BASE:

DOWN: 513 K SHEAR 47 K

UPLIFT -445 K SHEAR: 41 K

> AXIAL 281 K

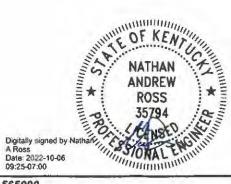
SHEAR MOMENT 2312 kip-ft 11 K

TORQUE 8 kip-ft 30 mph WIND - 1 0000 in ICE

> AXIAL 90 K

SHEAR MOMENT 71 K 12978 kip-ft

TORQUE 48 kip-ft REACTIONS - 106 mph WIND



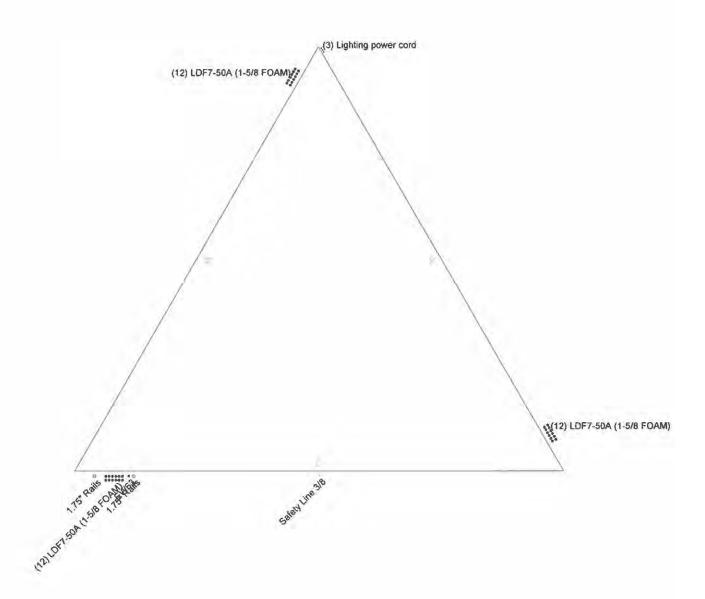
**STRUCTURES** 

1545 Pidco Dr. Plymouth, IN ont Industries, Inc. -Specialty Structures Group Phone: (574)-936-4221 FAX: (574)-936-6458

565090		Min so
roject H-31 x290' SST - 1	US-KY-5135 Fancy I	Farm
lient: VB BTS II, LLC	Drawn by: JL	App'd:
ode: TIA-222-G	Date 10/06/22	Scale NTS
ath:	N. A. S.	Dwg No. E-1

#### Feed Line Plan

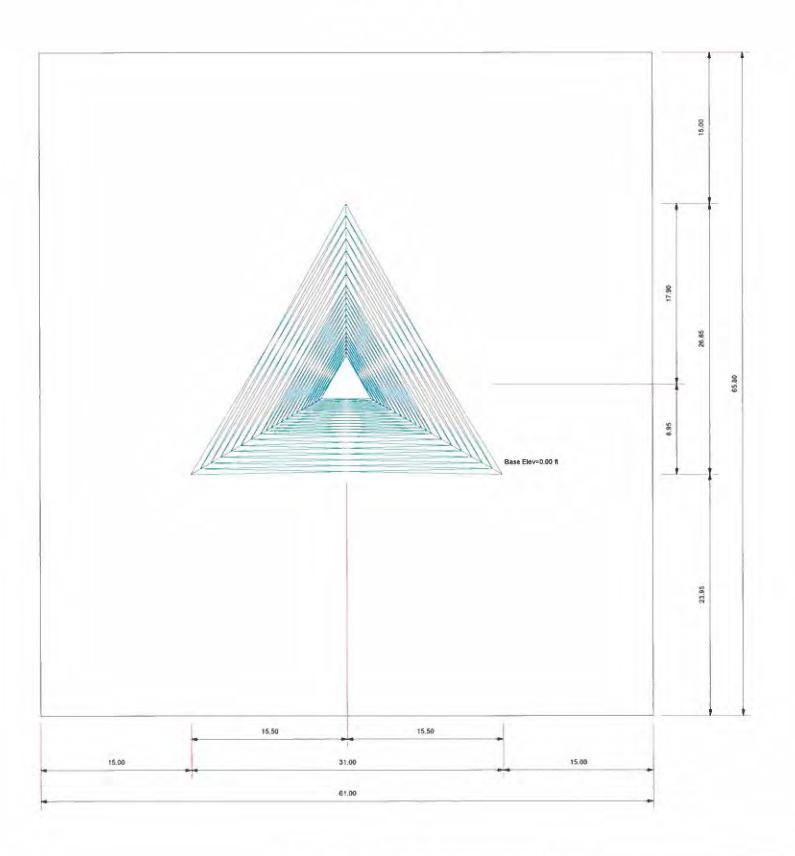
App Out Face Trass-Log





565090		
oject H-31 x290' SST - I	US-KY-5135 Fancy	Farm
ient: VB BTS II, LLC	Drawn by JL	App'd
ode TIA-222-G	Date 10/06/22	Scale NTS
ath:	e are the free from the second	Dwg No. E-7

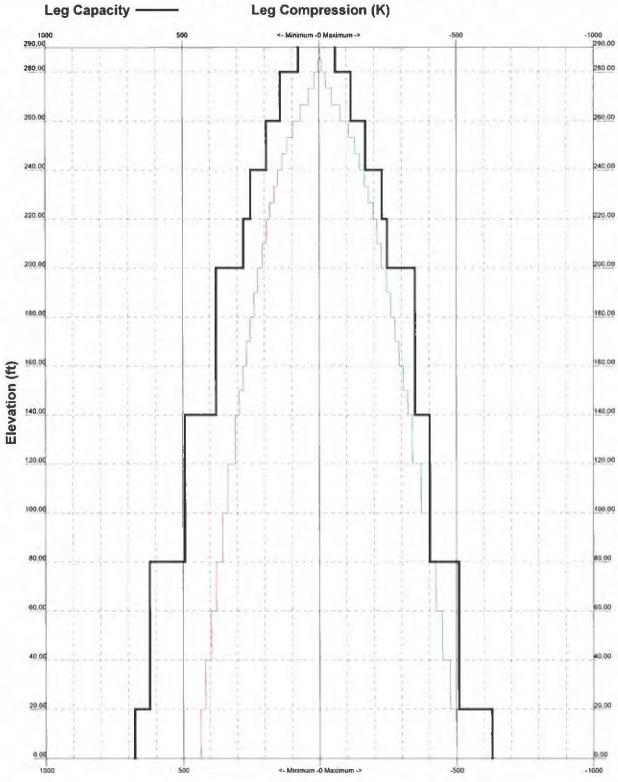
#### Plot Plan Total Area - 0.09 Acres





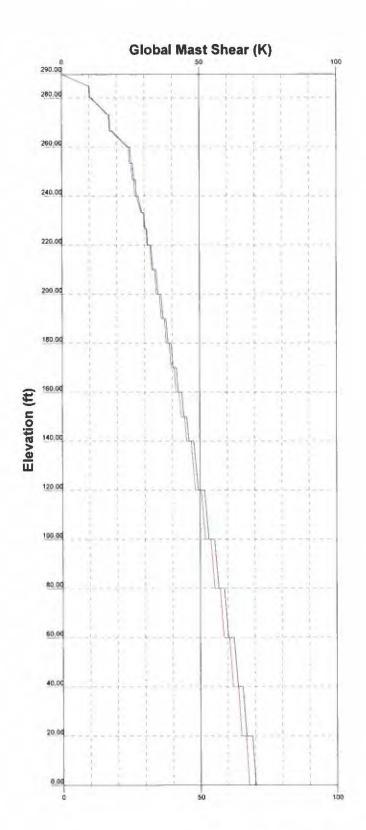
TIA-222-G - 106 mph/30 mph 1.0000 in Ice Exposure C

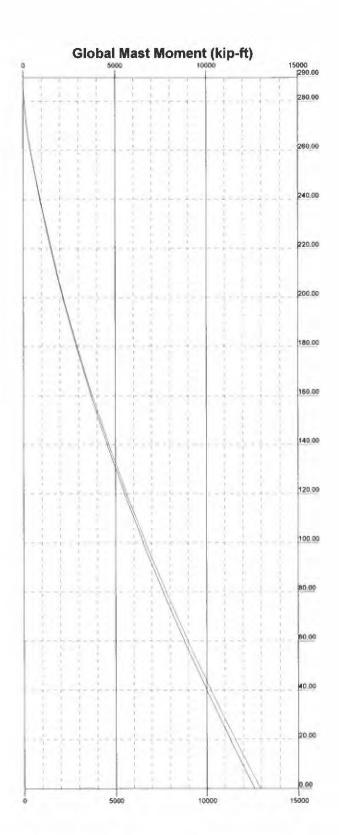
Leg Compression (K)





ob 565090		
Project H-31 x290' SST - I	US-KY-5135 Fancy i	Farm
Client: VB BTS II, LLC	Drawn by: JL	App'd
Code: TIA-222-G	Date: 10/06/22	Scale NTS
Path	6-KY-5180 Fancy Farm02 Tower Calcus	Dwg No. E-3

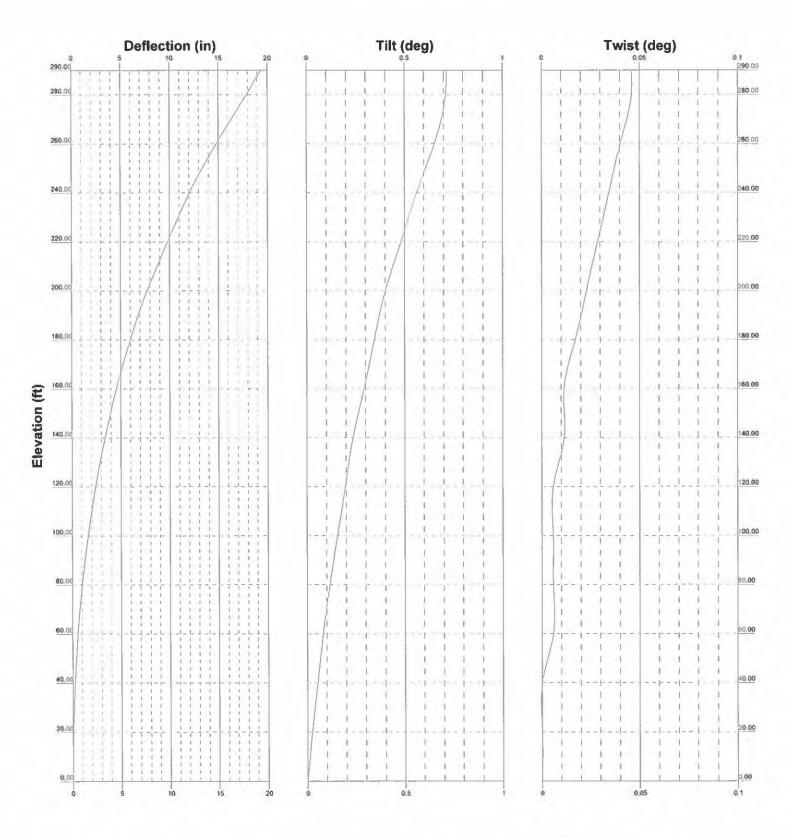






1545 Pidco Dr. Plymouth, IN nt Industries, Inc. -Specialty Structures Group Phone: (574)-936-4221 FAX: (574)-936-6458

<sup>Job.</sup> 565090		
Project: H-31 x290' SST -	US-KY-5135 Fancy	Farm
Client: VB BTS II, LLC	Drawn by: JL	App'd:
Code: TIA-222-G	Date 10/06/22	Scale NTS
Delhi	SAY 5180 Fancy Farming Tower Calcus	Dwg No. E-4





Valmont	Јо <b>в</b> 565090	Page 1 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Far	m Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

#### **Tower Input Data**

The main tower is a 3x free standing tower with an overall height of 290.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 5.00 ft at the top and 31.00 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Graves County, Kentucky.

ASCE 7-10 Wind Data is used.

Basic wind speed of 106 mph.

Risk Category II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 30 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A KA Factor of 1.00 has been applied to the anchor tenant EPA loading (for shielding)...

A KA factor of 0.82 has been applied to EPA loading for other 2 carriers (for shielding)...

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

#### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- √ Use Code Safety Factors Guys Escalate Ice Always Use Max Kz
- Use Special Wind Profile

  √ Include Bolts In Member Capacity
  Leg Bolts Are At Top Of Section
  Secondary Horizontal Braces Leg
  Use Diamond Inner Bracing (4 Sided)
  SR Members Have Cut Ends
  SR Members Are Concentric

Distribute Leg Loads As Uniform

- Assume Legs Pinned

  √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
- √ Retension Guys To Initial Tension Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt Autocalc Torque Arm Areas Add IBC 6D+W Combination
- √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing
- √ Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg Angle Legs

Use ASCE 10 X-Brace Ly Rules

- √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA
- √ SR Leg Bolts Resist Compression
- √ All Leg Panels Have Same Allowable Offset Girt At Foundation
- √ Consider Feed Line Torque
- √ Include Angle Block Shear Check Use TIA-222-G Bracing Resist Exemption Use TIA-222-G Tension Splice Exemption Poles

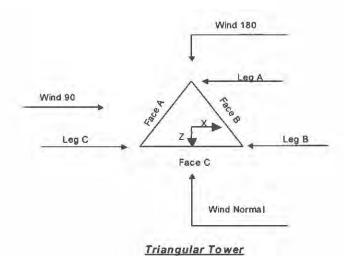
Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known

# Valmont

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Job	565090	Page 2 of 72
Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Client	VB BTS II, LLC	Designed by



### **Tower Section Geometry**

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length	
	ſ			fi		fi	
TI	290.00-280.00		V-Series Leg	5 00	1	10 00	
T2	280.00-260.00		V-Series Leg	5 00	1	20.00	
T3	260.00-240.00		V-Series Leg	5 00	1	20.00	
T4	240.00-220.00		V-Series Leg	7 00	1	20 00	
T5	220 00-200 00		PiRod 12BDFH Truss Leg	9.00	1	20 00	
T6	200 00-180 00		PiRod 12BDFH Truss Leg	11.00	1	20.00	
T7	180.00-160.00		PiRod 12BDFH Truss Leg	13 00	-1	20 00	
T8	160 00-140 00		PiRod 12BDFH Truss Leg	15.00	1	20 00	
T9	140 00-120 00		PiRod 12BDH2 Truss Leg	17.00	1	20.00	
T10	120 00-100 00		PiRod 12BDH2 Truss Leg	19 00	1	20 00	
TH	100 00-80 00		PiRod 12BDH2 Truss Leg	21 00	1	20 00	
T12	80 00-60 00		PiRod 12BDH2 Truss Leg	23 00	1	20.00	
T13	60 00-40 00		PiRod 12BDH2 Truss Leg	25 00	1	20.00	
T14	40 00-20 00		PiRod 12BDH2 Truss Leg	27.00	1	20 00	
TIS	20 00-0 00		PiRod 12BDH2 Truss Leg	29.00	1	20.00	

Tower	Tower	Diagonal	Bracing	Has	Has	Top Giri	Bottom Gir
Section	Elevation	Spacing	Type	K Brace	Horizontals	Offset	Offset
				End			
	ft	fi		Panels		in	in

Valmont	Job 565090	Page 3 of 72
1545 Pideo Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Tower Tower Section Elevation		Diagonal Spacing	Bracing Type	Has K Brace	Has Horizontals	Top Girt Offset	Bottom Giri Offset	
	fi	ji		End Panels		in	ın	
TI	290 00-280 00	4 79	X Brace	No	No	5 0000	0 0000	
T2	280 00-260 00	6 67	X Brace	No	No	0.0000	0 0000	
T3	260 00-240 00	6.67	X Brace	No	No	0 0000	0.0000	
T4	240.00-220.00	6.67	X Brace	No	No	0.0000	0.0000	
T5	220 00-200 00	10 00	X Brace	No	No	0 0000	0 0000	
T6	200 00-180 00	10.00	X Brace	No	No	0 0000	0 0000	
T7	180.00-160.00	10.00	X Brace	No	No	0.0000	0.0000	
T8	160.00-140.00	10.00	X Brace	No	No	0.0000	0.0000	
T9	140 00-120 00	20 00	X Brace	No	No	0.0000	0.0000	
T10	120 00-100 00	20 00	X Brace	No	No	0 0000	0 0000	
TII	100.00-80.00	20 00	X Brace	No	No	0.0000	0.0000	
T12	80.00-60 00	20 00	X Brace	No	No	0.0000	0.0000	
T13	60.00-40.00	20 00	X Brace	No	No	0 0000	0 0000	
T14	40.00-20 00	20.00	X Brace	No	No	0.0000	0.0000	
T15	20 00-0 00	20 00	X Brace	No	No	0.0000	0.0000	

Tower	Section	Geometry	(cont'd)
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Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 290 00-280.00	Pipe	P- 2.50" - 0.75" conn -10" -C-(Pirod 226172)	A572-50 (50 ksi)	Equal Angle	1.2x2x1/8	A572-50 (50 ksi)
T2 280 00-260 00	Pipe	P- 4.00"- 0.75" conn -20' -C-Trans-6B-4B-(Pirod 226184)	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A572-50 (50 ksi)
T3 260 00-240 00	Pipe	P- 5 00"- 0 75" conn -Trans-20' -C-(Pirod 226200)	A572-50 (50 ksi)	Equal Angle	L2x2x3/16	A572-50 (50 ksi)
T4 240 00-220 00	Pipe	P- 6.00"- 0.75" connHBD-Trans-20' -C-(Pirod 229377)	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A572-50 (50 ksi)
T5 220 00-200 00	Truss Leg	#12ZG-58 - 1 50" - 1 00" conn. (Pirod 194651)	A572-58 (58 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A572-50 (50 ksi)
T6 200 00-180 00	Truss Leg	#12ZG-58 - 1 75" - 1 00" conn -TR1-(Pirod 195213)	A572-58 (58 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A572-50 (50 ksi)
T7 180 00-160 00	Truss Leg	#12ZG-58 - 1 75" - 1 00" conn (Pirod 195217)	A572-58 (58 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A572-50 (50 ksi)
T8 160.00-140 00	Truss Leg	#12ZG-58 - 1.75" - 1.00" conn. (Ptrod 195217)	A572-58 (58 ksi)	Equal Angle	L3x3x3/16	A572-50 (50 ksi)
T9 140.00-120.00	Truss Leg	#12ZG-58 -2 00" - 0 875" conn -TR3-(Pirod 195637)	A572-58 (58 ksi)	Double Equal Angle	2L3x3x3/16	A572-50 (50 ksi)
T10 120 00-100 00	Truss Leg	#12ZG-58 -2 00" - 0 875" conn (Pirod 195639)	A572-58 (58 ksi)	Double Equal Angle	2L3x3x3/16	A572-50 (50 ksi)
T11 100.00-80.00	Truss Leg	#12ZG-58 -2 00" - 0 875" conn (Pirod 195639)	A572-58 (58 ksi)	Double Equal Angle	2L3x3x3/16	A572-50 (50 ksi)
T12 80 00-60 00	Truss Leg	#12ZG-58 -2 25" - 0 875" conn. (Pirod 195960)	A572-58 (58 ksi)	Double Equal Angle	2L3x3x3/16	A572-50 (50 ksi)
T13 60.00-40 00	Truss Leg	#12ZG-58 -2 25" - 0.875" conn (Pirod 195960)	A572-58 (58 ksi)	Double Equal Angle	2L3 I/2x3 I/2x1/4	A572-50 (50 ksi)
T14 40.00-20 00	Truss Leg	#12ZG-58 -2 25" - 0.875" conn. (Pirod 195960)	A572-58 (58 ksi)	Double Equal Angle	2L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T15 20 00-0 00	Truss Leg	#12ZG-58 BASE - 2.50" - 0 875" conn -TR4-(Pirod 281171)	A572-58 (58 ksi)	Double Equal Angle	2L3 1/2x3 1/2x1/4	A572-50 (50 ksi)

Valmont	Job	565090	Page 4 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth IN Phone (574) 936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Tower Section Geometry (cont'd)						
Tower Elevation fi	Top Giri Type	Top Girt Stze	Top Girl Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
TI 290 00-280 00	Equal Angle	L3x3x1/4	A572-50 (50 ksi)	Solid Round		A36 (36 ksi)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust Factor A <sub>j</sub>	Adjusi Factor A,	Weight Mult	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Ang Stitch Bolt Spacing Redundants
		in	127			1.00	in	<i>in</i>	in
90 00-280 00	0.00	0.2500	A36 (36 ksi)	- 1	- 1	1 05	36 0000	36 0000	36.0000
T2	0.00	0.2500	A36		1	1.05	36 0000	36.0000	36.0000
80 00-260 00	0.00	0.2300	(36 ksi)	1.	1	1.05	30 0000	30.0000	30.0000
T3	0.00	0 3750	A36	1	1	1.05	36 0000	36 0000	36.0000
60 00-240 00	0.00	0 3750	(36 ksi)		,	1.03	30 0000	30.0000	30.0000
T4	0.00	0 3750	A36	É	4	1.05	36 0000	36.0000	36.0000
40 00-220 00			(36 ksi)	3		4145	20 0000	50.000	20.0000
T5	0.00	0 5000	A36	1	1	1.05	36 0000	36.0000	36.0000
20 00-200 00			(36 ksi)				- E.W. 3-3-3-3-		
T6	0.00	0.5000	A36	1	1	1.05	36.0000	36.0000	36.0000
00 00-180 00			(36 ksi)						
T7	0.00	0 5000	A36	1	1	1.05	36 0000	36.0000	36.0000
80 00-160 00			(36 ksi)						
Т8	0.00	0.5000	A36	1	1	1.05	36 0000	36.0000	36.0000
60 00-140 00			(36 ksi)						
Т9	0.00	0 6250	A36	1	)	1.05	36 0000	36.0000	36.0000
40 00-120 00	2.21	0.000	(36 ksı)			V		23 2022	
T10	0.00	0.6250	A36	1	1	1.05	36 0000	36.0000	36.0000
20 00-100 00	0.00	0.4050	(36 ksi)	100	4	1 5 5	25.0000	22.6466	
T11	0.00	0 6250	A36	1	3	1,05	36 0000	36.0000	36.0000
100 00-80 00 T12	0.00	0.6250	(36 ksi) A36			1.05	77,0000	17.0000	76.0000
80.00-60.00	0.00	0.0230	(36 ksi)	1		1 05	36 0000	36.0000	36.0000
T13	0.00	0.6250	A36	i		1.05	36 0000	36.0000	36.0000
60.00-40 00	0.00	0.0230	(36 ksi)		1	1.05	30.0000	30.0000	30.0000
T14	0.00	0 6250	A36	1	1.	1.05	36 0000	36 0000	36.0000
40.00-20.00	0.00	0 0230	(36 ksi)	1	0.	1.03	30 0000	20.0000	30.0000
15 20 00-0.00	0.00	0.6250	A36	1:	b	1.05	36 0000	36.0000	36.0000
	0.00	0.0200	(36 ksi)			1103	20.0003	20.000	30,0000

to-the con-			
Tower	Section	Geometry	(cont'd)

Valmont	Job	565090	Page 5 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec Horiz	Inner Brace
	Angles	Rounds		X	X	X	X	X	X	X
ft				Y	Y	Y	Y	Y	Y	Y
T1	Yes	Yes	1	1	4	1	1	1	(1)	1
290.00-280.00				1	1	1	1	1	1	1
T2	Yes	Yes	T	10	1	1	1	1	1	1
280.00-260.00				100	1	1	1	1	1	0
T3	Yes	Yes	1	T	1	1	1	1	1	T
260 00-240.00				1	1	1	1	1	1	1
T4	Yes	Yes	T	1	1	1	1	1	1	T
240.00-220.00				1	1	10	1	1	1	1
T5	Yes	Yes	T	1	1	1.	1	1	1.	1
220.00-200.00				1	4	1	1	1	1	1
T6	Yes	Yes	1	1	1	1	1	1	1	1
200.00-180.00				1	4	1	1	1	1	1
T7	Yes	Yes	T	P		1	1	1	11	Î.
180 00-160.00				1	1	1	1	1	1	1
Т8	Yes	Yes	1	1	j.	1	1	1	1	1
160.00-140.00				1	1	1	î	1	1	1
Т9	Yes	Yes	Ĭ-	1	10	1	Í	I	1	1
40 00-120.00				1	1	1	1	1	1	1
T10	Yes	Yes	1	1	1	1	1	Î	1	1
120 00-100 00		1.75		1	1	1	1	1	1	1
T11	Yes	Yes	1	1	1	T.	1	1	î	1
100.00-80 00	,,,==		16	1	1	1	1	1	1	1
T12	Yes	Yes	I.	1	1	10	1	1	1	1
80.00-60.00				1	1	1	Î	1	1	1
T13	Yes	Yes	1	1	1	1	ì	1	T.	1
60 00-40.00	2 7896			1	1	1	1	1	1	1
T14	Yes	Yes	1	1	1	1	i.	ĭ	i	1
40 00-20 00	. 00	4 63		1	1	1	i	i	11	1
T15	Yes	Yes	1	1	1	10	i.	1	î	1
20.00-0.00	1 00	1 %:31	A .	1	Ŷ	1	1	1	Y	Ŷ

Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length

			Truss-Leg	K Factors		
	Trus	s-Legs Used As Leg Me	mbers	Truss	-Legs Used As Inner M	embers
Tower Elevation ft	Leg Panels	X Brace Diagonals	Z Brace Diagonals	Leg Panels	X Brace Diagonals	Z Brace Diagonals
T5 220.00-200.00	1	0.5	0.7	Î	0.5	0.7
T6 200.00-180.00	1	0.5	07	1	0.5	0.7
T7 180.00-160.00	1	0.5	0 7	1	0.5	0.7
T8 160.00-140.00	1	0.5	0.7	1	0.5	0.7
T9 140.00-120.00	1	0.5	0.7	1	0.5	0.7
T10 120 00-100.00	1	0.5	0.7	T-	0.5	07
T11 100.00-80 00	1	0.5	0.7	1)	0.5	0.7

Valmont	Job 565090	Page 6 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by

T12 80.00-60.00	Î	0.5	0.7	Î.	0.5	0.7
T13 60 00-40 00	1	0.5	0.7	l l	0.5	0.7
T14 40.00-20.00	1	0,5	0.7	T.	0.5	07
T15 20 00-0 00	j.	0.5	0 7	1>	0.5	0.7

Tower Elevation ft	Leg		Diago	nal	Top G	irt	Botton	Girt	Mid	Girt	Long Ho	rizontal	Short Ho	rizontal
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct	U	Net Width Deduct in	U	Net Width Deduct in	U
TI 290.00-280.00	0.0000	1	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 280.00-260.00	0.0000	1	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0 75	0.0000	0.75
T3 260 00-240 00	0.0000	1	0.0000	0 75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0 75
T4 240.00-220.00	0.0000	1	0.0000	0 75	0.0000	0.75	0.0000	0 75	0,0000	0.75	0.0000	0.75	0.0000	0.75
T5 220 00-200 00	0.0000	1	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 200.00-180.00	0.0000	1	0.0000	0 75	0.0000	0.75	0.0000	0 75	0.0000	0.75	0.0000	0 75	0.0000	0.75
T7 180.00-160.00	0.0000	1	0.0000	0.75	0.0000	0 75	0 0000	0.75	0,0000	0.75	0.0000	0.75	0.0000	0.75
T8 160 00-140 00	0.0000	T.	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0 75	0.0000	0 75	0 0000	0.75
T9 140 00-120.00	0.0000	Ť.	0.0000	0.75	0.0000	0 75	0 0000	0.75	0.0000	0.75	0.0000	0 75	0.0000	0.75
T10 120.00-100.00	0.0000	1	0.0000	0.75	0.0000	0.75	0 0000	0.75	0 0000	0 75	0.0000	0 75	0.0000	0 75
T11 100.00-80.00	0.0000	1	0.0000	0 75	0.0000	0.75	0 0000	0.75	0.0000	0 75	0.0000	0.75	0.0000	0 75
T12 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0 75	0 0000	0 75	0 0000	0 75	0.0000	0 75	0.0000	0 75
T13	0.0000	1	0.0000	0.75	0.0000	0 75	0.0000	0.75	0.0000	0 75	0.0000	0.75	0 0000	0.75
T14 40.00-20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0 75	0.0000	0.75
T15 20.00-0.00	0.0000	1.	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0.75

Tower Elevation st	Reduna Horizon		Redund Diago		Redund Sub-Diag		Redui Sub-Hoi		Redundan	t Vertical	Redunda	ant Hip	Redunda Diag	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 90.00-280.00	0.0000	0.75	0 0000	0 75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75

Valmont	Job	565090	Page 7 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Tower Elevation st	Reduna Hortzoi		Reduna Diago		Redund Sub-Diag	93,000	Redui Sub-Hoi		Redundan	ı Verncal	Redund	ant Hip	Redunda Diag	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T2	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0 75	0 0000	0.75
280.00-260.00														
T3	0.0000	0.75	0.0000	0.75	0.0000	0 75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75
260.00-240.00					0.2200			1.50	2.2622	0.00	e. 20200	225	5 4 5 7 5	2.00
T4	0.0000	0.75	0.0000	0.75	0.0000	0 75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75
240.00-220 00 T5	0 0000	0.75	0 0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
220.00-200.00	0.0000	0 /3	0 0000	0 /3	0.0000	0 13	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75
T6	0 0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75	0 0000	0.75	0 0000	0.75	0 0000	0 75
200.00-180.00	1.00 (2.2) (2.2)	0.15	0.0000	0.13	0.0000	0.75	0 0000	0.73	0 0000	013	0 0000	0 13	0 0000	9 13
T7	0 0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75
180.00-160.00					272.25				W-2026	333		187.53	8.0-23	
T8	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0 0000	0.75	0 0000	0.75
160.00-140.00	I have been been been been been been been be													
T9	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0 75	0 0000	0.75	0.0000	0.75	0 0000	0.75
140.00-120.00	1	0.20	44444		0.000			121	2222	2.5		55.	0.0000	-
T10	0.0000	0 75	0 0000	0.75	0 0000	0.75	0.0000	0 75	0.0000	0.75	0 0000	0.75	0 0000	0.75
120.00-100 00 T11	0 0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75
100.00-80.00	0.0000	0 13	0.0000	0.13	0.0000	0.73	0.0000	0.75	0.0000	0 /3	0.0000	0.73	0.0000	0.73
T12	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75
80 00-60 00								77.00			0.000		0.0000	0
T13	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0 0000	0.75	0 0000	0.75	0.0000	0.75
60 00-40 00	1000				1000000					37.5			100 m	
T14	0.0000	0.75	0 0000	0.75	0 0000	0.75	0.0000	0.75	0 0000	0.75	0 0000	0.75	0 0000	0.75
40 00-20 00	0.0007				5.06.04.0									
Γ15 20.00-0.00	0.0000	0.75	0.0000	0.75	0 0000	0.75	0.0000	0.75	0 0000	0.75	0 0000	0.75	0 0000	0.75

Tower				Connectio	on Offsets			
Elevation		Diag	onal			K-Br	acing	
	Vert. Top	Horiz Top	Vert Bot.	Horiz Bot.	Vert Top	Horiz. Top	Vert Bot	Horiz. Bot
ſi	in	in	in	in	in	ın	m	in
T1 290 00-280 00	5.0000	5 0000	5 0000	5 0000	0.0000	0.0000	0.0000	0 0000
T2 280.00-260.00	5.0000	5 0000	5 0000	5.0000	0.0000	0.0000	0 0000	0 0000
T3 260 00-240 00	5.0000	6.2500	5 0000	6.2500	0 0000	0.0000	0.0000	0 0000
T4 240 00-220 00	5.0000	6.2500	5 0000	6.2500	0.0000	0 0000	0.0000	0.0000
T5 220 00-200 00	5.0000	10 7500	5 0000	10 7500	0.0000	0.0000	0.0000	0 0000
T6 200 00-180 00	5 0000	10 7500	5 0000	10.7500	0 0000	0.0000	0.0000	0 0000
T7 180 00-160.00	5 0000	10 7500	5,0000	10.7500	0 0000	0.0000	0.0000	0 0000

Valmont	Job 565090	Page 8 of 72
1545 Pideo Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Tower				Connection	on Offsets	h		
Elevation		Diag	onal			K-Br	acing	
	Vert. Top	Horiz. Top	Vert Bot			Horiz Top	Vert Bot	Horiz, Bot
ft	in	ın	in	in	in	in	ın	in
T8 160 00-140 00	5.0000	10.7500	5.0000	10.7500	0 0000	0.0000	0.0000	0 0000
T9 140 00-120 00	5 0000	11 5000	5 0000	11 5000	0 0000	0.0000	0.0000	0 0000
T10 120 00-100 00	5.0000	11 5000	5 0000	11,5000	0.0000	0 0000	0.0000	0.0000
T11 100.00-80.00	5 0000	11 5000	5 0000	11 5000	0 0000	0 0000	0.0000	0 0000
T12 80 00-60 00	5.0000	11,5000	5 0000	11.5000	0 0000	0 0000	0.0000	0.0000
T13 60 00-40 00	5 0000	11 5000	5 0000	11,5000	0 0000	0 0000	0.0000	0 0000
T14 40.00-20.00	5 0000	11.5000	5.0000	11,5000	0.0000	0.0000	0 0000	0 0000
T15 20 00-0.00	5 0000	11.5000	5.0000	11.5000	0.0000	0.0000	0.0000	0.0000

Tower Elevation fi	Leg Connection Type	Leg		Diago	al	Top G	irt	Bottom	Girt	Mid G	irt	Long Hori	zontal	Short Hort	isontai
	Air	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
TI	Flange	0 7500	4	0.7500	1	0.7500	1	1.0000	0	1 0000	0	1.0000	0	1 0000	0
290 00-280 00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T2	Flange	0.7500	6	0 7500	1	0.0000	0	1.0000	0	1 0000	0	1 0000	0	1 0000	0
280 00-260 00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T3	Flange	0.7500	8	0.7500	1	0.0000	0	1.0000	0	1 0000	0	0000 1	0	1.0000	0
260 00-240 00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T4	Flange	1.0000	6	0.7500	1	0.0000	0	1.0000	0	1.0000	0	1.0000	0	1 0000	0
240 00-220 00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T5	Flange	1.0000	6	1.0000	1	0.0000	0	1.0000	0	1.0000	0	1.0000	0	1.0000	0
220 00-200 00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T6	Flange	1.2500	6	1 0000	1	0.0000	0	1.0000	0	1 0000	0	1.0000	0	1.0000	0
200.00-180.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7	Flange	1.2500	6	1 0000	1	0.0000	0	1 0000	0	1 0000	0	1 0000	0	1 0000	0
180.00-160.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8	Flange	1.2500	6	1 0000	1	0.0000	0	1.0000	0	1.0000	0	1.0000	0	1.0000	0
160.00-140.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T9	Flange	1 0000	12	0 8750	1	0.0000	0	1 0000	0	1 0000	0	1 0000	0	1.0000	0
140.00-120.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T10	Flange	1 0000	12	0 8750	1	0.0000	0	1 0000	0	1 0000	0	1 0000	0	1.0000	0
120 00-100 00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T11	Flange	1 0000	12	0.8750	1	0.0000	0	1 0000	0	1.0000	0	1.0000	0	1.0000	0
100.00-80.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T12	Flange	1.0000	12	0 8750	-1	0 0000	0	1 0000	0	1.0000	0	1 0000	0	1 0000	0
80.00-60.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T13	Flange	1 0000	12	0 8750	1	0.0000	0	1 0000	0	1 0000	0	1.0000	0	1.0000	0
60 00-40 00	l I v	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T14	Flange	1.0000	12	0 8750	1	0.0000	0	1,0000	0	1.0000	0	1 0000	0	1.0000	0
40.00-20.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

Valmont	Job 565090	Page 9 of 72
1545 Pidco Dr.	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Tower Elevation ft	Leg Connection Type	Leg	J	Diagoi	ıal	Top G	irt	Bottom	Girt	Mid G	irt	Long Hori	zontal	Short Hort	contai
,		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No	Bolt Size	No.
T15 20,00-0.00	Flange	1.7500 F1554-105	4	0 8750 A325N	1	0.0000 A325N	0	1 0000 A325N	0	1.0000 A325N	0	1.0000 A325N	0	I 0000 A325N	0

# Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Face Offset	Lateral Offset	#			Diameter	Perimeter	Weight
	Leg		Torque Calculation		ſì	in	(Frac FW)		Row	in	in	in	plf
Safety Line 3/8	C	No	No	Ar (CaAa)	290 00 - 0 00	3.0000	0	1	1	0 3750	0 3750		0 22
Lighting power cord	В	No	No	Ar (CaAa)	290.00 - 145.00	1,0000	-0.49	1	Ţ	0 8700	0 8700		0.15
Lighting power cord ****	В	No	No	Ar (CaAa)	145 00 - 0 00	1.0000	-0 49	3	3	0.8700	0.8700		0.15
LDF7-50A (1-5/8 FOAM)	C	No	No	Ar (CaAa)	285.00 - 0.00	3.0000	0.42	12	6	0 5200 1 0000	1 9800		0 82
LDF7-50A (1-5/8 FOAM)	В	No	No	Ar (CaAa)	275.00 - 0.00	3.0000	0 42	12	6	0.5200	1.9800		0.82
LDF7-50A (1-5/8 FOAM)	Α	No	No	Ar (CaAa)	265.00 - 0.00	3 0000	0.42	12	6	0.5200 1.0000	1.9800		0 82
EW63	C	No	No	Ar (CaAa)	240 00 - 0.00	3 0000	0.39	9	1	0.9300	1.5742		0.51
***													
1 75" Rails	C	No	No	Af (CaAa)	285 00 - 0 00	3 0000	0.46	1	1	32.2500 1.0000	1.7500		2 70
1 75" Rails	C	No	No	Af (CaAa)	285.00 - 0.00	3 0000	0.38	1	1	32 2500 1.0000	1 7500		2 70
***													

# Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Total Number	C.A.	Weigh
	Leg		Torque Calculation	315	fi	7,000	ft²/ft	plf
****								
***								

# Feed Line/Linear Appurtenances Section Areas

Valmont	Job 565090	Page 10 of 72
1545 Pidco Dr.	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Section	Tower Elevation	Face	$A_R$	$A_F$	C <sub>A</sub> A <sub>A</sub>	C <sub>t</sub> A <sub>1</sub> Out Face	Weight
	ſŧ		ſt²	SP	ft²	ft <sup>2</sup>	K
TI	290 00-280.00	A	0 000	0.000	0.000	0.000	0.00
		В	0 000	0.000	0.870	0 000	0.00
		C	0 000	0.000	15 172	0 000	0.08
T2	280 00-260.00	A	0.000	0.000	11.880	0.000	0.05
		В	0.000	0 000	37.380	0.000	0.15
		C	0.000	0.000	59 937	0.000	0.31
T3	260 00-240.00	A	0.000	0.000	47.520	0.000	0.20
		В	0.000	0.000	49 260	0.000	0.20
		C	0 000	0 000	59 937	0.000	0.31
T4	240.00-220.00	A	0 000	0.000	47 520	0.000	0.20
	and the armostic	В	0 000	0.000	49 260	0.000	0 20
		C	0 000	0 000	63.085	0.000	0.32
T5	220 00-200 00	A	0 000	0 000	47.520	0.000	0.20
	10-34-6-40-00-00-00-00-00-00-00-00-00-00-00-00-	В	0.000	0.000	49.260	0.000	0.20
		C	0 000	0.000	63.085	0.000	0.32
T6	200.00-180.00	A	0.000	0.000	47.520	0.000	0.20
C.A.	P27112-207171	В	0.000	0.000	49.260	0.000	0 20
		C	0.000	0.000	63.085	0.000	0 32
T7	180 00-160 00	A	0.000	0.000	47,520	0 000	0.20
* V	.00 00 ,00 00	В	0.000	0.000	49.260	0.000	0 20
		C	0.000	0.000	63.085	0.000	0 32
T8	160 00-140 00	A	0.000	0.000	47.520	0.000	0.20
	100 00-1-10 00	В	0.000	0.000	50.130	0.000	0 20
		C	0.000	0.000	63.085	0 000	0 32
T9	140 00-120 00	A	0.000	0.000	47.520	0.000	0 20
4.2	140.00-120.00	В	0 000	0 000	52.740	0 000	0 21
		C	0.000	0 000	63 085	0.000	0 32
T10	120 00-100 00	A	0.000	0 000	47 520	0.000	0 20
110	120 00-100 00	В	0.000	0 000	52.740	0.000	0.21
		C	0.000	0.000	63.085	0.000	0.32
TII	100 00-80 00	A	0 000	0 000	47 520	0 000	0 20
10.1	100 00-00 00	В	0.000	0 000	52.740	0.000	0 21
		C	0.000	0 000	63.085	0.000	0.32
T12	80.00-60.00	A	0 000	0 000	47 520	0 000	0.20
112	00.00-00.00	В	0.000	0 000	52 740	0.000	0.21
		C	0 000	0 000	63 085	0.000	0.32
T13	60 00-40 00	A	0 000	0 000	47 520	0.000	0.32
113	00.00-40.00	В	0.000	0 000	52 740	0 000	0.21
		Ĉ	0.000	0 000	63.085	0.000	0.21
T14	40.00-20.00	A	0.000	0.000	47 520	0.000	0.20
114	40.00-20.00	В	0.000	0 000	52 740	0.000	0.20
		C	0.000	0.000	63 085	0.000	0.32
TIE	20.00.0.00					0.000	0.20
T15	20 00-0 00	A	0.000	0.000	47 520		
		B	0.000	0.000	52 740 63 085	0.000	0.21

# Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation	Face or	Ice Thickness	AR	$A_{j}$	C <sub>1</sub> A <sub>.l</sub> In Face	C <sub>A</sub> A <sub>1</sub> Out Face	Weigh
P. Z + O. Z + O.	fi	Leg	in	ft <sup>2</sup>	Jt <sup>2</sup>	ft2	ft <sup>2</sup>	K
TI	290 00-280 00	A	2 481	0 000	0.000	0 000	0.000	0.00
		В		0 000	0.000	5 832	0.000	0 10
		C		0.000	0 000	26.859	0.000	0.61
T2	280 00-260 00	A	2 468	0.000	0 000	13 620	0.000	0.33
		В		0.000	0.000	52 471	0.000	1 20
		C		0.000	0 000	96.511	0.000	2 24
T3	260.00-240.00	A	2 449	0 000	0.000	54 353	0.000	1.32

Valmont	Јо <b>Б</b> 565090	Page 11 of 72
1545 Pidco Dr.	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by

Tower Section	Tower Elevation	Face or	Ice Thickness	$A_R$	$A_F$	C <sub>I</sub> A <sub>1</sub> In Face	C <sub>1</sub> A <sub>1</sub> Out Face	Weight
SECTION	fi fi	Leg	in	ft	ft <sup>2</sup>	fr <sup>2</sup>	P <sup>2</sup>	K
		В		0 000	0.000	65 888	0 000	1.52
		C		0 000	0.000	96.156	0 000	2.22
T4	240.00-220.00	Ã	2 429	0.000	0.000	54 216	0.000	1.31
4.4	240.00 220.00	В	2 (2)	0.000	0.000	65 670	0.000	1.51
		C		0 000	0.000	108.638	0 000	2 45
T5	220.00-200.00	A	2.407	0.000	0.000	54 068	0.000	1.30
	220,00 200,00	В	2.10)	0 000	0.000	65 435	0.000	1.49
		Č		0.000	0.000	108 139	0 000	2 42
T6	200.00-180.00	A	2.383	0.000	0.000	53 907	0 000	1.29
	200.00 100.00	В	2.303	0.000	0.000	65 178	0.000	1.48
		C		0 000	0.000	107 594	0 000	2 40
T7	180.00-160 00	Ä	2.356	0 000	0.000	53.730	0 000	1.28
	.50.00 100 00	В	4.450	0.000	0.000	64.895	0 000	1.47
		C		0.000	0.000	106 996	0.000	2.37
T8	160 00-140 00	A	2.327	0.000	0.000	53.533	0.000	1.27
	100 00 110.00	В		0.000	0.000	68.421	0.000	1.49
		C		0.000	0.000	106 330	0.000	2 34
T9	140.00-120.00	A	2.294	0.000	0 000	53.312	0 000	1 25
	1.0.00 .20.00	В		0.000	0 000	79 493	0 000	1 58
		č		0.000	0.000	105 579	0 000	2 30
T10	120.00-100.00	A	2.256	0.000	0.000	53.057	0.000	1 24
	120,00	В	2 82 9	0.000	0.000	78.977	0.000	1.56
		C		0.000	0.000	104 716	0.000	2 26
TII	100.00-80.00	A	2.211	0.000	0.000	52.756	0.000	1.22
	season Novel	В		0.000	0.000	78.368	0.000	1.53
		C		0.000	0 000	103 698	0.000	2 22
T12	80 00-60 00	A	2 156	0.000	0.000	52 389	0.000	1 20
	*********	В		0.000	0.000	77.624	0.000	1 50
		C		0 000	0 000	102.453	0.000	2 16
T13	60 00-40 00	A	2 085	0 000	0.000	51.911	0.000	1.17
		В		0 000	0 000	76 658	0.000	1 46
		C		0 000	0.000	100.834	0.000	2.09
T14	40 00-20 00	A	1 981	0.000	0 000	51.217	0.000	1.13
5.2 4	12021-2022	В	2.6(2.2)	0.000	0 000	75 254	0.000	1 40
		C		0.000	0 000	98 479	0.000	1.99
T15	20 00-0 00	Ā	1.775	0 000	0.000	49 843	0 000	1 05
232		В	1,100	0 000	0 000	72 474	0.000	129
		C		0 000	0.000	93 807	0.000	1.79

## **Feed Line Center of Pressure**

Section	Elevation	$CP_X$	CP2	$CP_X$	CP <sub>2</sub>
				Ice	lce
	ſi	in	in	in	in
TI	290 00-280.00	-6 0440	2.5269	-4 1991	1 9665
T2	280 00-260.00	-5 2859	3.4526	-4 4234	3,6711
T3	260 00-240.00	-5 2047	-2.0634	-4.6307	0 2373
T4	240.00-220.00	-6.6195	-1.6863	-7 0257	1 4224
T5	220 00-200 00	-7 5880	-1.8549	-7.2804	1 3960
T6	200 00-180 00	-8 7178	-2.0862	-7 9617	1 4695
T7	180 00-160 00	-9 7798	-2 3069	-9 3024	1 6656
T8	160 00-140 00	-10 1621	-2.8375	-10 0559	1 1845
T9	140 00-120 00	-11.7689	-4.6661	-12 1831	-0 5721
T10	120 00-100 00	-12 8075	-5.0598	-13 2136	-0 6878
T11	100.00-80.00	-13.8031	-5.4391	-14 1893	-0.8162
T12	80.00-60 00	-14 5254	-5.7082	-15 0567	-0.9602
T13	60.00-40.00	-14 7260	-5 8109	-15 5917	-1 1195

Valmont	Job	565090	Page 12 of 72
1545 Pidco Dr.	Project H	I-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 E4Y: (574)-936-6158	Client	VB BTS II, LLC	Designed by

Section	Elevation	$CP_X$	CP.	$CP_{V}$	$CP_{\ell}$	
				Ice	Ice	
	ſi	in	in	in	in	
T14	40.00-20.00	-15 5223	-6.1205	-16.3348	-1.3556	
T15	20 00-0 00	-16.1588	-6 3656	-16 9095	-1 7876	

# Shielding Factor Ka

K <sub>a</sub> lce	K <sub>a</sub> No Ice	Feed Line Segment Elev.	Description	Tower Feed Line Section Record No.			
0.477	0.6000	280.00 - 290.00	Safety Line 3/8	TI Safety Line 3/			
0.477	0.6000	280.00 - 290.00	Lighting power cord	2	TI		
0.477	0.6000	280.00 - 285.00	LDF7-50A (1-5/8 FOAM)	5	Ti		
0.477	0.6000	280.00 - 285.00	1 75" Rails	11	TI		
0.477	0.6000	280.00 - 285.00	1 75" Rails	12	T1		
0.497	0 6000	260.00 - 280.00	Safety Line 3/8	1	T2		
0.497	0.6000	260.00 - 280.00	Lighting power cord	2	T2		
0 497	0 6000	260 00 - 280 00	LDF7-50A (1-5/8 FOAM)	5	T2		
0.497	0 6000	260.00 - 275.00	LDF7-50A (1-5/8 FOAM)	6	T2		
0.497	0 6000	260 00 - 265 00	LDF7-50A (1-5/8 FOAM)	7	T2		
0 497	0 6000	260.00 - 280.00	1 1 75" Rails 260.00		T2		
0 497	0 6000	260 00 - 280 00	1.75" Rails	12	T2		
0 553	0 6000	240 00 - 260 00	Safety Line 3/8	.1	Т3		
0 553	0 6000	240 00 - 260 00		2	Т3		
0 553	0.6000	240 00 - 260 00	LDF7-50A (1-5/8 FOAM)	5	Т3		
0 553	0 6000	240 00 - 260 00	LDF7-50A (1-5/8 FOAM)	6	Т3		
0 553	0 6000	240 00 - 260 00	LDF7-50A (1-5/8 FOAM)	7	Т3		
0 553	0.6000	240 00 - 260.00	1.75" Rails	(1	Т3		
0 553	0.6000	240 00 - 260.00	1 75" Rails	12	Т3		
0 599	0.6000	220 00 - 240.00	T4 1 Safety Line 3/8		T4		
0 599	0 6000	220 00 - 240 00	Lighting power cord	2	T4		
0 599	0 6000	220 00 - 240.00	LDF7-50A (1-5/8 FOAM)	5	T4		
0 599	0 6000	220 00 - 240 00	LDF7-50A (1-5/8 FOAM)	6	T4		
0.599	0 6000	220.00 - 240.00	LDF7-50A (1-5/8 FOAM)	7	T4		

Valmont	Job	Page 13 of 72	
1545 Pidco Dr	Project H	-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>n</sub> No Ice	K.a Ice	
on Record No. T4 8		220 00 -	0 6000	0 5994	
11	1 75" Rails	220 00 -	0 6000	0.5994	
12	1 75" Rails	220 00 -	0 6000	0.5994	
i	Safety Line 3/8	200 00 -	0 6000	0 5668	
2	Lighting power cord	200 00 -	0 6000	0 5668	
5	LDF7-50A (1-5/8 FOAM)	200 00 -	0 6000	0 5668	
6	LDF7-50A (1-5/8 FOAM)	200 00 -	0.6000	0.566	
7	LDF7-50A (1-5/8 FOAM)	200 00 -	0 6000	0 5668	
8	EW63	200 00 -	0 6000	0.5668	
11	I 75" Rails	200 00 -	0 6000	0 5668	
12	1 75" Rails	200 00 -	0 6000	0 5668	
t	Safety Line 3/8	180 00 -	0 6000	0.5683	
2	Lighting power cord	180 00 -	0 6000	0.568	
5	LDF7-50A (1-5/8 FOAM)	180 00 -	0 6000	0.568	
6	6 LDF7-50A (1-5/8 FOAM) 180 00 -	0 6000	0.568		
7	LDF7-50A (1-5/8 FOAM)	180.00 -	0.6000	0.568	
8	EW63	180.00 -	0 6000	0.568	
11	1.75" Rails	180.00 -	0.6000	0.568	
12	1 75" Rails	180.00 -	0.6000	0 568	
1	Safety Line 3/8	160 00 -	0.6000	0.6000	
T7 2 Lighting power cor		160.00 -	0.6000	0.6000	
5	LDF7-50A (1-5/8 FOAM)	160.00 -	0.6000	0.6000	
		160.00 -	0.6000	0 6000	
7	LDF7-50A (1-5/8 FOAM)	160.00 -	0 6000	0 6000	
8	EW63	160 00 -	0.6000	0 6000	
11	1.75" Rails	160.00 -	0.6000	0 6000	
12	12 1.75" Rails 160 00 - 0.0	0.6000	0 600		
1	Safety Line 3/8	140 00 -	0 6000	0 600	
2	Lighting power cord	145.00 -	0 6000	0 600	
3	Lighting power cord	The state of the s	0 6000	0 600	
5	LDF7-50A (1-5/8 FOAM)	145.00 140.00 -	0,6000	0.600	
	8 11 12 1 2 5 6 7 8 11 12 1 2 5 6 7 8 11 12 1 2 5 6 7 8 11 12 1 2 1 1 2 1 1 1 2 1 1 1 1 1 1 1	8 EW63 11 1 75" Rails 12 1 75" Rails 13 1 75" Rails 14 Safety Line 3/8 2 Lighting power cord 25 LDF7-50A (1-5/8 FOAM) 26 LDF7-50A (1-5/8 FOAM) 27 LDF7-50A (1-5/8 FOAM) 28 EW63 29 Lighting power cord 29 LDF7-50A (1-5/8 FOAM) 20 LDF7-50A (1-5/8 FOAM) 21 LDF7-50A (1-5/8 FOAM) 22 Lighting power cord 25 LDF7-50A (1-5/8 FOAM) 26 LDF7-50A (1-5/8 FOAM) 27 LDF7-50A (1-5/8 FOAM) 28 EW63 29 Lighting power cord 29 LDF7-50A (1-5/8 FOAM) 20 LDF7-50A (1-5/8 FOAM) 21 LDF7-50A (1-5/8 FOAM) 22 Lighting power cord 23 LDF7-50A (1-5/8 FOAM) 25 Lighting power cord 26 LDF7-50A (1-5/8 FOAM) 27 LDF7-50A (1-5/8 FOAM) 38 EW63 39 Lighting power cord 30 Lighting power cord 31 Lighting power cord 31 Lighting power cord	S	B	

Valmont	Job	565090	Page 14 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev	K. No Ice	K. Ice	
T8	Record No.	LDF7-50A (1-5/8 FOAM)	140 00 -	0 6000	0 600	
10	Ò	LDF 7-30A (1-3/8 FOAM)	160.00	0.0000	0.000	
Т8	7	LDF7-50A (1-5/8 FOAM)	140 00 -	0 6000	0 600	
10		EDI 7-30N (1-3/01 O/M)	160.00	0 0000	0.000	
T8	8	EW63	140 00 -	0.6000	0 600	
300	1		160.00	4.0.00		
T8	11	1.75° Rails	140 00 -	0 6000	0 600	
			160.00			
T8	12	1.75° Rails	140 00 -	0 6000	0 600	
100		5 0 3 70	160 00	3 65.2	200	
Т9	1	Safety Line 3/8	120.00 -	0 6000	0 600	
TO		Linkson and acceptance	140.00	0 6000	0 600	
Т9	3	Lighting power cord	120.00 - 140.00	0 6000	0.000	
Т9	5	LDF7-50A (1-5/8 FOAM)	120.00 -	0.6000	0.600	
1.7	1	EDI 1-30A (1-3/6 I OAM)	140.00	0.0000	0.000	
T9	6	LDF7-50A (1-5/8 FOAM)	120 00 -	0 6000	0.600	
5.4		95 01 3319 <b>(</b> 1 513 1 519 1 <b>0</b>	140.00	2-2-2-5-5	13/3/24	
T9	7	LDF7-50A (1-5/8 FOAM)	120.00 -	0.6000	0.600	
			140 00			
T9	8	EW63	120 00 -	0.6000	0.600	
	1		140.00			
T9	11	1 75* Rails	120 00 -	0.6000	0.600	
ima	1.0	1	140 00		0.00	
T9	12	1 75" Rails	120 00 -	0 6000	0.600	
7710	-	C-C-+- 1 1/0	140.00	0.6000	0.600	
TIO	1	Safety Line 3/8	100 00 - 120 00	0.0000	0.000	
TIO	3	Lighting power cord	100 00 -	0 6000	0.600	
110		Eighting potter core	120.00	0.0000	0.000	
T10	5	LDF7-50A (1-5/8 FOAM)	100 00 -	0 6000	0 600	
1000		2000	120 00			
T10	6	LDF7-50A (1-5/8 FOAM)	100.00 -	0 6000	0.600	
100			120.00	5.00		
T10	7	LDF7-50A (1-5/8 FOAM)	100 00 -	0 6000	0 600	
		Pitter	120.00	0.4000	0 100	
TIO	8	EW63	100.00 -	0 6000	0.600	
T10	11	1 75" Rails	120.00	0 6000	0 600	
110	1.1	1 75 Kans	120.00	0 0000	0 000	
TIO	12	1.75" Rails		0.6000	0.600	
110	1.2	122 Kuns	120.00	0.0000	0.000	
T11	1	Safety Line 3/8		0 6000	0.600	
T11	3	Lighting power cord		0 6000	0.600	
TII	5	LDF7-50A (1-5/8 FOAM)		0 6000	0 600	
TII	6	LDF7-50A (1-5/8 FOAM)	80 00 - 100 00	0.6000	0 600	
TII	7	LDF7-50A (1-5/8 FOAM)	80 00 - 100 00	0.6000	0 600	
TIL	8		80 00 - 100 00	0.6000	0.600	
T11	11		80 00 - 100 00	0.6000	0.600	
TII	12		80 00 - 100 00	0.6000	0.60	
T12	1	Safety Line 3/8		0.6000	0.60	
T12	3	Lighting power cord		0.6000	0.60	
T12	5	LDF7-50A (1-5/8 FOAM) LDF7-50A (1-5/8 FOAM)		0.6000	0.60	
T12	7	LDF7-50A (1-5/8 FOAM)		0 6000	0 60	
T12	8	EW63		0 6000	0.60	
T12	11	1 75" Rails		0.6000	0.60	
T12	12	1.75" Rails	THE RESERVE OF THE PARTY OF THE	0 6000	0 60	
T13	1	Safety Line 3/8		0 6000	0.60	
T13	3	Lighting power cord	40.00 - 60.00	0 6000	0.60	
T13	5	LDF7-50A (1-5/8 FOAM)	The Control of the Co	0 6000	0 60	
T13	6	LDF7-50A (1-5/8 FOAM)	40 00 - 60.00	0 6000	0 60	

Valmont	Job 565090	Page 15 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev	K., No Ice	K. Ice	
T13	7	LDF7-50A (1-5/8 FOAM)	40.00 - 60.00	0.6000	0.6000	
T13	8	EW63	40.00 - 60 00	0.6000	0 6000	
T13	11	1.75" Rails	40.00 - 60.00	0 6000	0 6000	
T13	12	1 75" Rails	40.00 - 60.00	0.6000	0 6000	
T14	1	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000	
T14	3	Lighting power cord	20.00 - 40.00	0.6000	0.6000	
T14	5	LDF7-50A (1-5/8 FOAM)	20.00 - 40.00	0 6000	0 6000	
T14	6	LDF7-50A (1-5/8 FOAM)	20.00 - 40 00	0.6000	0.6000	
T14	7	LDF7-50A (1-5/8 FOAM)	20.00 - 40 00	0 6000	0.6000	
T14	8	EW63	20.00 - 40.00	0 6000	0 6000	
T14	11	1 75" Rails	20.00 - 40.00	0 6000	0 6000	
T14	12	1 75" Rails	20.00 - 40.00	0 6000	0.6000	
TIS	1	Safety Line 3/8	0.00 - 20.00	0 6000	0.6000	
T15	3	Lighting power cord	0.00 - 20.00	0.6000	0.6000	
TI5	5	LDF7-50A (1-5/8 FOAM)		0 6000	0.6000	
T15	6	LDF7-50A (1-5/8 FOAM)	0.00 - 20.00	0.6000	0.6000	
T15	7	LDF7-50A (1-5/8 FOAM)		0.6000	0.6000	
T15	8	EW63		0 6000	0 6000	
T15	11	1 75" Rails	(2020 M) 1000 (1000 M)	0 6000	0 6000	
T15	12	1.75" Rails		0 6000	0.6000	

Description	Face or Leg	Offset Type	Offsets. Horz Lateral	Azimuth Adjustinent	Placement		C <sub>1</sub> A <sub>.1</sub> Front	C <sub>v</sub> A <sub>A</sub> Side	Weight
			Vert fi fi fi	0	fi		ft²	ft <sup>2</sup>	К
5/8" x 10' lightning rod	C	From Leg	0 00 0 00 5 00	0 0000	290 00	No Ice 1/2" Ice 1" Ice	0.63 1.63 2.63	0.63 1.63 2.63	0 02 0 03 0 04
Beacon	В	From Leg	0 00 0 00 1 00	0 0000	290 00	No Ice 1/2" Ice 1" Ice	2.40 2.67 2.96	2 40 2 67 2 96	0 07 0 10 0 12
*						4 1-7			7.07
OB light	A	From Leg	0 00 0 00 1 00	0 0000	145 00	No Ice 1/2" Ice 1" Ice	0.50 0.60 0.70	0 50 0 60 0 70	0.03 0.04 0.04
OB light	В	From Leg	0.00 0.00 1.00	0 0000	145 00	No Ice 1/2" Ice 1" Ice	0.50 0.60 0.70	0 50 0 60 0 70	0.03 0.04 0.04
OB light	С	From Leg	0.00 0.00 1.00	0.0000	145 00	No Ice 1/2" Ice 1" Ice	0.50 0.60 0.70	0.50 0.60 0.70	0.03 0.04 0.04
40,000 sq in (277 8 sq ft EPA)	Α	None		0.0000	285 00	No Ice 1/2" Ice 1" Ice	277 80 347 25 416 70	277 80 347 25 416 70	4 50 5 50 6 50
30,000 sq in (208.3 sq ft EPA)	С	None		0.0000	275.00	No Ice 1/2" Ice 1" Ice	208 30 260 38 312 46	208 30 260 38 312 46	4 10 5 20 6 30
30,000 sq in (208.3 sq ft EPA)	В	None		0.0000	265 00	No Ice 1/2" Ice	208 30 260 38	208 30 260 38	4 10

**Discrete Tower Loads** 

Valmont	almont Job 565090			
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135	Date 07:03:40 10/06/22		
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by		

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Fert	Azimuth Adjustment	Placement		C <sub>1</sub> A <sub>1</sub> Front	C (A) Side	Weighi
			fi fi	a	ſŧ		fi	ft <sup>2</sup>	K
****						I = Ice	312.46	312.46	6 30
SP1 R5 (Includes 4 5"x72"	C	From Leg	0.50	0.0000	240.00	No Ice	2 85	3.15	0 14
Pipe)			0.00			1/2" Ice	3 36 3 90	3.69 4.26	0.17
2-1/2" x 7' Sch 40	В	From Face	0.00	0.0000	240 00	No Ice	2 0 1	2 01	0.04
			0.00			1/2" Ice 1" Ice	2 55	2.55 2.92	0 06
2-1/2" x 7 Sch 40	C	From Face	0.00	0.0000	240.00	No Ice	201	2.01	0 04
			0.00			1/2" Ice 1" Ice	2.55	2 55 2 92	0.06

Dishes											
Description	Face or Leg	Dish Type	Offset Type	Offsets Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				ft	0	0	JI	ft.		Jī <sup>2</sup>	K
6' HP	C	Paraboloid	From	1 00	0.0000		240.00	6 00	No Ice	28.27	0 30
		w/Shroud (HP)	Leg	0 00					1/2" Ice	29.07	0 52
			100	0 00					1" Ice	29.86	0.74

			Tru	ss-Leç	s-Leg Properties					
Section Designation	Area	Area Ice	Self Weight	lce Weight	Equiv Diameter	Equiv Diameter Ice	Leg Area			
	$m^{\Gamma}$	in <sup>2</sup>	K	K.	in	in	in <sup>2</sup>			
#12ZG-58 - 1 50" - 1 00" conn (Pirod 194651)	2010.3106	6026 8591	0.62	2.07	6 9802	20 9266	5 3014			
#12ZG-58 - 1.75" - 1.00" connTR1-(Pirod 195213)	2035 9652	7211 2772	0 79	2.09	7 0693	25 0392	7.2158			
#12ZG-58 - 1.75" - 1 00" conn (Pirod 195217)	2035 9652	7187 9803	0.79	2 06	7 0693	24.9583	7 2158			
#12ZG-58 - 1 75" - 1 00" conn (Pirod 195217)	2035 9652	7162.0721	0.79	2 01	7 0693	24.8683	7 2 1 5 8			
#12ZG-58 -2 00" - 0.875" conn -TR3-(Pirod 195637)	2339 7677	6176 2066	1.00	2 02	8 1242	21 4452	9 4248			
#12ZG-58 -2 00" - 0 875" conn. (Pirod	2339 7677	6154 8760	1 00	1 96	8 1242	21 3711	9 4248			

Valmont	Job	565090	Page 17 of 72
1545 Pideo Dr.	Project H-31 x290' S	ST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Section Designation	Area	Area Ice	Self Weight	Ice Weight	Equiv Diameter	Equiv. Diameter Ice	Leg Area
	in <sup>2</sup>	in <sup>2</sup>	K	K	in	in	in
195639)		- NAME OF			1 2 2 2 2 2	- Care C. /	
#12ZG-58 -2.00" -	2339 7677	6129 7197	1 00	1 90	8 1242	21 2837	9 4248
0 875" conn. (Pirod 195639)							
#12ZG-58 -2 25" -	2475 7141	6170 9183	1.17	1 84	8 5962	21 4268	11 9282
0.875" conn (Pirod 195960)							
#12ZG-58 -2 25" -	2475 7141	6130 8735	117	1 74	8 5962	21 2878	11 9282
0.875" conn (Pirod 195960)							
#12ZG-58 -2 25" -	2475.7141	6072 5831	1.17	1.60	8.5962	21 0854	11.9283
0.875" conn (Pirod 195960)							
#12ZG-58 BASE -	2548 5522	5961 8648	1.29	1.32	8.8491	20 7009	14 7262
2 50" - 0.875"							
conn -TR4-(Pirod							
281171)							

### Tower Pressures - No Ice

 $G_H = 0.850$ 

Section	2	K <sub>≥</sub>	$q_z$	An,	F	AF	As	Aire	Leg	C,A,	$C_{AA,i}$
Elevation				100	a				%	ln	Out
				6.2	C	6.5	A)	1.5		Face	Face
fl	ft		psf	ft <sup>2</sup>	2	ft	ſŧ	ft'	1000	fi <sup>7</sup>	fi.
T1	285.00	1.578	39	52.396	A	5 022	4.792	4.792	48 83	0.000	0.000
290.00-280.00		0.00		1334	В	5 022	4.792		48.83	0.870	0.000
	200 Ed 1 200		5.41	55.036	C	5 022	4 792	3.5945.6	48 83	15 172	0.000
T2	270.00	1.56	38	107.500	A	8.961	15.000	15,000	62 60	11 880	0.000
280.00-260.00	100	-		1000000	В	8.961	15 000	-	62 60	37 380	0.000
	VG6.85	56.25	Lile.	0.00.00	C	8.961	15 000	000.00	62 60	59 937	0.000
T3	250 00	1 535	38	129 283	A	7 669	18 574	18 574	70 78	47 520	() 000
260 00-240 00					В	7 669	18 574		70 78	49 260	0.000
	to contain	and the second			C	7 669	18 574	200	70 78	59.937	0 000
T4	230.00	1 508	37	171 054	A	11.361	22 120	22 120	66.07	47 520	0.000
240 00-220 00			17.21	6-00	B	11.361	22 120		66.07	49 260	0.00
277.1					C	11.361	22 120	1	66.07	63 085	0.00
TS	210 00	1 48	36	222 527	A	10.261	23 306	23 306	69 43	47 520	0.00
220 00-200 00	4.00			-	В	10.261	23.306		69 43	49.260	0.00
	100			9.779	C	10.261	23.306		69 43	63 085	0.00
T6	190 00	1 449	35	262 944	A	11 439	23 604	23 604	67 36	47 520	0.00
200 00-180 00	27,752		97.	1.3.2	В	11.439	23 604	4.700	67 36	49 260	0.00
					C	11.439	23 604		67.36	63 085	0.000
T7	170 00	1 415	35	302.944	A	12.727	23 604	23 604	64 97	47.520	0.000
180.00-160.00	340745	100000		-	В	12 727	23.604	1200000	64.97	49 260	0.000
22002					C	12 727	23.604		64.97	63.085	0.000
T8	150.00	1.378	34	342 944	A	16 913	23.604	23 604	58.26	47 520	0.000
160.00-140.00	*********	13,347,24	117	100000	В	16 913	23.604		58.26	50 130	0.000
			201	100	C	16 913	23 604		58.26	63.085	0.000
T9	130.00	1.337	33	383 361	A	12 514	27 126	27 126	68 43	47 520	0.00
140 00-120 00		2000			В	12 514	27 126		68.43	52 740	0.00
					C	12 514	27.126		68.43	63.085	0 00
T10	110.00	1 291	32	423 361	A	13 178	27 126	27 126	67 30	47.520	0.00
120 00-100 00	110.00	00.0	-		В	13 178	27 126	27,129	67 30	52 740	0 00
123.00 100 00		1 1			Č	13.178	27 126		67 30	63.085	0.00

Valmont	Job 565090	Page 18 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5	Date 07:03:40 10/06/22
Plymouth, 1N Phone (574)-936-4221 FAX (574)-936-6458	Client VB BTS II, L	LC Designed by

Section Elevation	s	Kz	qz	Ac	F a	$A_F$	AR	Airy	Leg %	C <sub>1</sub> A <sub>3</sub> In	C <sub>4</sub> A <sub>4</sub> Out
fi	ſŧ		psf	ft <sup>2</sup>	e	ſt²	ft²	ſt²		Face ft <sup>2</sup>	Face fr
T11	90.00	1.238	30	463 361	A	13 884	27 126	27.126	66.15	47 520	0 000
100 00-80 00	2.74			Vertical A	В	13.884	27 126		66.15	52 740	0.000
		1.00		20.00	C	13.884	27 126	77.00	66 15	63 085	0 000
T12	70 00	1.174	29	503.778	A	14.623	28 702	28 702	66.25	47 520	0.000
80 00-60 00	100		40		В	14.623	28.702	45.44.5	66.25	52 740	0.000
200000000000000000000000000000000000000			1.00		C	14.623	28.702		66.25	63 085	0 000
T13	50 00	1.094	27	543 778	A	17.958	28 702	28 702	61 51	47.520	0 000
60.00-40.00	100				В	17 958	28.702	C75 C25.	61.51	52 740	0 000
20.00.00.00.00.00.00.00.00.00.00.00.00.0	- 4			100	C	17 958	28.702		61.51	63 085	0.000
T14	30 00	0.982	24	583 778	A	18 884	28 702	28 702	60.32	47 520	0 000
40.00-20.00		4.355	- 61		В	18 884	28.702		60.32	52 740	0 000
In the same of the		200		9.00	C	18.884	28.702		60.32	63 085	0 000
T15 20 00-0 00	10.00	0.85	21	624 196	A	19.836	29.546	29.546	59.83	47 520	0.000
	1	1,000	0.51		В	19 836	29.546		59.83	52 740	0.000
		150			C	19.836	29 546		59.83	63 085	0.000

## **Tower Pressure - With Ice**

 $G_H = 0.850$ 

Section Elevation	2	Kz	$q_z$	12	Air	F	AF	Ak	Aleg	Leg %	C.A.	C <sub>s</sub> A <sub>s</sub>
270,000	1					c				5,4	Face	Face
fi	fi		psf	in	ft2	e	ft <sup>2</sup>	fr	fr		ft <sup>2</sup>	ſŕ
TI	285 00	1.578	3	2.4812	56.531	A	5.022	24.538	13 062	44.19	0.000	0.000
290.00-280.00	72.00				200	В	5 022	24.538		44.19	5.832	0.000
200						C	5 022	24 538		44.19	26 859	0.000
T2	270.00	1.56	3	2.4678	115 726	A	8 961	49.143	31 452	54.13	13.620	0.000
280.00-260.00		200	100			В	8 961	49.143		54.13	52 471	0.000
-22,7-12,000		1		A		C	8 961	49 143		54.13	96 511	0.000
Т3	250.00	1 535	3	2 4489	137 456	A	7 669	53 709	34 927	56.91	54 353	0.000
260 00-240.00				1		В	7.669	53 709		56.91	65.888	0.000
						C	7 669	53 709		56 91	96 156	0.000
T4	230.00	1 508	3	2.4286	179 160	A	11 361	60 410	38.338	53.42	54 216	0.000
240 00-220 00		6.7		10000	200	В	11 361	60 410		53.42	65.670	0.000
					100	C	11.361	60 410	7.00	53.42	108.638	0.000
TS	210.00	1.48	3	2,4066	230.559	A	10 261	89 627	69 871	69 95	54 068	0.000
220 00-200 00	-11.00	100			2000	В	10 261	89 627	12.32.3	69 95	65.435	0.000
		100	1			C	10 261	89 627		69 95	108 139	0.000
T6	190.00	1 449	3	2.3826	270 896	A	11 439	105 407	83.603	71 55	53 907	0.000
200 00-180 00	-		-		1387.575	В	11 439	105 407	220,000	71.55	65 178	0.000
SAN A PACACA						C	11 439	105 407		71.55	107 594	0.000
T7	170.00	1.415	3	2.3563	310 808	A	12 727	107 323	83.333	69 42	53 730	0.000
180 00-160 00		200			773-5-0-3	В	12 727	107 323	1,000	69.42	64 895	0.000
,						C	12.727	107 323		69.42	106 996	0.000
T8	150.00	1 378	3	2 3270	350 710	A	16 913	109 270	83.032	65 80	53 533	0 000
160 00-140 00	-			a.v.		В	16 913	109 270	10.00	65 80	68 421	0 000
	100					C	16.913	109 270		65 80	106 330	0.000
Т9	130.00	1 337	3	2.2939	391 017	A	12 514	90 739	71 603	69 35	53 312	0 000
140.00-120.00		9.0			1/2/2/2	В	12.514	90 739	20.00	69 35	79 493	0.000
1.0450-00414.1			0.00			C	12 514	90.739		69 35	105 579	0.000
T10	110 00	1 291	3	2 2559	430 890	A	13 178	91.175	71.356	68 38	53 057	0 000
120.00-100.00	7.5	10000		The second	20715	В	13 178	91 175		68 38	78 977	0 000
377453043430		2.7				C	13 178	91 175		68 38	104 716	0.000
TII	90.00	1 238	2	2 2111	470 741	A	13 884	91 529	71.064	67.42	52 756	0 000
100 00-80.00	100.03		-		3.500.301.0	В	13 884	91 529	22,000	67 42	78 368	0 000

Valmont	Job	565090	Page 19 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Section Elevation ft	s ft	Kz	q <sub>i</sub>	1 <sub>2</sub>	Aci	F a c	A <sub>F</sub>	A <sub>R</sub>	Alex	Leg %	C <sub>1</sub> A <sub>1</sub> In Face It <sup>2</sup>	C <sub>1</sub> A <sub>.1</sub> Out Face fr
,	-		1.5		1	C	13.884	91.529		67.42	103.698	0.000
T12 80:00-60:00	70.00	1 174	2	2 1562	510.975	A	14.623	92.562	71 542	66.75	52 389	0.000
	* 4.000	25.13			0.8263%	В	14 623	92.562	36.5	66.75	77.624	0.000
		1.0			-	C	14.623	92.562		66.75	102,453	0 000
T13 60.00-40.00	50.00	1.094	2	2.0849	550.737	A	17.958	92.471	71 077	64.36	51.911	0.000
ALD OLLOWING THE					333.31	В	17.958	92.471	-321300	64.36	76.658	0.000
					100	C	17.958	92.471		64.36	100.834	0.000
T14 40.00-20.00	30.00	0 982	2	1 9810	590.390	A	18 884	91.779	70 402	63.62	51 217	0.000
				0.31	7.3.3	В	18 884	91.779	-	63.62	75 254	0 000
						C	18 884	91 779		63.62	98 479	0.000
T15 20.00-0.00	10.00	0 85	2	1 7749	630 119	A	19 836	89 237	69 118	63.37	49 843	0.000
				11.47.5	11100	В	19 836	89.237		63.37	72.474	0.000
						C	19.836	89.237		63.37	93 807	0.000

# Tower Pressure - Service

 $G_H=0.850$ 

Section Elevation	3	Kz	q <sub>z</sub>	Aa	Fa	AF	$A_R$	Aleg	Leg %	C <sub>1</sub> A <sub>A</sub> In	C <sub>1</sub> A <sub>4</sub> Out
5	1.0			4.5	C		44			Face	Face
ft	fi		psf	ft2	e	fi²	ft <sup>2</sup>	fr <sup>2</sup>		ft²	ft <sup>-</sup>
TI	285.00	1.578	12	52 396	A	5 022	4 792	4 792	48 83	0 000	0.000
290.00-280.00				10.00	В	5.022	4 792	1,011	48 83	0.870	0.000
- The state of the	200	5.65	3.5		C	5 022	4.792	1.777	48 83	15.172	0.000
T2	270.00	1 56	12	107 500	A	8.961	15 000	15 000	62 60	11 880	0.000
280 00-260.00					B	8.961	15.000		62 60	37.380	0.000
					C	8.961	15.000		62 60	59.937	0.000
T3	250 00	1 535	12	129 283	A	7.669	18 574	18 574	70 78	47.520	0 000
260 00-240 00				( )	В	7 669	18 574		70.78	49,260	0.000
				100	C	7.669	18 574	45.70	70.78	59 937	0.000
T4	230.00	1 508	12	171 054	A	11 361	22.120	22 120	66.07	47.520	0.000
240 00-220.00			100	1	В	11 361	22 120	-	66 07	49 260	0 000
	F (7)	1		A	C	11.361	22 120		66 07	63.085	0.000
T5	210 00	1.48	12	222 527	A	10 261	23 306	23 306	69 43	47.520	0.00
220 00-200.00	0.00		9.7	1000	В	10 261	23 306		69 43	49 260	0.000
					C	10 261	23 306		69 43	63.085	0.000
T6	190 00	1 449	11	262 944	A	11.439	23 604	23 604	67 36	47.520	0.000
200 00-180 00	· COLUMN				В	11 439	23.604		67 36	49 260	0.000
A 10 A 5 COLUMN	0.00		. 654	1000	C	11 439	23 604		67 36	63.085	0.000
T7	170 00	1415	11	302 944	A	12 727	23.604	23.604	64.97	47.520	0.000
180 00-160 00	- 10 CM		100		В	12 727	23.604	100000	64.97	49.260	0.000
					C	12 727	23.604		64.97	63.085	0.000
T8	150 00	1.378	11	342 944	A	16 913	23.604	23.604	58.26	47 520	0.000
160 00-140.00	25000	10000	100		В	16 913	23.604		58.26	50 130	0.000
-0.2 × 20.00 do 10.00					C	16 913	23.604		58.26	63 085	0.000
T9	130.00	1.337	10	383.361	A	12 514	27.126	27 126	68.43	47.520	0.000
140.00-120.00	1.50	11000	-	2000	В	12 514	27.126		68.43	52 740	0.000
137171074817170					C	12 514	27 126		68 43	63 085	0.000
T10	110.00	1.291	10	423.361	A	13.178	27 126	27 126	67.30	47.520	0.00
120.00-100.00	100000				В	13.178	27.126	2,0-7.52	67.30	52 740	0.000
				107-4	c	13 178	27 126		67 30	63 085	0.000
TII	90.00	1 238	10	463.361	A	13.884	27 126	27 126	66 15	47 520	0.00
100.00-80.00		1,000	2.5	235 538 0	В	13.884	27.126	201192	66 15	52 740	0.00
					C	13.884	27 126		66 15	63 085	0 00

Valmont	Job 565090	Page 20 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fa	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Section Elevation ft	î fi	Kz	q <sub>z</sub>	A <sub>G</sub>	F a c	A <sub>F</sub>	A <sub>R</sub>	A <sub>log</sub>	Leg %	C.A. In Face ft <sup>2</sup>	C <sub>1</sub> A <sub>1</sub> Out Face ft
T12	70.00	1.174	9	503 778	A	14.623	28.702	28 702	66.25	47.520	0.000
80.00-60.00		200		00.784.47	В	14.623	28 702	0.00	66.25	52.740	0 000
5.54.54.36.3					C	14.623	28 702		66.25	63.085	0.000
T13	50.00	1.094	9	543.778	A	17.958	28.702	28.702	61.51	47.520	0.000
60.00-40.00					В	17.958	28.702	100	61.51	52.740	0.000
				100	C	17.958	28.702		61.51	63.085	0.000
T14	30.00	0.982	8	583 778	A	18.884	28.702	28.702	60.32	47.520	0 000
40.00-20.00				7.77	В	18 884	28.702	200	60.32	52.740	0.000
					C	18 884	28 702		60.32	63.085	0.000
T15 20.00-0.00	10 00	0.85	7	624 196	A	19.836	29 546	29 546	59.83	47.520	0.000
					В	19.836	29 546		59.83	52 740	0.000
					C	19.836	29.546		59.83	63.085	0.000

# **Tower Forces - No Ice - Wind Normal To Face**

Section Elevation	Add Weight	Self Weight	F a	e	$C_F$	q:	$D_F$	$D_R$	AE	F	W	Ctrl. Face
ft	K	K	c e			psf			St <sup>2</sup>	K	plf	
TI	0 08	0.38	A	0 187	2.639	39	1	1	7 764	0.92	91 74	C
290 00-280.00		2.50	В	0 187	2.639	11000	1	1	7.764		20,000	
7 11 11 11			C	0.187	2 639		1	1	7.764		200	
T2	0.51	1.10	A	0 223	2 521	38	1	1	16.654	2.64	132.05	C
280.00-260.00	2.0		В	0 223	2 521		1	1	16.654			
	7.4		C	0 223	2 521		1	1	16 654	-		
T3	0.71	1 28	A	0 203	2.586	38	1	1	16.212	2 95	147 38	C
260.00-240.00	2.7	9.22	В	0 203	2 586		1	1	16 212	-		
			C	0.203	2 586		1	1	16 212			
T4	0.72	1.73	Α	0.196	2.61	37	1	1.	21.167	3.37	168 67	C
240.00-220 00		-	В	0 196	2.61		1	1	21 167		0.00	
			C	0 196	2.61	100	1	1	21 167			
T5	0.72	2.33	A	0 151	2.768	36	-1	1	23 490	3.61	180.50	C
220 00-200 00		100	В	0.151	2.768	1	1	15	23 490	2500	27.327	100
2.604.4.75.4.4.4.4			C	0.151	2.768		î	i i	23 490			100
T6	0.72	3.07	A	0.133	2 834	35	I	1.1	24 800	3.69	184.64	C
200 00-180 00	3.00		В	0.133	2.834	201	1	Ĺ	24 800		130.000	
			C	0 133	2 834		1	i i	24 800			
T7	0.72	3 15	A	0.12	2.885	35	1	1	26.067	3 75	187.62	C
180 00-160 00		2007	В	0.12	2.885		1	Ť.	26 067	7.75	13.1,160	100
			C	0 12	2 885		1	î	26 067			
T8	0 72	3 16	A	0.118	2 892	34	1	î	30.251	4.02	201 07	В
160.00-140.00		2.10	В	0.118	2.892	1000	i	Î.	30 251	1.02	40.00	125
140,00			c	0 118	2 892		i	1	30 251			
Т9	0.72	4 16	A	0 103	2 95	33	î	1	27 825	3.80	189 78	В
140.00-120.00	0,12	7.10	8	0 103	2 95	-	1	ĩ	27.825	3.00	102 10	
140.00 120.00			C	0 103	2 95		1	i	27 825	7.1		
T10	0 72	4 22	A	0.095	2 983	32	1	î	28 484	3 74	187 10	В
120.00-100.00	0.72	7 22	В	0.095	2 983	3.5	1	î	28 484	277	107 10	D
120.00-100.00	- 3		č	0.095	2 983		1	1	28 484			
TII	0 72	4 29	A	0.089	3 011	30	î	1	29 187	3.66	183 08	В
100.00-80.00	0 72	4.4.2	В	0.089	3.011	20	- 3	i i	29.187	5.00	102.00	
100.00-80.00			c	0.089	3.011		1	i	29.187			
T12	0 72	4 89	Ã	0.086	3 021	29	1	10	30 816	3.60	180 01	В
80 00-60 00	0 72	4.07	B	0.086	3 021	29	1	- 5	30.816	5.00	100.01	D
80 00-00.00			C	0.086	3 021		1	1	30.816			
T13	0 72	5 75	1	0.086	3 022	27	1		34 150	3 58	179 18	В
60 00-40 00	0 /2	3 /3	A B	0.086		21	1	1	34.150	3 38	1/7 18	D
00.00-40.00			D	0.000	3.022	1		4	34.130		_	

Valmont	Job 565090	Page 21 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Section Elevation	Add Weight	Self Weight	Fa	e	$C_F$	q. psf	$D_{\mathbb{F}}$	$D_{\kappa}$	As	F	15	Ctrl. Face
ft	K	K	e		1				ſt²	K	plf	
	1.5		C	0 086	3 022		1	1	34 150	100	TO THE	
T14	0.72	5.87	A	0.082	3 039	24	4	1	35 077	3 29	164 41	В
40 00-20 00	2.7	2.00	В	0.082	3.039	i nan	1	1	35 077		2017	
	100		C	0 082	3 039	1.0	1	1	35 077	4.3		
T15	0.72	6.32	A	0.079	3.049	21	1	1	36 506	2 93	146 43	В
20 00-0 00	14		В	0.079	3.049		1	1	36 506	15.43		
			C	0.079	3 049		1	T.	36 506	0.0		
Sum Weight	9.93	51.68		5.070				ОТМ	6981 92 kip-ft	49 56		

## Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a	e	C,	q:	$D_{\ell}$	$D_{k}$	AE	F	19	Ctrl. Face
û	K	K	e			psf			R	K	plf	
TI	0 08	0.38	A	0.187	2.639	39	0.8	1	6.760	0.83	83.05	Α
290.00-280.00	0.00	0.00	В	0 187	2 639	30	0.8	1	6 760			30
200.00	10.00		C	0 187	2 639		0.8	il	6 760			
T2	0.51	1 10	A	0 223	2 521	38	0.8	i	14 861	2 49	124 72	A
280.00-260 00	0.5.		В	0.223	2 521	20	0.8	î	14.861	-	2011/4	2.0
200.00 200 00			C	0 223	2 521		0.8	i	14 861			
Т3	0.71	1 28	A	0 203	2 586	38	08	1	14 678	2 82	141 06	C
260 00-240 00	0,71	7.20	В	0 203	2 586	30	0.8	1	14 678	2 02	11100	
200 00 210 00			C	0 203	2.586		08	1	14 678			
T4	0.72	1.73	A	0 196	2.61	37	08	1	18 895	3.19	159 37	C
240 00-220 00	0.72	1,10	В	0 196	2.61	2.	0.8	1	18 895	2,117	10.01	
240 00-220 00			c	0.196	2.61		0.8	i	18 895			
T5	0.72	2 3 3	A	0.151	2 768	36	0.8	1	21 438	3 44	171 77	C
220 00-200 00	0.72	2 32	B	0.151	2 768	50	0.8	1	21 438	234		-
220 00-200 00			c	0.151	2.768		08	1	21.438	-		
Т6	0.72	3 07	Ã	0.133	2 834	35	08	- 11	22 512	3 50	174 88	C
200 00-180 00	0.72	307	B	0.133	2.834	55	0.8	- 2	22 512	3.30	174 00	-
200 00-180.00			č	0.133	2.834	1	0.8	il	22 512			
17	0.72	3 15	A	0.133	2.885	35	0.8	1	23 521	3.54	176 82	C
180.00-160.00	0.72	3.13	B	0 12	2 885	33	0.8	1	23 521	2.24	170.02	-
160.00-100.00			C	0 12	2 885		0.8	1	23 521			
Т8	0.72	3 16	A	0.118	2 892	34	0.8	1	26.868	3 74	187 06	C
160 00-140 00	0.72	310	B	0.118	2 892	34	0.8	i	26.868	3.74	107 00	-
100 00-140 00			C	0 118	2 892		0.8	1	26.868			
Т9	0 72	4 16	A	0 103	2.95	33	0.8	î	25.322	3 59	179 52	C
140.00-120.00	0 /2	410	B	0 103	2 95	22	0.8	1	25.322	3.37	117 52	-
140.00-120.00	-		c	0 103	2.95		0.8	1	25 322			
T10	0.72	4 22	A	0.095	2.983	32	0.8	1	25.849	3 53	176.55	C
120.00-100 00	0.12	4 22	B	0 095	2 983	32	0.8	1	25 849	333	110.55	~
120.00-100.00			C	0.095	2.983		0.8	1	25 849	1		
TIL	0.72	4 29	A	0.033	3.011	30	0.8	1	26 411	3 45	172 33	C
100 00-80 00	0 12	4.43	B	0.089	3 011	50	08	7	26 411	3.43	11233	
100.00-80.00			C	0.089	3 011		0.8	1	26 411			
T12	0 72	4.89	A	0 086	3 021	29	0.8	1	27 891	3 38	169 23	C
80 00-60 00	0.72	4.89	B	0 086	3.021	29	0.8	1	27 891	3 38	109 23	
90 00-00 00			C	0.086	3 021	1	0.8	1	27 891			
7713	0.72	5.75	- 60	0.086	3.021	27	08		30 559	3.34	166 85	С
T13	0.72	3.75	A		100000000000000000000000000000000000000	21	0.8	1	30 559	3.34	100 83	C
60.00-40.00			B	0.086	3 022		0.8	14	30 334			

Valmont	Job 565090	Page 22 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Fa	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Section Elevation	Add Weight	Self Weight	F a c	e	C <sub>F</sub>	q. psf	D <sub>F</sub>	D <sub>B</sub>	$A_E$	F	1V	Ctrl Face
fi	K	K	e		1				ft <sup>2</sup>	K	plf	
			C	0.086	3.022		0.8	1	30.559			
T14	0.72	5 87	A	0.082	3.039	24	0.8	1	31.300	3.05	152.69	C
40.00-20.00	30.7	- 2.5	В	0.082	3.039	1.00	0.8	1	31.300		1	
COLOR STATE			C	0.082	3.039		08	1.	31 300		100.00	
TI5	0.72	6 32	A	0.079	3.049	21	0.8	1	32 539	2.71	135.74	C
20 00-0.00	1 1 1 1 1 1 1		В	0.079	3.049		0.8	1	32.539		3.000	
120000000000000000000000000000000000000			C	0.079	3.049		0.8	1	32 539			
Sum Weight	9.93	51.68						ОТМ	6584 84 kip-ft	46.60		

### Tower Forces - No Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F	е	C <sub>F</sub>	q.	$D_F$	D <sub>k</sub>	Ai	F	19	Ctrl. Face
ſ	K	K	e			psf			ft <sup>2</sup>	K	plf	
TI	0.08	0.38	A	0 187	2 639	39	0.85	- 1	7011	0.82	82 30	В
290 00-280 00	2000	3.4.5	В	0.187	2 639		0.85	1	7011		4.4	
		100	C	0.187	2.639		0.85	1	7.011			
T2	0.51	1.10	A	0.223	2,521	38	0.85	1	15 309	2 56	128 00	A
280.00-260.00		35.00	В	0.223	2.521	7.77	0.85	1	15 309	10000		
	7.1		C	0 223	2,521		0.85	1	15 309			
T3	0.71	1.28	A	0.203	2 586	38	0.85	1	15 061	2 85	142 64	C
260 00-240.00	137-517	7.04	В	0 203	2.586		0.85	1	15.061	30.7	2050	
creation in city of	1		C	0 203	2.586		0.85	î	15 061	- 3		
T4	0.72	1.73	A	0.196	2.61	37	0.85	1	19 463	3 23	161 70	C
240 00-220.00		-12042	В	0.196	2.61		0.85	1.	19 463			
			C	0 196	2.61		0.85	T	19 463			
T5	0 72	2.33	A	0 151	2.768	36	0.85	1	21 951	3 48	173 95	C
220 00-200.00		-100	В	0.151	2.768	16.6	0.85	1	21 951	7776	6,000,000	1000
			C	0.151	2.768		0.85	1	21.951			
T6	0 72	3.07	A	0.133	2.834	35	0.85	1	23 084	3 55	177 32	C
200.00-180.00		2.0.1	В	0.133	2.834	2.0	0.85	1	23 084	2.00	313188	
200.00 100.00			$\tilde{c}$	0.133	2.834		0.85	1	23.084			
T7	0.72	3 15	Ā	0.12	2.885	35	0.85	1	24 158	3 59	179 52	C
180.00-160.00	0.72	2.12	В	0.12	2.885		0.85	1	24 158	2.2.		-
100.00			C	0 12	2.885		0.85	1	24.158			
Т8	0 72	3 16	A	0.118	2.892	34	0 85	11	27 714	3 81	190.57	C
160.00-140.00		2.0	В	0.118	2.892	-	0.85	1	27 714			-
100.00 110.00			c	0.118	2.892		0 85	il	27 714			
Т9	0 72	4 16	A	0.103	2 95	33	0.85	Ŷ.	25 948	3 64	182 09	C
140.00-120.00		,,,,	В	0.103	2.95	-	0 85	11	25 948			
1.0.00 120.00			C	0.103	2.95		0 85	1	25 948			
T10	0 72	4 22	A	0.095	2 983	32	0 85	î	26 508	3 58	179.18	C
120.00-100.00	0,12	,	В	0.095	2 983	0.2	0 85	1	26 508		173.10	
120.00-100.00			C	0.095	2 983		0.85	î	26 508	1		
TIL	0 72	4 29	A	0.089	3.011	30	0.85	î	27 105	3.50	175.01	C
100.00-80.00	0 72	427	В	0.089	3 011	50	0 85	í	27 105	3,50	172.01	
100.00-00 00	100		C	0.089	3.011		0.85	1	27 105			
T12	0 72	4.89	A	0.086	3 021	29	0.85	1	28 622	3 44	171 93	C
80.00-60.00	0 72	4.07	B	0.086	3.021	49	0.85	1	28 622	2.79	171.93	-
60.00*00.00			C	0.086	3.021		0.85	î	28 622			
T13	0.72	5.75	A	0.086	3.022	27	0.85	î	31 457	3.40	169 93	С
60.00-40.00	0.72	3.13	B	0.086		41	0.85	i	31 457	3.40	107.73	
00.00-40.00			D	0.080	3 022		0.63	1	31437	1		

Valmont	Job 565090	Page 23 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by

Section Elevation	Add Weight	Self Weight	F a c	e	C,	q: psf	$D_F$	$D_R$	AE	F	34	Ctrl Face
ft	K	K	e		1				ſt²	K	plf	
		7	C	0.086	3.022		0.85	1	31 457			
TI4	0.72	5.87	A	0.082	3.039	24	0.85	12	32.244	3.11	155 62	C
40.00-20 00		1	В	0.082	3 039	1-4	0.85	1	32.244	1000	A. A. C.	
A		100	C	0.082	3 039	100	0 85	1	32.244		5	
T15	0.72	6.32	A	0.079	3.049	21	0.85	1	33.531	2.77	138.41	C
20 00-0 00			В	0.079	3.049	7.5	0.85	1	33.531		100000	
			C	0.079	3 049		0.85	1	33 531			
Sum Weight	9 93	51 68						OTM	6683 58 kip-ft	47.34		

## Tower Forces - With Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F	ę	Cr	q:	$D_{\ell}$	Da	A	F	10	Cırl. Face
fi .	K	K	e			psf			ft <sup>2</sup>	K	plf	
TI	071	2.28	A	0.523	1.872	3	1	1	22 139	0.15	14 80	C
290 00-280 00	8.77		В	0 523	1 872	-	T.	1	22 139			
220.00.200.00	100		C	0.523	1.872		Ŷ	1	22 139			
T2	3.76	4.69	A	0 502	1.897	3	î	1	42 679	0.38	19.10	C
280.00-260.00	5.70		В	0 502	1.897	-	Ť	1	42 679			1.7
			C	0 502	1.897		1	1	42 679			
Т3	5.05	4.90	Ā	0.447	1.979	3	1	1	42 988	0.45	22.39	C
260 00-240 00	5.05	1.70	В	0.447	1.979		1	1	42 988	5		
200 00 21000			c	0.447	1 979		1	1	42 988			
T4	5 26	6.24	A	0.401	2.063	3	1	î	49 811	0 52	26.08	C
240 00-220 00		4,5.	В	0.401	2.063		1	î	49.811	3.5		
210 00 220 00			C	0 401	2 063		1	11	49 811			
T5	5 22	11 15	A	0 433	2.002	3	1	11	68 631	0.58	29 09	C
220 00-200 00			В	0.433	2.002	-	î	1	68 631	2.00		_
220 00 200 00			C	0 433	2.002	1	il	1	68 631	1	- 27	
Т6	5 17	12.20	A	0.431	2.005	3	1	1	79 991	0.62	31.25	C
200 00-180.00	337	12.20	В	0.431	2.005	-	1	1	79 991	0.02	3, 25	_
200 00 100.00			c	0.431	2.005		1	1	79 991			
T7	511	12 43	A	0 386	2 092	3	1	1	80 383	0.64	32 01	C
180 00-160.00			В	0.386	2 092		Ť	1	80 383		200,	-
100 00 100.00			Č	0 386	2 092		i	1	80.383			
Т8	5 09	13.00	Ā	0 36	2 149	3	î	1	84 636	0 66	32 88	В
160.00-140.00	3.07	15.00	В	0 36	2 149	- 5	i	1	84 636	0.00	52 00	-
100.00 110.00			c	0 36	2 149	4.77	i	1	84 636			
Т9	5 13	13 72	Ā	0 264	2 396	3	1	11	65 921	0.59	29.73	В
140.00-120.00	27.2		В	0 264	2 396	~	1	1	65.921	***		-
110.00 120.00			c	0 264	2 396	1111	1	1	65.921			
T10	5.06	13.72	A	0 242	2 461	3	1	1	66 345	0.58	29.19	В
120.00-100.00	5.00	13.7.2	В	0 242	2 461		1	i	66 345	0.00	27-17	
120.00 100.00			č	0 242	2 461	W	1	1	66 345			
T11	4 97	13 69	A	0 224	2518	2.	1	i	66 886	0.57	28.41	В
100 00-80 00		10.02	В	0 224	2518		1	1	66 886		20.41	-
100 00 00 00			C	0 224	2 518		T	1	66.886			
T12	4 86	14 20	A	021	2.564	2	1	1	67.963	0.55	27.40	В
80 00-60.00	4.00	14.20	В	0.21	2.564		1	1	67 963	0,55	27-70	
20 00 00.00			C	0.21	2.564		1	1	67.963			
T13	4.72	15.26	A	0.201	2.594	2	î	1	71 089	0.53	26.32	В
60.00-40.00	7.12	13.20	В		2.594	2	1	- 31	71 089	0.55	40.34	u

Valmont	Job 565090	Page 24 of 72
1545 Pideo Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth. 1N Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Section Elevation	Add Weight K	Self Weight K	F a c	e	$C_F$	q: psf	$D_F$	$D_R$	A <sub>#</sub>	F K	w plf	Cirl Face
	T		C	0.201	2 594		1	1	71 089		1000	
T14	4.51	14.91	A	0 187	2 638	2	1	1	71 418	0 48	23 78	В
40.00-20.00	100		В	0 187	2 638	12.0	1	1	71 418		200	
-			C	0 187	2 638		1	1	71.418			
T15	4 13	14 16	A	0.173	2 688	2	1	T.	70 727	041	20 38	В
20.00-0.00		100	В	0.173	2 688		1	1.	70 727		100	
			C	0.173	2.688		1	- 1	70 727			
Sum Weight	68 75	166.54						ОТМ	1101 37 kip-ft	7.71		

# Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F	ę	C <sub>F</sub>	q:	$D_F$	$D_{\tilde{R}}$	AE	F	3/2	Ctrl Face
G	K	K	c e			psf			ft2	K	plf	
TI	0.71	2 28	A	0.523	1.872	3	0.8	1	21 135	0.14	14 30	A
290.00-280.00	0.71	2 20	В	0.523	1.872	3	0.8	T)	21.135	0.14	14 20	-11
270.00-260.00			C	0.523	1 872		0.8	1	21 135			
T2	3.76	4.69	A	0 502	1.897	3	0.8	1	40.887	0.37	18 66	A
280 00-260 00	5.70	4.02	B	0.502	1.897	2	0.8	1	40.887	03,	10.00	
200 00-200.00			C	0.502	1 897		0.8	1	40 887			
Т3	5.05	4 90	Ă	0 447	1 979	3	0.8	î	41 454	0 44	22 01	C
260 00-240 00	5.05	4 30	B	0.447	1979	- 1	0.8	1	41 454	2.13	22.01	~
200 00 210 00			c	0 447	1 979	1	0.8	1	41.454			
T4	5.26	6 24	Ā	0 401	2 063	3	0.8	i	47.539	0.51	25.49	C
240 00-220 00	2.20	0.24	В	0.401	2 063	- 2	0.8	1	47 539	0.51	22.12	_
210 00 220 00			č	0 401	2 063		0.8	1	47 539			
TS	5 22	11 15	A	0 433	2 002	3	0.8	1	66 579	0.57	28.59	c
220 00-200 00	322	11.15	В	0 433	2 002		0.8	1	66 579	37,2,7	40.57	-
220 00 200 00			C	0.433	2.002	100	0.8	1	66.579			
Т6	5 17	12.20	A	0.431	2.005	3	0.8	i	77 703	0.61	30.69	C
200 00-180 00	211	12.54	В	0 431	2 005		0.8	1	77 703	5,64		
200 00 100 00			C	0 431	2 005	1.7	0.8	1	77 703			
T7	5.11	12.43	A	0 386	2.092	3	0.8	1	77 837	0.63	31.38	C
180.00-160.00		1200	В	0 386	2.092	-	0.8	1	77.837	2.52		
.00.00 100 00			c	0 386	2 092		0.8	1	77.837			
T8	5.09	13 00	Ā	0.36	2 149	3	08	1	81 254	0 64	32 05	C
160.00-140.00	0.07	19.90	В	0.36	2.149		0.8	1	81 254			
,00,00 1,0.00	100		C	0 36	2 149	1 0	0.8	1	81 254			
Т9	5.13	13 72	A	0 264	2.396	3	0.8	1	63 418	0.58	29 07	C
140.00-120.00	393	-	В	0.264	2.396		0.8	1	63.418	2.50		1
			C	0 264	2 396		0.8	1	63 418			
T10	5.06	13.72	A	0.242	2.461	3	0.8	1	63 709	0.57	28 49	C
120 00-100 00	87.50	1400	В	0.242	2.461	139	0.8	1	63.709	44.		
Sec. 29. 27. 27. 27. 27. 27. 27. 27. 27. 27. 27			C	0.242	2 461		0.8	1	63.709			
T11	4 97	13 69	A	0.224	2 518	2	08	1	64 109	0.55	27 69	C
100 00-80 00	100	18000	В	0.224	2 5 1 8	-	0.8	1	64 109	75.40-6-1	54793	
			C	0 224	2 5 1 8	1. (4)	0.8	1	64 109			
T12	4 86	14 20	A	0.21	2 564	2	0.8	1	65 038	0.53	26.67	C
80.00-60.00	1 (2)	-	В	0.21	2 564	1	0.8	1	65.038			
576755730730			C	0.21	2 564		0.8	1	65 038			
T13	4 72	15.26	A	0 201	2 594	2	0.8	1	67 497	0.51	25.47	C
60 00-40 00			В	0.201			0.8	11	67.497	7.0		

Valmont	Job	565090	Page 25 of 72
1545 Pidco Dr	Project H-	-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, 1N Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Section Elevation	Add Weight K	Self Weight K	F a c e	e	$C_F$	q: psf	$D_F$	$D_R$	A <sub>E</sub>	F K	nv plf	Cirl. Face
		- 10	C	0 201	2 594		0.8	T	67 497		Po	
T14	4.51	14.91	A	0.187	2.638	2	0.8	î	67 641	0.46	22 96	C
40.00-20.00	0.00	0,41354	В	0.187	2.638		0.8	1	67.641	2000	. C. S. M. S.	
			C	0 187	2.638		0.8	1	67 641		1	
T15	4.13	14.16	A	0.173	2.688	2	08	1	66.760	0.39	19 63	C
20.00-0.00	2000	33,777	В	0.173	2.688		0.8	1	66.760		40000	
	2.5		C	0.173	2.688		0.8	-1	66.760			
Sum Weight	68.75	166 54	20					ОТМ	1076 99 kip-ft	7 52		

# Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weighi	Self Weight	F a c	e	C <sub>#</sub>	q: psf	D <sub>F</sub>	D <sub>ii</sub>	AE	F	IV.	Ctrl Face
ft	K	K	e			404			fi <sup>2</sup>	K	plf	
TI	0.71	2 28	Ä	0.523	1 872	3	0.85	1	21.386	0.14	14.24	В
290.00-280.00	- 200	9.33	В	0 523	1.872		0.85	1	21.386	200	1,000	
2,0.00 200,00			C	0 523	1 872		0.85	1	21 386			
T2	3.76	4 69	A	0 502	1 897	3	0.85	1	41 335	0.38	18.87	A
280.00-260.00	-	9751	В	0 502	1.897	-	0.85	1	41.335	2.2.5	1200	12.2
24.700 502/10			C	0.502	1 897		0.85	1	41 335			
T3	5 05	4 90	A	0.447	1 979	3	0.85	1.	41 837	0.44	22 10	C
260 00-240 00	10.76	1,111,111	В	0.447	1 979	100	0.85	1	41 837	26.2.	20000	2,4
20,000,000,000			C	0.447	1.979	1.5	0.85	1	41 837			
T4	5 26	6.24	A	0 401	2.063	3	0.85	1	48 107	0.51	25 64	C
240 00-220 00	3997	200	В	0 401	2 063		0.85	1	48 107	4375,0		100
			C	0 401	2 063	7.5	0.85	1	48 107			
TS	5 22	11 15	A	0 433	2.002	3	0.85	1	67 092	0.57	28 72	C
220 00-200 00	10,22	1000	В	0.433	2 002		0 85	1	67 092	1000		1.7
2000 2000			C	0 433	2.002	10.0	0.85	1	67 092	1		
T6	5 17	12.20	A	0 431	2 005	3	0.85	1	78 275	0.62	30 83	C
200 00-180 00	2130	135/03	В	0.431	2.005		0.85	1	78 275	12.62	24 (35)	1 65
563020 554655			C	0 431	2.005		0.85	1	78 275			
T7	5.11	12.43	A	0.386	2.092	3	0.85	Û	78.474	0.63	31.54	C
180 00-160 00	2227	100	В	0.386	2 092		0.85	T.	78.474		100	1
A			C	0.386	2.092		0.85	1	78.474			
T8	5.09	13.00	A	0.36	2 149	3	0 85	1	82 099	0.64	32 22	C
160 00-140.00	27.444	2000	В	0.36	2 149		0.85	1	82 099	100	- 200	
750000000000000000000000000000000000000			C	0.36	2 149		0.85	1	82 099			
T9	5 13	13 72	A	0 264	2 396	31	0.85	1	64.044	0.58	29 09	C
140 00-120.00	100		В	0.264	2 396		0.85	1	64.044		200	
	100		C	0.264	2 396		0 85	- 1	64.044			
T10	5.06	13.72	Α	0.242	2 461	3	0.85	- 1	64 368	0.57	28.53	C
120.00-100.00	375	7.44.7	В	0.242	2 461		0.85	-1	64 368	1,000		
			C	0 242	2 461		0.85	1	64 368			
T11	4 97	13 69	A	0.224	2 5 1 8	2	0.85	- 1	64 804	0.55	27 74	C
100 00-80 00	100		В	0.224	2518		0.85	- 1	64 804			
A - Charles State (St.)			C	0 224	2518		0.85	1	64 804			
T12	4 86	14 20	A	0.21	2 564	2	0.85	1	65 769	0.53	26.73	C
80 00-60 00	Y		В	0.21	2 564		0.85	1	65.769			
			C	021	2.564		0.85	1	65 769			
T13	4 72	15.26	A	0 201	2 594	2	0.85	3	68 395	0.51	25 56	C
60 00-40 00			В	0.201			0.85	11	68 395	***		

Valmont	Јо <b>ь</b> 565090	Page 26 of 72
1545 Pidco Dr.	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

Section Elevation	Add Weight K	Self Weight K	F a c	e	C <sub>F</sub>	q: psf	$D_F$	$D_{\mathcal{R}}$	A <sub>E</sub>	F	nv plf	Cirl Face
<i>J</i> .			C	0.201	2.594		0.85	1	68.395	- 1	Po	
T14	4 51	14.91	A	0.187	2.638	2	0.85	1	68.585	0.46	23 06	C
40 00-20 00		96 (350)	В	0.187	2.638		0.85	1	68.585			
2.000			C	0.187	2.638		0.85	1	68.585			
T15	4 13	14 16	A	0.173	2.688	2	0.85	1	67 752	0.39	19.72	C
20 00-0 00			В	0.173	2.688		0.85	Ť	67 752		2000	
			C	0.173	2.688		0.85	1	67.752			
Sum Weight	68.75	166 54		Laborate and	6.00			OTM	1081.65 kip-ft	7 55		

# **Tower Forces - Service - Wind Normal To Face**

Section Elevation	Add Weight	Self Weight	F a	e	Cr	$q_{\scriptscriptstyle I}$	$D_{\tilde{r}}$	$D_{x}$	AE	F	36'	Ctrl. Face
ſŧ	K	K	e			psf			ft <sup>2</sup>	K	plf	
TI	0.08	0 38	A	0.187	2 639	12	1	1	7.764	0.29	29 39	C
290.00-280.00	-		В	0.187	2.639		1	1	7.764	-	1.00	
			C	0 187	2.639		- 1	1.1	7 764	-		
T2	0.51	1 10	A	0 223	2.521	12	- 1	11	17 644	087	43 61	C
280 00-260 00	1		В	0 223	2.521	1.00	- 1	£3	17.644	- A-34	1,444	40
200,000,000,000			C	0.223	2.521	1 2	1	T	17.644			
T3	071	1 28	A	0 203	2.586	12	1	Ĩ	18 168	1.00	49.81	C
260 00-240 00			В	0 203	2 586	1,722	1	1	18 168	-		
24.24.07.27.27.00.1			C	0 203	2.586		3)	1	18 168			
T4	0.72	1 73	A	0.196	2.61	12	1	1	23.269	1.14	56 80	C
240 00-220 00	1800000	2.42	В	0.196	261	2.2	1	1	23 269	45.0		
202086 20205			C	0 196	261		1	1	23 269			
T5	0.72	2 33	A	0.151	2.768	12	3	1	23 490	1 16	57 83	C
220 00-200 00	3.40	10000	В	0.151	2 768		3	1	23 490	0.004	7.5.00	
	1 - +1		c	0.151	2 768		1	1.	23 490			
Т6	0.72	3 07	A	0.133	2.834	11	- d	1	24 800	1 18	59 16	C
200 00-180 00	37.32	100	В	0 133	2 834	201	i i	i.	24 800		7.0	
			C	0.133	2 834	- 1	1	1	24 800			
T7	0.72	3.15	A	0.12	2 885	H	1.1	1.1	26.067	1 20	60 11	C
180.00-160.00	0.12	10000	В	0.12	2.885	2.5	1	i i	26.067	1.00	43.27	100
244,44,444,44			C	0.12	2.885		1	1	26.067			
T8	0 72	3 16	Ā	0.118	2 892	11	1	1	30.251	1 29	64.42	В
160 00-140.00			В	0 118	2 892	130	1	1	30.251	0,000	3.11.00	1
100 00 170.00			C	0 118	2 892		1	1	30.251			
Т9	0 72	4 16	Ă	0 103	2 95	10	1	1	27 825	1 22	60.81	В
140 00-120 00	0.74	,,,,,	В	0 103	2.95	100	i i	1	27 825		-	
110.00	7.		C	0.103	2.95		1	1	27.825			
T10	0.72	4 22	A	0 095	2.983	10	1	1	28 484	1.20	59.95	В
120 00-100 00	0,72	7,65	В	0.095	2.983		î	1	28 484			
120.00.100.00			C	0.095	2.983	0.0	1	Î	28 484	1 1		
T11	0 72	4 29	A	0 089	3.011	10	- 7	1	29 187	1.17	58.66	В
100.00-80.00	0.72	7.47	В	0.089	3.011	1.0	1	i	29 187		50.00	
100.00 00 00			c	0.089	3.011		1	1	29 187			
T12	0.72	4 89	Ā	0 086	3 021	9	Ŷ	1	30 816	1.15	57 68	В
80 00-60 00	0.72	7.07	B	0.086	3 021	-	1	1	30 816		21.00	
50 00-00.00			C	0.086	3.021		1	1	30 816			
T13	0.72	5 75	A	0.086	3 022	9	i i	1	34 150	1.15	57.41	В
60 00-40.00	0.72	3/3	B	0.086	3 022	7	1	1	34 150	1.13	3.1.41	D
00.00-40.00			1 0	0.000	3 022			4.1	100			1

Valmont	Јо <b>ь</b> 565090	Page 27 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client  VB BTS II, LLC	Designed by JL

Section Elevation	Add Weight K	Self Weight K	F a c	е	$C_F$	q: psf	D <sub>+</sub>	D <sub>R</sub>	A <sub>k</sub>	F K	w plf	Cirl Face
Ji .	Λ.		C	0.086	3.022			- 1	34 150	Λ.	рij	-
T14	0.72	5.87	A	0.082	3.039	8	1	1	35 077	1.05	52 68	В
40.00-20.00	0,72	2.07	B	0.082	3.039		1	i	35 077	1.02	52 00	
10.00 20.00			C	0.082	3.039		i	1	35.077			
T15	0.72	6.32	A	0.079	3.049	7	1	1	36 506	0 94	46.92	В
20 00-0.00	12.44	1,700	В	0.079	3.049	1 M	1	1	36.506	91.77	2477.0	
			C	0.079	3.049		1	1	36.506			
Sum Weight	9 93	51 68		7.0				OTM	2269 60 kip-ft	16.01		

## Tower Forces - Service - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a	e	$C_{\ell}$	q:	$D_{i}$	$D_h$	$A_{\tilde{\kappa}}$	F	W	Ctrl Face
fi	K	K	e			psf			(t <sup>2</sup>	K	plf	
TI	0.08	0.38	A	0.187	2.639	12	0.8	-11	6.760	0.27	26 61	A
290 00-280 00			В	0.187	2 639	-	0.8	1	6.760	2.45		1
270 00 200 00	1		C	0.187	2.639		0.8	1.1	6 760			
T2	0.51	1 10	A	0 223	2.521	12	0.8	T	15 852	0.83	41 26	A
280 00-260 00		1405-	В	0 223	2 521		0.8	I.	15 852	1.5	V. 1. O. Z.	11.2.4
221/12/02/15/			C	0.223	2 521		0.8	1	15.852			
Т3	0.71	1 28	A	0.203	2.586	12	0.8	1	16.634	0 96	47 78	C
260 00-240.00		1,000	В	0.203	2 586	7.2	0.8	i	16.634	7 27	24/4/4	
	1		C	0.203	2 586		0.8	1	16.634	- 4		
T4	0.72	1.73	A	0.196	2 61	12	0.8	i i	20 997	1 08	53.82	C
240.00-220.00		10.5	В	0 196	2.61		0.8	1	20 997			
	- 31		c	0.196	261		0.8	1	20.997			
TS	0.72	2 33	A	0 151	2 768	12	0.8	1	21 438	1.10	55 03	C
220 00-200 00	19.10	9,00	В	0 151	2 768	35	0.8	Ť.	21 438	10,000	10000	
200 00 200 00			C	0 151	2.768		0.8	1	21.438			
T6	0.72	3 07	Ă	0 133	2 834	11	0.8	1	22 512	1.12	56.03	C
200 00-180 00			В	0 133	2.834		0.8	1	22 512	0.5	4.44	
200 00 100 00			c	0 133	2 834	1.2	0.8	1	22 512			
T7	0.72	3 15	A	0.12	2 885	11	0.8	1	23 521	1 13	56.65	C
180 00-160 00			В	0 12	2 885	100	0.8	1	23 521			1.3
10000100			c	0.12	2.885	-	0.8	- 11	23 521			
T8	0 72	3 16	Ā	0.118	2.892	11	0.8	1	26 868	1 20	59 93	C
160 00-140 00	202	2,10	В	0.118	2 892		0.8	1	26 868	10.0	40.00	-
			C	0.118	2.892		0.8	1	26 868		100	
Т9	0.72	4 16	Ā	0 103	2.95	10	0.8	1	25 322	1 15	57.52	C
140 00-120 00	2.12	7.50	В	0.103	2 95	15	0.8	1	25.322	299	2,130	100
110.00	7.71		C	0.103	2 95		0.8	1	25.322			
T10	0.72	4 22	A	0.095	2 983	10	0.8	1	25.849	1.13	56.57	C
120.00-100.00		3,100	В	0.095	2 983		0.8	i	25.849			-
			C	0.095	2 983		0.8	1	25 849			
TII	0.72	4.29	A	0.089	3 011	10	0.8	1	26.411	1.10	55 21	C
100 00-80.00	7 (2		В	0.089	3 011		0.8	1	26 411	0.00		1
			C	0.089	3 011		0.8	Î	26 411			
T12	0.72	4 89	A	0.086	3 021	9	0.8	1	27 891	1 08	54 22	C
80 00-60 00	9.72		В	0.086	3 021	- 5	0.8	1	27.891	. 55		-
20 00 00 00			C	0 086	3 021		0.8	1	27 891			
T13	0 72	5.75	Ā	0.086	3 022	9	0.8	i	30 559	1.07	53.46	C
60 00-40 00	0.72	2.73	B	0 086			0.8	â	30 559		33,70	~

Valmont	Job	565090	Page 28 of 72
1545 Pidco Dr	Project H-31 x290' SST -	US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB I	BTS II, LLC	Designed by JL

Section Elevation	Add Weighi K	Self Weight K	F a c	e	C)	q: psf	Dr	$D_R$	A <sub>k</sub>	F	w plf	Ctrl Face
			C	0.086	3.022		0.8	1	30 559		-	
T14	0 72	5 87	A	0.082	3 039	8	0.8	1	31.300	0 98	48.92	C
40.00-20.00	3.00		В	0.082	3.039	1000	0.8	1	31.300	100	200	
1000	100		C	0.082	3.039		0.8	1	31 300	4 2.1	1.00	
T15	0 72	6 32	A	0.079	3.049	7	0.8	1	32.539	0.87	43 49	C
20.00-0.00			В	0.079	3.049		0.8	1	32 539	2.00		
			C	0.079	3.049		0.8	1	32 539			
Sum Weight	9 93	51 68						ОТМ	2142 37 kip-ft	15 06		

### Tower Forces - Service - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F	e	C,	q.	$D_F$	$D_k$	A	F	W.	Ctrl Face
0	K	K	E			psf			Q <sup>2</sup>	K	plf	
TI	0.08	0 38	A	0.187	2.639	12	0.85	- 1	7.011	0.26	26.37	В
290 00-280.00	0,00	0.50	В	0 187	2.639	10	0.85	1.1	7011	0.20	20.57	
270 00-200.00	11		Č	0.187	2.639		0.85	11	7011		10.22	
T2	0.51	110	A	0.223	2.521	12	0.85	i	16.300	0.85	42 31	A
280 00-260 00	0.51	110	В	0.223	2.521	1.2	0.85	i	16.300	0.05	72.21	
200 00-200 00			C	0.223	2.521		0.85	1	16.300		-	
Т3	0 71	1.28	A	0.203	2.586	12	0.85	1	17017	0 97	48 29	C
260 00-240 00	0 /1	1 20	B	0.203	2 586	12	0 85	1	17017	0.27	40 27	
200 00-240.00			c	0.203	2 586		0 85	3	17.017			
T4	0.72	1 73	A	0.196	2.61	12	0.85		21 565	1 09	54 56	C
240.00-220.00	0.72	1.73	B	0.196	2.61	12	0.85	1	21.565	1 07	24 20	-
240.00-220.00			C	0 196	261		0.85	11	21.565			8
T5	0.72	2 33	A	0.151	2.768	12	0 85	1	21 951	1.11	55 73	С
220.00-200.00	0.72	2.33	B	0.151	2 768	1.2	0 85	il	21 951	3-11	20 12	
220.00-200.00			C	0 151	2 768	1	0.85	1	21.951		-2	
Т6	0 72	3 07	A	0 131	2 834	11	0.85	1	23.084	1 14	56 81	С
200 00-180 00	0.72	307	B	0 133	2 834	1.1	0.85	1	23.084	1.14	20.01	-
200.00-180.00	1.0		Č	0 133	2.834	- 1 -	0.85	1	23.084		-	
T7	0.72	3 15	A	0.12	2.834	11	0.85	1	24 158	1.15	57.52	C
	0.72	3 13	B	0.12	2.885	11	0.85	1	24 158	1.13	31.32	-
180 00-160 00			C	0.12	2.885		0.85	1	24 158			
70	0.72	2.12			2.892	11	0.85		27 714	1 22	61 06	С
T8	0.72	3 16	A B	0.118		11	0.85	1		1 22	01.00	C
160 00-140 00			0.0	0.118	2.892		0.85	1	27 714 27 714	1		
Т9	0.72	4 16	C	0.118	2.892	10	0.85	- 1	25 948	1.17	58.34	С
	0 /2	4 10	A		2 95	10	0.85			1/17	38,34	
140 00-120 00	-		B	0.103				1	25 948 25 948			
m. ( a)	0.774		C	0.103	2 95	10	0.85	1		1.15	57.41	С
T10	0 72	4 22	A	0.095	2 983	10		1	26.508	1.15	3/41	C
120 00-100 00			В	0.095	2 983		0.85	1	26.508			
557	10.00	25.2.2	C	0.095	2.983	1.5	0.85	1	26.508			-
T11	0 72	4 29	A	0.089	3.011	10	0.85	- 1	27.105	1.12	56 07	C
100 00-80.00			В	0.089	3.011		0.85	1	27 105		776	11.5
200	4.50	1.22	C	0.089	3.011	-	0 85	1	27 105		****	-
T12	0 72	4 89	Α	0.086	3 021	9	0.85	1	28 622	1 10	55.09	C
80 00-60 00			В	0.086	3 021		0.85	1	28.622			
	- Circl	2	C	0.086	3.021		0.85	1	28.622	100	4	1
T13	0.72	5 75	A	0.086	3.022	9	0.85	1	31.457	1.09	54 45	C
60 00-40 00			В	0 086	3 022		0.85	1	31 457			

Valmont	Јо <b>b</b> 565090	Page 29 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by

Section Elevation	Add Weight	Self Weight	F a c	ę	$C_F$	q. psf	$D_F$	$D_R$	A <sub>E</sub>	F	34'	Ctrl Face
fl	K	K	е						ft²	K	plf	
			C	0 086	3 022		0.85	1	31.457			
T14	0.72	5.87	A	0 082	3 039	8	0.85	1	32 244	1 00	49 86	C
40 00-20.00	200		В	0 082	3 039	0.00	0.85	1.	32 244	100	100	
			C	0.082	3.039		0.85	1	32 244			
T15	0 72	6 32	A	0.079	3.049	7	0.85	1	33 531	0.89	44.35	C
20 00-0.00	10000		В	0.079	3 049		0.85	1	33 531	9-20-	2010	
			C	0 079	3.049		0.85	1	33 531			
Sum Weight:	9.93	51.68						ОТМ	2174.01 kip-ft	15 30		

#### Mast Vectors - No Ice

Section	Section	Wind	Directionality	F	$V_{ij}$	15	OTM,	ОТМ-	Torque
No.	Elevation ft	Azimuth		K	K	K	kip-ft	kip-fi	kip-ft
TI	290 00-280 00	D	Wind Normal	0 92	0.00	-0 92	-261 33	0.16	-0 46
	9-73-0	30	Wind 90	0.82	0.41	-0.71	-202 99	-117.12	-0 19
	)	60	Wind 60	0 74	0.64	-0 37	-105.70	-183 16	0.04
		90	Wind 90	0.74	0.74	0.00	0.14	-209 37	0.13
- 4		120	Wind Normal	0.83	0.72	0.41	118.37	-204 61	0.33
		150	Wind 90	0.82	0.41	0.71	203 27	-117 12	0.5
		180	Wind 60	0 83	0.00	0.83	236.85	0.16	0.40
		210	Wind 90	0.82	-0.41	0.71	203.27	117 44	0.19
		240	Wind Normal	0.83	-0.72	0.41	118 37	204.93	-0.04
		270	Wind 90	0.74	-0 74	0.00	0 14	209 69	-0.13
		300	Wind 60	0.74	-0 64	-0.37	-105.70	183 48	-0 3
	and the second second	330	Wind 90	0.82	-0.41	-0.71	-202.99	117.44	-0.5
T2	280.00-260.00	0	Wind Normal	2 64	0.00	-2 64	-712 51	0.27	-1.10
100	100000	30	Wind 90	2.56	1 28	-2 22	-598.05	-345 34	0.40
		60	Wind 60	2.41	2.09	-1 20	-324 48	-562 71	1.10
		90	Wind 90	2 30	2 30	0.00	0.56	-620.63	0.6
1		120	Wind Normal	2.38	2 06	119	321 93	-556 37	0.8
		150	Wind 90	2 39	1 19	2.07	558 57	321 90	1.5
		180	Wind 60	2 49	0.00	2.49	674.07	0 27	1.1
		210	Wind 90	2.56	-1 28	2 22	599 16	345.88	-0.4
		240	Wind Normal	2 55	-2 21	1 28	345 37	597 51	-1.19
		270	Wind 90	2 30	-2.30	0.00	0.56	621.18	-0.6
		300	Wind 60	2 23	-1 93	-1 12	-301 05	522.66	-0.8
		330	Wind 90	2 39	-1 19	-2 07	-557 46	322 44	-1.5
T3	260.00-240.00	0	Wind Normal	2 95	0.00	-2 95	-736 70	0.27	-1.2
96	1 - 2 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	30	Wind 90	2 85	1 43	-2 47	-617 42	-356 33	0.5
		60	Wind 60	2.82	2.44	-141	-352 42	-610.53	0.8
4		90	Wind 90	2 85	2.85	0.00	0 22	-712 92	-0.4
		120	Wind Normal	2 95	2.55	1 47	368 68	-637 92	-0.4
		150	Wind 90	2.85	1 43	2.47	617 87	-356 33	1.2
		180	Wind 60	2.82	0.00	2.82	705.51	0.27	1.2
		210	Wind 90	2 85	-1 43	2 47	617 87	356.87	-0.5
		240	Wind Normal	2 95	-2 55	1.47	368 68	638 46	-0.8
		270	Wind 90	2 85	-2 85	0.00	0.22	713 47	0.4
		300	Wind 60	2 82	-2 44	-141	-352 42	611.07	0.4
		330	Wind 90	2 85	-1 43	-2 47	-617.42	356 87	-12
T4	240.00-220.00	0	Wind Normal	3.37	0.00	-3 37	-775 56	0.39	-1.8
6.5		30	Wind 90	3.23	1.62	-2.80	-643 85	-371 51	0.5
		60	Wind 60	3.19	2 76	-1.59	-366.25	-634 50	1.1

## Valmont

1545 Pidco Dr.

Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458

Job		Page
	565090	30 of 72
Project		Date
	H-31 x290' SST - US-KY-5135 Fancy Farm	07:03:40 10/06/22
Client		Designed by
	VB BTS II, LLC	JL

Section No.	Section Elevation	Wind Azimuth	Directionality	F	$V_x$	T,	OTM.	OTM:	Torque
	ſŧ	0		K	K	K	kip-ft	kip-ft	kıp-ft
		90	Wind 90	3 23	3.23	0.00	0.31	-743 41	-0.4
1		120	Wind Normal	3.37	2 92	1.69	388 24	-671 53	-0.3
		150	Wind 90	3.23	1.62	2.80	644 46	-371 51	1.8
		180	Wind 60	3.19	0.00	3 19	733 42	0.39	1.8
		210	Wind 90	3.23	-1 62	2.80	644.46	372.30	-0.5
		240	Wind Normal	3.37	-2.92	1.69	388.24	672.32	41:1
		270	Wind 90	3.23	-3 23	0.00	031	744 20	0.4
		7.77				16-7-46-76-1		635.29	0.3
		300	Wind 60	3 19	-2.76	-1.59	-366.25		
97	colo con plan par	330	Wind 90	3 23	-1 62	-2.80	-643.85	372 30	-1.
T5	220.00-200.00	0	Wind Normal	3.61	0 00	-3 61	-757 73	0.49	-2
2.0		30	Wind 90	3.48	1 74	-3 01	-632 33.	-364 80	0
		60	Wind 60	3 44	2.98	-1 72	-360.33	-624.27	L
		90	Wind 90	3 48	3.48	0.00	0.38	-730.10	-0
		120	Wind Normal	3.61	3.13	181	379 43	-656 04	-0
		150	Wind 90	3 48	1.74	3.01	633.08	-364 80	2
		180	Wind 60	3 44	0.00	3 44	721 79	0 49	2
		210	Wind 90	3 48	-1 74	3.01	633 08	365 79	-0
		The second second				200			
		240	Wind Normal	3.61	-3.13	1.81	379.43	657 03	-1
		270	Wind 90	3.48	-3.48	0.00	0 38	731 08	0
		300	Wind 60	3 44	-2.98	-1 72	-360 33	625 26	0
200	William Street, and	330	Wind 90	3 48	-1.74	-3 01	-632 33	365 79	-2
T6	200.00-180.00	0	Wind Normal	3 69	0.00	-3 69	-701 21	0.59	-2
	20000 00000	30	Wind 90	3.55	1.77	-3 07	-583 11	-336 32	0
		60	Wind 60	3 50	3 03	-1 75	-331 84	-574 93	1.
		90	Wind 90	3.55	3 55	0 00	0.44	-673 24	-0
		120	Wind Normal	3 69	3 20	1 85	351 27	-607 05	-0
		1,000						100 300 100 200	2
		150	Wind 90	3 55	1.77	3 07	584 00	-336 32	
		180	Wind 60	3 50	0.00	3 50	665 00	0.59	2
1		210	Wind 90	3.55	-1 77	3 07	584 00	337 51	-0
		240	Wind Normal	3.69	-3 20	1.85	351 27	608 24	-1.
		270	Wind 90	3.55	-3.55	0.00	0.44	674 42	0.
		300	Wind 60	3.50	-3 03	-1 75	-331 84	576 12	0.
		330	Wind 90	3.55	-1.77	-3.07	-583 11	337 51	-2.
T7	180 00-160 00	0	Wind Normal	3.75	0.00	-3 75	-637 38	0.69	-3.
1.1	100 00-100 00	30	Wind 90	3 59	1 80	-3 11	-528.07	-304.49	0
		40.00	Wind 60		3 06	90,000	-300.08	-519 94	1.
		60		3 54		-1 77			
		90	Wind 90	3 59	3 59	0 00	0.51	-609 66	-0.
		120	Wind Normal	3 75	3 25	1.88	319 46	-551.74	-0.
		150	Wind 90	3 59	1.80	3 11	529 09	-304 49	2.
		180	Wind 60	3 54	0.00	3 54	601.68	0.69	3.
		210	Wind 90	3 59	-1.80	3 11	529 09	305.87	-0
	ľ	240	Wind Normal	3 75	-3.25	1 88	319.46	553 12	-1.
		270	Wind 90	3 59	-3.59	0.00	0.51	611.04	0
		300	Wind 60	3.54	-3 06	-1 77	-300.08	521 32	0.
		The second secon					-528 07		-2
TO.	170.00 140.00	330	Wind 90	3 59	-1 80	-3 11		305 87	
T8	160 00-140.00	0	Wind Normal	4 02	0.00	-4 02	-602 10	0 79	-3
		30	Wind 90	3 81	191	-3 30	-494 54	-285.06	0
		60	Wind 60	3 74	3 24	-1 87	-280 03	-485 21	1
		90	Wind 90	3.81	3.81	0.00	0 56	-570 91	-0
		120	Wind Normal	4.02	3 48	201	301 90	-521 13	-0
		150	Wind 90	3.80	1 90	3.29	493 97	-284 08	3
		180	Wind 60	3.74	0.00	3 74	561 19	0 79	3
		210	Wind 90	3.81	-1 91	3.30	495 67	286 64	-0
									-1
		240	Wind Normal	4 02	-3 48	201	302 18	523 20	-1
		270	Wind 90	3.81	-3.81	0.00	0.56	572 48	0
		300	Wind 60	3 74	-3 24	-1.87	-279.75	486 30	0
		330	Wind 90	3 80	-1.90	-3.29	-492 84	285 66	-3
T9	140 00-120 00	0	Wind Normal	3 78	0.00	-3 78	-490 96	0.89	-3
4.5		30	Wind 90	3 64	1.82	-3.15	-409.41	-235 83	0.
		60	Wind 60	3 59	3.11	-1 80	-232.79	-403 33	1
		90		3 64	3 64	0 00	0.58	-472 54	-1

Valmont	Job	565090	Page 31 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Section No.	Section Elevation	Wind Azımuth	Directionality	F	V <sub>x</sub>	1.5	OTM <sub>x</sub>	OTM-	Torque
_	ſ		107 151 1	K 170	K 2.27	K	kip-ft	kip-ft	kip-ft
		120 150	Wind Normal Wind 90	3.78 3.59	3.27 1.80	189	246 36 404 86	-424.81	-1 I 3 3
							465.45	-232 52	3 7
		180	Wind 60	3.58	0.00	3 58 3 15	410.58	0 89 237 60	-0.5
		210	Wind 90	3.64	-1 82				
		240	Wind Normal	3.80	-3 29	1 90	247.30	428.21	-1.7
		270	Wind 90	3.64	-3.64	0 00	0.58	474 31	1.4
		300	Wind 60	3.58	-3.10	-1 79	-231 85	403.47	1.1
20.00	version reducted	330	Wind 90	3.59	-1.80	-3.11	-403 70	234 30	-3 3
T10	120.00-100.00	0	Wind Normal	3 73	0.00	-3 73	-409 43	0 98	-3.9
-		30	Wind 90	3 58	1.79	-3 10	-340 75	-196 12	0.5
		60	Wind 60	3 53	3.06	-1 77	-193.56	-335 38	1.8
		90	Wind 90	3 58	3 58	0 00	0 64	-393 22	-15
1		120	Wind Normal	3 73	3.23	1 86	205.68	-354 15	-12
		150	Wind 90	3.53	1.77	3 06	337 36	-193 42	3.5
		180	Wind 60	3 52	0 00	3 52	387 50	0.98	3.9
		210	Wind 90	3 58	-1 79	3 10	342 03	198 09	-0.5
		240	Wind Normal	3 74	-3 24	187	206.45	357 45	-1.8
		270	Wind 90	3.58	-3.58	0 00	0.64	395.19	1.5
		300	Wind 60	3.52	-3 05	-1 76	-192 79	336.02	12
	340,000	330	Wind 90	3.53	-1 77	-3 06	-336 08	195 39	-3.5
T11	100.00-80.00	0	Wind Normal	3.65	0.00	-3 65	-327.63	1 08	-4 2
		30	Wind 90	3.50	1.75	-3.03	-272 12	-156 43	0.5
		60	Wind 60	3 45	2 98	-1 72	-154 39	-267 55	1.9
		90	Wind 90	3 50	3 50	0.00	0 70	-313.94	-1.5
1		120	Wind Normal	3.65	3.16	1 82	164 87	-283 26	-1.3
		150	Wind 90	3.45	1.73	2 99	269 86	-154 31	3.7
		180	Wind 60	3.43	0 00	3 43	309 68	1 08	4.2
		210	Wind 90	3 50	-1 75	3.03	273 53	158.60	-0.5
		240	Wind Normal	3 66	-3 17	1.83	165.48	286.48	-1.9
		270	Wind 90	3 50	-3.50	0 00	0.70	316.11	1.5
		300	Wind 60	3 43	-2 97	-1 72	-153.78	268.67	1.3
		330	Wind 90	3.45	-1.73	-2 99	-268 45	156.48	-3 7
T12	80 00-60 00	0	Wind Normal	3.59	0.00	-3 59	-250.36	1.18	-4.3
112	80 00-00 00	30	Wind 90	3 44	1.72	-2 98	-207 69	-119.17	0.6
		60	Wind 60	3 38	2 93	-1 69	-117 70	-204 00	2.0
		90	Wind 90	3.44	3.44	0.00		-239 52	-1.6
1							0.77		
		120	Wind Normal	3 59	3 11	1 79	126 33	-216 30	-1.3
		150	Wind 90	3 39	1 70	2 94	206 52	-11761	3 8
		180	Wind 60	3 37	0.00	3.37	236 80	1 18	4 3
		210	Wind 90	3.44	-1.72	2 98	209 22	121 53	-0.6
		240	Wind Normal	3.60	-3 12	1 80	126 77	219 44	-2 0
		270	Wind 90	3 44	-3.44	0 00	0 77	241 88	16
		300	Wind 60	3 37	-2 92	-1.69	-117.25	205.60	1.3
77.00	as as sais as	330	Wind 90	3 39	-1 70	-2 94	-204 98	119 97	-3 8
T13	60 00-40 00	0	Wind Normal	3 57	0 00	-3 57	-177 76	1 28	-4 3
		30	Wind 90	3 40	1 70	-2 94	-146 34	-83 68	0.5
		60	Wind 60	3 34	2 89	-1 67	-82 60	-143 21	2 (
	}	90		3.40	3 40	0.00	0.83	-168.65	-16
		120	Wind Normal	3.57	3.09	1.79	90 12	-153 38	-1.3
		150	Wind 90	3 36	1 68	2.91	146.19	-82 65	3 3
		180	Wind 60	3 33	0 00	3 33	167 08	1 28	4 :
		210	Wind 90	3 40	-1 70	2 94	147 99	86 25	-0
		240	Wind Normal	3 58	-3 10	1.79	90 42	156 46	-2 (
		270	Wind 90	3 40	-3 40	0.00	0 83	171 21	1 6
		300	Wind 60	3.33	-2 88	-1 66	-82 30	145 26	1.
		330	Wind 90	3.36	-1 68	-2 91	-144 54	85 21	-3
T14	40 00-20 00	0	Wind Normal	3 28	0.00	-3 28	-97 44	1.38	-4
-1		30		3 11	1 56	-2 70	-79 97	-45 31	0.5
		60	Wind 60	3.05	2.64	-1.53	-44.92	-77 96	1.9
		90		3 11	3 11	0 00	0.89	-91.99	-1.5
		120		3 28	2.84	1 64	50.05	-83.77	-1.3

Valmont	Job	565090	Page 32 of 72
1545 Pidco Dr.	Project H-31 x290' S	SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth. IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Section No.	Section Elevation	Wind Azımuth	Directionality	F	$V_{\star}$	17,	OTM <sub>c</sub>	OTM.	Torque	
177	fi	0		K	K	K	kip-ft	kip-fi	kip-ft	
		150	Wind 90	3 08	1 54	2.66	80 78	-44.75	3 76	
		180	Wind 60	3 04	0 00	3.04	92 18	1 38	4 24	
		210	Wind 90	3.11	-1.56	2.70	81 75	48.06	-0.56	
		240	Wind Normal	3.29	-2.85	1 64	50.21	86 81	-1.98	
		270	Wind 90	3 11	-3 11	0.00	0.89	94 75	1.59	
		300	Wind 60	3.04	-2 64	-1.52	-44 76	80.44	1.35	
		330	Wind 90	3 08	-1 54	-2.66	-79.01	47.51	-3.76	
T15	20 00-0 00	0	Wind Normal	2.92	0.00	-2 92	-28 25	1 48	-3 93	
		30	Wind 90	2 77	1.38	-2.40	-23 03	-12.36	0.52	
1		60	Wind 60	2.71	2 35	-1.36	-12 63	-22.03	1.83	
		90	Wind 90	2.77	2 77	0.00	0.95	-26 20	-1.47	
1		120	Wind Normal	2 92	2 53	1 46	15 54	-23 80	-1 25	
		150	Wind 90	2.74	1.37	2 37	24 64	-12 20	3 47	
		180	Wind 60	2.71	0.00	2.71	28 00	1.48	3.93	
		210	Wind 90	2.77	-1 38	2.40	24 92	15 32	-0 52	
		240	Wind Normal	2 93	-2 54	1.46	15 59	26 84	-1.83	
		270	Wind 90	2.77	-2 77	0.00	0.95	29 16	1 47	
		300	Wind 60	2 71	-2 34	-1.35	-12.58	24.91	1 25	
		330	Wind 90	2.74	-1 37	-2 37	-22 75	15.16	-3.47	

#### Mast Totals - No Ice

Wind Azimuth	$V_x$	V <sub>z</sub>	OTM <sub>x</sub>	OTM <sub>z</sub>	Torque
0	K	K	kip-fi	kip-ft	kip-ft
0	0.00	-49.47	-6966.35	11.93	-44.97
30	23.67	-41.00	-5779 67	-3329 85	8 00
60	40.21	-23.21	-3259 71	-5648 73	23 61
90	46.99	0.00	8.48	-6576.31	-13.73
120	42.54	24.56	3448.22	-5945 88	-11 14
150	23.43	40 57	5734 53	-3294 00	41.38
180	0.00	46 51	6586.22	11 93	44 97
210	-23.67	41.00	5796.62	3353 72	-8.00
240	-42.77	24.69	3475.20	6016 49	-23.61
270	-46 99	0.00	8.48	6600 18	13.73
300	-39 98	-23 08	-3232.73	5625 87	11.14
330	-23 43	-40 57	-5717.58	3317.87	-41 38

## Mast Vectors - With Ice

Section Section No. Elevation	Wind Azimuth	Directionality	F	$V_{\kappa}$	$F_z$	OTM,	OTM:	Torque	
1.0.	ft	0		K	K	K	kip-ft	kip-ft	kip-ft
TI	290.00-280.00	0	Wind Normal	0 15	0.00	-0.15	-41 36	1.07	-0 05
W 1		30	Wind 90	0 14	0 07	-0 12	-34 33	-19 22	-0 02
		60	Wind 60	0 14	0.12	-0.07	-18 77	-32 85	0.00
		90	Wind 90	0 14	0.14	0.00	0.81	-37 91	0.02
		120	Wind Normal	0 14	0.12	0.07	21 10	-34 06	0 04
		150	Wind 90	0.14	0.07	0.12	35 95	-19 22	0 06
		180	Wind 60	0.14	0.00	0 14	41 57	1 07	0.05
		210	Wind 90	0 14	-0.07	0 12	35 95	21 36	0 02
		240	Wind Normal	0 14	-0.12	0.07	21 10	36 21	-0.00

Valmont	Job	565090	Page 33 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

ection No.	Section Elevation	Wind Azimuth	Directionality	F	V <sub>e</sub>	$V_x$	OTM,	OTM.	Torque
	ſŧ	8		K	K	K	kip-fi	kip-ft	kip-ft
		270	Wind 90	0 14	-0.14	0.00	0.81	40 06	-0.02
		300	Wind 60	0 14	-0 12	-0.07	-18 77	34 99	-0.04
Gr.	02312246574	330	Wind 90	0 14	-0 07	-0.12	-34 33	21 36	-0.06
T2	280 00-260 00	0	Wind Normal	0.38	0 00	-0 38	-99.69	1 82	-0.14
		30	Wind 90	0 38	0 19	-0.33	-84 77	-49 13	0.00
		60	Wind 60	0.37	0 32	-0.18	-46 14	-84 11	0.10
		90	Wind 90	0.36	0.36	0.00	3 47	-95.40	0.11
		120	Wind Normal	0 36	0 32	0 18	52 71	-83 47	0.14
		150 180	Wind 90 Wind 60	0 37	0 18	0 32	89.01	-47 57	0.19
		210	Wind 90	038	-0 19	0 3 3	91.71	1 82 52 76	-0.00
		240	Wind Normal	0 38	-0 33	0.19	54.27	89 81	-0.10
		270	Wind 90	0.36	-0.36	0.00	3.47	99 04	-0.11
		300	Wind 60	0 36	-0.31	-0.18	-44 58	85 05	-0.14
		330	Wind 90	037	-0 18	-0 32	-82.07	51 21	-0.19
T3	260.00-240.00	0	Wind Normal	0.45	0 00	-0.45	-110.82	181	-0.17
100	Local State of Control	30	Wind 90	0 44	0 22	-0.38	-94 56	-53 45	-0.0
		60	Wind 60	0 44	0.38	-0 22	-53 87	-93.48	0.06
		90	Wind 90	0 44	0 44	0 00	114	-108 70	0.0
	110	120	Wind Normal	0.45	0 39	0 22	57.12	-95 16	0.05
		150	Wind 90	0.44	0 22	0.38	96 85	-53.45	0.18
		180	Wind 60	0.44	0.00	0 44	111.17	1.81	0.17
		210	Wind 90	0 44	-0 22	0 38	96 85	57 06	0.01
		240	Wind Normal	0.45	-0 39	0.22	57 12	98 77	-0.00
		270	Wind 90	0.44	-0 44	0.00	1 14	112.32	-0.01
		300	Wind 60	0.44	-0 38	-0.22	-53 87	97 09	-0.05
TA	240.00.220.00	330	Wind 90	0.44	-0.22	-0.38	-94 56	57.06	-0.18
T4	240.00-220 00	30	Wind Normal Wind 90	0.52	0.00	-0.52 -0.44	-11791	3 16	-0.3 -0.04
		60	Wind 60	0.51	0.26	-0.44	-100 07 -56 57	-55 81 -98 39	0.10
		90	Wind 90	0.51	0.51	0.00	2 07	-114 78	0.00
		120	Wind Normal	0.52	0.45	0.26	62 06	-100.74	0.13
		150	Wind 90	0.51	0.26	0.44	104 21	-55 81	0.33
		180	Wind 60	0.51	0.00	0.51	119 34	3 16	0.31
		210	Wind 90	0.51	-0.26	0.44	104 21	62.14	0.04
		240	Wind Normal	0.52	-0.45	0.26	62 06	107.06	-0.10
		270	Wind 90	0.51	-0.51	0.00	2 07	121 11	-0 06
		300	Wind 60	0.51	-0.44	-0 25	-56 57	104.72	-0 13
	CONTRACTOR STATES	330	Wind 90	0.51	-0.26	-0 44	-100 07	62.14	-0 33
T5	220 00-200 00	0	Wind Normal	0.58	0.00	-0.58	-119 73	3.91	-0 35
	Mary Control of the Control	30	Wind 90	0.57	0.29	-0.50	-101 98	-56.39	-0.06
		60	Wind 60	0.57	0.50	-0.29	-57.57	-100 08	0.11
		90	Wind 90	0.57	0.57	0 00	2 47	-116.69	0.0
		120	Wind Normal	0.58	0.50	0 29	63 57	-101 92	0 15
		150	Wind 90	0.57	0.29	0.50	106.92	-56 39	0.3
		180	Wind 60 Wind 90	0.57	-0.29	0 57 0 50	122.54 106.92	3.91 64.21	0 33
		210 240	Wind Normal	0.58	-0.29	0.29	63.57	109.74	-0.1
		270	Wind 90	0.58	-0.57	0 00	2.47	124 51	-0.07
		300	Wind 60	0.57	-0.50	-0 29	-57.57	107 90	-0 15
		330	Wind 90	0.57	-0.29	-0 50	-101 98	64.21	-0 37
T6	200 00-180.00	0	Wind Normal	0.62	0.00	-0 62	-115.88	4 63	-0 41
	220123148898	30	Wind 90	0.62	0.31	-0 53	-98.60	-53 95	-0 0
		60	Wind 60	0.61	0.53	-0.31	-55 46	-96 37	0.13
		90	Wind 90	0.62	0.62	0.00	2.86	-112 53	0.08
		120	Wind Normal	0.62	0 54	0 31	62 22	-98 19	0 17
		150	Wind 90	0.62	0.31	0.53	104 32	-53 95	0 43
		180	Wind 60	0.61	0.00	0.61	119.49	4.63	0.41
		210	Wind 90	0.62	-0 31	0.53	104 32	63 21	0.07
		240	Wind Normal	0.62	-0 54	0.31	62 22	107 46	-0.13
		270	Wind 90	0.62	-0.62	0.00	2.86	121.79	-0.08

Valmont	Job	565090	Page 34 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth. IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Section No.	Section Elevation	Wind Azimuth	Directionality	F	$V_{\hat{e}}$	T's	OTM <sub>x</sub>	OTM.	Torque
11	ſı	0		K	K	K	kıp-fi	kip-ft	kip-ft
		300	Wind 60	0.61	-0.53	-0.31	-55.46	105 64	-0.17
7-1	Protect Control	330	Wind 90	0.62	-0.31	-0 53	-98.60	63 21	-0.43
T7	180 00-160 00	0	Wind Normal	0 64	0.00	-0 64	-105 60	5 32	-0.50
		30	Wind 90	0 63	0.32	-0 55	-89.64	-48 29	-0.09
		60	Wind 60	0 63	0.54	-0.31	-50 12	-87 08	0.15
		90	Wind 90	0 63	0 63	0 00	3.23	-101 91	0.09
		120	Wind Normal	0 64	0.55	0 32	57.65	-88 93	0.20
		150	Wind 90	0.63	0 32	0.55	96 10	-48 29	0.52
		180	Wind 60	0 63	0 00	0 63	109 93	5 32	0.50
		210	Wind 90	0.63	-0 32	0.55	96 10	58 94	0.09
		240	Wind Normal	0.64	-0.55	0 32	57 65	99 58	-0.15
		270	Wind 90	0.63	-0.63	0.00	3 23	112 56	-0.09
		300	Wind 60	0.63	-0.54	-0.31	-50 12	97 73	-0.20
		330	Wind 90	0.63	-0 32	-0 55	-89 64	58 94	-0.52
18	160.00-140.00	0	Wind Normal	0.66	0.00	-0.66	-95.07	5.98	-0.55
		30	Wind 90	0.64	0.32	-0.56	-80.47	-42.35	-0.11
		60	Wind 60	0.64	0.56	-0 32	-44.83	-77.28	0.13
		90	Wind 90	0.64	0.64	0.00	3 24	-90.68	0.06
		120	Wind Normal	0.66	0.57	0.33	52 39	-79 16	0.20
		150	Wind 90	0 64	0 32	0.56	86.65	-42.18	0.56
		180	Wind 60	0.64	0.00	0.64	99 04	5 98	0 55
		210	Wind 90	0.64	-0.32	0 56	86.94	54.30	0.11
		240	Wind Normal	0.66	-0.57	0 33	52.56	91.40	-0 13
		270	Wind 90	0 64	-0 64	0.00	3 24	102.63	-0.06
		300	Wind 60	0.64	-0 55	-0 32	-44.67	88.95	-0 20
		330	Wind 90	0.64	-0 32	-0 56	-80 18	54 14	-0.56
79	140 00-120 00	0	Wind Normal	0.59	0.00	-0 59	-73.74	6.58	-0.59
	100 M 2 2 1 1 1 1 1 1	30	Wind 90	0.58	0 29	-0 50	-63.06	-31.24	-0 17
		60	Wind 60	0 58	0.50	-0 29	-35 35	-58.87	0.04
		90	Wind 90	0.58	0 58	0.00	2.43	-69.05	-0 03
		120	Wind Normal	0.59	0.51	0 29	40.52	-59.39	0.17
		150	Wind 90	0 57	0 29	0.50	66 95	-30.67	0.59
		180	Wind 60	0.57	0.00	0 57	76.87	6 58	0.59
		210	Wind 90	0.58	-0 29	0.50	67 93	44 39	0.17
		240	Wind Normal	0 59	-0 52	0 30	41 09	73 53	-0.04
		270	Wind 90	0.58	-0 58	0.00	2.43	82 21	0.03
		300	Wind 60	0.57	-0 50	-0 29	-34 79	71 04	-0 17
		330	Wind 90	0.57	-0 29	-0 50	-62 08	43 83	-0.59
10	120.00-100.00	0	Wind Normal	0.58	0 00	-0.58	-60 70	715	-0 63
. 7	195163 113163	30		0.57	0 29	-0 49	-51 75	-24 23	-0 18
		60	Wind 60	0.57	0.49	-0.28	-28.74	-47 14	0.04
		90	Wind 90	0.57	0.57	0 00	2.60	-55 61	-0.03
		120	Wind Normal	0.58	0.50	0.29	34 25	-47.67	0.18
		150	Wind 90	0.56	0.28	0.49	56.15	-23.77	0.63
	l.	180	1. 256.63.73.63.53	0 56	0.00	0.56	64 36	7.15	0.63
		210		0 57	-0 29	0.49	56.95	38.53	0.18
		240	Wind Normal	0.58	-0.51	0 29	34 71	62 77	0.18 -0.04
		270		0.57	-0 57	0.00	2.60	69.91	0.03
		300	Wind 60	0.56	-0 49	-0 28	-28.28	60 64	-0.18
		330		0.56	-0 28	-0 49	-50.95	38.07	-0.63
TIT	100 00-80 00	0		0.56	0 00	-0.56	-47 68	7 66	-0.66
	100 00-00 00	30		0.55	0 28	-0 48	-40.50	-17.31	-0.19
		60	Wind 60	0.55	0.48	-0.28	-22.18	-35 51	0.04
		90		0.55	0.48	0 00	2.74	-42.27	-0.04
		120		0 56	0 49	0 28	27 95	-36.01	0.18
		150		0.55	0.27	0.47	45.36	-16 94	0.65
		180		0.55	0.00	0.55	51.87	7.66	0.66
				0.55	50,000,000	0.33	45 98	32.62	0.00
		210			-0 28				-0 04
		240		0.57	-0 49	0.28	28 32	51 95	
		270		0.55	-0 55	0.00	2 74	57.59	0.04
		300	Wind 60	0.55	-0 47	-0 27	-21.82	50.20	-0.11

Valmont	Job	565090	Page 35 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Section No.	Section Elevation	Wind Azimuth	Directionality	F	V <sub>a</sub>	L.	OTM <sub>x</sub>	OTM.	Torque
	ft	0		K	K	K	kip-ft	kıp-ft	kip-ft
		330	Wind 90	0.55	-0 27	-0 47	-39 87	32.26	-0.63
T12	80.00-60.00	0	Wind Normal	0.54	0.00	-0 54	-34 97	8 08	-0.68
		30	Wind 90	0.53	0.27	-0.46	-29 55	-10 62	-0.19
1		60	Wind 60	0.53	0 46	-0 27	-15 81	-24 25	0.04
		90	Wind 90	0.53	0 53	0.00	2.85	-29 33	-0 04
		120	Wind Normal	0.54	0.47	0.27	21 77	-24 68	0.13
1		150	Wind 90	0 53	0.26	0.46	34 79	-10 36	0.6
1		180	Wind 60	0.53	0.00	0.53	39 66	8 0 8	0.6
		210	Wind 90	0.53	-0 27	0.46	35 26	26 79	0.19
		240	Wind Normal	0.55	-0 47	0.27	22.04	41 31	-0.04
		270	Wind 90	0.53	-0 53	0 00	2.85	45 50	0.0
		300	Wind 60	0.53	-0 46	-0.26	-15 55	39 95	-0 18
		330	Wind 90	0.53	-0.26	-0.46	-29 08	26 52	-0 6
T13	60.00-40.00	0	Wind Normal	0 52	0.00	-0.52	-23.04	8.38	-0 6
		30	Wind 90	0.51	0 26	-0.44	-19 22	-4 40	-0 19
		60	Wind 60	0.51	0 44	-0.25	-9 82	-13 68	0.0
		90	Wind 90	0.51	0.51	0.00	2 92	-17 18	-0.0
		120	Wind Normal	0.52	0.45	0.26	15 90	-14.10	0.1
		150	Wind 90	0.50	0 25	0.44	24 74	-4 22	0.60
		180	Wind 60	0.50	0.00	0.50	28 03	8 38	0.6
		210	Wind 90	0.51	-0.26	0.44	25 05	21.16	0.19
		240	Wind Normal	0.53	-0 46	0.26	16 07	31.17	-0.0:
		270	Wind 90	0.51	-0 51	0.00	2 92	33 94	0.0
		300	Wind 60	0.50	-0 43	-0.25	-9 64	30.13	-0.1
		330	Wind 90	0.50	-0.25	-0 44	-18.91	20.98	-0.66
T14	40 00-20 00	0	Wind Normal	0.47	0 00	-0.47	-11 18	8 45	-0.64
		30	Wind 90	0.46	0.23	-0.40	-9 09	1.53	-0.13
		60	Wind 60	0.46	0.40	-0 23	-4 00	-3 48	0.0.
		90	Wind 90	0.46	0.46	0 00	2 89	-5 39	-0.0
		120	Wind Normal	0.47	0.41	0 23	9.93	-3 74	0.1
		150	Wind 90	0.45	0.23	0 39	14.71	1.63	0.6
		180	Wind 60	0.45	0.00	0 45	16.48	8 45	0.6
		210	Wind 90	0.46	-0.23	0 40	14 87	15.37	0.1
- 3		240	Wind Normal	0.48	-0.41	0 24	10.02	20 80	-0.0
1		270	Wind 90	0.46	-0.46	0 00	2 89	22 29	0.0
		300	Wind 60	0.45	-0.39	-0 23	-3.90	20 21	-0.1:
		330	Wind 90	0.45	-0.23	-0 39	-8.92	15 27	-0.6
T15	20 00-0.00	0	Wind Normal	0.40	0.00	-0 40	-1 39	7 89	-0.5
000		30	Wind 90	0.39	0.20	-0 34	-0.78	5.92	-0.1
		60	Wind 60	0.39	0 34	-0 20	0.67	4 49	0.0
		90	Wind 90	0.39	0.39	0.00	2.63	3 95	-0.0
		120	Wind Normal	0.40	0.35	0.20	4.64	4.41	0.13
		150	Wind 90	0.39	0.19	0 34	6.00	5 95	0.5
		180	Wind 60	0.39	0.00	0.39	6 50	7 89	0.5
		210	Wind 90	0.39	-0.20	0.34	6.05	9 86	01
1		240	Wind Normal	0.41	-0 35	0 20	4 67	11.42	-0 0
		270	Wind 90	0.39	-0 39	0 00	2.63	11 84	0.0
		300	Wind 60	0.39	-0.34	-0.19	0.70	11.24	-0.1
		330	Wind 90	0.39	-0 19	-0.34	-0 74	9 84	-0.5

## Mast Totals - With Ice

Wind Azimuth	V,	V <sub>s</sub>	OTM,	OTM <sub>t</sub>	Torque kip-st
0	0.00	-7.65	-1058 76	81 90	-6 93
30	3 77	-6.54	-898.39	-458 93	-1.65

Valmont	Job 565090	Page 36 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 F	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by

Wind Azimuth	$V_{\tau}$	V2.	OTM.	OTM <sub>t</sub>	Torque
o	K	K	kip-ft	kip-ft	kip-ft
60	6.50	-3 75	-498 57	-848 08	1.09
90	7.53	0.00	38.35	-993 49	0.20
120	6.61	3 82	583.77	-862.80	2.25
150	3 74	6 48	968 70	-455 24	7.03
180	0.00	7 47	1111.08	81.90	6.93
210	-3 77	6.54	975.09	622.72	1.65
240	-6.67	3.85	587 46	1032.98	-1.09
270	-7.53	0.00	38 35	1157.28	-0 20
300	-6.45	-3.72	-494.88	1005.48	-2.25
330	-3 74	-6.48	-892.00	619 03	-7.03

### Mast Vectors - Service

Elevation ft	0 30 60 90 120 150 180 210 240 270 300 330	Wind Normal Wind 90 Wind 60 Wind 90 Wind Normal Wind 90 Wind 60 Wind 90 Wind 90 Wind 90 Wind Normal	0 29 0 26 0 24 0 24 0 27 0 26 0 27 0 26 0 27 0 26	0.00 0 13 0 21 0 24 0 23 0 13 0 00 -0 13 -0 23	-0 29 -0 23 -0 12 0 00 0 13 0 23 0 27 0 23 0 13	kip-ft -83.64 -64.94 -33.77 0.14 38.02 65.22 75.98 65.22	8ip-ft 0 16 -37 42 -58 58 -66 97 -65 45 -37 42 0 16 37 74	kip-ft -0.15 -0.06 0.00 0.01 0.16 0.15
	30 60 90 120 150 180 210 240 270 300 330	Wind 90 Wind 60 Wind 90 Wind Normal Wind 90 Wind 60 Wind 90 Wind 90 Wind 90	0 26 0 24 0 24 0 27 0 26 0 27 0 26 0 27	0 13 0 21 0 24 0 23 0 13 0 00 -0 13 -0 23	-0 23 -0 12 0 00 0 13 0 23 0 27 0 23	-64 94 -33 77 0 14 38.02 65 22 75 98	-37.42 -58.58 -66.97 -65.45 -37.42 0.16	-0 00 0 0 0 0 0 10 0 10
80.00-260.00	60 90 120 150 180 210 240 270 300 330	Wind 60 Wind 90 Wind Normal Wind 90 Wind 60 Wind 90 Wind Normal Wind 90	0 24 0 24 0 27 0 26 0 27 0 26 0 27	0 21 0 24 0 23 0 13 0 00 -0 13 -0 23	-0 12 0 00 0 13 0 23 0 27 0 23	-33 77 0 14 38 02 65 22 75 98	-58 58 -66 97 -65 45 -37 42 0 16	0 0 0 0 0 1 0 1 0 1
30.00-260.00	90 120 150 180 210 240 270 300 330	Wind 90 Wind Normal Wind 90 Wind 60 Wind 90 Wind Normal Wind 90	0 24 0 27 0 26 0 27 0 26 0 27	0 24 0 23 0 13 0 00 -0 13 -0 23	0 00 0 13 0 23 0 27 0 23	0 14 38.02 65 22 75.98	-66 97 -65 45 -37 42 0 16	0 0 0 1 0 1 0 1
30.00-260.00	120 150 180 210 240 270 300 330	Wind Normal Wind 90 Wind 60 Wind 90 Wind Normal Wind 90	0 27 0 26 0 27 0 26 0 27	0 23 0 13 0 00 -0 13 -0 23	0 13 0 23 0 27 0 23	38.02 65.22 75.98	-65 45 -37 42 0 16	0 1 0 1 0 1
80.00-260.00	150 180 210 240 270 300 330	Wind 90 Wind 60 Wind 90 Wind Normal Wind 90	0.26 0.27 0.26 0.27	0 13 0 00 -0 13 -0 23	0 23 0 27 0 23	65 22 75 98	-37 42 0 16	01
80.00-260.00	180 210 240 270 300 330	Wind 60 Wind 90 Wind Normal Wind 90	0 27 0 26 0 27	0 00 -0 13 -0 23	0 27 0 23	75.98	0.16	0.1
80.00-260.00	210 240 270 300 330	Wind 90 Wind Normal Wind 90	0 26 0 27	-0 13 -0 23	0 23			
30.00-260.00	240 270 300 330	Wind Normal Wind 90	0.27	-0 23		65 22	37 74	
30.00-260.00	270 300 330	Wind 90	6.000		0.13		21.14	0.0
30.00-260.00	300 330		0.24		0.15	38 02	65 77	-0 0
30.00-260.00	330	Wind 60		-0 24	0 00	0.14	67 29	-0 0
30.00-260.00			0 24	-0 21	-0 12	-33 77	58.90	-0.1
30.00-260.00	1.00	Wind 90	0 26	-0 13	-0 23	-64 94	37 74	-0 1
	0	Wind Normal	0.87	0.00	-0.87	-234 91	0.27	-0.3
	30	Wind 90	0.85	0.42	-0.73	-197.30	-113.96	0.1
	60	Wind 60	0 80	0 69	-0.40	-107 09	-186 17	0.3
	90	Wind 90	0.76	0.76	0.00	0 56	-205 67	0.2
	120	Wind Normal	0.79	0.68	0.39	107 03	-184 14	0.2
	150	Wind 90	0.79	0.40	0.68	185.41	-106.45	0.5
	180	Wind 60	0.83	0.00	0.83	223 35	0 27	0.3
	210	Wind 90	0.85	-0.42	0 73	198.41	114.50	-0 1
	240	Wind Normal	0.84	-0.73	0 42	114 53	197 69	-03
	270	Wind 90	0.76	-0.76	0.00	0.56	206.21	-0.2
	300	Wind 60	0.74	-0.64	-0 37	-99 58	173.71	-0.2
	330	Wind 90	0 79	-0 40	-0.68	-184 30	106 99	-0.5
60 00-240 00	0	Wind Normal	1.00	0.00	-1 00	-248.81	0.27	-0.4
21/2/4 = 21/11/2	30	Wind 90	0 97	0.48	-0 84	-208 86	-120.44	0.1
	60	Wind 60	0.96	0.83	-0.48	-119 23	-206.62	0.2
	90	Wind 90	0 97	0 97	0 00	0.22	-241.16	-0 1
	120	Wind Normal	1 00	0.86	0.50	124 74	-215 39	-0.1
	150	Wind 90	0.97	0.48	0 84	209 30	-120.44	0.4
	180	Wind 60	0.96	0.00	0 96	239.12	0.27	0.4
								-0.1
								-0.2
						0.22		0.1
								0 1
								-0.4
40.00-220.00								-0.6
10.00-420.00								0 1
								0.3
								-0 1
								-0 1
40	0.00-220.00	210 240 270 300 330	210 Wind 90 240 Wind Normal 270 Wind 90 300 Wind 60 330 Wind Normal 30 Wind Normal 30 Wind 90 60 Wind 60 90 Wind 90	210 Wind 90 0 97 240 Wind Normal 1 00 270 Wind 90 0 97 300 Wind 60 0 96 330 Wind 90 0 97 0 00-220.00 0 Wind Normal 1 14 30 Wind 90 1 09 60 Wind 60 1 08 90 Wind 90 1 .09	210 Wind 90 0 97 -0 48 240 Wind Normal 1 00 -0.86 270 Wind 90 0 97 -0 97 300 Wind 60 0 96 -0.83 330 Wind 90 0 97 -0 48 0.00-220.00 0 Wind Normal 1 14 0 00 30 Wind 90 1 09 0.55 60 Wind 60 1 08 0 93 90 Wind 90 1.09 1 09	210 Wind 90 0 97 -0 48 0 84 240 Wind Normal 1 00 -0 86 0 50 270 Wind 90 0 97 -0 97 0 00 300 Wind 60 0 96 -0 83 -0 48 330 Wind 90 0 97 -0 48 -0 84 0 00-220.00 0 Wind Normal 1 14 0 00 -1 14 30 Wind 90 1 09 0 55 -0 95 60 Wind 60 1 08 0 93 -0 54 90 Wind 90 1 09 1 09 0 00	210 Wind 90 0 97 -0 48 0 84 209 30 240 Wind Normal 1 00 -0 86 0 50 124 74 270 Wind 90 0 97 -0 97 0 00 0 22 300 Wind 60 0 96 -0 83 -0 48 -119 23 330 Wind 90 0 97 -0 48 -0 84 -208 86 0 00-220.00 0 Wind Normal 1 14 0 00 -1 14 -260 96 30 Wind 90 1 09 0 55 -0 95 -217 05 60 Wind 60 1 08 0 93 -0 54 -123 47 90 Wind 90 1 09 1 09 0 00 0 31	210 Wind 90 0 97 -0.48 0.84 209 30 120 99 240 Wind Normal 1 00 -0.86 0.50 124 74 215 94 270 Wind 90 0 97 -0.97 0.00 0.22 241 70 300 Wind 60 0.96 -0.83 -0.48 -119 23 207 16 330 Wind 90 0.97 -0.48 -0.84 -208.86 120 99 0.00-220.00 0 Wind Normal 1 14 0.00 -1 14 -260 96 0.39 30 Wind 90 1 0.9 0.55 -0.95 -217 0.5 -125 10 60 Wind 60 1 0.8 0.93 -0.54 -123 47 -214 00 90 Wind 90 1 0.9 1 0.9 0.00 0.31 -250 60

Valmont	Job	565090	Page 37 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Section No.	Section Elevation	Wind Azimuth	Directionality	F	$V_{\kappa}$	12	OTM <sub>x</sub>	OTM <sub>2</sub>	Torque
-	fi	0		K	K	K	kip-ft	kip-ft	kip-ft
		150	Wind 90	1 09	0.55	0 95	217.67	-125 10	0.58
		180	Wind 60	1.08	0.00	1.08	247 87	0.39	0.60
		210	Wind 90	1.09	-0.55	0.95	217 67	125.89	-0.17
		240	Wind Normal	1.14	-0 98	0.57	130.94	226.66	-0.38
		270	Wind 90	1.09	-1 09	0.00	0.31	251.38	0.15
		300	Wind 60	1.08	-0.93	-0 54	-123 47	214.79	0.11
	Vanna en el	330	Wind 90	1.09	-0.55	-0.95	-217.05	125.89	-0.58
T5	220.00-200 00	0	Wind Normal	1 16	0.00	-1 16	-242 52	0.49	-0 73
	100000000000000000000000000000000000000	30	Wind 90	1.11	0.56	-0.97	-202 34	-116.55	0.19
		60	Wind 60	1.10	0.95	-0.55	-115.19	-199.68	0.46
		90	Wind 90	1.11	1.11	0 00	0.38	-233 59	-0 17
		120	Wind Normal	1.16	1.00	0.58	121.82	-209.86	-0 14
		150	Wind 90	1.11	0.56	0.97	203.10	-116.55	0.69
		180	Wind 60	1.10	0.00	1.10	231.52	0.49	0.73
		210	Wind 90	1.11	-0 56	0 97	203 10	117.53	-0 19
		240	Wind Normal	1 16	-1.00	0.58	121.82	210 85	-0 46
		270	Wind 90	1.11	-1.11	0 00	0 38	234 57	0 17
		300	Wind 60	1 10	-0.95	-0.55	-115 19	200.67	0.14
		330	Wind 90	111	-0 56	-0 97	-202 34	117.53	-0 69
T6	200 00-180 00	0	Wind Normal	1.18	0.00	-1 18	-224 36	0 59	-0 86
	200 00-100.00	30	Wind 90	1 14	0.57	-0 98	-186 53	-107.36	0 22
		60	Wind 60	1.12	0.97	-0 56	-106.02	-183 81	0.53
		90	Wind 90	1 14	1 14	0 00	0 44	-215 30	-0 20
	120	Wind Normal	1.18	1.02	0.59	112 85	-194.10	-0 17	
	150	Wind 90	1.14	0.57	0 98	187 41	-107.36	0.80	
		180	Wind 60	1 12	0.00	1 12	213 37	0.59	0 86
		1/2/2017 CARSES	100,000,000				108 54	-0 22	
		210	Wind 90	1 14	-0.57	0.98	187.41		-0.53
		240	Wind Normal	1 18	-1.02	0.59	112 85	195 28	
	270	Wind 90	1 14	-1 14	0.00	0 44	216.49	0 20	
	300	Wind 60	1 12	-0.97	-0 56	-106 02	184 99	0 17	
22	122.22.122.22	330	Wind 90	1.14	-0.57	-0.98	-186 53	108 54	-0 80
17	180 00-160 00	0	Wind Normal	1 20	0.00	-1 20	-203.87	0.69	-0 98
		30	Wind 90	1 15	0 58	-1 00	-168 85	-97 09	0 24
		60	Wind 60	1 13	0.98	-0 57	-95 80	-166 12	0.60
		90	Wind 90	1.15	1.15	0.00	0.51	-194 87	-0 22
		120	Wind Normal	1 20	1 04	0.60	102.70	-176 31	-0.20
		150	Wind 90	1.15	0.58	1 00	169 87	-97 09	0 90
		180	Wind 60	1.13	0.00	1 13	193 13	0 69	0 98
		210	Wind 90	1.15	-0 58	1.00	169 87	98 47	-0 24
		240	Wind Normal	1 20	-1.04	0.60	102.70	177.69	-0 60
		270	Wind 90	1 15	-1 15	0.00	0.51	196 25	0 22
		300	Wind 60	1 13	-0 98	-0 57	-95 80	167 50	0.20
		330	Wind 90	1 15	-0.58	-1.00	-168.85	98.47	-0.90
T8	160 00-140 00	0	Wind Normal	1.29	0.00	-1 29	-192 53	0.79	-1 09
3.00		30	Wind 90	1.22	0.61	-1.06	-158 07	-90.80	0 23
		60	Wind 60	1 20	1 04	-0 60	-89.34	-154.93	0.63
		90		1 22	1 22	0 00	0 56	-182 38	-0 29
		120		1.29	1.11	0.64	97 11	-166.43	-0 25
		150	Wind 90	1.22	0.61	1 05	158.65	-90.48	1 00
		180	Wind 60	1 20	0 00	1 20	180 19	0.79	1 09
		210	Wind 90	1 22	16.0-	1.06	159 19	92 37	-0 23
	1	240	Wind Normal	1 29	-1 12	0.64	97.20	168 17	-0 63
		270	Wind 90			0.00	0.56	183 96	0 29
				1 22	-1 22				
		300	Wind 60	1 20	-1.04	-0 60	-89 25	156 35	0.25
TO	140.00 100.00	330		1 22	-0 61	-1 05	-157.52	92.06	-1 00
T9	140 00-120 00	0	Wind Normal	1 21	0.00	-1.21	-156 91	0.89	-1.19
		30		1 17	0.58	-101	-130 78	-74 96	0 18
		60		1 15	1.00	-0 58	-74.19	-128.63	0.56
		90		1.17	1.17	0 00	0.58	-150 80	-0.45
	1	120	Wind Normal	1.21	1.05	0.61	79 33	-135 51	-0.37
		150	Wind 90	1.15	0.58	1 00	130 11	-73 90	1.07

Valmont	Јо <b>ь</b> 565090	Page 38 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135	Date 07:03:40 10/06/22
Plymouth IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by

Section No.	Section Elevation	Wind Azimuth	Directionality	F	V,	I's	OTM <sub>x</sub>	OTM:	Torque
17.00	ſì	9		K	K	K	kip-fi	kip-ft	kip-ft
		180	Wind 60	1.15	0.00	1.15	149 53	0 89	1.
		210	Wind 90	1.17	-0.58	1.01	131 95	76.73	-0.
		240	Wind Normal	1 22	-1 05	061	79 63	137 80	-0.
		270	Wind 90	1.17	-1 17	0.00	0.58	152 57	0.4
		300	Wind 60	1.15	-0.99	-0.57	-73 89	129 87	0.
T10 120 00-100.00	330	Wind 90	1.15	-0.58	-1.00	-128 95	75.67	-1,0	
	0	Wind Normal	1 19	0.00	-1.19	-130 74	0.98	-1.3	
	30	Wind 90	1.15	0.57	-0 99	-108.74	-62 17	0	
		60	Wind 60	1.13	0.98	-0.57	-61.58	-106.79	0.
		90	Wind 90	1.15	1.15	0.00	0.64	-125.32	-0
-		120	Wind Normal	1 19	1 03	0.60	66.34	-112 80	-0
1		150	Wind 90	1.13	0.57	0 98	108.53	-61 30	1
4		180	Wind 60	1.13	0.00	1.13	124.59	0.98	L.
- 1		210	Wind 90	1 15	-0 57	0 99	110.03	64 14	-0
		240	Wind Normal	1.20	-1.04	0 60	66.58	115.20	-0
		270	Wind 90	1.15	-1.15	0.00	0.64	127 29	0
T11 100.00-80.00	300	Wind 60	1.13	-0.98	-0 56	-61.33	108 33	0.	
	330	Wind 90	1 13	-0.57	-0 98	-107 24	63.27	-1	
	0	Wind Normal	1.17	0.00	-1 17	-104.49	1 08	-1	
		30	Wind 90	1.12	0.56	-0.97	-86 71	-49 38	0.
		60	Wind 60	1 10	0 96	-0 55	-48 99	-84.99	0.
		90	Wind 90	1.12	1 12	0.00	0 70	-99 85	-0
Y.	120	Wind Normal	1.17	1.01	0.58	53 30	-90 02	-0	
T.	150	Wind 90	1.11	0.55	0.96	86.94	-48 71	1.	
	180	Wind 60	1 10	0.00	1 10	99.70	1 08	1	
	210	Wind 90	1 12	-0 56	0.97	88 12	51.55	-0	
	240	Wind Normal	117	-1.02	0 59	53 50	92.52	-0	
	270	Wind 90	1 12	-1.12	0.00	0 70	102 02	0	
	300	Wind 60	1 10	-0 95	-0.55	-48 79	86 82	0	
	330	Wind 90	1.11	-0.55	-0 96	-85 53	50 87	-1	
T12	80.00-60.00	0	Wind Normal	1.15	0.00	-1.15	-79 70	1.18	-1
		30	Wind 90	1.10	0 55	-0.95	-66 02	-37 38	0
		60	Wind 60	1 08	0 94	-0 54	-37 19	-64 56	0
		90	Wind 90	1 10	1.10	0.00	0.77	-75 94	-0
	1	120	Wind Normal	1.15	1 00	0.57	41.00	-68 50	-0
	,	150	Wind 90	1 09	0 54	0 94	66 69	-36 88	1
1		180	Wind 60	1 08	0.00	1.08	76 39	1 18	1
		210	Wind 90	1.10	-0.55	0.95	67 55	39 74	-0
		240	Wind Normal	1.15	-1.00	0.58	41.14	71.11	-0
		270	Wind 90	1.10	-1 10	0.00	0 77	78 30	0
		300	Wind 60	1.08	-0.94	-0.54	-37 05	66 68	0
Person.	a special series	330	Wind 90	1 09	-0 54	-0 94	-65.16	39 24	-1
T13	60 00-40 00	0	Wind Normal	1.14	0.00	-1 14	-56.39	1.28	-1
-		30	Wind 90	1.09	0.54	-0 94	-46.33	-25 94	0
		60	Wind 60	1.07	0.93	-0 53	-25 90	-45.01	0
		90	Wind 90	1 09	1.09	0.00	0 83	-53.16	-0
		120	Wind Normal	1 14	0 99	0 57	29 44	-48.27	-0
3		150	Wind 90	1.08	0.54	0.93	47.40	-25.61	1
		180	Wind 60	1.07	0 00	1.07	54.09	1.28	1
		210	Wind 90	1.09	-0.54	0 94	47 98	28.50	-0
		240	Wind Normal	1.15	-0 99	0.57	29 53	51.00	-0
		270	Wind 90	1 09	-1.09	0 00	0.83	55.73	0
		300	Wind 60	1.07	-0 92	-0 53	-25.81	47 41	0
2	Charles and	330	Wind 90	1.08	-0 54	-0 93	-45.75	28 17	-1
T14	40 00-20 00	0	Wind Normal	1 05	0 00	-1 05	-30 62	1,38	+1
		30	Wind 90	1 00	0.50	-0.86	-25.02	-13 58	0
		60	Wind 60	0.98	0.85	-0 49	-13 79	-24 04	0
		90	Wind 90	1.00	1.00	0.00	0.89	-28 54	-0
		120	Wind Normal	1.05	0.91	0.53	16.64	-25 90	-0
		150	Wind 90	0.99	0.49	0.85	26.48	-13 40	1
		180	Wind 60	0.98	0.00	0.98	30 14	1.38	1

Valmont	Job 565090	Page 39 of 72
1545 Pideo Dr.	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX (574)-936-6458	Client VB BTS II, LLC	Designed by

Section No.	Section Elevation	Wind Azimuth	Directionality	F	V <sub>x</sub>	$\Gamma_z$	OTM <sub>x</sub>	OTM <sub>2</sub>	Torque	
140-	ſ	0		K	K		kip-ft	kip-fi	kip-ft	
		210	Wind 90	1.00	-0 50	0 86	26.79	16 34	-0 18	
		240	Wind Normal	1.05	-0.91	0.53	16.69	28 75	-0 63	
		270	Wind 90	1.00	-1 00	0.00	0.89	31.30	0.51	
		300	Wind 60	0 98	-0.84	-0 49	-13.74	26.71	0.43	
		330	Wind 90	0 99	-0.49	-0.85	-24 71	16 16	-1 20	
T15 20 00-0 00	0	Wind Normal	0.94	0.00	-0.94	-8 41	1.48	-1.26		
100		30	Wind 90	0.89	0.44	-0.77	-6.73	-2.96	0.17	
		60	Wind 60	0.87	0.75	-0 43	-3.40	-6 06	0 59	
		90	Wind 90	0.89	0.89	0 00	0.95	-7.39	-0 47	
		120	Wind Normal	0.94	0.81	0.47	5.62	-6 62	-0 40	
		150	Wind 90	0 88	0.44	0.76	8 54	-2 91	1.11	
		180	Wind 60	0.87	0.00	087	9 62	1 48	1 26	
		210	Wind 90	0.89	-0.44	0.77	8.63	5 91	-0.17	
		240	Wind Normal	0 94	-0 81	047	5 64	9.60	-0 59	
		270	Wind 90	0.89	-0.89	0.00	0 95	10 35	0 47	
		300	Wind 60	0.87	-0.75	-0.43	-3 39	8 99	0.40	
		330	Wind 90	0.88	-0.44	-0.76	-6.64	5 86	-1.11	

## Mast Totals - Service

Wind Azimuth	$V_{\star}$	V <sub>z</sub>	OTM,	OTM,	Torque
0	K	K	kip-ft	kip-ft	kip-fi
0	0.00	-15 98	-2258 85	11.93	-14 41
30	7.65	-13 25	-1874 27	-1075.07	2.56
60	13.00	-7 50	-1054 95	-1829.97	7.56
90	15 19	0.00	8.48	-2131 53	-4 40
120	13.74	7.93	1126.87	-1925.17	-3 57
150	7.57	13.11	1871 33	-1063 58	13 26
180	0.00	15.04	2148 58	11.93	14 41
210	-7.65	13.25	1891.22	1098 94	-2.56
240	-13.82	7.98	1135 51	1964.02	-7.56
270	-15 19	0.00	8.48	2155 40	4.40
300	-12,92	-7.46	-1046.30	1838.86	3.57
330	-7.57	-13.11	-1854.38	1087.45	-13.26

### Discrete Appurtenance Pressures - No Ice $G_H = 0.850$

Description	Aiming Azimuth	Weight K	Offset <sub>z</sub>	Offset,	2	K	q.	C <sub>1</sub> 4c Front	C <sub>A</sub> A; Side ft²
	0				fi		psf	fi <sup>2</sup>	
5/8" x 10' lightning rod	240.0000	0.02	-2.50	1.44	295.00	1.589	39	0.63	0.63
Beacon	120.0000	0.07	2.50	1.44	291 00	1.585	39	2.40	2 40
OB light	0 0000	0 03	0.00	-9 53	146 00	1371	34	0.50	0.50
OB light	120.0000	0.03	8.25	4 76	146 00	1 371	34	0.50	0.50
OB light	240.0000	0.03	-8 25	4.76	146.00	1 371	34	0.50	0.50
40,000 sq in. (277 8 sq ft EPA)	0.0000	4 50	0 00	0.00	285 00	1 578	39	277.80	277 80
30,000 sq.in. (208.3 sq ft. EPA)	0.0000	4.10	0 00	0.00	275.00	1.566	38	208.30	208 30
30,000 sq.in (208.3 sq ft. EPA)	0.0000	4.10	0.00	0.00	265 00	1 554	38	208 30	208 30

Valmont	Јо <b>в</b> 565090	Page 40 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL.

Description	Aiming Azimuth	Weight K	Offset,	Offset,	ž p	K <sub>i</sub>	q: psf	C <sub>1</sub> Ac Front ft <sup>2</sup>	C <sub>1</sub> A <sub>1</sub> Side ft <sup>2</sup>
SP1 R5 (Includes 4 5"x72" Pipe)	240 0000	0.14	-3 93	2.27	240 00	1.522	37	2 85	3.15
2-1/2" x 7' Sch. 40 2-1/2" x 7' Sch. 40	60 0000 180 0000 Sum Weight:	0.04 0.04 13.10	1.75 0.00	-1.01 2.02	240 00 240.00	1.522 1.522	37 37	201	2 01 2 01

## Discrete Appurtenance Vectors - No Ice

	7.000	5 8'	x 10' lightning rod -	Elevation 295 - Fre	m Leg C		
Wind Azimuth	Fo	$F_{r}$	Ve	V <sub>z</sub>	OTM <sub>x</sub>	OTM:	Torque
0	K	K	K	K	kip-fi	kip-ft	kip-fi
0	0.01	0.02	0.00	-0.02	-6.06	0.06	-0.05
30	0.02	0.01	0.01	-0.02	-5.24	-2 99	-0.03
60	0.02	0.00	0.02	-0.01	-3 01	-5 22	0.00
90	0.02	0.01	0.02	0.00	0 03	-6 03	0.03
120	0.01	0.02	0.02	0.01	3 08	-5.22	0.05
150	0.00	0.02	0.01	0.02	5 31	-2.99	0.06
180	0.01	0.02	0.00	0.02	6 12	0 06	0.05
210	0.02	0.01	-0.01	0.02	5.31	3 10	0.03
240	0.02	0.00	-0.02	0.01	3.08	5 33	0.00
270	0.02	0.01	-0.02	0.00	0 03	6.15	-0.03
300	0.01	0.02	-0.02	-0.01	-3 01	5 33	-0.05
330	0.00	0.02	-0.01	-0.02	-5 24	3.10	-0.06

			Reacon - Elevan	on 291 - From Leg I	H			
Wind Azimuth	Fa	$F_s$	ν.	V <sub>z</sub>	OTM,	OTM:	Torque	
• K	K	K	K	K K		kip-ft	kip-fi	
0	0.04	0.07	0.00	-0 08	-22.90	-0 18	0.20	
30	0.00	0.08	0.04	-0 07	-19 81	-11.68	0 23	
60	0.04	0.07	0.07	-0.04	-11.40	-20 10	0.20	
90	0 07	0.04	0.08	0.00	0.11	-23 18	0.11	
120	0.08	0.00	0.07	0.04	11.61	-20 10	0 00	
150	0.07	0.04	0.04	0.07	20.03	-11.68	-0.11	
180	0 04	0.07	0.00	0.08	23.11	-0 18	-0 20	
210	0 00	0 08	-0.04	0.07	20.03	11.32	-0 23	
240	0 04	0.07	-0.07	0.04	11.61	19 74	-0 20	
270	0.07	0.04	-0.08	0.00	0.11	22 82	-0.11	
300	0.08	0 00	-0.07	-0.04	-11 40	19 74	0.00	
330	0.07	0.04	-0.04	-0.07	-19.81	11.32	0 11	

OB light - Flevation 146 - From Leg A									
Wind Azimuth	F <sub>a</sub>	F.	V.	$V_{\varepsilon}$	OTM <sub>e</sub>	OTM:	Torque		
0	K	K	K	K	kip-ft	kip-fi	kip-fi		
0	0.01	0.00	0.00	-0.01	-2 37	0.00	0.00		
30	0.01	0.01	0.01	-0.01	-2.09	-1 04	-0.07		
60	0.01	0.01	0.01	-0.01	-1 33	-1.80	-0.12		
90	0.00	0.01	0.01	0.00	-0 29	-2 08	-0.14		
120	0.01	0.01	0.01	0.01	0.75	-1.80	-0.12		
150	0 01	0.01	0.01	0.01	1.51	-1 04	-0.07		
180	0.01	0.00	0.00	0.01	1 79	0 00	0.00		

Valmont	Job	565090	Page 41 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, 1N Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

OB light - Elevation 146 - From Leg A									
Wind Azimuth	$F_{\alpha}$	F,	Ve	V <sub>2</sub>	OTM <sub>x</sub>	OTM:	Torque		
0	K	K	K	K	kip-ft	kip-ft	kip-ft		
210	0.01	0.01	-0.01	0.01	1.51	1.04	0.07		
240	0.01	0 01	-0.01	0.01	0.75	1.80	0.12		
270	0 00	0 01	-0.01	0.00	-0 29	2 08	0 14		
300	0.01	0.01	-0.01	-0.01	-1 33	1.80	0 12		
330	0 01	0.01	-0.01	-0.01	-2.09	1.04	0.07		

			OB light - Elevat	ion 146 - From Leg	В		
Wind Azimuth	Fa	Fi	V <sub>e</sub>	$V_z$	OTM,	OTM <sub>2</sub>	Torque
0	K	K	K	K	kip-ft	kip-fi	kip-ft
0	0.01	0.01	0.00	-0.01	-1 94	-0.25	0.12
30	0.00	0.01	0.01	-0.01	-1 66	-1 29	0 14
60	0.01	0.01	0.01	-0.01	-0 90	-2 05	0.12
90	0.01	0.01	0.01	0.00	0.14	-2 33	0.07
120	0.01	0.00	0.01	0.01	1.18	-2.05	0.00
150	0.01	0.01	0.01	0.01	1 94	-1 29	-0 07
180	001	0.01	0.00	0.01	2 22	-0 25	-0 12
210	0.00	0.01	-0.01	0.01	1 94	0.79	-0 14
240	0.01	0.01	-0.01	0.01	1.18	1.55	-0.12
270	0.01	0.01	-0.01	0.00	0.14	1.83	-0.07
300	0.01	0.00	-0.01	-0.01	-0 90	1 55	0.00
330	0.01	0.01	-0.01	-0.01	-1.66	0.79	0.07

			OB light - Elevat	ion 146 - From Leg	C		
Wind Azimuth	$F_{u}$	F <sub>s</sub>	$\nu_{\cdot}$	V <sub>a</sub>	ОТМ,	OTM:	Torque
0	K	K	A.	K	kip-ft	kip-ft	kip-ft
0	0.01	0.01	0.00	-0 01	-1 94	0.25	-0.12
30	0.01	0.01	0.01	-0.01	-1.66	-0 79	-0.07
60	0.01	0.00	0.01	-0.01	-0.90	-1.55	0.00
90	0.01	0.01	0.01	0.00	0.14	-1.83	0.07
120	100	0.01	0.01	0.01	1.18	-1 55	0.12
150	0 00	0.01	0.01	0.01	1.94	-0 79	0 14
180	0.01	0.01	0 00	0.01	2.22	0.25	0.12
210	0.01	0 01	-0.01	0.01	1.94	1 29	0.07
240	0.01	0.00	-0.01	0.01	1.18	2.05	0.00
270	0.01	0.01	-0.01	0.00	0.14	2 33	-0.07
300	0.01	0.01	-0 01	-0.01	-0.90	2 0 5	-0.12
330	0.00	0.01	-0 01	-0.01	-1.66	1.29	-0 14

		40,00	0 sq.m. (277.8 sq.ft	nras - cievation 28	J = None A		
Wind Azimuth	$F_a$	F,	$\nu_{\epsilon}$	<i>V</i> _	OTM,	OTM:	Torque
ď	K	K	K	K	kip-ft	kip-ft	kip-ft
0	911	0.00	0.00	-911	-2596 13	0 00	0.00
30	9 1 1	0.00	4 55	-7 89	-2248 31	-1298 06	0.00
60	9.11	0.00	7.89	-4.55	-1298 06	-2248 31	0.00
90	9 11	0.00	9.11	0.00	0.00	-2596.13	0.00
120	9.11	0.00	7.89	4 55	1298 06	-2248 31	0.00
150	9.11	0.00	4.55	7.89	2248 31	-1298.06	0.00
180	9.11	0.00	0 00	9 11	2596.13	0.00	0.00
210	9.11	0.00	-4.55	7 89	2248 31	1298 06	0.00
240	9.11	0.00	-7.89	4.55	1298.06	2248.31	0.00
270	911	0.00	-9 11	0 00	0.00	2596.13	0.00

Valmont	Job	565090	Page 42 of 72
1545 Pidco Dr.	Project H	-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 F4V: (574)-936-6458	Client	VB BTS II, LLC	Designed by

		40,00	0 sq in (277,8 sq.ft	EPA) - Elevation 28	5 - None A		
Wind Azimuth	F <sub>a</sub>	F,	ν,	1/2	OTM,	OTM:	Torque
0	K	K	K	K	kip-ft	kip-ft	kip-ft
300	9.11	0 00	-7.89	-4.55	-1298 06	2248 31	0.0
330	911	0.00	-4.55	-7.89	-2248.31	1298 06	0.0

		30,00	0 sq m. (208,3 sq.fr	EFA) - Elevation 2			
Wind Azimuth	Fa	F.	V <sub>*</sub>	$V_z$	OTM,	OTM	Torque
o K	λ*	A.	K	kip-ft	kip-ft	kip-ft	
0	5,56	0.00	0.00	-5 56	-1528.69	0.00	0.00
30	5 56	0.00	2.78	-4 81	-1323.88	-764 34	0.00
60	5 56	0.00	4.81	-2 78	-764 34	-1323.88	0.00
90	5.56	0.00	5.56	0.00	0.00	-1528.69	0.00
120	5 56	0.00	4.81	2 78	764.34	-1323 88	0.00
150	5 56	0.00	2.78	4.81	1323.88	-764 34	0.00
180	5 56	0.00	0.00	5.56	1528.69	0.00	0.00
210	5 56	0.00	-2.78	4 81	1323 88	764 34	0.00
240	5 56	0.00	-4.81	2.78	764.34	1323 88	0.00
270	5 56	0.00	-5.56	0.00	0.00	1528 69	0.00
300	5 56	0.00	-4.81	-2.78	-764.34	1323.88	0.00
330	5 56	0.00	-2.78	-4 81	-1323.88	764.34	0.00

		30,00	0 sq.in. (208,3 sq.ft.	EPA) Elevation 26	5 - None B	- KA - 4	0.0 x 35x
Wind Azimuth	$F_{\phi}$	F,	$V_{i}$	V	OTM <sub>s</sub>	OTM;	Torque
0	K	K	K	K	kip-fi	kip-ft	kip-ft
0	5.52	0.00	0.00	-5 52	-1461 66	0.00	0.00
30	5.52	0.00	2.76	-4.78	-1265.83	-730.83	0.00
60	5 52	0.00	4.78	-2.76	-730 83	-1265.83	0.00
90	5.52	0.00	5.52	0 00	0.00	-1461.66	0.00
120	5.52	0.00	4.78	2.76	730 83	-1265.83	0.00
150	5.52	0 00	2.76	4.78	1265 83	-730.83	0.00
180	5 52	0.00	0.00	5.52	1461 66	0.00	0.00
210	5.52	0.00	-2 76	4.78	1265.83	730.83	0.00
240	5.52	0.00	-4 78	2.76	730 83	1265.83	0.00
270	5 52	0 00	-5 52	0 00	0.00	1461 66	0.00
300	5.52	0.00	-4 78	-2.76	-730.83	1265.83	0.00
330	5.52	0.00	-2.76	-4.78	-1265 83	730.83	0.00

Wind Azimuth	Fa	Fi	Vx	V <sub>z</sub>	OTM <sub>x</sub>	OTM:	Torque
0	K	K	K	A.	kip-ft	kip-fi	kip-ft
0	0.05	0.09	-0.00	-0 10	-23.03	1.52	-0.3
30	0 08	0.05	0.04	-0 08	-19.41	-9 71	-0.23
60	0 09	0.00	0.08	-0 05	-10.50	-18 19	0.00
90	0.08	0.05	0.09	0.00	1.30	-21.66	0.23
120	0.05	0.09	0.08	0.05	12.83	-19 18	0.39
150	0.00	0.10	0.05	0.09	21.02	-11.42	0.43
180	0.05	0.09	0.00	0.10	23.65	-0 45	0.39
210	0.08	0.05	-0.04	0.08	20 03	10 79	0.23
240	0 09	0.00	-0.08	0.05	11.13	19 27	0.00
270	0.08	0.05	-0.09	-0 00	-0.67	22 74	-0.2
300	0.05	0.09	-0.08	-0.05	-12 21	20 26	-0.39
330	0.00	0 10	-0.05	-0 09	-20 39	12 49	-0 4

Valmont	Job	565090	Page 43 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, 1N Phone (574)-936-4221 FAX (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

		2.	) 2" x 7' Sch, 40 - Et	levation 240 - From	Face B		
Wind Azimuth	F <sub>a</sub>	F,	P <sub>x</sub>	V	OTM.	OTM;	Torque
0	K	K	K	K	kip-ft	kip-ft	kip-fi
0	0.03	0.06	0.00	-0 06	-15 32	-0 07	0.1
30	0.06	0.03	0.03	-0.06	-13.27	-7.71	0.0
60	0.06	0.00	0.06	-0.03	-7 68	-13.30	0.0
90	0.06	0.03	0.06	0 00	-0 04	-15.35	-0 0
120	0.03	0.06	0.06	0.03	7 60	-13 30	-0 1
150	0.00	0.06	0.03	0.06	13.19	-7.71	-0.1
180	0.03	0.06	0 00	0.06	15.23	-0.07	-0.1
210	0.06	0.03	-0 03	0.06	13.19	7.57	-00
240	0.06	0.00	-0 06	0.03	7.60	13.16	0.0
270	0.06	0.03	-0 06	0.00	-0.04	15 20	0.0
300	0.03	0.06	-0.06	-0.03	-7 68	13 16	0.1
330	0.00	0.06	-0 03	-0 06	-13.27	7.57	0.1

		2-	1 2" x 7' Sch. 40 - El	evation 240 - From	Face C		
Wind Azimuth	F <sub>a</sub>	F <sub>a</sub>	$\nu_{\epsilon}$	$V_x$	OTM,	OTM <sub>2</sub>	Torque
0	K	K	K	K	kip-ft	kip-fi	kip-fi
0	0.06	0:00	0.00	-0 06	-15.19	0 00	0.00
30	0.06	0.03	0.03	-0.06	-13.15	-7.64	0.06
60	0.03	0.06	0.06	-0.03	-7 56	-13.23	0.11
90	0.00	0.06	0 06	0.00	0.08	-15.28	0.13
120	0.03	0.06	0.06	0.03	7.72	-13.23	0.11
150	0.06	0.03	0.03	0.06	13.31	-7 64	0.06
180	0.06	0.00	0.00	0.06	15.36	0.00	0 00
210	0.06	0.03	-0.03	0.06	13.31	7.64	-0 06
240	0.03	0.06	-0 06	0 03	7.72	13 23	-0.11
270	0 00	0.06	-0 06	0.00	0.08	15 28	-0.13
300	0 03	0.06	-0.06	-0.03	-7.56	13.23	-0 11
330	0.06	0.03	-0.03	-0.06	-13.15	7.64	-0.06

## Discrete Appurtenance Totals - No Ice

Wind Azimuth	$V_{\tau}$	$V_z$	OTM,	OTM <sub>z</sub>	Torque
0	K	K	kip-ft	kip-ft	kip-ft
0	-0.00	-20.55	-5675.21	1.33	-0.13
30	10.27	-17.80	-4914 31	-2836.08	0.10
60	17 79	-10 27	-2836 50	-4913 48	0.31
90	20 55	0.00	1.48	-5674.22	0.43
120	17.80	10.28	2839 19	-4914.46	0.44
150	10 28	17 80	4916 28	-2837 79	0.33
180	0.00	20 55	5676 19	-0.64	0.13
210	-10.27	17 80	4915 30	2836.77	-0.10
240	-17.79	10 27	2837 49	4914 16	-0.31
270	-20.55	-0.00	-0.50	5674.90	-0.43
300	-17.80	-10.28	-2838 21	4915.15	-0.44
330	-10.28	-17.80	-4915 30	2838 48	-0 33

Valmont	Job	565090	Page 44 of 72
1545 Pidco Dr	Project H-31 x	(290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Description	Aiming Azimuth	Weight	Offset,	Offset,		$K_{\epsilon}$	q-	C <sub>t</sub> A <sub>C</sub> Front	C <sub>4</sub> A <sub>c</sub> Side	$l_{\phi}$
	-0-	K	fi	ft	ft		psf	ft <sup>2</sup>	ſŕ	in
5/8" x 10' lightning rod	240.0000	0.13	-2 50	1 44	295.00	1.589	3	5 17	5 17	2 4898
Beacon	120 0000	0 19	2 50	1 44	291.00	1 585	3	3 74	3 74	2 4864
OB light	0 0000	0.05	0 00	-9 53	146 00	1.371	3	0 96	0 96	2 3207
OB light	120.0000	0.05	8 25	4.76	146 00	1 371	3	0 96	0 96	2 3207
OB light	240 0000	0.05	-8 25	4.76	146 00	1.371	3	0.96	0.96	2 3207
40,000 sq in. (277 8 sq ft	0 0000	9 46	0 00	0 00	285.00	1 578	3	622 44	622 44	2 4812
EPA)										100
30,000 sq in. (208 3 sq ft	0.0000	9.54	0.00	0 00	275.00	1.566	3	465 82	465 82	2 4724
EPA)						1000				
30,000 sq in. (208 3 sq ft EPA)	0 0000	9 52	0.00	0 00	265 00	1 554	3	464 87	464 87	2 4632
SPI R5 (Includes	240 0000	0.32	-3.93	2.27	240 00	1 522	3	5 45	5.90	2 4389
4 5"x72" Pipe)	(0.0000	0.12	1.00	1.01	240.00	1.500		4.00	4.00	2 4200
2-1/2" x 7' Sch 40	60 0000	0.17	1 75	-1.01	240.00	1.522	3	4 00	4 00	2 4389
2-1/2" x 7' Sch 40	180 0000 Sum Weight	0.17 29.65	0.00	2.02	240 00	1 522	3	4.00	4.00	2 4389

# Discrete Appurtenance Vectors - With Ice

		58'	'x 10' lightning rod -	Elevation 295 - Fre	om Leg (		
Wind Azimuth	Fa	F,	Vx	$\nu_z$	OTM,	OTM:	Torque
0	K	K	K	K	kip-ft	kip-ft	kip-ft
0	0.01	0.01	0.00	-0.01	-3 85	0.33	-0.03
30	0.01	0.01	0.01	-0.01	-3 30	-1 69	-0 02
60	0.01	0.00	0.01	-0.01	-1 83	-3.16	0.00
90	0.01	0.01	0.01	0.00	0.19	-3.71	0.02
120	0.01	0.01	0.01	0.01	2 21	-3 16	0.03
150	0.00	0.01	10.0	0.01	3.69	-1 69	0.04
180	0.01	0.01	0.00	0.01	4 23	0.33	0.03
210	0.01	0.01	-0 01	0.01	3 69	2 35	0 02
240	0.01	0.00	-0.01	0.01	2 21	3.83	0.00
270	0.01	0.01	-0.01	0.00	0.19	4.37	-0.02
300	0.01	0 01	-0.01	-0.01	-1.83	3.83	-0.03
330	0 00	0.01	-0.01	-0 01	-3.30	2 35	-0.04

			Beacon - Elevan	on 291 - From Leg	B		
Wind Azimuth	Fa	F,	$\nu_{\star}$	V <sub>z</sub>	OTM,	OTM.	Torque
0	K	K	K	K	kip-fi	kip-ft	kip-fi
0	0.00	0 01	0.00	-0.01	-2.60	-0.48	0.02
30	0.00	0.01	0.00	-0.01	-2.21	-1 92	0.03
60	0.00	0.01	0.01	-0.00	-1.16	-2 97	0.02
90	0.01	0.00	0.01	0.00	0.28	-3.35	0.01
120	0.01	0.00	0.01	0.00	1.71	-2 97	0.00
150	0.01	0.00	0.00	0.01	2.77	-1 92	-0.01
180	0.00	0.01	0.00	0.01	3 15	-0 48	-0.02
210	0.00	0.01	-0.00	0.01	2 77	0 96	-0.03
240	0.00	0.01	-0.01	0.00	1.71	2 01	-0.02
270	0.01	0.00	-0.01	0.00	0.28	2 39	-0.01
300	0.01	0.00	-0.01	-0 00	-1 16	2 01	0.00
330	0.01	0.00	-0.00	-0.01	-2.21	0.96	0.01

Valmont	Job 565090	Page 45 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

			OB light - Elevat	ion 146 - From Leg	A		
Wind Azimuth	Fu	F,	V.	$\nu_{z}$	OTM,	OTM <sub>=</sub>	Torque
0.	K	A'	K	K	kip-fi	kip-ft	kip-ft
0	0.00	0.00	0.00	-0.00	-0.83	0.00	0.00
30	0.00	0.00	0.00	-0 00	-0.78	-0 16	-0.01
60	0.00	0.00	0.00	-0 00	-0 67	-0 28	-0.02
90	0.00	0.00	0.00	0.00	-0.51	-0 32	-0.02
120	0.00	0.00	0.00	0 00	-0 35	-0 28	-0.02
150	0.00	0.00	0.00	0.00	-0 23	-0 16	-0.01
180	0.00	0.00	0.00	0 00	-0 19	0 00	0.00
210	0.00	0.00	-0.00	0.00	-0 23	0 16	0.01
240	0.00	0.00	-0.00	0 00	-0.35	0.28	0.02
270	0.00	0.00	-0.00	0 00	-0.51	0 32	0.02
300	0.00	0.00	-0.00	-0 00	-0.67	0 28	0.02
330	0.00	0.00	-0.00	-0 00	-0.78	0.16	0.01

21.8	N-1		OB light - Elevat	ion 146 - From Leg	B		
Wind Azimuth	Fa	$F_{i}$	V <sub>x</sub>	V.	OTM <sub>x</sub>	OTM <sub>2</sub>	Torque
0	K	K	K	K	kip-ft	kip-ft	kip-ft
0	0.00	0.00	0.00	-0.00	-0 07	-0 44	0.02
30	0.00	0.00	0.00	-0.00	-0.02	-0 60	0.02
60	0.00	0.00	0.00	-0.00	0.09	-0.72	0.02
90	0.00	0.00	0.00	0.00	0.25	-0.76	0.01
120	0.00	0.00	0.00	0.00	0 41	-0 72	0.00
150	0.00	0.00	0.00	0.00	0.53	-0 60	-0 01
180	0.00	0.00	0.00	0.00	0.57	-0.44	-0.02
210	0.00	0.00	-0.00	0.00	0.53	-0 28	-0.02
240	0.00	0.00	-0.00	0.00	0 41	-0.16	-0 02
270	0.00	0.00	-0.00	0.00	0 25	-0.12	-0 01
300	0.00	0.00	-0.00	-0.00	0 09	-0.16	0.00
330	0.00	0.00	-0.00	-0.00	-0 02	-0 28	0.01

			OB light - Elevat	ion 146 - From Leg	C		
Wind Azimuth	Fa	F <sub>1</sub>	ν,	V <sub>z</sub>	OTM <sub>x</sub>	OTM:	Torque
9	K	K	K	K	kip-fi	kip-fi	kip-ft
0	0.00	0.00	0.00	-0.00	-0.07	0.44	-0.02
30	0.00	0.00	0.00	-0.00	-0.02	0.28	-0.01
60	0.00	0.00	0.00	-0.00	0.09	0 16	0.00
90	0.00	0.00	0 00	0.00	0.25	0 12	0.01
120	0.00	0.00	0.00	0.00	0 41	0.16	0.02
150	0.00	0.00	0.00	0.00	0.53	0 28	0.02
180	0.00	0.00	0.00	0.00	0.57	0.44	0.02
210	0.00	0.00	-0.00	0.00	0.53	0 60	0.01
240	0.00	0.00	-0.00	0.00	0.41	0 72	0.00
270	0.00	0.00	-0.00	0.00	0.25	0 76	-0 01
300	0.00	0.00	-0.00	-0 00	0.09	0.72	-0.02
330	0.00	0.00	-0.00	-0.00	-0.02	0.60	-0 02

Wind	F <sub>a</sub>	F <sub>a</sub>	0 sq m. (277.8 sq ft. V.	P <sub>2</sub>	OTM.	OTM;	Torque
Azimuth	K	К	K	K	kip-ft	kip-fi	kıp-fi
0 30	1.63 1.63	0.00	0.00 0.82	-1 63 -1.42	-465 93 -403 51	0.00 -232 97	0.0

Valmont	Job	565090	Page 46 of 72
1545 Pideo Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Wind Azımuth	$F_{o}$	F,	V <sub>x</sub>	$V_z$	OTM <sub>r</sub>	OTM-	Torque
o o	K	K	K	K	kip-fi	kip-ft	kip-ft
60	1 63	0.00	1.42	-0.82	-232 97	-403 51	0.00
90	1.63	0.00	1.63	0.00	0.00	-465.93	0.00
120	1 63	0.00	1.42	0.82	232 97	-403 51	0.00
150	1.63	0.00	0.82	1.42	403 51	-232.97	0.00
180	1.63	0.00	0.00	1.63	465 93	0.00	0.00
210	1.63	0.00	-0.82	1.42	403 51	232 97	0.00
240	1.63	0.00	-1.42	0.82	232 97	403.51	0.00
270	1.63	0.00	-1.63	0.00	0.00	465 93	0.00
300	1.63	0.00	-1.42	-0.82	-232 97	403 51	0.00
330	1.63	0.00	-0.82	-1.42	-403 51	232 97	0.00

		30,00	0 sq.in. (208.3 sq.ft.	EPA) - Elevation 27.	5 - None C		
Wind Azimuth	Fu	F.	V.	V <sub>2</sub>	OTM <sub>x</sub>	OTM:	Torque
0	K	K	K	K	kip-ft	kip-fi	kip-fi
0	1.00	0.00	0.00	-1.00	-273 83	0.00	0.00
30	1.00	0.00	0.50	-0.86	-237 14	-136.91	0.00
60	1.00	0.00	0.86	-0.50	-136 91	-237 14	0.00
90	1.00	0.00	1.00	0.00	0.00	-273 83	0.00
120	1.00	0.00	0.86	0.50	136.91	-237 14	0.00
150	1.00	0.00	0.50	0.86	237.14	-136 91	0.00
180	1.00	0.00	0.00	1.00	273.83	0.00	0.00
210	1.00	0.00	-0 50	0.86	237.14	136 91	0.00
240	1.00	0.00	-0 86	0.50	136.91	237 14	0.00
270	1.00	0.00	-1.00	0.00	0.00	273 83	0.00
300	1.00	0.00	-0.86	-0.50	-136.91	237 14	0.00
330	1.00	0.00	-0.50	-0.86	-237.14	136 91	0.00

		30,00	0 sq.m. (208,3 sq.ft )	EPA) - Elevation 26	5 - None B		
Wind Azimuth	Fa	F,	$V_{\epsilon}$	V.	OTM,	OTM:	Torque
٥	K	K	K	K	kip-ft	kip-ft	kip-fi
0	0.99	0.00	0.00	-0.99	-261 29	0.00	0.00
30	0 99	0.00	0.49	-0.85	-226 28	-130 64	0.00
60	0 99	0.00	0.85	-0.49	-130 64	-226 28	0.00
90	0 99	0.00	0.99	0.00	0.00	-261 29	0.00
120	0.99	0.00	0.85	0.49	130.64	-226 28	0.00
150	0.99	0.00	0.49	0.85	226 28	-130.64	0.00
180	0.99	0.00	0.00	0.99	261 29	0 00	0.00
210	0.99	0.00	-0.49	0.85	226.28	130.64	0.00
240	0 99	0 00	-0 85	0 49	130.64	226.28	0.00
270	0 99	0.00	-0 99	0.00	0.00	261.29	0.00
300	0.99	0.00	-0 85	-0.49	-130.64	226.28	0.00
330	0.99	0.00	-0 49	-0.85	-226 28	130.64	0.00

Wind Azimuth	$F_a$	F,	Vx	$V_{z}$	OTM,	OTM:	Torque
a	K	K	K	K	kip-ft	kip-ft	kip-ft
0	0.01	0.01	-0:00	-0 01	-2.80	1.37	-0.06
30	0.01	0.01	0.01	-0.01	-2.27	-0 34	-0 03
60	0.01	0.00	0.01	-0 01	-0.94	-1.62	0.00
90	0.01	0.01	0.01	0.00	0.84	-2 13	0.03
120	0.01	0.01	10.0	0.01	2.58	-1 74	0.06

Valmont	Job	565090	Page 47 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Wind Azimuth	$F_o$	F <sub>3</sub>	ν,	V <sub>2</sub>	OTM,	OTM:	Torque
0	K	K	K	K	kip-ft	kıp-ft	kip-fi
150	0.00	0.01	0.01	0.01	3.83	-0.54	0.07
180	0.01	0.01	0.00	0.01	4 24	1.13	0.06
210	0.01	0.01	-0.01	0.01	3.71	2.84	0 03
240	0.01	0.00	-0.01	0.01	2.38	4 12	0.00
270	0.01	0.01	-0.01	-0.00	061	4.64	-0 03
300	0.01	0.01	-0.01	-0.01	-1 14	4.24	-0.06
330	0.00	0.01	-0.01	-0.01	-2 38	3.05	-0 07

		2-	1 2"x 7" Sch. 40 - El	evotion 240 - From	Face B		
Wind Azimuth	Fa	F,	ν,	V <sub>2</sub>	OTM <sub>r</sub>	OTM:	Torque
0	K	K	K	K	kip-ft	kip-ft	kip-fi
0	0.01	0.01	0.00	-0.01	-2 60	-0 29	0.02
30	0.01	0.01	0.01	-0.01	-2 27	-1.51	0.01
60	0.01	0.00	0.01	-0.01	-1 38	-2 40	0.00
90	0.01	0.01	10.0	0.00	-0.17	-2 72	-0.01
120	0.01	0.01	0.01	0.01	1.05	-2 40	-0.02
150	0.00	0.01	0.01	0.01	1.94	-1.51	-0.02
180	0.01	0.01	0.00	0.01	2.27	-0.29	-0.02
210	0.01	0.01	-0.01	0.01	1.94	0.93	-0.01
240	0.01	0 00	-0.01	0.01	1.05	1 82	0.00
270	0.01	0.01	-0.01	0.00	-0.17	2 14	0.01
300	0.01	0.01	-0 01	-0 01	-1.38	1 82	0.02
330	0 00	0.01	-0.01	-0 01	-2.27	0.93	0.02

	-	2-	1 2" x 7' Seh. 40 - Et	evation 240 - From	Face C		
Wind Azımuth	Fo	F,	ν,	ν-	OTM <sub>x</sub>	OTM:	Torque
0	K	K	Α-	K	kip-ft	kip-ft	kip-ft
0	0.01	0.00	0.00	-0.01	-2.10	0.00	0.00
30	0.01	0.01	0.01	-0.01	-1.77	-1.22	0.01
60	0.01	0.01	0.01	-0.01	-0 88	-2 11	0.02
90	0.00	0 01	0.01	0.00	0.33	-2.43	0.02
120	0 0 1	0.01	0.01	0.01	1 55	-2.11	0.02
150	0.01	0.01	0.01	0.01	2 44	-1.22	0.01
180	0.01	0.00	0.00	0.01	2.77	0.00	0.00
210	0.01	0.01	-0.01	0.01	2 44	1.22	-0.01
240	0.01	0.01	-0.01	0.01	1.55	2 11	-0.02
270	0.00	0.01	-0.01	0.00	0.33	2.43	-0 02
300	0.01	0.01	-0.01	-0.01	-0 88	2 11	-0.02
330	0.01	0.01	-0 01	-0.01	-1 77	1 22	-0.01

## Discrete Appurtenance Totals - With Ice

Wind Azımuth	V <sub>x</sub> K	V.	OTM, kip-ft	OTM <sub>t</sub> kip-fi	Torque kip-ft
0	-0 00	-3 68	-1015 95	0.93	-0.05
30	1 84	-3 19	-879.60	-507 67	-0.00
60	3 19	-1 84	-507 19	-880 03	0.04
90	3 68	0 00	1 48	-1016.36	0 08
120	3.19	1 84	510.11	-880 14	0.09

Valmont	Job	565090	Page 48 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Wind Azimuth	V <sub>x</sub>	V <sub>i</sub>	OTM, kip-ft	OTM <sub>2</sub> kip-ft	Torque kip-ft
150	1.84	3 19	882.43	-507.88	0.08
180	0.00	3 68	1018 67	0.69	0.05
210	-1 84	3 19	882.31	509 30	0.00
240	-3 19	1 84	509.91	881.65	-0 04
270	-3 68	-0 00	1 24	1017.98	-0 08
300	-3 19	-1 84	-507.40	881.77	-0.09
330	-1 84	-3.19	-879.72	509 50	-0.08

#### Discrete Appurtenance Pressures - Service $G_H = 0.850$

Description	Aiming Azimuth	Weight	Offset,	Offset:	z .	K.	q.	C <sub>1</sub> A <sub>1</sub> Front	C <sub>A</sub> A <sub>U</sub> Side
	0	K	ft	ft	fi		psf	fi <sup>2</sup>	ſť
5/8" x 10' lightning rod	240.0000	0.02	-2 50	1.44	295 00	1 589	12	0.63	0 63
Beacon	120.0000	0 07	2.50	1.44	291 00	1 585	12	2 40	2.40
OB light	0 0000	0 03	0 00	-9.53	146 00	1 371	-11	0.50	0.50
OB light	120 0000	0.03	8 25	4.76	146 00	1 371	111	0 50	0.50
OB light	240 0000	0 03	-8 25	4 76	146 00	1 371	111	0.50	0.50
40,000 sq in (277 8 sq ft EPA)	0 0000	4 50	0.00	0.00	285 00	1 578	12	277 80	277.80
30,000 sq in (208 3 sq ft EPA)	0 0000	4 10	0.00	0.00	275 00	1 566	12	208 30	208.30
30,000 sq.in (208 3 sq.ft EPA)	0 0000	4.10	0.00	0.00	265 00	1 554	12	208 30	208.30
SPI R5 (Includes 4.5"x72" Pipe)	240 0000	0 14	-3 93	2.27	240.00	1 522	12	2 85	3.15
2-1/2" x 7' Sch 40	60 0000	0.04	1 75	-1 01	240.00	1 522	12	2.01	2.01
2-1/2" x 7' Sch 40	180 0000 Sum Weight:	0.04 13.10	0 00	2 02	240 00	1 522	12	2.01	201

## Discrete Appurtenance Vectors - Service

		5.8"	x 10' lightning rod	Flevation 295 Fra	ım Leg C		
Wind Azımuth	Fa	F,	Ve	ν.	OTM <sub>e</sub>	OTM;	Torque
0	K	K	K	K	kip-ft	kip-fi	kip-ft
0	0.00	0.01	0.00	-0.01	-1.92	0.06	-0.0
30	0.01	0.00	0.00	-0 01	-1 66	-0 92	-00
60	0.01	0.00	0.01	-0 00	-0.94	-1 63	0.00
90	0.01	0.00	0.01	0 00	0.03	-1.89	0.0
120	0 00	0.01	0.01	0 00	1.01	-1.63	0.03
150	0.00	0.01	0.00	0.01	1.72	-0 92	0.02
180	0 00	0.01	0.00	0.01	1.98	0.06	0.02
210	0.01	0.00	-0 00	0.01	1 72	1 03	0.01
240	0.01	0.00	-0.01	0.00	101	1 75	0.00
270	0.01	0.00	-0.01	0.00	0.03	2.01	-0.0
300	0.00	0.01	-0 01	-0.00	-0 94	1 75	-0.02
330	0.00	0.01	-0.00	-0.01	-1.66	1 03	-0 02

Beacon - Elevation 291 - From Leg E

Valmont	doL	565090	Page 49 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Wind Azimuth	Fu	F,	$\nu_*$	$V_s$	OTM.	OTM <sub>z</sub>	Torque
0	K	K	K	K	kip-fi	kip-ft	kip-ft
0	0.01	0.02	0.00	-0 03	-7.26	-0 18	0.00
30	0.00	0.03	0.01	-0 02	-6.28	-3.87	0.0
60	0.01	0.02	0.02	-0.01	-3.58	-6.56	0.00
90	0 02	0.01	0.03	0 00	0.11	-7 55	0.04
120	0.03	0.00	0.02	0.01	3.79	-6 56	0.00
150	0.02	0.01	0.01	0.02	6.49	-3.87	-0.0
180	001	0 02	0.00	0 03	7 48	-0.18	-0.00
210	0.00	0 03	-0.01	0 02	6.49	3 50	-0.0
240	0.01	0.02	-0.02	0.01	3 79	6.20	-0.0
270	0 02	0.01	-0.03	0.00	0.11	719	-0.0
300	0 03	0.00	-0.02	-0.01	-3 58	6 20	0.0
330	0.02	0.01	-0.01	-0.02	-6.28	3.50	0.0

			OB light - Elevat	ion 146 - From Leg.	Ä		
Wind Azimuth	Fa	F,	V	V2	OTM,	OTM:	Torque
0	K	K	A.	K	kip-ft	kip-fi	kip-ft
0	0 00	0.00	0.00	-0.00	-0 95	0.00	0.00
30	0 00	0.00	0.00	-0.00	-0 86	-0 33	-0 02
60	0.00	0.00	0.00	-0.00	-0.62	-0.58	-0 04
90	0.00	0.00	0.00	0.00	-0 29	-0 67	-0 04
120	0.00	0.00	0.00	0.00	0.05	-0 58	-0 04
150	0.00	0.00	0.00	0.00	0 29	-0.33	-0 02
180	0.00	0.00	0.00	0.00	0.38	0.00	0.00
210	0.00	0.00	-0 00	0.00	0.29	0 33	0.02
240	0.00	0.00	-0 00	0.00	0.05	0.58	0.04
270	0.00	0.00	-0.00	0.00	-0 29	0.67	0.04
300	0.00	0.00	-0 00	-0.00	-0 62	0.58	0.04
330	0.00	0.00	-0.00	-0.00	-0.86	0.33	0.02

			OB light - Elevat	ion 146 - From Leg	В		
Wind Azimuth	Fa	F,	$V_x$	V.	OTM <sub>x</sub>	OTM;	Torque
6	K	K	K	K	kip-fi	kip-ft	kip-ft
0	0.00	0.00	0.00	-0.00	-0.52	-0 25	0.0
30	0.00	0.00	0.00	-0.00	-0.43	-0.58	0.0
60	0.00	0.00	0.00	-0 00	-0.19	-0 82	0.0
90	0.00	0.00	0.00	0.00	0.14	-0 91	0.0
120	0.00	0.00	0.00	0.00	0.48	-0 82	0.0
150	0.00	0.00	0.00	0 00	0.72	-0.58	-00
180	0.00	0.00	0.00	0 00	0.81	-0.25	-0 0
210	0.00	0.00	-0.00	0.00	0.72	0.09	-00
240	0.00	0.00	-0.00	0 00	0.48	0 33	-00
270	0.00	0.00	-0.00	0.00	0.14	0.42	-00
300	0.00	0.00	-0.00	-0.00	-0.19	0.33	0.0
330	0.00	0.00	-0.00	-0.00	-0 43	0 09	0.0

Wind Azimuth	F <sub>a</sub>	F,	V <sub>x</sub> K	V:	OTM <sub>s</sub>	OTM: kip-fi	Torque kıp-fi
0	0.00	0.00	0 00	-0 00	-0.52	0.25	-00
30	0.00	0.00	0.00	-0.00	-0 43	-0 09	-0.0
60	0.00	0.00	0.00	-0.00	-0 19	-0 33	0.0
90	0 00	0.00	0.00	0.00	0.14	-0.42	0.03

Valmont	Јо <b>ь</b> 565090	Page 50 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX. (574)-936-6458	Client VB BTS II, LLC	Designed by JL

			()B light - Eleval	ion 146 - From Leg	(		
Wind Azimuth	Fa	F,	$V_{\tau}$	$V_{\tau}$	OTM,	OTM:	Torque
0	K	K	K	K	kip-ft	kip-ft	kip-fi
120	0.00	0.00	0.00	0.00	0.48	-0 33	0 04
150	0 00	0.00	0.00	0.00	0.72	-0 09	0.04
180	0 00	0.00	0.00	0.00	0.81	0.25	0 04
210	0.00	0 00	-0 00	0 00	0.72	0.58	0 02
240	0.00	0.00	-0.00	0.00	0 48	0 82	0 00
270	0 00	0 00	-0.00	0 00	0 14	0 91	-0 02
300	0.00	0 00	-0.00	-0 00	-0 19	0 82	-0.04
330	0.00	0.00	-0.00	-0 00	-0 43	0.58	-0 04

Wind Azimuth	Fu	F,	$V_{\mathfrak{r}}$	V.	OTM <sub>t</sub>	OTM:	Torque
o o	K	K	K	K	kip-fi	kip-fi	kip-ft
0	2.92	0.00	0.00	-2.92	-831 80	0.00	0.0
30	2.92	0.00	1.46	-2 53	-720 36	-415 90	0.0
60	2.92	0.00	2.53	-1.46	-415 90	-720 36	0.0
90	2.92	0.00	2.92	0.00	0.00	-831 80	0.0
120	2 92	0.00	2 53	1.46	415 90	-720 36	0.0
150	2.92	0.00	1 46	2 53	720 36	-415 90	0.0
180	2.92	0.00	0.00	2 92	831.80	0 00	0.0
210	2 92	0.00	-1 46	2.53	720.36	415.90	0.0
240	2 92	0.00	-2 53	1 46	415.90	720 36	0.0
270	2.92	0.00	-2.92	0.00	0.00	831 80	0.0
300	2 92	0.00	-2 53	-1.46	-415.90	720.36	0.0
330	2 92	0.00	-1 46	-2 53	-720 36	415.90	0.0

Wind Azimuth	Fu	F.	Ve	V <sub>z</sub>	OTM <sub>x</sub>	OTM:	Torque
0	K	K	K	K	kip-fi	kip-ft	kip-fi
0	1.78	0.00	0.00	-1.78	-489.79	0.00	0.00
30	1.78	0.00	0.89	-1.54	-424.17	-244 90	0.00
60	1.78	0.00	1 54	-0 89	-244 90	-424 17	0.00
90	1.78	0 00	1 78	0.00	0.00	-489 79	0.00
120	1 78	0.00	1.54	0.89	244 90	-424.17	0.00
150	1.78	0.00	0.89	1 54	424 17	-244 90	0.00
180	1 78	0.00	0.00	1 78	489 79	0.00	0.00
210	1.78	0.00	-0 89	1.54	424 17	244 90	0.00
240	1 78	0.00	-1.54	0.89	244.90	424 17	0.00
270	1.78	0 00	-1 78	0 00	0.00	489 79	0.00
300	1.78	0 00	-1.54	-0.89	-244 90	424 17	0.00
330	1.78	0.00	-0 89	-1.54	-424 17	244 90	0.00

		30,00	0 sq.m. (208.3 sq.ft.	EPA) - Elevanon 26	5 - None B		
Wind	$F_{\mu}$	F,	V <sub>k</sub>	V.,	OTM.	OTM:	Torque
Azimuth	K	K	K	K	kip-fi	kip-ft	kip-ft
0	1.77	0.00	0.00	-1 77	-468.31	0.00	0.0
30	177	0.00	0.88	-1 53	-405.57	-234 16	0.0
60	177	0.00	1.53	-0 88	-234.16	-405 57	0.
90	1 77	0.00	1.77	0.00	0.00	-468.31	0.
120	1 77	0.00	1.53	0 88	234 16	-405 57	0.
150	177	0.00	0.88	1.53	405 57	-234 16	0.
180	1 77	0.00	0.00	1.77	468 31	0.00	0.

Valmont	Job	565090	Page 51 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Wind Azimuth	F <sub>a</sub> K	F, K	V <sub>x</sub>	V <sub>2</sub>	OTM <sub>x</sub>	OTM: kip-ft	Torque kip-fi
210	1.77	0.00	-0.88	1.53	405 57	234 16	0.00
240	1.77	0.00	-1.53	0.88	234 16	405.57	0.00
270	1.77	0.00	-1.77	0.00	0.00	468.31	0.00
300	1 77	0 00	-1 53	-0.88	-234 16	405 57	0.00
330	1.77	0.00	-0.88	~1.53	-405 57	234.16	0.00

Wind	E I	7	Includes 4.5"x72" P	1	OTM,	OTM	Towner
Wind	Fa	$F_s$	$\nu_{\star}$	1/2	O'I M's	OTM:	Torque
Azimuth	K	К	K	K	kip-ft	kip-ft	kip-fi
0	0.01	0 03	-0.00	-0.03	-7.17	0.85	-0.13
30	0.03	0 02	0.01	-0.03	-6 01	-2.74	-0.07
60	0.03	0 00	0.03	-0.01	-3 15	-5 46	0.00
90	0.03	0.02	0.03	0.00	0.63	-6.57	0.07
120	0.01	0.03	0.03	0.02	4 32	-5 78	0 13
150	0.00	0.03	0.02	0.03	6.95	-3.29	0 14
180	0.01	0.03	0.00	0.03	7 79	0.22	0 13
210	0.03	0.02	-0.01	0.03	6 6 3	3.82	0 07
240	0.03	0.00	-0 03	0.01	3.78	6.54	0 00
270	0.03	0.02	-0.03	-0.00	-0 00	7 65	-0.07
300	0.01	0.03	-0 03	-0.02	-3.70	6.86	-0 13
330	0.00	0.03	-0 02	-0.03	-6.32	4.37	-0 14

		2-	1 2"x 7' Sch. 40 - FI	evation 240 - From	Face B		
Wind Azimuth	F <sub>u</sub>	F. F.	Ve	V <sub>=</sub>	OTM <sub>e</sub>	OTM:	Torque kıp-fi
0	K	K	K	K	kip-fi	kip-ft	
0	0.01	0.02	0.00	-0.02	-4.94	-0 07	0 04
30	0 02	0.01	0.01	-0 02	-4.28	-2 52	0.02
60	0 02	0.00	0.02	-0.01	-2.49	-4.31	0.00
90	0 02	0.01	0.02	0.00	-0.04	-4 97	-0.02
120	0.01	0.02	0.02	0.01	2.41	-4.31	-0.04
150	0 00	0 02	0.01	0.02	4 20	-2.52	-0.04
180	0.01	0.02	0.00	0.02	4.85	-0 07	-0.04
210	0.02	0.01	-0.01	0.02	4.20	2 38	-0.02
240	0.02	0.00	-0.02	0.01	2.41	4.17	0.00
270	0.02	0.01	-0.02	0.00	-0.04	4 82	0.02
300	0.01	0.02	-0.02	-0.01	-2.49	4.17	0.04
330	0.00	0.02	-0.01	-0.02	-4.28	2.38	0.04

		2-	1 2" x 7' Sch. 40 - El	evation 240 - From .	Face C			
Wind Azimuth	Fa	F,	V.	$\nu_{\cdot}$	OTM <sub>x</sub>	OTM:	Torque	
0	K	K	K	K	kip-fi	kip-ft	kip-ft	
0	0.02	0.00	0.00	-0.02	-4 81	0.00	0.00	
30	0.02	0.01	0.01	-0 02	-4 16	-2 45	0.02	
60	0.01	0.02	0.02	-0.01	-2 37	-4.24	0.04	
90	0.00	0.02	0.02	0.00	0 08	-4.89	0.04	
120	0.01	0.02	0.02	0.01	2 53	-4 24	0.04	
150	0 02	0.01	0.01	0.02	4.32	-2.45	0.02	
180	0.02	0.00	0.00	0.02	4.98	0.00	0.00	
210	0.02	0.01	-0.01	0 02	4 32	2 45	-0.02	
240	0.01	0.02	-0.02	0.01	2.53	4.24	-0 04	
270	0 00	0.02	-0.02	0 00	0.08	4.89	-0 04	

Valmont	Јов 565090	Page 52 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX (574)-936-6458	Client VB BTS II, LLC	Designed by JL

		2-	1 2" x 7" Sch. 40 - Ei	levation 240 - From	Face C		
Wind Azimuth	F <sub>u</sub>	F.	V.	V <sub>2</sub>	OTM,	OTM.	Torque
0	K	K	K	K	kip-ft	kip-ft	kip-fi
300	0.01	0.02	-0.02	-0 01	-2 37	4 24	-0 04
330	0.02	0.01	-0.01	-0.02	-4.16	2.45	-0.02

## Discrete Appurtenance Totals - Service

Wind Azimuth	$V_{\tau}$	V <sub>2</sub>	OTM <sub>e</sub>	OTM <sub>2</sub>	Torque
0	K	K	kip-fl	kip-fi	kip-ft
0	-0.00	-6 58	-1818 00	0 66	-0 04
30	3.29	-5.70	-1574 21	-908 44	0 03
60	5.70	-3.29	-908 48	-1574 04	0.10
90	6.58	0.00	0.81	-1817.78	0 14
120	5.70	3 29	910.01	-1574.35	0.14
150	3.29	5.70	1575.50	-908.99	0.11
180	0.00	6.58	1818 98	0.03	0.04
210	-3.29	5 70	1575 19	909 13	-0 03
240	-5 70	3 29	909.46	1574.72	-0 10
270	-6.58	-0.00	0 17	1818.46	-0.14
300	-5 70	-3 29	-909 03	1575 04	-0 14
330	-3 29	-5.70	-1574 52	909 68	-0 1

#### Dish Pressures - No Ice

Elevation	Dish	Aiming	Weight	Offset,	Offset:	К.	A,i	q <sub>T</sub>
ft	Description	Azimuth	K	ft	ft		fi <sup>2</sup>	psf
240.00	6' НР	240 0000 Sum Weight	0 30 0 30	-4.37	2 52	1.522	28 27	37

### Dish Vectors - No Ice

	- 0			6' HP -	Elevation 2	40 - From Le	g C		lan a		
Wind Azimuth	Cr	('3	Cs Cu	F <sub>4</sub>	Fs	Fu	Vt	T <sub>2</sub>	OTM.	OTM: kip-ft	Inrque kip fi
					K	kip-fi	K	K	kip-fi		
0	0.002420	-0.000940	0.000022	0.85	-0.33	0.05	0.57	-0.71	-168.90	-134.96	-161
30	0.003100	-0.000600	0 000133	1 08	-0.21	0.28	0.83	-0 72	-172.71	-198 57	-0.78
60	0.003230	0.000000	0.000000	1.13	0.00	0.00	0 98	-0.56	-134 61	-233 15	0.00
90	0.003100	0.000600	-0.000133	1 08	0.21	-0.28	1 04	-0 36	-85.61	-248.86	0.78
120	0.002420	0.000940	-0.000022	0 85	0.33	-0.05	0 90	-0 14	-32 43	-213 75	1.61
150	-0.000280	0.001600	0 000251	-0 10	0.56	0.53	0 19	0.53	128.63	-45 42	3.34
180	-0.001820	0.001120	0.000266	-0.64	0.39	0.56	-0 35	0.66	158 33	86.48	2.53
210	-0.002450	0.000450	0.000158	-0.86	0.16	0.33	-0.66	0.56	136 10	160.29	1 12
240	-0.002600	0.000000	0.000000	-0.91	0.00	0.00	-0.79	0.45	109.72	190.04	0.00
270	-0.002450	-0,000450	-0 000158	-0 86	-0.16	-0.33	-0 82	0.29	70.77	198.01	-1.12
300	-0.001820	-0.001120	-0.000266	-0.64	-0.39	-0.56	-0.75	-0.02	-4.27	180.36	-2.53
330	-0.000280	-0.001600	-0.000251	-0.10	-0.56	-0.53	-0.36	-0.44	-103,65	88.69	-3.34

Valmont	Job 565090	Page 53 of 72
1545 Pidco Dr	Project H-31 x290' SST - US-KY-5135 F	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

### Dish Totals - No Ice

Wind Azimuth	$V_{\rm v}$	ν,	OTM,	OTM,	Torque	
•	K	K	kip-ft	kip-ft	kip-fi	
0	0.57	-0.71	-168 90	-134 96	-1.61	
30	0.83	-0.72	-172 71	-198 57	-0.78	
60	0 98	-0.56	-134.61	-233 15	0.00	
90	1.04	-0.36	-85 61	-248 86	0.78	
120	0.90	-0.14	-32 43	-213 75	1.61	
150	0.19	0.53	128 63	-45.42	3.34	
180	-0 35	0 66	158 33	86.48	2.53	
210	-0.66	0.56	136 10	160.29	1.12	
240	-0.79	0.45	109 72	190.04	0.00	
270	-0.82	0.29	70.77	198.01	-1:12	
300	-0 75	-0.02	-4 27	180.36	-2.53	
330	-0.36	-0 44	-103 65	88 69	-3.34	

#### Dish Pressures - With Ice

Elevation	Dish	Aiming	Weight	Offset.	Offset:	K <sub>∓</sub>	A <sub>A</sub>	q:	t <sub>s</sub>
ft	Description	Azimuth	K	ft	ft		ft <sup>2</sup>	psf	in
240.00 6' I	{P	240 0000 Sum Weight:	1.37 1.37	-4.37	2.52	1.522	32.13	3	2 4389

#### Dish Vectors - With Ice

				6' HP -	Elevation 2	10 - From Les	3 C				
Wind Azimuth	Ca	Cs	Cu	$F_{\mathcal{A}}$	Fs	FM	ν,	V <sub>z</sub>	OTM:	OTM <sub>2</sub>	Torque
0				K	K	kip-fi	K	K	kip-fi	kip-fi	kip-ft
0	0.002420	-0.000940	0.000022	0.08	-0.03	0.00	0.05	-0.06	-11.98	-6.41	-0.15
30	0.003100	-0.000600	0.000133	0.10	-0.02	0.03	0.08	-0.07	-12.33	-12 20	-0.07
60	0.003230	0.000000	0.000000	0.10	0.00	0.00	0.09	-0.05	-8.86	-15 35	0.00
90	0.003100	0.000600	-0.000133	0.10	0.02	-0.03	0.09	-0.03	-4.40	-16.78	0.03
120	0.002420	0.000940	-0.000022	0.08	0.03	-0.00	0.08	-0.01	0.44	-13.59	0.15
150	-0.000280	0.001600	0.000251	-0.01	0.05	0.05	0.02	0.05	15 10	1.74	0.30
180	-0.001820	0.001120	0.000266	-0.06	0.04	0.05	-0.03	0.06	17 81	13.75	0.23
210	-0.002450	0.000450	0.000158	-0.08	0.01	0.03	-0.06	0.05	15 78	20.47	0.10
240	-0.002600	0.000000	0.000000	-0.08	0.00	0.00	-0.07	0.04	13 38	23.18	0.00
270	-0 002450	-0.000450	-0.000158	-0.08	-0.01	-0.03	-0.07	0.03	9.84	23.90	-0.10
300	-0.001820	-0.001120	-0.000266	-0.06	-0.04	-0.05	-0.07	-0.00	3.00	22.30	-0.23
330	-0.000280	-0.001600	-0.000251	-0.01	-0.05	-0.05	-0.03	-0.04	-6.04	13.95	-0.30

#### Dish Totals - With Ice

Wind Azimuth	V <sub>r</sub>	V <sub>a</sub> K	OTM <sub>v</sub>	OTM <sub>2</sub>	Torque kip-ft
0	0.05	-0.06	-11.98	-6 41	-0.15
30	0.08	-0.07	-12 33	-12.20	-0.07

Valmont	Job	565090	Page 54 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Wind Azimuth	$V_{\epsilon}$	$V_{z}$	OTM,	OTM <sub>e</sub>	Torque	
0	K	K	kıp-ft	kıp-fi	kip-ft	
60	0.09	-0.05	-8 86	-15.35	0.00	
90	0 09	-0 03	-4 40	-16 78	0 07	
120	0.08	-0.01	0.44	-13 59	0 15	
150	0.02	0.05	15.10	1.74	0.30	
180	-0.03	0 06	17.81	13.75	0.23	
210	-0 06	0.05	15.78	20.47	0.10	
240	-0.07	0.04	13 38	23 18	0.00	
270	-0 07	0.03	9 84	23.90	-0.10	
300	-0 07	-0.00	3 00	22 30	-0 23	
330	-0.03	-0.04	-6.04	13.95	-0.30	

#### Dish Pressures - Service

Elevation	Dish	Ainung	Weight	Offset,	Offset-	Κ.	A <sub>1</sub>	q:
ft	Description	Azımuth	K	fi	ft		ft <sup>2</sup>	psf
240 00	6' HP	240 0000 Sum Weight	0 30 0 30	-4.37	2.52	1 522	28.27	12

### Dish Vectors - Service

				6' HP -	Elevation 2	40 - From Ley	g('				
Wind Azimuth	Cit	Cs	Csy	$F_A$	Fs	$F_M$	ν.	Va	OTM,	OTM.	Torque
٥				K	K	kip-ji	K 1	K	kip-fi	kip-fi	kip-ft
0	0.002420	-0.000940	0.000022	0.27	-0.11	0.01	0.18	-0.23	-53.60	-42 35	-0.52
30	0.003100	-0.000600	0.000133	0.35	-0.07	0.09	0.27	-0.23	-54 82	-62 73	-0.25
60	0.003230	0.000000	0.000000	0.36	0.00	0.00	0.31	-0 18	42 61	-73.81	0.00
90	0.003100	0.000600	-0.000133	0.35	0.07	-0.09	0.33	-0.12	-26.91	-78.84	0.25
120	0.002420	0.000940	-0.000022	0.27	0.11	-0.01	0.29	-0.04	-9.88	-67 59	0.52
150	-0.000280	0.001600	0.000251	-0.03	0.18	0.17	0.06	0.17	41 73	-13.66	1.07
180	-0.001820	0.001120	0.000266	-0.20	0.13	0.18	-0.11	0.21	51 24	28 60	0.81
210	-0.002450	0.000450	0.000158	-0.27	0.05	0.11	-0.21	0 18	44.12	52 25	0.36
240	-0 002600	0.000000	0.000000	-0.29	0.00	0.00	-0.25	0.15	35 67	61.78	0.00
270	-0.002450	-0.000450	-0.000158	-0.27	-0.05	-0.11	-0.26	0.09	23 19	64 33	-0.36
300	-0.001820	-0.001120	-0.000266	-0.20	-0.13	-0.18	-0.24	-0.01	-0.85	58 68	-0.81
330	-0.000280	-0,001600	-0.000251	-0.03	-0.18	-0.17	-0.12	-0 14	-32.70	29.31	-1.07

### Dish Totals - Service

Wind Azimuth	$V_{\epsilon}$	V <sub>z</sub>	OTM,	OTM <sub>z</sub>	Torque
0	K	K	kip-ft	kip-ft	kip-ft
0	0.18	-0.23	-53.60	-42.35	-0.52
30	0.27	-0.23	-54.82	-62.73	-0.25
60	0.31	-0.18	-42.61	-73.81	0.00
90	0.33	-0.12	-26.91	-78.84	0.25
120	0.29	-0.04	-9.88	-67.59	0.52
150	0.06	0.17	41.73	-13.66	1.07
180	-0.11	0.21	51.24	28 60	0.81
210	-0.21	0.18	44.12	52.25	0.36

Valmont	Job	565090	Page 55 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Wind Azimuth	V <sub>x</sub> K	V: K	OTM, kip-fi	OTM <sub>s</sub> kip-ft	Torque kip-st
240	-0.25	0.15	35.67	61 78	0.00
270	-0.26	0.09	23.19	64 33	-0.36
300	-0.24	-0.01	-0.85	58 68	-0.81
330	-0.12	-0.14	-32 70	29 31	-1.07

### Force Totals

Load Case	Vertical Forces	Sum of Forces X	Sum of Forces Z	Sum of Overturning Moments, M <sub>x</sub>	Sum of Overturning Moments, M <sub>2</sub>	Sum of Torques
	K	K	K	kip-ft	kip-fi	kip-ft
Leg Weight	35.35					
Bracing Weight	16.33			0.72	12.50	
Total Member Self-Weight	51.68	- 535		9.72 9.72	13.59 13.59	
Total Weight	75.02	0.56	-70.72	-12810.45	-121.70	
Wind 0 deg - No Ice		34.77	-59.52	-12810.45	-6364 50	
Wind 30 deg - No Ice		58.98	-34.05	-6230.83	-10795 36	0.50
Wind 60 deg - No Ice Wind 90 deg - No Ice	A	68.58	-0.36	-75.66	-10793 38	
Wind 120 deg - No Ice	1	61 23	34.70	6254 98	-11074.09	
		33 90	58.91	10779.44	-6177.21	
Wind 150 deg - No Ice	3	-0.35	67.72		97.77	1 VET VIET
Wind 180 deg - No Ice		-34 60		12420 74		1,000,000
Wind 210 deg - No Ice		-61.34	59.36	10848.02 6422.41	6350 78	
Wind 240 deg - No Ice		-68.36	35.42 0.29	78 75	11120.69 12473.09	
Wind 270 deg - No Ice			E 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	-6075 21		
Wind 300 deg - No Ice		-58.52 -34.07	-33.38		10721 37 6245 04	
Wind 330 deg - No Ice	114.85	-34.07	-58.81	-10736 53	6243.04	-45.06
Member Ice				42.17	88.70	
Total Weight Ice	266.31	0.05	11.40	43 17		
Wind 0 deg - Ice	10	0.05	-11 40 -9 79	-2086 69	76.41 -978.81	-1 72
Wind 30 deg - Ice		5.69		-1790 32		
Wind 60 deg - Ice		9.78	-5.65	-1014.63	-1743.45	
Wind 90 deg - Ice		11.30	-0.03	35 43	-2026.63	
Wind 120 deg - Ice		9.88	5 64	1094 33	-1756.53	
Wind 150 deg - Ice		5.60	9.72	1866 24	-961.38	
Wind 180 deg - Ice		-0.03	11 21	2147 56	96.34	
Wind 210 deg - Ice		-5.68	9 78	1873 19	1152 49	
Wind 240 deg - Ice	1	-9.93	5.73	1110.75	1937.81	-1.13
Wind 270 deg - Ice		-11.28	0.03	49 43	2199.17	
Wind 300 deg - Ice		-9.70	-5 56	-999 27	1909.54	
Wind 330 deg - Ice	77.00	-5.62	-9.71	-1777.76	1142.48	
Total Weight	75.02	0.10	22.50	9.72	13.59	
Wind 0 deg - Service		0 18	-22 79	-4138 92	-41.69	[ (300)101)
Wind 30 deg - Service		11.21	-19 18	-3511 78	-2058.18	
Wind 60 deg - Service		19 01	-10.98	-2014 52	-3489.75	
Wind 90 deg - Service		22 11	-0 11	-26 11	-4040.09	
Wind 120 deg - Service		19 73	11 18	2018 52	-3579.06	
Wind 150 deg - Service		10.93	18 99	3480 08	-1998.17	
Wind 180 deg - Service		-0 11	21 83	4010 32	28.63	
Wind 210 deg - Service	V	-11.15	19 13	3502.05	2048.38	
Wind 240 deg - Service		-19.77	11.41	2072.16	3588.58	
Wind 270 deg - Service		-22 03	0 09	23 36	4026.26	1.000
Wind 300 deg - Service	1	-18.86	-10.76	-1964.66	3460.64	
Wind 330 deg - Service		-10.98	-18.96	-3470.07	2014.50	-14.44

Valmont	Job	565090	Page 56 of 72
1545 Pidco Dr	Project  -	I-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

# **Load Combinations**

Comb	Description	
No.	D:-10-1-	
1 2	Dead Only	
2	1.2 Dead+1.0 Wind 0 deg - No Ice	
3	0 9 Dead+1 0 Wind 0 deg - No Ice	
4	1.2 Dead+1.0 Wind 30 deg - No Ice	
	0 9 Dead+1 0 Wind 30 deg - No Ice	
6	1.2 Dead+1.0 Wind 60 deg - No Ice	
	0 9 Dead+1 0 Wind 60 deg - No Ice	
8	1.2 Dead+1.0 Wind 90 deg - No Ice 0.9 Dead+1.0 Wind 90 deg - No Ice	
10	1.2 Dead+1.0 Wind 120 deg - No Ice	
11	0 9 Dead+1 0 Wind 120 deg - No Ice	
12	1 2 Dead+1 0 Wind 150 deg - No ice	
13	0.9 Dead+1.0 Wind 150 deg - No Ice	
14	12 Dead+1 0 Wind 180 deg - No Ice	
15	0 9 Dead+1 0 Wind 180 deg - No Ice	
16	1 2 Dead+1 0 Wind 210 deg - No Ice	
17	0.9 Dead+1.0 Wind 210 deg = No Ice	
18	1.2 Dead+1.0 Wind 240 deg - No Ice	
19	0 9 Dead+1 0 Wind 240 deg - No Ice	
20	1 2 Dead+1 0 Wind 270 deg - No Ice	
21	0.9 Dead+1 0 Wind 270 deg - No Ice	
22	1 2 Dead+1 0 Wind 300 deg - No Ice	
23	0 9 Dead+1 0 Wind 300 deg - No Ice	
24	1 2 Dead+1 0 Wind 330 deg - No Ice	
25	0.9 Dead+1.0 Wind 330 deg - No Ice	
26	1.2 Dead+1 0 Ice+1.0 Temp	
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	
28	1.2 Dead+1 0 Wind 30 deg+1 0 Ice+1 0 Temp	
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	
31	1 2 Dead+1 0 Wind 120 deg+1 0 Ice+1 0 Temp	
32	1 2 Dead+1 0 Wind 150 deg+1.0 Ice+1 0 Temp	
33	1 2 Dead+1 0 Wind 180 deg+1 0 Ice+1 0 Temp	
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	
35	1.2 Dead+1.0 Wind 240 deg+1.0 lce+1.0 Temp	
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	
37	1 2 Dead+1 0 Wind 300 deg+1 0 Ice+1 0 Temp	
38	1 2 Dead+1 0 Wind 330 deg+1 0 Ice+1 0 Temp	
39	Dead+Wind 0 deg - Service	
40	Dead+Wind 30 deg - Service	
41	Dead+Wind 60 deg - Service	
42	Dead+Wind 90 deg - Service	
43	Dead+Wind 120 deg - Service	
44	Dead+Wind 150 deg - Service	
45	Dead+Wind 180 deg - Service	
46	Dead+Wind 210 deg - Service	
47	Dead+Wind 240 deg - Service	
48	Dead+Wind 270 deg - Service	
49	Dead+Wind 300 deg - Service	
50	Dead+Wind 330 deg - Service	

Valmont	Јо <b>ь</b> 565090	Page 57 of 72
1545 Pideo Dr	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB BTS II, LLC	Designed by JL

lection No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axıal K	Major Axis Moment kip-fi	Minor Axis Moment kip-fi
TI	290 - 280	Leg	Max Tension	15	5.07	0.01	-0.31
	230 - 200	Leg	Max Compression	18	-7.59	-0.28	0 17
			Max. Mx	20	-2 00	-0.56	0.02
					-7 51	-0 02	0.58
			Max. My	2			
			Max Vy	20	-3 05	0.26	-0 12
			Max. Vx	2	-3.07	-0.01	0.34
		Diagonal	Max Tension	20	4 42	0.00	0 00
			Max Compression	8	-5.00	0.00	0 00
			Max Mx	14	-0 49	-0.05	0.00
			Max My	20	-4.99	-0.01	0.02
			Max Vy	38	-0 03	0.02	0 00
			Max Vx	20	0.01	0.00	0.00
		Top Gut	Max Tension	22	1.13	0.00	0.00
		2,00	Max Compression	18	-1.34	0.00	0.00
			Max Mx	26	0.07	-0.08	0 00
			Max My	12	-0.23	0.00	0.00
			Max Vy	26	0 07	0 00	0 00
			Max Vx	12	-0.00	0 00	0.00
-	280 - 260	1.44	Max Tension	15	69 91	-0 02	0 60
12	200 - 200	Leg					
			Max Compression	2	-78.36	-0 11	2.53
			Max Mx	8	32 64	-2 50	0 23
			Max My	14	39.07	0 04	-2.62
			Max Vy	20	-1.90	1 92	-0 65
			Max Vx	2	-2.00	-0.03	2.31
		Diagonal	Max Tension	20	1231	0.12	-0.01
			Max Compression	8	-12 84	0.00	0.00
			Max. Mx	17	-5 10	-0.14	-0.01
			Max My	20	-9.54	-0.06	0.05
			Max Vy	35	-0 05	0.08	-0.00
			Max Vx	20	0.01	0.00	0.00
3	260 - 240	Leg	Max Tension	15	136 21	-0.18	-0.02
3	200 - 240	LEE		2	-148.35	2.47	-0.04
			Max Compression				
			Max Mx	2	-107.69	3.52	0.04
			Max My	12	-6 30	-0.00	-2 29
			Max Vv	2	-0 80	3.52	0.04
			Max Vx	12	0.50	-0.00	-2.29
		Diagonal	Max Tension	8	7.60	0.04	-0 00
			Max Compression	20	-8.44	0.00	0.00
			Max Mx	17	-4 16	-0.07	0.00
			Max My	2	-8 26	-0.04	0.01
			Max Vv	35	-0.04	0.04	-0 00
			Max Vx	2	-0 00	0.00	0.00
14	240 - 220	Leg	Max Tension	15	181 29	-3 78	0.00
	東下の 事業が		Max. Compression	18	-197 25	-5 73	0 02
				18	-197 25	-5 73	0.02
			Max Mx Max My			-0 09	-2.87
			Max My	4	-7 27		
			Max Vy	18	1 54	4 18	0 02
		W-1112	Max Vx	12	-0 57	0.12	-0.85
		Diagonal	Max Tension	2	7.77	0.00	0.00
			Max Compression	14	-7 72	0.00	0.00
			Max. Mx	16	2.37	0.11	0.00
			Max My	14	-7.71	-0.01	-0.04
			Max. Vy	34	0.05	0.05	-0.01
			Max Vx	14	0.01	0.00	0.00
15	220 - 200	Leg	Max Tension	15	208 95	-5 89	0 17
1.2	240 - 200	24.0	Max Compression	18	-226 56	10.50	0 18
			Max Mx			13.67	0 15
				18	-210 72		
			Max My	12	-9 77	0.04	-11.58
			Max Vy	18	-2 31	13.67	0 15
		120000	Max Vx	12	1.70	0.04	-11 58
		Diagonal	Max Tension	14	6 82	0.00	0.00

Valmont	Job	565090	Page 58 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, 1N Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

Section No.	Elevation ft	Component Type	Condition	Gov. Load	Axial v	Major Axis Moment	Minor Ax Moment
				Comb.	K	kip-ft	kıp-fi
			Max. Compression	2	-7 89	0.00	0 00
			Max Mx	35	101	0 10	-0.01
			Max My	27	0 20	0.07	0 01
			Max Vy	33	0 06	0.09	0.01
-25	. C. C. 122.2	0.44	Max, Vx	35	0.00	0.00	0.00
T6	200 - 180	Leg	Max Tension	7	239.43	-4.90	0.08
			Max Compression	18	-261 49	9 22	0.11
			Max Mx	18	-245.76	10.62	0.12
			Max My	12	-1191	0 00	-7 82
			Max Vy	2	-0 90	10.62	0 30
			Max Vx	24	-0 42	0.00	7.82
		Diagonal	Max Tension	2	6.51	0 00	0.00
			Max Compression	2	-7 07	0 00	0.00
			Max Mx	35	0.79	0.12	0.02
			Max My	27	0.06	0.10	0.02
			Max Vy	33	0.08	0.12	0 02
			Max Vx	35	0.01	0 00	0 00
17	180 - 160	Leg	Max Tension	7	266 90	-4.68	0.05
			Max Compression	18	-292 70	6.93	0.06
			Max Mx	18	-277.47	9 07	0 10
			Max My	4	-14.22	-0 13	-8.11
			Max Vy	2	-0.61	9 06	0.25
			Max Vx	24	-0 42	-0.17	8 09
		Diagonal	Max Tension	2	6.57	0 00	0.00
			Max Compression	8	-7.06	0.00	0.00
			Max Mx	35	1.10	0.15	-0.02
			Max. My	27	0.12	0.13	0.02
			Max Vy	33	0.09	0.15	0 02
			Max. Vx	35	0.01	0.00	0.00
T8	160 - 140	Leg	Max Tension	7	293.09	-5.70	0.06
			Max Compression	18	-322 99	5.47	0 26
			Max Mx	18	-306 81	9 75	0 04
			Max My	12	-15.34	-0.20	-7.42
			Max Vy	2	-0 74	9 73	0.09
			Max Vx	24	-0 69	-0 20	7.42
		Diagonal	Max Tension	2	7.74	0.00	0 00
			Max Compression	2	-7 90	0.00	0 00
			Max Mx	33	0.89	0 19	-0 03
			Max My	14	-761	0.01	-0.03
			Max Vy	33	0.11	0 19	-0.03
			Max Vx	32	-0 01	0.00	0.00
79	140 - 120	Leg	Max Tension	7	308 42	0.42	0.04
	110 120	2.0	Max Compression	18	-339.90	13 72	0.30
			Max Mx	2	-339 19	13.72	0.67
			Max My	12	-16 80	011	-10.16
			Max Vy	3	-0.96	13.68	0.67
			Max Vx	24	-0 67	0.11	10.16
		Diagonal	Max Tension	15	10 28	0.00	0.00
		13 rafform	Max Compression	2	-11.77	0.00	0.00
						-0.45	0.08
			Max Mx	33	0.71		
			Max My	33	-1 97	-0 42	0.09
			Max Vy	33	-0.17	-0 45	-0.08
TOLY	100 100	F 400	Max Vx	38	-0.01	0.00	0.00
T10	120 - 100	Leg	Max Tension	7	334.87	0.84	0 16
			Max Compression	18	-371 60	12 31	031
			Max Mx	2	-370 78	12 32	0.67
			Max My	12	-19.39	-0 01	-8 18
			Max Vy	2	-0 93	12 32	0.67
		22000	Max Vx	24	-0 36	-0.01	8 18
		Diagonal	Max Tension	2	10 26	0.00	0 00
			Max Compression	14	-10.46	0.00	0.00

Valmont	Job	565090	Page 59 of 72
1545 Pidco Dr	Project H	-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Cilent	VB BTS II, LLC	Designed by

Section No.	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axi. Moment
				Comb	K	kip-fi	kip-fi
			Max. Mx	33	1.20	-0.53	-0 08
			Max My	27	-0 41	-0 52	-0.09
			Max Vy	33	-0 19	-0 53	-0 08
			Max Vx	35	-0.01	0.00	0.00
TH	100 - 80	Leg	Max Tension	7	354.96	-1.89	0.10
			Max Compression	18	-396 17	11.15	0.23
			Max. Mx	2	-395 26	11.15	0.52
			Max. My	12	-20.92	0.11	-7.48
			Max Vy	3	-0.71	11.11	0.52
			Max Vx	24	-0 53	0.11	7.48
		Diagonal	Max Tension	15	10.02	0.00	0.00
			Max Compression	2	-1149	0.00	0 00
			Max Mx	33	1 25	-0.59	-0.09
			Max. My	27	-0.07	-0 58	-0.09
			Max. Vy	33	-0.21	-0.59	-0.09
			Max Vx	27	-0 01	0.00	0 00
T12	80 - 60	Leg	Max Tension	7	377 92	-0.09	011
112	00 - 00	Lieb	Max Compression	18	-424.77	10.83	0 30
			Max Mx	19	-417.72	10.85	0.30
			Max My	12	-23 54	-0.16	-9 22
			Max Vy	6	0.79	-10.59	-0 29
			Max Vx	24	-0.53	-0.16	9 22
		Discount					0.00
		Diagonal	Max Tension	2	10 31	0.00	
			Max Compression	4	-10.51	0 00	0 00
			Max Mx	33	0.59	-0.69	-0 10
			Max My	27	-0.96	-0.68	-0.11
			Max Vy	33	-0.22	-0.69	-0 10
	80 30		Max Vx	27	-0 01	0 00	0 00
T13	60 - 40	Leg	Max Tension	7	397 27	-2.46	0.02
			Max Compression	18	-449 05	11.05	0.19
			Max Mx	18	-449 05	11.05	0 19
			Max My	4	-25.30	-0 36	-5 15
			Max Vy	3	-0 66	10.93	0.40
			Max Vx	12	-0 31	-0 36	-5.07
		Diagonal	Max Tension	15	10.60	0 00	0 00
			Max Compression	2	-12.21	0 00	0.00
			Max Mx	33	1 74	-091	-0 13
			Max My	27	0 37	-0.89	-0 13
			Max Vy	33	-0 29	-0.91	-0.13
			Max. Vx	27	-0 02	0 00	0.00
T14	40 - 20	Leg	Max Tension	7	418.66	-1 92	0.07
			Max Compression	18	-477 13	7.99	0 18
			Max Mx	6	412.04	-8.34	-0.19
			Max My	12	-28 72	-0.38	-12 09
			Max Vy	35	-0 57	-4 12	-0 01
			Max Vx	12	0.76	-0 38	-12 09
		Diagonal	Max Tension	2	11 29	0.00	0.00
		- mBorimi	Max Compression	14	-11 06	0.00	0.00
			Max Mx	37	-0.48	-1.06	-0.15
			Max My	38	-2 76	-1.03	-0.16
				37	-0 31	-1.06	-0.15
			Max Vy	38	-0.02	0.00	0 00
THE	20 - 0	1	Max Vx				
T15	20 - 11	l.eg	Max Tension	7	436.01	-2.39	0.04
			Max Compression	18	-499 88	4.39	0.11
			Max Mx	18	-499 88	4 39	0 11
			Max My	4	-30.80	-0 72	-8 80
			Max Vy	10	-0.32	4 38	0.03
			Max Vx	12	-0.60	-0 73	-8 63
		Diagonal	Max Tension	15	11.73	0.00	0 00
			Max Compression	2	-13.55	0 00	0.00
			Max Mx	36	2.40	-1 02	0 13

Valmont	Job	565090	Page 60 of 72	
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22	
Plymouth IN Phone: (574)-936-4221	Client	VB BTS II, LLC	Designed by	

Section No.	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
			Comb.	K	kip-ft	kip-fi	
			Max My	27	1.92	-1.02	-0.14
			Max Vy	36	-0 30	-1 02	0 13
			Max Vx	27	-0 02	0.00	0.00

#### **Maximum Reactions**

Location	Condition	Gov Load Comb	Vertical K	Horizonial, X K	Horizontal, 2 K
Leg C	Max Vert	18	513.42	40.83	-23 06
2.25	Max H <sub>x</sub>	18	513.42	40.83	-23 06
	Max H	7	-445 44	-35 68	20 08
	Min Vert	7	-445 44	-35.68	20 08
	Min H.	7	-445.44	-35 68	20 08
	Min H	18	513 42	40 83	-23 06
Leg B	Max Vert	10	508 57	-40.26	-23 41
	Max H	23	-440.03	34 97	20 38
	Max. H.	23	-440.03	34 97	20.38
	Min Vert	23	-440.03	34 97	20 38
	Min. H.	10	508 57	-40 26	-23 41
	Min H.	10	508 57	-40 26	-23 41
Leg A	Max Vert	2	512.04	-0 88	46.81
100	Max H,	21	19.74	2.89	1.45
	Max. H.	2	512 04	-0 88	46.81
	Min Vert	15	-443 71	0.89	-40.71
	Min He	9	25.54	-2.91	1 88
	Min H	15	-443 71	0.89	-40 71

### **Tower Mast Reaction Summary**

Load Combination	l'ertical	Shear,	Shear-	Overturning Moment Me	Overturning Moment, Mz	Torque
	K	K	K	kip-ft	kip-fi	kip-ft
Dead Only	75.02	0.00	-0.00	9.57	13.40	-0.00
1 2 Dead+1 0 Wind 0 deg - No	90.02	0.56	-70 72	-12941.06	-119.60	-46 94
Ice						
0 9 Dead+1 0 Wind 0 deg - No	67.52	0.56	-70 72	-12909.96	-123 39	-46 88
Ice						
1 2 Dead+1 0 Wind 30 deg - No	90 02	34.77	-59 51	-10975 57	-6432 11	7.47
Ice						
0 9 Dead+1 0 Wind 30 deg - No	67 52	34 77	-59.51	-10949 52	-6419 05	7.47
Ice						
1.2 Dead+1.0 Wind 60 deg - No	90.02	58 97	-34 05	-6293 54	-10905 56	23 95
lce						
0 9 Dead+1 0 Wind 60 deg - No	67.52	58.97	-34.05	-6279 78	-10880 70	23 94
Ice						
1.2 Dead+1 0 Wind 90 deg - No	90.02	68 58	-0.36	-78 75	-12626.65	-12 57
Ice						
0 9 Dead+1 0 Wind 90 deg - No	67 52	68 58	-0.36	-81 35	-12597 37	-12 60
Ice						
1 2 Dead+1.0 Wind 120 deg -	90.02	61 23	34 70	6321 63	-11185.76	-8.82
No Ice						
0.9 Dead+1.0 Wind 120 deg -	67.52	61 23	34 70	6302.08	-11160.49	-8.89

#### Valmont

1545 Pidco Dr.

Plymouth, IN Phone (574)-936-4221 FAX (574)-936-6458

Job	565090	Page 61 of 72
Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Client	VB BTS II, LLC	Designed by

Load Combination	l'ertical	Shear,	Shear,	Overturning Moment M.	Overturning Moment, M <sub>z</sub>	Torque
	K	K	K	kip-fi	kip-ft	kip-ft
No Ice I 2 Dead+1 0 Wind 150 deg -	90 02	33 90	58.90	10896 37	-6234 35	45.5
No Ice	90 02	32 70	36.70	10070 37	-0234 33	43,3
9 Dead+1 0 Wind 150 deg -	67.52	33 90	58.90	10864.47	-6222.09	45.4
No Ice						
1 2 Dead+1 0 Wind 180 deg -	90 02	-0 35	67.72	12552 56	102 22	47.8
No Ice 0 9 Dead+1 0 Wind 180 deg -	67.52	-0.35	67.72	12516 24	97.82	47.7
No Ice	07.32	-0.33	01.12	12310 24	71.02	4.7.7
1 2 Dead+1 0 Wind 210 deg -	90.02	-34 60	59.36	10965 33	6415 76	-7 1
No Ice						
9 Dead+1 0 Wind 210 deg -	67.52	-34 60	59.36	10933 26	6394 78	-71
No Ice	90.02	-61 34	35.42	6491.21	11238 18	-23.9
1.2 Dead+1.0 Wind 240 deg - No Ice	90.02	-01 34	33.42	0491.21	11238 18	-23.9
9 Dead+1 0 Wind 240 deg -	67.52	-61 34	35.42	6471 19	11204 53	-23 9
No Ice						
1 2 Dead+1 0 Wind 270 deg -	90.02	-68 36	0.29	77 39	12605 62	12.2
No Ice	20.00		2.22	21.72	10500 10	100
0.9 Dead+1 0 Wind 270 deg -	67 52	-68 36	0.29	74.36	12568 18	12 2
No Ice I 2 Dead+1 0 Wind 300 deg -	90.02	-58.52	-33 38	-6136.48	10836.11	7.9
No Ice	70.02	-50.52	-33.30	-0130.40	10050.11	11.2
9 Dead+1.0 Wind 300 deg -	67.52	-58 52	-33 38	-6123.16	10803 24	8.0
No Ice		0.173				
2 Dead+1 0 Wind 330 deg -	90.02	-34 06	-58 81	-10843 85	6317.37	-45 5
No Ice 0 9 Dead+1 0 Wind 330 deg -	67.52	-34 07	-58.81	-10818 15	6296 38	-45 4
No Ice	01.32	-34.07	-30.01	-1001013	0270 38	-43 4
1 2 Dead+1 0 Ice+1 0 Temp	281.31	0.00	-0 00	47.03	94 41	-0.0
1 2 Dead+1 0 Wind 0 deg+1 0	281 31	0.05	-1140	-2154 09	81 91	-73
lce+1 0 Temp	444.45	- 200		- 00/200		-
1 2 Dead+1 0 Wind 30 deg+1 0	281.31	5 69	-9 79	-1847 87	-1008 76	-1.8
lce+1 0 Temp I 2 Dead+1 0 Wind 60 deg+1 0	281 31	9.78	-5 65	-1046 15	-1799 06	(1)
Ice+1 0 Temp	20131	2.75	-5.05	104015	1722.00	
1 2 Dead+1 0 Wind 90 deg+1 0	28131	11 30	-0 03	39 13	-2091 69	0 4
Ice+1 0 Temp						
1.2 Dead+1.0 Wind 120	281 31	9.88	5 64	1133.51	-1812 43	27
deg+1 0 lce+1 0 Temp	281 31	5.60	9 72	1931 50	-990 72	7.6
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	201 31	2.00	9 12	1931 30	-990 72	7.6
1 2 Dead+1 0 Wind 180	281 31	-0.03	11.21	2222 32	102.52	74
deg+1 0 lce+1 0 Temp						
1.2 Dead+1.0 Wind 210	281 31	-5 67	9.78	1938 62	1194 10	1.8
deg+1 0 lce+1 0 Temp	201.01	0.00	2.00	1100.00	2005.00	
1 2 Dead+1 0 Wind 240 deg+1 0 Ice+1 0 Temp	281 31	-9 92	5.73	1150 47	2005 60	-1.1
1.2 Dead+1.0 Wind 270	281.31	-11 28	0.03	53 62	2275.88	-0.5
deg+1 0 Ice+1 0 Temp		1.40	0.03	25 02		
1.2 Dead+1.0 Wind 300	281 31	-9 70	-5 56	-1030 30	1976.65	-2.7
deg+1 0 Ice+1 0 Temp						
1 2 Dead+1 0 Wind 330	281.31	-5 62	-9.71	-1834 94	1183.83	-7.6
deg+1 0 Ice+1 0 Temp	75.02	0.18	-22 79	-4166.39	-29 99	-150
Dead+Wind 0 deg - Service Dead+Wind 30 deg - Service	75.02 75.02	11.21	-19 18	-3533.93	-29 99	2.3
Dead+Wind 60 deg - Service	75.02	1901	-10 98	-2023 60	-3508 32	7.6
Dead+Wind 90 deg - Service	75.02	22.11	-0.11	-17.83	-4063 36	-3.9
Dead+Wind 120 deg - Service	75.02	19 73	11 18	2044 62	-3598 13	-2.8
Dead+Wind 150 deg - Service	75.02 75.02	10.93	18 99	3519 18	-2003 55	14.5
Dead+Wind 180 deg - Service	75.02	-0.11	21.83	4054.09	40.96	15 3

Valmont	Job	565090	Page 62 of 72	
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22	
Plymouth 1N Phone (574)-936-4221 FAX (574)-936-6458	Client	VB BTS II, LLC	Designed by	

Load Combination	Vertical	Shear,	Shear:	Overturning Moment, M.	Overturning Moment, M.	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead+Wind 210 deg - Service	75.02	-11.15	19.13	3541.31	2078.32	-2.22
Dead+Wind 240 deg - Service	75.02	-19 77	11.41	2098 77	3631 84	-7 68
Dead+Wind 270 deg - Service	75.02	-22 03	0.09	32.09	4073 50	3 86
Dead+Wind 300 deg - Service	75.02	-18.86	-10.76	-1973.29	3503.02	2 55
Dead+Wind 330 deg - Service	75.02	-10.98	-18 96	-3491 85	2044 27	-14 52

### **Solution Summary**

		n of Applied Forces		Sum of Reaction	S		
Load	PX	PY	PY PZ	PX	PY	PZ	% Erro
Comb.	K	K	K	K	K	K	
1	0.00	-75.02	0.00	-0.00	75.02	0.00	0.001%
2 3	0.56	-90.02	-70 72	-0.56	90.02	70 72	0.002%
3	0.56	-67,52	-70 72	-0 56	67.52	70 72	0.002%
4	34.77	-90.02	-59.52	-34 77	90.02	59 51	0 002%
5	34.77	-67.52	-59.52	-34.77	67.52	59.51	0.002%
6	58.98	-90.02	-34.05	-58 97	90.02	34 05	0.002%
7	58.98	-67.52	-34.05	-58 97	67.52	34 05	0.002%
8	68.58	-90.02	-0 36	-68 58	90.02	0.36	0.002%
9	68.58	-67.52	-0 36	-68 58	67.52	0.36	0.002%
10	61.23	-90.02	34 70	-61 23	90.02	-34.70	0.002%
11	61.23	-67.52	34 70	-61 23	67.52	-34 70	0 002%
12	33.90	-90 02	58 91	-33 90	90 02	-58 90	0.002%
13	33.90	-67.52	58 91	-33 90	67.52	-58.90	0.002%
14	-0.35	-90 02	67 72	0.35	90.02	-67.72	0.002%
15	-0.35	-67 52	67 72	0.35	67.52	-67 72	0.002%
16	-34.60	-90.02	59 36	34 60	90.02	-59 36	0.002%
17	-34 60	-67.52	59 36	34 60	67.52	-59.36	0.002%
18	-61 34	-90 02	35 42	61 34	90 02	-35.42	0.002%
19	-61 34	-67.52	35 42	61 34	67.52	-35 42	0.002%
20	-68 36	-90.02	0 29	68.36	90 02	-0.29	0.002%
21	-68 36	-67 52	0.29	68 36	67.52	-0.29	0.002%
22	-58 52	-90 02	-33 38	58 52	90 02	33.38	0.002%
23	-58 52	-67 52	-33.38	58.52	67.52	33.38	0.002%
24	-34 07	-90.02	-58.81	34 06	90.02	58.81	0.002%
25	-34 07	-67.52	-58 81	34 07	67.52	58.81	0.002%
26	0.00	-281.31	0.00	-0.00	281.31	0.00	0.000%
27	0.05	-281.31	-11 40	-0.05	281 31	11.40	0.000%
28	5 69	-281 31	-9.79	-5.69	281 31	9.79	0.000%
29	9.78	-281 31	-5.65	-9 78	281 31	5.65	0.000%
30	11.30	-281 31	-0.03	-11 30	281 31	0.03	0.000%
31	9.88	-281 31	5 64	-9.88	281 31	-5.64	0.000%
32	5.60	-281 31	9 72	-5.60	281 31	-9.72	0.000%
33	-0.03	-281 31	11.21	0.03	281.31	-11.21	0.000%
34	-5 68	-281 31	9.78	5 67	281 31	-9.78	0.000%
35	-9.93	-281 31	5 73	9 92	281 31	-5.73	0.000%
36	-11 28	-281 31	0.03	11 28	281 31	-0.03	0.000%
37	-9 70	-281 31	-5 56	9.70	281 31	5 56	0.000%
38	-5 62	-281 31	-9 71	5 62	281 31	9.71	0 000%
39	0.18	-75.02	-22 79	-0.18	75.02	22.79	0 000%
40	11.21	-75.02	-19 18	-11.21	75.02	19 18	0 001%
41	19.01	-75 02 -75 02	-10 98	-19 01	75.02	10 98	0.001%
	22.11		-0 11	-19 01	75.02	0 11	0 001%
42		-75.02 75.02	11.18	-19 73	75.02	-11.18	0.001%
43	19 73	-75.02 75.02					
44	10 93	-75 02 76 03	18 99	-10 93	75 02	-18 99	0 001%
45	-0.11	-75 02 75 03	21 83	0.11	75 02	-21 83	0 001%
46	-11 15	-75 02	19 13	11.15	75.02	-19 13	0 001%
47	-19 77	-75 02	11.41	19.77	75 02	-11 41	0 001%

Valmont	Job	565090	Page 63 of 72	
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22	
Plymouth 1N Phone. (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL	

	Sui	n of Applied Force.	s		5		
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
48	-22 03	-75.02	0.09	22.03	75 02	-0.09	0.001%
49	-18.86	-75 02	-10.76	18 86	75 02	10.76	0.001%
50	-10.98	-75 02	-18.96	10.98	75 02	18.96	0.001%

## Non-Linear Convergence Results

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	6	0.00000001	0.00014355
2	Yes	14	0 00003009	0 00007784
3	Yes	14	0 00002591	0 00006759
4	Yes	14	0.00003229	0.00008276
5	Yes	14	0 00002793	0.00007219
6	Yes	14	0 00003398	0 00008666
7	Yes	14	0 00002948	0 00007582
8	Yes	14	0 00003227	0.00008270
9	Yes	14	0 00002791	0.00007215
10	Yes	14	0.00003011	0 00007786
11	Yes	14	0.00002592	0.00006761
12	Yes	14	0 00003229	0.00008275
13	Yes	14	0 00002793	0 00007217
14	Yes	14	0.00003400	0 00008668
15	Yes	14	0.00002949	0.00007583
16	Yes	14	0.00003223	0.00008261
17	Yes	14	0.00002787	0 00007205
18	Yes	14	0.00003009	0.00007783
19	Yes	14	0.00002591	0.00006758
20	Yes	14	0.00003220	0 00008255
21	Yes	14	0.00002785	0.00007200
22	Yes	14	0.00003397	0.00008660
23	Yes	14	0 00002947	0 00007576
24	Yes	14	0 00003229	0 00008274
25	Yes	14	0.00002792	0 00007216
26	Yes	11	0 00000001	0 00008862
27	Yes	14	0 00000001	0 00010924
28	Yes	14	0.00000001	0 00010829
29	Yes	14	0.00000001	0 00010840
30	Yes	14	0 00000001	0.00010839
31	Yes	14	0.00000001	0.00010963
32	Yes	14	0 00000001	0 00011230
33	Yes	14	0.00000001	0 00011499
34	Yes	14	0 00000001	0 00011567
35	Yes	14	0 00000001	0 00011554
36	Yes	14	0.00000001	0.00011505
37	Yes	14	0.00000001	0.00011409
38	Yes	14	0.00000001	0.00011155
39	Yes	14	0.00000001	0.00007291
40	Yes	14	0.00000001	0.00007433
41	Yes	14	0.00000001	0.00007560
42	Yes	14	0.00000001	0 00007428
43	Yes	14	0.00000001	0 00007289
44	Yes	14	0 00000001	0 00007435
45	Yes	14	0 00000001	0.0000756
46	Yes	14	0 00000001	0 00007433
47	Yes	14	0 00000001	0.00007294
48	Yes	14	0 00000001	0 00007427

Valmont	Job	565090	Page 64 of 72
1545 Pidco Dr.	Project I	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by

50	Yes	14	0.00000001	0 00007433
49	Yes	14	0.00000001	0 00007559

# **Maximum Tower Deflections - Service Wind**

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist	
	fi	in	Comb.	0	0	
TI	290 - 280	19 359	47	0.7164	0.0464	
T2	280 - 260	17 861	47	0.7132	0.0461	
T3	260 - 240	14 777	47	0.6504	0 0420	
T4	240 - 220	12.019	47	0.5627	0.0358	
T5	220 - 200	9 779	47	0.4792	0 0277	
T6	200 - 180	7.683	47	0.4030	0 0213	
T7	180 - 160	5.965	47	0.3465	0.0173	
T8	160 - 140	4.537	47	0.2895	0.0137	
T9	140 - 120	3.367	47	0.2334	0 0 1 0 2	
T10	120 - 100	2.382	47	0 1928	0 0081	
TII	100 - 80	1.598	47	0 1526	0.0063	
T12	80 - 60	0.994	47	0 1133	0 0045	
T13	60 - 40	0.545	47	0.0828	0 0029	
T14	40 - 20	0.238	47	0.0528	0.0019	
T15	20 - 0	0.054	47	0.0231	0 0009	

# Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ſŧ		Comb	in	0	0	fi
290 00	5/8" x 10' lightning rod	47	19 359	0 7164	0 0464	31165
285 00	40,000 sq in. (277.8 sq ft EPA)	47	18.615	0.7165	0.0464	31165
275 00	30,000 sq in (208 3 sq ft EPA)	47	17 092	0.7040	0.0455	35777
265 00	30,000 sq in (208 3 sq ft EPA)	47	15 539	0.6710	0 0433	21926
240.00	6' HP	47	12.019	0.5627	0 0358	7287
145.00	OB light	47	3.640	0.2462	0.0110	26137

# **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov	Tilt	Twist
No.	ft	Deflection in	Load Comb.	0	6
TI	290 - 280	59 840	18	2.2174	0.1457
T2	280 - 260	55 206	18	2 2076	0.1448
T3	260 - 240	45 664	18	20121	0 1319
T4	240 - 220	37.135	18	1.7390	0.1123
T5	220 - 200	30.215	18	1 4806	0.0871
T6	200 - 180	23.741	18	1 2451	0.0671
T7	180 - 160	18 435	18	1 0703	0.0545
T8	160 - 140	14 022	18	0.8945	0.0431
T9	140 - 120	10.409	18	0.7210	0.0319
T10	120 - 100	7.365	18	0.5957	0.0254

Valmont	Јо <b>Б</b> 565090	Page 65 of 72
1545 Pidco Dr.	Project H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
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Section	Elevation	Horz.	Gov:	Tih	Twist
No.		Deflection	Load		
	ſt	in	Comb	9	'n
TII	100 - 80	4.942	18	0.4716	0.0196
T12	80 - 60	3 075	18	0.3502	0.0142
T13	60 - 40	1 686	18	0.2560	0.0091
T14	40 - 20	0.736	18	0 1633	0 0060
T15	20 - 0	0.168	18	0 0716	0 0029

# Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tili	Twist	Radius of Curvature
fi		Comb.	in	0	0	fi
290.00	5/8" x 10' lightning rod	18	59 840	2 2 1 7 4	0 1457	10103
285.00	40,000 sq in. (277 8 sq ft. EPA)	18	57.538	2.2178	0 1456	10103
275.00	30,000 sq in (208 3 sq ft EPA)	18	52.825	2 1788	0.1428	11714
265.00	30,000 sq in. (208 3 sq ft. EPA)	18	48 021	2 0764	0.1361	6982
240.00	6' HP	18	37 135	1.7390	0.1123	2348
145.00	OB light	18	11.252	0.7605	0 0343	8438

# **Bolt Design Data**

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size	Number Of Bolts	Maximum Load per Bolt	Allowable Load per Bolt	Rati Loa Allow	d	Allowable Ratio	Criteria
						K	K	4 4 4 4 4 4 4	2772		
TI	290	Leg	A325N	0.7500	4	1.27	29.82	0 043	V	1	Bolt Tension
		Diagonal	A325N	0.7500	1	4.42	7 46		V	1	Member Block Shear
		Top Girt	A325N	0.7500	T-	1 13	12 62	0 090	V	1	Gusset Bearing
T2	280	Leg	A325N	0.7500	6	11.65	29.82	0 391	V	1	Bolt Tension
		Diagonal	A325N	0 7500	Ţ	12 31	12 62	0 976	V	1	Gusset Bearing
T3	260	Leg	A325N	0 7500	8	17 03	29.82	0 571	V	1	Bolt Tension
		Diagonal	A325N	0.7500	1	7.60	11.20	0.678	V	J.	Member Block Shear
T4	240	Leg	A325N	1.0000	6	30.22	53 01	0.570	V	1	Bolt Tension
		Diagonal	A325N	0 7500	Ţ	7 77	13 48	0.577	1	į.	Member Block Shear
T5	220	Leg	A325N	1 0000	6	34 82	53 01	0.657	V	1	Bolt Tension
		Diagonal	A325N	1 0000	T	6 82	13.03	0 524	V	1	Member Block Shear
Т6	200	Leg	A325N	1 2500	6	3991	82 83	0 482	V	1	Bolt Tension
		Diagonal	A325N	1.0000	1	651	17.37	0 375	1	1.	Member Block Shear
T7	180	Leg	A325N	1.2500	6	44 48	82.83	0 537	V	1.	<b>Bolt Tension</b>
		Diagonal	A325N	1.0000	1	6.57	17.37	0 378	1	1	Member Block Shear
T8	160	Leg	A325N	1.2500	6	48.85	82 83	0 590	V	1	Bolt Tension
		Diagonal	A325N	1 0000	1	7 74	14.17	0 547	V	Ţ	Member Block

Valmont	Job	565090	Page 66 of 72
1545 Pidco Dr.	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth. IN Phone (5 <sup>-4</sup> 4)-936-4221 FAX: (5 <sup>7</sup> 4)-936-6458	Client	VB BTS II, LLC	Designed by JL

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size	Number Of Bolts	Maximum Load per Bolt	Allowable Load per Bolt	Rat Loa Allow	nd	Allowable Ratio	Criteria
	1111					K	K				
T9	140	Leg	A325N	1 0000	12	25 70	53.01	0.485	V	1	Shear Bolt Tension
		Diagonal	A325N	0 8750	1	10.28	24.68	0416	~	1	Member Block Shear
TIO	120	Leg	A325N	1 0000	12	27.91	53.01	0.526	V	1	Bolt Tension
		Diagonal	A325N	0.8750	1	10.26	24.68	0.416	~	1	Member Block Shear
T11	100	Leg	A325N	1 0000	12	29.58	53.01	0.558	V	1	Bolt Tension
		Diagonal	A325N	0.8750	1	10,02	24 68	0.406		1	Member Block Shear
T12	80	Leg	A325N	1.0000	12	31.49	53.01	0.594	V	1	Bolt Tension
		Diagonal	A325N	0 8750	1	10 31	24 68	0 418	1	1	Member Block Shear
T13	60	Leg	A325N	1.0000	12	33 11	53 01	0.624	V	1	Bolt Tension
		Diagonal	A325N	0 8750	1	10 60	36.98	0 287	V	1	Gusset Bearing
T14	40	Leg	A325N	1.0000	12	34 89	53 01	0.658	V	1	Bolt Tension
		Diagonal	A325N	0.8750	1	11 29	36.98	0 305	V	1	Gusset Bearing
T15	20	Leg	F1554-10	1.7500	4	109.00	169 12	0.645	V	T.	Bolt Tension
		Diagonal	A325N	0 8750	1	11 73	36.98	0317	V	1	Gusset Bearing

# Compression Checks

# Leg Design Data (Compression)

Section No.	Elevation	Size	L	$L_{\epsilon}$	Kl/r	A	$P_n$	φP_	Ratio P <sub>n</sub>
	ft		fi	ft		$m^2$	K	K	φP <sub>n</sub>
TI	290 - 280	P- 2 50" - 0.75" conn10" -C-(Pirod 226172)	10.00	4.79	60 7 K=1.00	1 7040	-7 59	58.58	0.130
T2	280 - 260	P- 4.00"- 0.75" conn -20' -C-Trans-6B-4B-(Pirod 226184)	20.00	6 67	53 () K=1 00	3.1741	-78.36	116 32	0 674
Т3	260 - 240	P- 5.00"- 0.75" connTrans-20' -C-(Pirod 226200)	20.03	6 68	42 7 K=1 00	4.2999	-148 35	169 37	0 876
T4	240 - 220	P- 6 00"- 0 75" connHBD-Trans-20' -C-(Ptrod 229377)	20 03	6.68	35.7 K=1.00	5 5813	-197 25	228 83	0 862
T5	220 - 200	#12ZG-58 - 1.50" - 1.00" conn (Pirod 194651)	20.03	10.02	35 7 K=1.00	5 3014	-226.56	248 43	0.912
T6	200 - 180	#12ZG-58 - 1 75" - 1 00" connTR1-(Pirod 195213)	20.03	10 02	30 6 K-1 00	7.2158	-261.49	347 96	0.751
17	180 - 160	#12ZG-58 - 1 75" - 1 00" conn. (Pirod 195217)	20.03	10.02	30 6 K=1.00	7 2 1 5 8	-292 70	347 96	0 841
T8	160 - 140	#12ZG-58 - 1 75" - 1 00"	20 03	10.02	30 6	72158	-322 99	347 96	0 928

# Valmont

1545 Pidco Dr.

Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458

Job		Page
	565090	67 of 72
Project	P. C.	Date
	H-31 x290' SST - US-KY-5135 Fancy Farm	07:03:40 10/06/22
Client	200 DOLD 200 A	Designed by
	VB BTS II, LLC	JL

Section No.	Elevation	Size	L	L	Kl/r	A	$P_{\scriptscriptstyle \rm M}$	$\phi P_n$	Ratio P,,
	fi		fi	st		in'	K	K	$\Phi P_n$
		conn (Pirod 195217)			K=1 00				V
T9	140 - 120	#12ZG-58 -2 00" - 0.875" connTR3-(Pirod 195637)	20 03	20 03	48.8 K=1.00	9 4248	-339 90	401 94	0.846
TIO	120 - 100	#12ZG-58 -2.00" - 0.875" conn. (Pirod 195639)	20 03	20 03	48.8 K=1.00	9 4248	-371 60	401 94	0.925
TII	100 - 80	#12ZG-58 -2 00" - 0 875" conn (Pirod 195639)	20.03	20.03	48.8 K=1 00	9 4248	-396.17	401 94	0.986
T12	80 - 60	#12ZG-58 -2 25" - 0 875" conn (Pirod 195960)	20 03	20.03	48.8 K=1.00	11 9282	-424 77	508 98	0.835
T13	60 - 40	#12ZG-58 -2 25" - 0 875" conn (Pirod 195960)	20.03	20.03	48 8 K=1 00	11 9282	-449 05	508 98	0.882
T14	40 - 20	#12ZG-58 -2 25" - 0.875" conn (Pirod 195960)	20.03	20.03	48 8 K=1.00	11 9282	-477 13	508.98	0.937
T15	20 - 0	#12ZG-58 BASE - 2.50" - 0.875" connTR4-(Pirod 281171)	20 03	20 03	48 7 K=1 00	14.7262	-499,88	628.76	0.795

<sup>1</sup> P , / \phi P , controls

			Truss-	Leg D	iagon	al Data	1		
Section No.	Elevation fi	Diagonal Size	L <sub>J</sub> fi	Kl/r	φ <i>P<sub>n</sub> K</i>	A m²	ľ'. K	$\phi V_n$ $K$	Stress Ratio
T5	220 - 200	0.5	1.42	95 2	276 74	0.1963	231	4 57	0 507
Т6	200 - 180	0.5	1.40	94 4	376.67	0.1963	0.90	461	0 196
T7	180 - 160	0.5	1.40	94 4	376.67	0.1963	0.62	4.61	0.135
T8	160 - 140	0.5	1 40	94,4	376.67	0 1963	0 74	461	0 163
Т9	140 - 120	0.5	1.39	93.2	491.97	0.1963	0 96	4.67	0 208
T10	120 - 100	0.5	1 39	93.2	491.97	0.1963	0 93	4.67	0 201
TH	100 - 80	0.5	1 39	93 2	491 97	0 1963	0.72	4 67	0 155
T12	80 - 60	0.5	1.38	92.4	622.65	0 1963	0.80	4.71	0.171
T13	60 - 40	0.5	1 38	92 4	622.65	0 1963	0.67	4.71	0.144
T14	40 - 20	0.5	1.38	92 4	622.65	0 1963	0 76	471	0.175
T15	20 - 0	0.5	1.34	90.2	768.71	0.1963	0.56	4.87	0.129

# Diagonal Design Data (Compression)

Valmont	Job 56	65090	Page 68 of 72
1545 Pidco Dr	Project H-31 x290' SST - U	S-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458	Client VB B	TS II, LLC	Designed by

Section No.	Elevation	Size	L	$L_{\nu}$	Kl/r	A	$P_{u}$	$\phi P_n$	Ratio P <sub>u</sub>
	fi		ft	ft		in <sup>2</sup>	K	K	$\phi P_n$
TI	290 - 280	L2x2x1/8	5.75	2.69	90 8 K=1.12	0.4844	-5.00	11 03	0 453
T2	280 - 260	L2 1/2x2 1/2x3/16	7 17	3 40	91.8 K=1.11	0 9020	-12.84	21 92	0.586
Т3	260 - 240	L2x2x3/16	7.24	3.66	113 6 K=1 02	0 7150	-8 44	12 51	0 675
T4	240 - 220	L2 1/2x2 1/2x3/16	9.60	4 80	117.2 K=1.01	0.9020	-7 72	14 82	0 521
T5	220 - 200	L2 1/2x2 1/2x3/16	12 65	6.42	155.5 K=1 00	0.9020	-7 89	8.42	0.937
T6	200 - 180	L2 1/2x2 1/2x1/4	14 10	7.12	174 1 K=1 00	1 1900	-7 07	8.87	0.797
T7	180 - 160	1,2 1/2x2 1/2x1/4	15 67	7.90	193.0 K=1.00	1 1900	-7 06	7.22	0.978
T8	160 - 140	L3x3x3/16	17 33	8.72	175.6 K=1.00	1 0900	-7 64	7.99	0.957
Т9	140 - 120	2L3x3x3/16	25 03	13 04	166 7 K=1.00	2 1800	-11 77	17.73	0.664
T10	120 - 100	2L3x3x3/16	26.36	13,67	174 7 K=1.00	2.1800	-10 46	16.14	0.648
TH	100 - 80	2L3x3x3/16	27.77	14.35	183.4 K=1.00	2.1800	-11.49	14.65	0 784
T12	80 - 60	2L3x3x3/16	29 25	15.07	192.5 K=1.00	2.1800	-10 51	13 29	0 791
T13	60 - 40	2L3 1/2x3 1/2x1/4	30 78	15.82	173 9 K=1 00	3 3750	-12 21	25 21	0 484
T14	40 - 20	2L3 1/2x3 1/2x1/4	32.37	16.60	182 5 K=1 00	3 3750	-11.06	22 89	0 483
T15	20 - 0	2L3 1/2x3 1/2x1/4	34.01	17 40	191 3 K=1 00	3 3750	-13.55	20.82	0.651

 $P_n / \phi P_n$  controls

Top Girt Design Data (Compression)									-
Section No.	Elevation	Size	L	$L_{i}$	Kl/r	A	$P_{u}$	φ <i>P</i> .,	Ratio P <sub>u</sub>
	fi		ſŧ	ſì		in <sup>J</sup>	K	K	$\phi P_m$
TI	290 - 280	L3x3x1/4	5.00	4.49	105.5 K=1.16	1 4400	-1 34	28 72	0.047

 $<sup>^{1}</sup>$  P  $_{u}$  /  $\phi$ P $_{u}$  controls

# **Tension Checks**

# Valmont

1545 Pidco Dr.

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Job		Page
	565090	69 of 72
Project		Date
	H-31 x290' SST - US-KY-5135 Fancy Farm	07:03:40 10/06/22
Client	Construe Parker Anna	Designed by
	VB BTS II, LLC	11

# Leg Design Data (Tension)

Section No.	Elevation	Size	L	Lu	Klir	A	$P_{\kappa}$	$\phi P_n$	Ratio P <sub>u</sub>
	fi		ſi	ft		$ua^2$	K	K	φP.,
Tl	290 - 280	P- 2.50" - 0.75" conn10" -C-(Pirod 226172)	10 00	4 79	60.7	1,7040	5.07	76,68	0 066
T2	280 - 260	P- 4.00"- 0.75" conn20' -C-Trans-6B-4B-(Pirod 226184)	20 00	6 67	53 0	3,1741	69.91	142 83	0 489
Т3	260 - 240	P- 5 00"- 0 75" connTrans-20' -C-(Pirod 226200)	20 03	6 68	42.7	4.2999	136.21	193 49	0 704
T4	240 - 220	P- 6.00"- 0.75" connHBD-Trans-20' -C-(Pirod 229377)	20.03	6.68	35.7	5 5813	181 29	251 16	0 722
T5	220 - 200	#12ZG-58 - 1.50" - 1.00" conn (Pirod 194651)	20.03	10 02	35 7	5 3014	208 95	276 74	0.755
T6	200 - 180	#12ZG-58 - 1.75" - 1.00" connTR1-(Pirod 195213)	20.03	10.02	30.6	72158	239.43	376.67	0.636
T7	180 - 160	#12ZG-58 - 1 75" - 1.00" conn. (Pirod 195217)	20.03	10.02	30.6	7 2 1 5 8	266.90	376.67	0.709
Т8	160 - 140	#12ZG-58 - 1.75" - 1.00" conn. (Pirod 195217)	20.03	10 02	30 6	72158	293.09	376 67	0.778
Т9	140 - 120	#12ZG-58 -2 00" - 0 875" conn -TR3-(Pirod 195637)	20.03	20.03	48 8	9 4248	308.42	491 97	0.627
TIO	120 - 100	#12ZG-58 -2 00" - 0.875" conn. (Pirod 195639)	20 03	20.03	48 8	9 4248	334 87	491 97	0 681
T11	100 - 80	#12ZG-58 -2 00" - 0 875" conn. (Pirod 195639)	20 03	20.03	488	9 4248	354 96	491 97	0 722
T12	80 - 60	#12ZG-58 -2 25" - 0.875" conn. (Pirod 195960)	20 03	20.03	48.8	11 9282	377 92	622 65	0 607
T13	60 - 40	#12ZG-58 -2 25" - 0.875" conn. (Pirod 195960)	20 03	20.03	48.8	11 9282	397 27	622 65	0 638
T14	40 - 20	#12ZG-58 -2 25" - 0.875" conn (Pirod 195960)	20 03	20 03	48.8	11 9282	418.66	622.65	0.672
TIS	20 - 0	#12ZG-58 BASE - 2.50" - 0.875" connTR4-(Pirod 281171)	20 03	20 03	48.7	14.7262	436.01	768 71	0 567

 $P_u / \phi P_n$  controls

# Truss-Leg Diagonal Data

Section No.	Elevation fi	Diagonal Size	L.j ft	Kl/r	$\phi P_n$ $K$	A in²	ľ, K	φν. Κ	Stress Ratio
T5	220 ~ 200	0.5	1 42	95 2	276 74	0 1963	231	4.57	0.507
T6	200 - 180	0.5	1 40	94 4	376 67	0 1963	0 90	461	0 196
T7	180 - 160	0.5	1.40	94.4	376.67	0.1963	0.62	4.61	0.135
T8	160 - 140	0 5	1 40	94.4	376.67	0 1963	0.74	461	0 163

				-1
100	m	0	34	ø
668	776	U	r.	ø.

1545 Pidco Dr.

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Jop	565090	Page 70 of 72
Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Client	VB BTS II, LLC	Designed by JL

Section No.	Elevation ft	Diagonal Size	L,i fi	Klr	φP <sub>n</sub> K	A in	I' <sub>n</sub> K	φV <sub>n</sub> K	Stress Ratio
					1.00		10.5		V
T9	140 - 120	0.5	1 39	93.2	491 97	0.1963	0 96	4 67	0.208
T10	120 - 100	0.5	1.39	93.2	491.97	0 1963	0 93	4 67	0.201
TII	100 - 80	0.5	1 39	93.2	491 97	0 1963	0 72	4.67	0.155
T12	80 - 60	0.5	1 38	92 4	622 65	0 1963	0.80	471	0 171
T13	60 - 40	0.5	1.38	92 4	622.65	0 1963	0.67	4.71	0 144
T14	40 - 20	0.5	1 38	92 4	622.65	0 1963	0.76	4.71	0 175
T15	20 - 0	0.5	1 34	90 2	768 71	0 1963	0 56	4.87	0 129

Diagonal	Design	Data	(Tension)
Diagonai	Pesign	PRIL	11011011

Section No.	Elevation	Size	L	$L_{\mu}$	Kl/r	A	$P_n$	$\phi P_n$	Ratio P <sub>*</sub>
	fi		fi	fi		ın²	K	K	$\phi P_n$
Tt	290 - 280	L2x2x1/8	5 75	2 69	55.1	0.2813	4 42	13.71	0 322
T2	280 - 260	L2 1/2x2 1/2x3/16	7.17	3 40	55 3	0 5535	1231	26 98	0 456
Т3	260 - 240	L2x2x3/16	7 24	3 66	74 8	0.4132	7 60	20 14	0.377
T4	240 - 220	L2 1/2x2 1/2x3/16	9 60	4.80	76.9	0 5535	7 77	26 98	0 288 1
TS	220 - 200	L2 1/2x2 1/2x3/16	12 65	6.42	102 2	0 5183	6.82	25.27	0.270 1
T6	200 - 180	L2 1/2x2 1/2x1/4	13 35	6.76	108.7	0 6816	651	33 23	0 196 1
T7	180 - 160	L2 1/2x2 1/2x1/4	15 67	7 90	126 5	0 6816	6.57	33 23	0 198 1
Т8	160 - 140	L3x3x3/16	17.33	8 72	114 1	0.6593	7.74	32 14	0.241
T9	140 - 120	2L3x3x3/16	25.03	13.04	168 8	1 3537	10 28	66.00	0 156 1
T10	120 - 100	2L3x3x3/16	26.36	13.67	176.8	1.3537	10 26	66.00	0.156
TU	100 - 80	2L3x3x3/16	27 77	14.35	185 5	1 3537	10.02	66.00	0 152
T12	80 - 60	2L3x3x3/16	29.25	15.07	194.7	1 3537	10.31	66.00	0 156 '
T13	60 - 40	2L3 1/2x3 1/2x1/4	30 78	15 82	175 8	2.1563	10.60	105.12	0 101 1
T14	40 - 20	2L3 1/2x3 1/2x1/4	32 37	16 60	184.3	2 1563	11.29	105 12	0 107
TIS	20 - 0	2L3 1/2x3 1/2x1/4	34 01	17.40	193.2	2.1563	11 73	105.12	0.112

#### Page Job 71 of 72 565090 Project Date H-31 x290' SST - US-KY-5135 Fancy Farm 07:03:40 10/06/22 Plymouth, IN Phone: (574)-936-4221 FAX: (574)-936-6458 Client Designed by VB BTS II, LLC JL

Section No.	Elevation	Size	L	$L_{\nu}$	Kl/r	A	$P_w$	$\phi P_n$	Ratio P.,
3.45	ſŧ		ſì	ſŧ		in <sup>2</sup>	K	K	$\phi P_n$
									V

P , / \phi P , controls

Valmont

1545 Pidco Dr.

		To	p Girt [	)esigi	n Data	a (Tens	ion)		initial and
Section No.	Elevation	Size	L	L	Kl/r	A	Pin	$\phi P_n$	Ratio P.,
110.	ft		ſŧ	fi		in <sup>2</sup>	K	K	$\phi P_n$
TI	290 - 280	L3x3x1/4	5.00	4 49	614	0.9159	1 13	44 65	0.025

<sup>&</sup>lt;sup>1</sup>  $P_u / \phi P_u$  controls

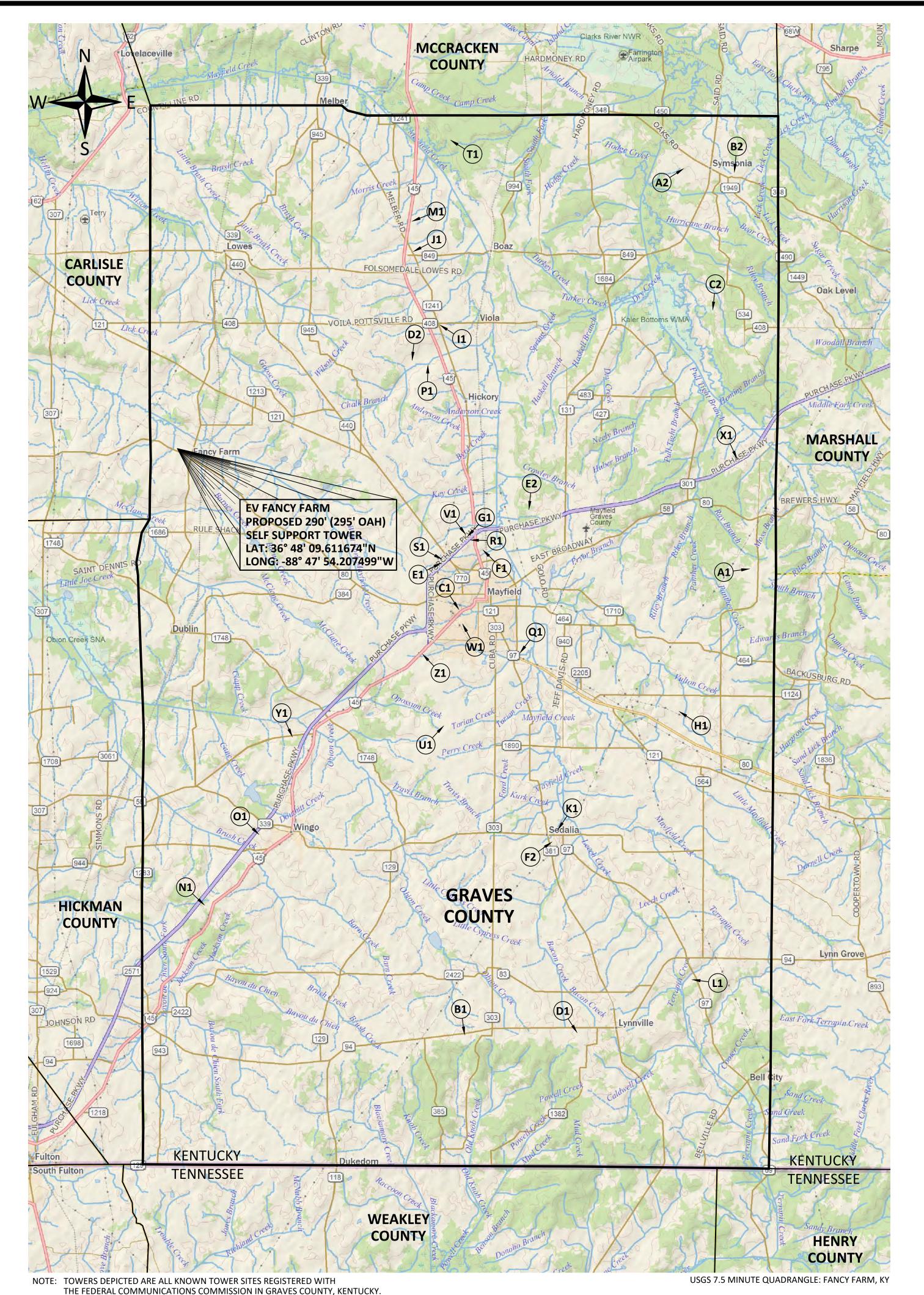
Section	Capa	city	Table
	- 60 lo 40	The same in	It also note, if you,

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ØP <sub>allow</sub> K	% Capacity	Pass Fail
Tì	290 - 280	Leg	P- 2 50" - 0 75" conn -10' -C-(Pirod 226172)	1	-7.59	58.58	13.0	Pass
T2	280 - 260	Leg	P- 4 00"- 0 75" conn -20' -C-Trans-6B-4B-(Pirod 226184)	21	-78 36	116.32	674	Pass
T3	260 - 240	Leg	P- 5 00"- 0 75" conn -Trans-20' -C-(Pirod 226200)	42	-148 35	169 37	87.6	Pass
T4	240 - 220	Leg	P- 6.00"- 0.75" conn -HBD-Trans-20' -C-(Pirod 229377)	61	-197 25	228 83	86.2	Pass
T5	220 - 200	Leg	#12ZG-58 - 1 50" - 1.00" conn (Pirod 194651)	82	-226.56	248 43	912	Pass
T6	200 - 180	Leg	#12ZG-58 - 1 75" - 1 00" connTR1-(Pirod 195213)	97	-261 49	347.96	75 1	Pass
T7	180 - 160	Leg	#12ZG-58 - 1.75" - 1.00" conn (Pirod 195217)	112	-292.70	347.96	84 1	Pass
T8	160 - 140	Leg	#12ZG-58 - 1.75" - 1.00" conn. (Pirod 195217)	127	-322 99	347 96	92 8	Pass
T9	140 - 120	Leg	#12ZG-58 -2 00" - 0 875 conn -TR3-(Pirod 195637)	142	-339 90	401 94	84.6	Pass
T10	120 - 100	Leg	#12ZG-58 -2 00" - 0 875" conn (Pirod 195639)	151	-371.60	401 94	92.5	Pass
TH	100 - 80	Leg	#12ZG-58 -2 00" - 0 875" conn (Pirod 195639)	160	-396 17	401 94	98.6	Pass
T12	80 - 60	Leg	#12ZG-58 -2.25" - 0.875" conn (Pirod 195960)	169	-424 77	508 98	83.5	Pass
T13	60 - 40	Leg	#12ZG-58 -2.25" - 0 875" conn (Pirod 195960)	178	-449 05	508 98	88.2	Pass
T14	40 - 20	Leg	#12ZG-58 -2 25" - 0 875" conn (Pirod 195960)	187	-477 13	508 98	93.7	Pass
T15	20 - 0	Leg	#12ZG-58 BASE - 2.50" - 0.875" conn -TR4-(Pirod 281171)	196	-499 88	628 76	79 5	Pass
TI	290 - 280	Diagonal	L2x2x1/8	8	-5.00	11.03	45.3	Pass

Valmont	Job	565090	Page 72 of 72
1545 Pidco Dr	Project	H-31 x290' SST - US-KY-5135 Fancy Farm	Date 07:03:40 10/06/22
Plymouth, IN Phone (574)-936-4221 FAX: (574)-936-6458	Client	VB BTS II, LLC	Designed by JL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	oP <sub>allow</sub>	% Capacity	Pass Fail
				172	- J. L.	Park and	59.2 (b)	
T2	280 - 260	Diagonal	L2 1/2x2 1/2x3/16	23	-12 84	21 92	58.6	Pass
T3	260 - 240	Diagonal	L2x2x3/16	55	-8 44	12 51	97.6 (b) 67.5	Pass
13	200 - 240	Diagonal	L2X2X3/10	33	-0.44	14 31	67.8 (b)	£ 435
T4	240 - 220	Diagonal	L2 1/2x2 1/2x3/16	69	-7.72	14.82	52.1	Pass
				- 77	40:02		577(b)	2000
T5	220 - 200	Diagonal	1,2 1/2x2 1/2x3/16	89	-7.89	8.42	93.7	Pass
T6	200 - 180	Diagonal	L2 1/2x2 1/2x1/4	104	-7 07	8.87	79 7	Pass
T7	180 - 160	Diagonal	L2 1/2x2 1/2x1/4	116	-7 06	7 22	978	Pass
T8	160 - 140	Diagonal	L3x3x3/16	135	-7.64	7.99	95 7	Pass
T9	140 - 120	Diagonal	2L3x3x3/16	149	-11,77	17.73	66.4	Pass
TIO	120 - 100	Diagonal	2L3x3x3/16	159	-10 46	16.14	64.8	Pass
T11	100 - 80	Diagonal	2L3x3x3/16	167	-11.49	14.65	78 4	Pass
T12	80 - 60	Diagonal	2L3x3x3/16	176	-10.51	13.29	79 1	Pass
T13	60 - 40	Diagonal	2L3 1/2x3 1/2x1/4	185	-12.21	25.21	484	Pass
T14	40 - 20	Diagonal	2L3 1/2x3 1/2x1/4	195	-11 06	22.89	483	Pass
T15	20 - 0	Diagonal	2L3 1/2x3 1/2x1/4	203	-13.55	20.82	65 1	Pass
TI	290 - 280	Top Girt	L3x3x1/4	5	-1 34	28.72	4.7	Pass
							9.0 (b)	
							Summary	
						Leg (TII)	986	Pass
						Diagonal (T7)	978	Pass
						Top Girt (T1)	9.0	Pass
						Bolt Checks	976	Pass
						RATING =	98.6	Pass

Program Version 8 I 1 0 - 6/3/2021 File Z /Documents/565/565090 VB BTS II - US-KY-5180 Fancy Farm/02 Tower Calcs/565090 eri



# GRAVES COUNTY, KENTUCKY VERIZON WIRELESS SITE NAME: EV FANCY FARMS

# **EXISTING TOWER LEGEND**

(GRANTED) FCC REGISTRATION #: 1018328 SUN MEDIA INC dba = WRIK RADIO LAT: 36° 45' 09.0"N LONG: 88° 29' 58.0"W

FCC REGISTRATION #: 1039661 B1 TEXAS GAS 11..... LAT: 36° 33' 26.0"N TEXAS GAS TRANSMISSION, LLC

LONG: 88° 38' 54.0"W

FCC REGISTRATION #: 1039661 GRAVES CO....
LAT: 36° 44' 07.0"N **GRAVES COUNTY CO-OP** 

LONG: 88° 39' 05.0"W

FCC REGISTRATION #: 1041880 BELLSOUTH (D1) TELECOMMUNICATIONS, LLC LAT: 36° 33′ 30.0″N

FCC REGISTRATION #: 1043138 MELVIN N SHOLAR LAT: 36° 45' 19.2"N LONG: 88° 39' 36.8"W

LONG: 88° 35' 22.0"W

FCC REGISTRATION #: 1043916 BRISTOL BROADCASTING (**F1**) COMPANY, INC. LAT: 36° 45′ 37.0″N LONG: 88° 38' 20.0"W

FCC REGISTRATION #: 1043917 **BRISTOL BROADCASTING** (**G1**) COMPANY, INC. LAT: 36° 45′ 58.0″N LONG: 88° 38' 50.0"W

FCC REGISTRATION #: 1044036 KENTUCKY AUTHORITY FOR EDUCATIONAL TELEVISION dba = MKMU LAT: 36° 41′ 34.0″N LONG: 88° 32' 11.0"W

FCC REGISTRATION #: 1044824 COMMONWEALTH OF KENTUCKY dba = EMERGENCY WARNING SYSTEM KEWS

FCC REGISTRATION #: 1201350 WEST KENTUCKY RURAL TELEPHONE COOP CORP INC LAT: 36° 53′ 08.0″N LONG: 88° 40' 29.0"W

LAT: 36° 51′ 17.0″N

LONG: 88° 39' 40.0"W

FCC REGISTRATION #: 1201356 WEST KENTUCKY RURAL (K1) TELEPHONE COOP CORP INC LAT: 36° 38′ 35.0″N LONG: 88° 35' 58.0"W

FCC REGISTRATION #: 1202399 KENTUCKY RSA NO. 1 PARTNERSHIP L1 KENTUCKY K5A NO. 1 LAT: 36° 34' 49.2"N LONG: 88° 31' 45.2"W

FCC REGISTRATION #: 1213964 CROWN CASTLE SOUTH LLC LAT: 36° 53' 53.3"N LONG: 88° 40' 32.2"W

FCC REGISTRATION #: 1215493 (N1) CROWN CASTLE SOUTH LLC LAT: 36° 36' 41.4"N LONG: 88° 47' 03.9"W

FCC REGISTRATION #: 1215862 O1 SBA PROPERTIES, LLC LAT: 36° 38' 28.6"N LONG: 88° 45' 21.4"W

FCC REGISTRATION #: 1215910 P1 SBA PROPERTIES, LLC LAT: 36° 50' 15.7"N LONG: 88° 40' 02.5"W

SBA PROPERTIES, LLC LAT: 36° 43' 02.0"N LONG: 88° 37' 10.0"W

FCC REGISTRATION #: 1217408

FCC REGISTRATION #: 1222179 SBA PROPERTIES, LLC LAT: 36° 45' 51.9"N LONG: 88° 38' 42.5"W

FCC REGISTRATION #: 1223176 CROWN CASTLE SOUTH LLC LAT: 36° 45' 23.0"N LONG: 88° 39' 36.1"W

FCC REGISTRATION #: 1223623 SBA PROPERTIES, LLC LAT: 36° 55' 55.2"N LONG: 88° 39' 19.1"W

(GRANTED) FCC REGISTRATION #: 1244198 FCC REGISTRATION #: 1314081 (U1) KENTUCKY RSA NO. 1 PARTNERSHIP LAT: 36° 41′ 12.0″N (C2) KENTUCKY RSA NO. 1 PARTNERSHIP LAT: 36° 51′ 39.0″N LONG: 88° 39' 33.5"W

FCC REGISTRATION #: 1261078 KENTUCKY RSA NO. 1 PARTNERSHIP LAT: 36° 46' 00.1"N LONG: 88° 38' 51.9"W

FCC REGISTRATION #: 1264848 MAYFIELD ELECTRIC & WATER LAT: 36° 43' 45.5"N

LONG: 88° 38' 57.2"W

FCC REGISTRATION #: 1266082 SBA TOWERS III LLC LAT: 36° 47' 54.5"N LONG: 88° 30' 22.2"W

(GRANTED) FCC REGISTRATION #: 1320153 (F2) TILLMAN INFRASTRUCTURE, LLC LAT: 36° 38, 16.2"N LONG: 88° 36' 09.6"W

FCC REGISTRATION #: 1271762

FCC REGISTRATION #: 1277402

FCC REGISTRATION #: 1287188

KENTUCKY RSA NO. 1 PARTNERSHIP LAT: 36° 55' 12.8"N

FCC REGISTRATION #: 1305782

B2 TILLMAN INFRASTRUCTURE, LLC LAT: 36° 55' 07.1"N

LONG: 88° 30' 26.8"W

LONG: 88° 31' 07.0"W

LAT: 36° 50′ 23.7″N

LONG: 88° 40' 32.7"W

LONG: 88° 36' 52.8"W

FCC REGISTRATION #: 1315881

(**D2**) VERTICAL BRIDGE DEVELOPMENT, LLC

FCC REGISTRATION #: 1317446

VERTICAL BRIDGE DEVELOPMENT, LLC LAT: 36° 46' 38.0"N

(GRANTED)

WEST KENTUCKY RURAL ELECTRIC LAT: 36° 42' 59.9"N

LONG: 88° 40' 13.0"W

LONG: 88° 32' 02.2"W

AMERICAN TOWERS LLC LAT: 36° 40' 56.3"N

LONG: 88° 44' 18.6"W



PREPARED FOR:

# **REVISIONS**

REV.	DATE	DESCRIPTION
Α	9.14.22	ISSUED FOR REVIEW

SITE INFORMATION:

**EV FANCY FARM KENTUCKY HIGHWAY 80** FANCY FARM, KY 42039 **GRAVES COUNTY** 

**TAX PARCEL NUMBER:** 006.00.00.005.00

**PROPERTY OWNER:** KM & K FARMS LLC P O BOX 48035 COON RAPIDS, MN 55448

SOURCE OF TITLE: DEED BOOK 506, PAGE 639

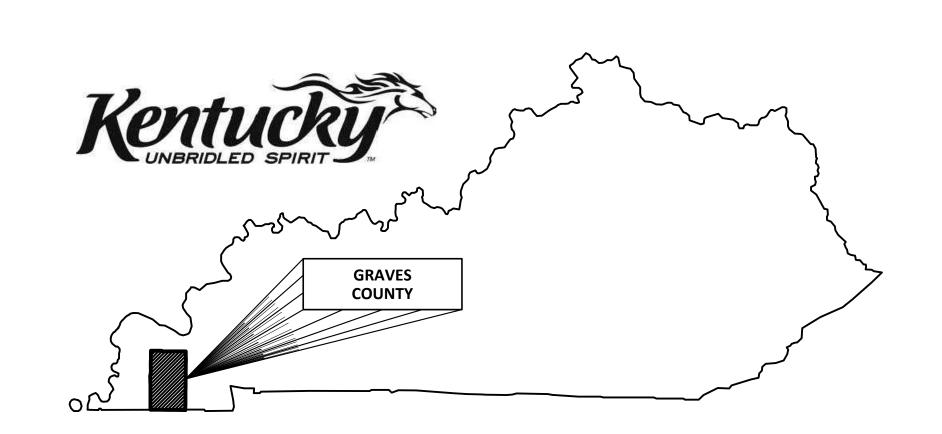
22-123884 POD NUMBER: DRAWN BY: DAP MEP CHECKED BY: SURVEY DATE: 6.24.20 PLAT DATE: 9.14.22

SHEET TITLE:

**TOWER GRID MAP** 

SHEET NUMBER: (1 page)

**L**-.





Mail Processing Center
Federal Aviation Administration
Southwest Regional Office
Obstruction Evaluation Group
10101 Hillwood Parkway
Fort Worth, TX 76177

Issued Date: 01/06/2023

Network Regulatory Cellco Partnership 5055 North Point Pkwy NP2NE Network Engineering Alpharetta, GA 30022

## \*\* DETERMINATION OF NO HAZARD TO AIR NAVIGATION \*\*

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure: Antenna Tower EV Fancy Farm (16207023)

Location: Fancy Farm, KY

Latitude: 36-48-09.61N NAD 83

Longitude: 88-47-54.21W

Heights: 431 feet site elevation (SE)

295 feet above ground level (AGL) 726 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

Emissions from this site must be in compliance with the parameters set by collaboration between the FAA and telecommunications companies and reflected in the FAA 5G C band compatibility evaluation process (such as power, frequencies, and tilt angle). Operational use of this frequency band is not objectionable provided the Wireless Providers (WP) obtain and adhere to the parameters established by the FAA 5G C band compatibility evaluation process. **Failure to comply with this condition will void this determination of no hazard.** 

As a condition to this Determination, the structure is to be marked/lighted in accordance with FAA Advisory circular 70/7460-1 M, Obstruction Marking and Lighting, a med-dual system-Chapters 4,8(M-Dual),&15.

Any failure or malfunction that lasts more than thirty (30) minutes and affects a top light or flashing obstruction light, regardless of its position, should be reported immediately to (877) 487-6867 so a Notice to Airmen (NOTAM) can be issued. As soon as the normal operation is restored, notify the same number.

It is required that FAA Form 7460-2, Notice of Actual Construction or Alteration, be e-filed any time the project is abandoned or:

	At least 10 days prior to start of construction (7460-2, Part 1)	
X_	Within 5 days after the construction reaches its greatest height (7460-2, Part 2	.)

## See attachment for additional condition(s) or information.

This determination expires on 07/06/2024 unless:

- (a) the construction is started (not necessarily completed) and FAA Form 7460-2, Notice of Actual Construction or Alteration, is received by this office.
- (b) extended, revised, or terminated by the issuing office.
- (c) the construction is subject to the licensing authority of the Federal Communications Commission (FCC) and an application for a construction permit has been filed, as required by the FCC, within 6 months of the date of this determination. In such case, the determination expires on the date prescribed by the FCC for completion of construction, or the date the FCC denies the application.

NOTE: REQUEST FOR EXTENSION OF THE EFFECTIVE PERIOD OF THIS DETERMINATION MUST BE E-FILED AT LEAST 15 DAYS PRIOR TO THE EXPIRATION DATE. AFTER RE-EVALUATION OF CURRENT OPERATIONS IN THE AREA OF THE STRUCTURE TO DETERMINE THAT NO SIGNIFICANT AERONAUTICAL CHANGES HAVE OCCURRED, YOUR DETERMINATION MAY BE ELIGIBLE FOR ONE EXTENSION OF THE EFFECTIVE PERIOD.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power, except those frequencies specified in the Colo Void Clause Coalition; Antenna System Co-Location; Voluntary Best Practices, effective 21 Nov 2007, will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA. This determination includes all previously filed frequencies and power for this structure.

If construction or alteration is dismantled or destroyed, you must submit notice to the FAA within 5 days after the construction or alteration is dismantled or destroyed.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

A copy of this determination will be forwarded to the Federal Communications Commission (FCC) because the structure is subject to their licensing authority.

If we can be of further assistance, please contact our office at (817) 222-5928, or chris.smith@faa.gov. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2022-ASO-27278-OE.

Signature Control No: 542760464-567380305 (DNE)

Chris Smith Specialist

Attachment(s)
Additional Information
Frequency Data
Map(s)

cc: FCC

## Additional information for ASN 2022-ASO-27278-OE

At a distance of 15.9 nautical miles from the site emissions from the 2496-2690 MHz transmitters must be less than -155 dBm in the 2700-3100 MHz Surveillance Radar frequency band.

Additionally, Part 77 authorizes the FAA to evaluate a structure or object's potential electromagnetic effects on air navigation, communication facilities, and other surveillance systems. It also authorizes study of impact on arrival, departure, and en route procedures for aircraft operating under visual or instrument flight rules, as well as the impact on airport traffic capacity at existing public use airports. Broadcast in the 3.7 to 3.98 GHz frequency (5G C band) currently causes errors in certain aircraft radio altimeters and the FAA has determined they cannot be relied upon to perform their intended function when experiencing interference from wireless broadband operations in the 5G C band. The FAA has adopted Airworthiness Directives for all transport and commuter category aircraft equipped with radio altimeters that prohibit certain operations when in the presence of 5G C band.

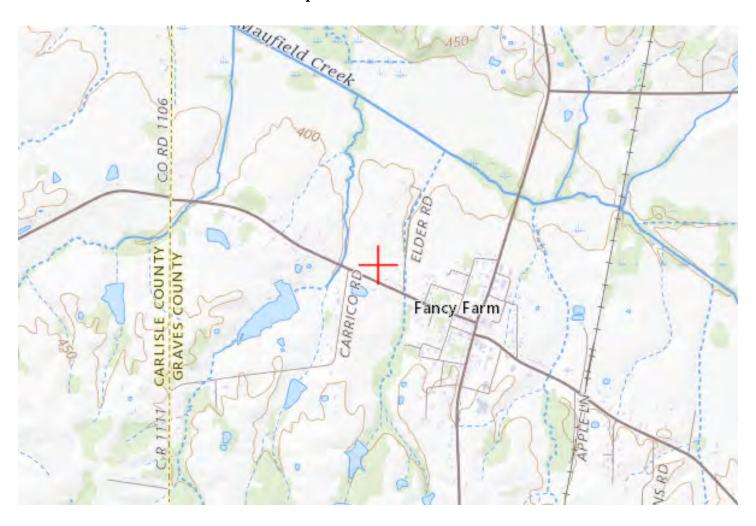
This determination of no hazard is based upon those mitigations implemented by the FAA and operators of transport and commuter category aircraft, and helicopters operating in the vicinity of your proposed location. It is also based on telecommunication industry and FAA collaboration on acceptable power levels and other parameters as reflected in the FAA 5G C band evaluation process.

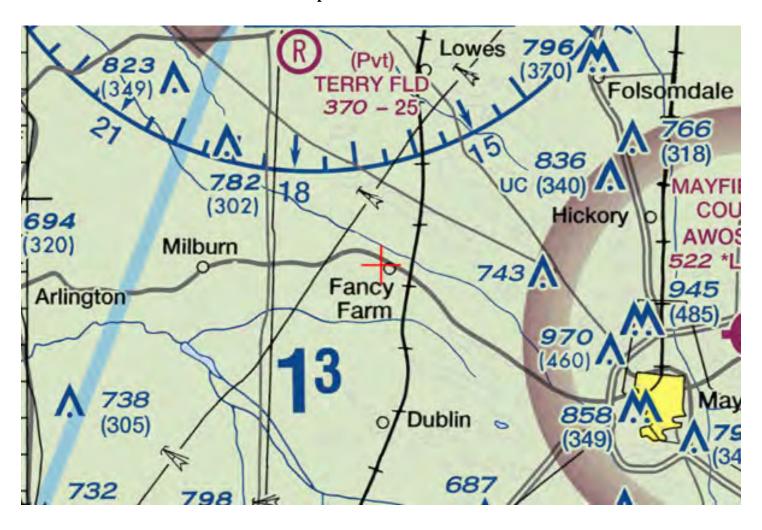
The FAA 5G C band compatibility evaluation is a data analytics system used by FAA to evaluate operational hazards related to aircraft design. The FAA 5G C band compatibility evaluation process refers to the process in which the telecommunication companies and the FAA have set parameters, such as power output, locations, frequencies, and tilt angles for antenna that mitigate the hazard to aviation. As the telecommunication companies and FAA refine the tools and methodology, the allowable frequencies and power levels may change in the FAA 5G C band compatibility evaluation process. Therefore, your proposal will not have a substantial adverse effect on the safe and efficient use of the navigable airspace by aircraft provided the equipment and emissions are in compliance with the parameters established through the FAA 5G C band compatibility evaluation process.

Any future changes that are not consistent with the parameters listed in the FAA 5G C band compatibility evaluation process will void this determination of no hazard.

FREQUENCY   FREQUENCY   UNIT   ERP   UNIT	LOW	HIGH	FREQUENCY		ERP
6 7 GHz 42 dBW 10 11.7 GHz 55 dBW 10 11.7 GHz 42 dBW 17.7 19.7 GHz 55 dBW 17.7 19.7 GHz 55 dBW 17.7 19.7 GHz 42 dBW 21.2 23.6 GHz 42 dBW 21.2 23.6 GHz 55 dBW 21.2 23.6 GHz 42 dBW 21.2 23.6 GHz 42 dBW 614 698 MHz 2000 W 614 698 MHz 1000 W 698 806 MHz 1000 W 806 824 MHz 500 W 806 901 MHz 500 W 824 849 MHz 500 W 851 866 MHz 500 W 851 866 MHz 500 W 851 866 MHz 500 W 869 894 MHz 500 W 879 932 MHz 7 W 899 932 MHz 7 W 901 902 MHz 3500 W 901 902 MHz 7 W 901 902 MHz 3500 W 901 901 MHz 3500 W 901 902 MHz 3500 W 901 900 MHz 1000 W 901 901 MHz 500 W 901 902 MHz 1000 W 901 903 903 MHz 1000 W 904 904 MHz 1000 W 905 905 MHz 1000 W 906 906 MHz 1000 W 907 907 MHz 1000 W 908 908 MHz 1000 W 909 MHz 1000 MHz 1000 W 909 MHz 1000 MHz 10000 MHz 1000 MHz 10000 MHz 1000 MHz 10000 MHz 10000 MHz 10000 MHz 10000 MHz 10000 MHz 10000 M	FREQUENCY	FREQUENCY	UNIT	ERP	UNIT
6 7 GHz 42 dBW 10 11.7 GHz 55 dBW 10 11.7 GHz 42 dBW 17.7 19.7 GHz 55 dBW 17.7 19.7 GHz 55 dBW 17.7 19.7 GHz 42 dBW 21.2 23.6 GHz 42 dBW 21.2 23.6 GHz 55 dBW 21.2 23.6 GHz 42 dBW 21.2 23.6 GHz 42 dBW 614 698 MHz 2000 W 614 698 MHz 1000 W 698 806 MHz 1000 W 806 824 MHz 500 W 806 901 MHz 500 W 824 849 MHz 500 W 851 866 MHz 500 W 851 866 MHz 500 W 851 866 MHz 500 W 869 894 MHz 500 W 879 932 MHz 7 W 899 932 MHz 7 W 901 902 MHz 3500 W 901 902 MHz 7 W 901 902 MHz 3500 W 901 901 MHz 3500 W 901 902 MHz 3500 W 901 900 MHz 1000 W 901 901 MHz 500 W 901 902 MHz 1000 W 901 903 903 MHz 1000 W 904 904 MHz 1000 W 905 905 MHz 1000 W 906 906 MHz 1000 W 907 907 MHz 1000 W 908 908 MHz 1000 W 909 MHz 1000 MHz 1000 W 909 MHz 1000 MHz 10000 MHz 1000 MHz 10000 MHz 1000 MHz 10000 MHz 10000 MHz 10000 MHz 10000 MHz 10000 MHz 10000 M	_	_			
10         11.7         GHz         55         dBW           10         11.7         GHz         42         dBW           17.7         19.7         GHz         42         dBW           17.7         19.7         GHz         42         dBW           21.2         23.6         GHz         55         dBW           21.2         23.6         GHz         42         dBW           614         698         MHz         2000         W           614         698         MHz         1000         W           698         806         MHz         1000         W           806         824         MHz         1000         W           806         824         MHz         500         W           806         901         MHz         500         W           824         849         MHz         500         W           851         866         MHz         500         W           869         894         MHz         500         W           896         901         MHz         500         W           929         932         MHz					
10 11.7 GHz 42 dBW 17.7 19.7 GHz 55 dBW 17.7 19.7 GHz 42 dBW 21.2 23.6 GHz 55 dBW 21.2 23.6 GHz 55 dBW 21.2 23.6 GHz 42 dBW 614 698 MHz 2000 W 614 698 MHz 1000 W 698 806 824 MHz 1000 W 806 824 MHz 500 W 8806 991 MHz 500 W 8851 866 MHz 500 W 8851 866 MHz 500 W 8851 866 MHz 500 W 8869 894 MHz 500 W 8860 901 MHz 500 W 8860 901 MHz 500 W 8860 MHz 500 W 8860 MHz 500 W 8860 MHz 500 W 8851 866 MHz 500 W 8860 MHz 500 W 8660 MHz 500 MHz 500 W 8660 MHz 500 MHz 500 MHz 500 W 8660 MHz 500 MHz					
17.7					
17.7         19.7         GHz         42         dBW           21.2         23.6         GHz         55         dBW           614         698         MHz         2000         W           614         698         MHz         1000         W           614         698         MHz         1000         W           698         806         MHz         1000         W           806         824         MHz         500         W           806         824         MHz         500         W           806         901         MHz         500         W           824         849         MHz         500         W           851         866         MHz         500         W           869         894         MHz         500         W           896         901         MHz         500         W           901         902         MHz         7         W           930         931         MHz         3500         W           931         932         MHz         3500         W           932         932.5         MHz					
21.2         23.6         GHz         55         dBW           21.2         23.6         GHz         42         dBW           614         698         MHz         2000         W           614         698         MHz         1000         W           698         806         MHz         1000         W           806         824         MHz         500         W           806         901         MHz         500         W           806         901         MHz         500         W           824         849         MHz         500         W           851         866         MHz         500         W           869         894         MHz         500         W           896         901         MHz         500         W           901         902         MHz         7         W           930         931         MHz         3500         W           931         932         MHz         3500         W           932         932.5         MHz         17         dBW           940         MHz         1000					
21.2         23.6         GHz         42         dBW           614         698         MHz         2000         W           614         698         MHz         1000         W           614         698         MHz         1000         W           698         806         MHz         1000         W           806         824         MHz         500         W           806         901         MHz         500         W           824         849         MHz         500         W           851         866         MHz         500         W           869         894         MHz         500         W           896         901         MHz         7         W           929         932         MHz         7         W           930         931         MHz         3500         W           931         932         MHz         3500         W           932         932.5         MHz         17         dBW           935         940         MHz         1000         W           940         941         MHz <td< td=""><td></td><td></td><td></td><td></td><td></td></td<>					
614 698 MHz 2000 W 614 698 MHz 1000 W 698 806 MHz 1000 W 806 824 MHz 1000 W 806 824 MHz 500 W 806 901 MHz 500 W 824 849 MHz 500 W 851 866 MHz 500 W 8896 901 MHz 500 W 896 901 MHz 500 W 897 8984 MHz 500 W 8986 901 MHz 500 W 898 901 MHz 77 W 901 902 MHz 7 W 929 932 MHz 3500 W 931 MHz 3500 W 932 932.5 MHz 3500 W 932 932.5 MHz 17 dBW 935 940 MHz 17 dBW 940 941 MHz 1000 W 940 941 MHz 500 W 1670 1675 MHz 500 W 1710 1755 MHz 500 W 1850 1910 MHz 1640 W 1850 1990 MHz 1640 W 1930 1990 MHz 1640					
614 698 MHz 1000 W 698 806 MHz 1000 W 806 824 MHz 500 W 806 901 MHz 500 W 824 849 MHz 500 W 824 849 MHz 500 W 851 866 MHz 500 W 869 894 MHz 500 W 896 901 MHz 500 W 896 901 MHz 500 W 896 901 MHz 500 W 901 902 MHz 7 W 929 932 MHz 3500 W 931 MHz 3500 W 931 932 MHz 3500 W 931 932 MHz 3500 W 932 932.5 MHz 3500 W 935 940 MHz 17 dBW 940 941 MHz 3500 W 1670 1675 MHz 500 W 1710 1755 MHz 500 W 1850 1910 MHz 1640 W 1850 1990 MHz 1640 W 1850 1990 MHz 1640 W 1990 2025 MHz 500 W 2110 2200 MHz 500 W 2305 2310 MHz 500 W 2315 2360 MHz 500 W 2496 2690 MHz 2000 W 2496 2690 MHz 2000 W 257500 28350 MHz 500 W 25100 29250 MHz 500 W 25100 29250 MHz 500 W 25100 3880 MHz 500 W 25100 3880 MHz 500 W 25100 3880 MHz 5500 W 25100 3880 MHz 5500 W 25100 29250 MHz 5500 W 25100 3880 MHz 5500 W 25100 3880 MHz 5500 W 25100 3880 MHz 5500 W 25100 29250 MHz 5500 W 25100 29250 MHz 5500 W 25100 3880 MHz 5500 W 25100 3880 MHz 5500 W 25100 29250 MHz 5500 W 25100 29250 MHz 5500 W 25100 29250 MHz 5500 W 25100 3880 MHz 5500 W 25100 3980 MHz 5500 W 25100 29250 MHz 5500 W 25100 5500 MHz 5500 W 25100 5500 MHz 5500 MHz 5500 W 25100 5500 MHz 5500					
698         806         MHz         1000         W           806         824         MHz         500         W           806         901         MHz         500         W           824         849         MHz         500         W           851         866         MHz         500         W           869         894         MHz         500         W           896         901         MHz         500         W           901         902         MHz         7         W           929         932         MHz         3500         W           930         931         MHz         3500         W           931         932         MHz         3500         W           931         932         MHz         3500         W           932         932.5         MHz         17         dBW           935         940         MHz         1000         W           940         941         MHz         3500         W           1670         1675         MHz         500         W           1850         1910         MHz					
806         824         MHz         500         W           806         901         MHz         500         W           824         849         MHz         500         W           851         866         MHz         500         W           869         894         MHz         500         W           896         901         MHz         500         W           901         902         MHz         7         W           929         932         MHz         3500         W           930         931         MHz         3500         W           931         932         MHz         3500         W           932         932.5         MHz         17         dBW           935         940         MHz         1000         W           940         941         MHz         3500         W           1670         1675         MHz         500         W           1710         1755         MHz         500         W           1850         1910         MHz         1640         W           1850         1990         MHz			MHz		
806         901         MHz         500         W           824         849         MHz         500         W           851         866         MHz         500         W           869         894         MHz         500         W           896         901         MHz         500         W           991         902         MHz         7         W           929         932         MHz         3500         W           930         931         MHz         3500         W           931         932         MHz         3500         W           932         932.5         MHz         17         dBW           935         940         MHz         1000         W           940         941         MHz         3500         W           1670         1675         MHz         500         W           1710         1755         MHz         500         W           1850         1910         MHz         1640         W           1930         1990         MHz         1640         W           1930         1990         MHz	698	806	MHz	1000	W
824       849       MHz       500       W         851       866       MHz       500       W         869       894       MHz       500       W         896       901       MHz       500       W         901       902       MHz       7       W         929       932       MHz       3500       W         930       931       MHz       3500       W         931       932       MHz       3500       W         932       932.5       MHz       17       dBW         935       940       MHz       1000       W         940       941       MHz       3500       W         1670       1675       MHz       500       W         1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1880       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2305       2310       MHz       2000       W	806	824	MHz	500	W
851       866       MHz       500       W         869       894       MHz       500       W         896       901       MHz       500       W         901       902       MHz       7       W         929       932       MHz       3500       W         930       931       MHz       3500       W         931       932       MHz       3500       W         932       932.5       MHz       17       dBW         935       940       MHz       1000       W         940       941       MHz       3500       W         1670       1675       MHz       500       W         1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       MHz       500       W         2310       200       MHz       500       W         2305       2310       MHz       2000       W	806	901	MHz	500	W
869       894       MHz       500       W         896       901       MHz       500       W         901       902       MHz       7       W         929       932       MHz       3500       W         930       931       MHz       3500       W         931       932       MHz       3500       W         932       932.5       MHz       17       dBW         935       940       MHz       1000       W         940       941       MHz       3500       W         1670       1675       MHz       500       W         1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2345       2360       MHz       2000       W	824	849	MHz	500	W
896       901       MHz       7       W         901       902       MHz       7       W         929       932       MHz       3500       W         930       931       MHz       3500       W         931       932       MHz       3500       W         932       932.5       MHz       17       dBW         935       940       MHz       1000       W         940       941       MHz       1000       W         1670       1675       MHz       500       W         1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1930       1990       MHz       1640       W         1990       MHz       1640       W         1990       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       47       dBm         3700	851	866	MHz	500	W
901         902         MHz         7         W           929         932         MHz         3500         W           930         931         MHz         3500         W           931         932         MHz         3500         W           932         932.5         MHz         17         dBW           935         940         MHz         1000         W           940         941         MHz         3500         W           1670         1675         MHz         500         W           1710         1755         MHz         500         W           1850         1910         MHz         1640         W           1850         1990         MHz         1640         W           1930         1990         MHz         1640         W           1990         2025         MHz         500         W           2305         2310         MHz         500         W           2305         2360         MHz         2000         W           2345         2360         MHz         500         W           2496         2690 <td< td=""><td>869</td><td>894</td><td>MHz</td><td>500</td><td>W</td></td<>	869	894	MHz	500	W
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931       932       MHz       3500       W         932       932.5       MHz       17       dBW         935       940       MHz       1000       W         940       941       MHz       3500       W         1670       1675       MHz       500       W         1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75	929	932	MHz	3500	W
932         932.5         MHz         17         dBW           935         940         MHz         1000         W           940         941         MHz         3500         W           1670         1675         MHz         500         W           1710         1755         MHz         500         W           1850         1910         MHz         1640         W           1850         1990         MHz         1640         W           1930         1990         MHz         1640         W           1990         2025         MHz         500         W           2110         2200         MHz         500         W           2305         2310         MHz         2000         W           2305         2360         MHz         2000         W           2345         2360         MHz         500         W           2496         2690         MHz         500         W           3550         3700         MHz         47         dBm           3700         3980         MHz         3280         W           27500         28350	930	931	MHz	3500	W
935         940         MHz         1000         W           940         941         MHz         3500         W           1670         1675         MHz         500         W           1710         1755         MHz         500         W           1850         1910         MHz         1640         W           1850         1990         MHz         1640         W           1930         1990         MHz         1640         W           1990         2025         MHz         500         W           2110         2200         MHz         500         W           2305         2310         MHz         2000         W           2305         2360         MHz         2000         W           2345         2360         MHz         2000         W           2496         2690         MHz         500         W           3550         3700         MHz         47         dBm           3700         3980         MHz         3280         W           27500         28350         MHz         75         dBm           31000         31300 </td <td>931</td> <td>932</td> <td>MHz</td> <td>3500</td> <td>W</td>	931	932	MHz	3500	W
940       941       MHz       3500       W         1670       1675       MHz       500       W         1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         31000       31300       MHz       75       dBm	932	932.5	MHz	17	dBW
1670       1675       MHz       500       W         1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         31000       31300       MHz       75       dBm	935	940	MHz	1000	W
1710       1755       MHz       500       W         1850       1910       MHz       1640       W         1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	940	941	MHz	3500	W
1850       1910       MHz       1640       W         1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         31000       31300       MHz       75       dBm	1670	1675	MHz	500	W
1850       1990       MHz       1640       W         1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	1710	1755	MHz	500	W
1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	1850	1910	MHz	1640	W
1930       1990       MHz       1640       W         1990       2025       MHz       500       W         2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	1850	1990	MHz	1640	W
2110       2200       MHz       500       W         2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	1930	1990	MHz	1640	
2305       2310       MHz       2000       W         2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	1990	2025	MHz	500	W
2305       2360       MHz       2000       W         2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	2110	2200	MHz	500	W
2345       2360       MHz       2000       W         2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	2305	2310	MHz	2000	W
2496       2690       MHz       500       W         3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	2305	2360	MHz	2000	W
3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	2345	2360	MHz	2000	W
3550       3700       MHz       47       dBm         3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm					W
3700       3980       MHz       3280       W         27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm	3550				
27500       28350       MHz       75       dBm         29100       29250       MHz       75       dBm         31000       31300       MHz       75       dBm					
29100 29250 MHz 75 dBm 31000 31300 MHz 75 dBm					
31000 31300 MHz 75 dBm					

# $TOPO\ Map\ for\ ASN\ 2022\text{-}ASO\text{-}27278\text{-}OE$







« OE/AAA

#### Circle Search for Cases Results Form 7460-1 for ASN 2022-ASO-27278-OE

Overview

Study (ASN): 2022-ASO-27278-OE

**Prior Study:** 

Status: Determined

Letters: Determination

Supplemental Form 7460-2: Please login to add a Supplemental Form 7460-2.

**Completion Date:** 01/06/2023 **Expiration Date:** 07/06/2024

07/13/2022

07/13/2022

Map: View Map

#### **Sponsor Information**

Sponsor: Cellco Partnership
Attention Of: Network Regulatory
Address: 5055 North Point Pkwy
Address2: NP2NE Network Engineering

City: Alpharetta
State: GA
Postal Code: 30022
Country: US

**Phone:** 210-488-2623 **Fax:** 770-797-1034

,,,,,

## Sponsor's Representative Information

Representative:

Received Date:

Entered Date:

Attention Of: Network Regulatory
Address: 5055 North Point Pkwy
Address2: NP2NE Network Engineering

City: Alpharetta
State: GA
Postal Code: 30022
Country: US

**Phone:** 770-797-1070

Fax:

#### **Construction Info**

Notice Of: CONSTR

**Duration:** PERM (Months: 0 Days: 0)

Work Schedule:

Date Built:

## **Structure Summary**

Structure Type: Antenna Tower

Structure Name: EV Fancy Farm (16207023)

FCC Number: 1324370

FCC ASR Registration

## Structure Details

Latitude (NAD 83):	36° 48' 09.61" N
Longitude (NAD 83):	88° 47' 54.21" W
Horizontal Datum:	NAD 83
Survey Accuracy:	1A
Marking/Lighting:	Dual-red and medium intensit

Other Description:

Current Marking/Lighting: N/A Proposed Structure
Current Marking/Lighting Other Description:

Fancy Farm

63732.99 feet

N of State Rt 80 between Carrico Rd and Elder Rd

Proposing new 295 ft lattice tower owned by Sponsor of this study. Emissions from this site will adhere to parameters set by collaboration between the FAA and telecomm companies and reflected in the FAA 5G C-Band compatibility evaluation process. Questions to juliane.madsen@vzw.com

Graves

M25

No

280.94°

Name:
City:
State:
Nearest County:
Nearest Airport:
Distance to Structure:
On Airport:

Direction to Structure:
Description of Location:
Description of Proposal:

Description of Proposal:

## **Height and Elevation**

	Proposed	DNE	DET
Site Elevation:	431		
Structure Height:	295	0	295
Total Height (AMSL):	726	0	726

#### Frequencies

rrequenc	ies				
Low Freq	High Freq	Unit	ERP	Unit	
6	7	GHz	55	dBW	
6	7	GHz	42	dBW	
10	11.7	GHz	55	dBW	
10	11.7	GHz	42	dBW	
17.7	19.7	GHz	55	dBW	
17.7	19.7	GHz	42	dBW	
21.2	23.6	GHz	55	dBW	
21.2	23.6	GHz	42	dBW	
614	698	MHz	2000	W	
614	698	MHz	1000	W	
698	806	MHz	1000	W	
806	824	MHz	500	W	
806	901	MHz	500	W	
824	849	MHz	500	W	
851	866	MHz	500	W	
869	894	MHz	500	W	
896	901	MHz	500	W	
901	902	MHz	7	W	
929	932	MHz	3500	W	
930	931	MHz	3500	W	
931	932	MHz	3500	W	
932	932.5	MHz	17	dBW	
935	940	MHz	1000	W	
940	941	MHz	3500	W	

1670	1675	MHz	500	w
1710	1755	MHz	500	w
1850	1910	MHz	1640	W
1850	1990	MHz	1640	W
1930	1990	MHz	1640	w
1990	2025	MHz	500	W
2110	2200	MHz	500	W
2305	2310	MHz	2000	W
2305	2360	MHz	2000	W
2345	2360	MHz	2000	W
2496	2690	MHz	500	W
3550	3700	MHz	47	dBm
3700	3980	MHz	3280	W
27500	28350	MHz	75	dBm
29100	29250	MHz	75	dBm
31000	31300	MHz	75	dBm
38600	40000	MHz	75	dBm

Previous Back to Search Result

Next



## KENTUCKY TRANSPORTATION CABINET

TC 55-2 Rev. 06/2020

## KENTUCKY AIRPORT ZONING COMMISSION

Page 2 of 2

APPLIC	ATION FOR	PERMIT TO COM	<b>NSTRUCT OR AL</b>	TER A STRU	CTURE
APPLICANT (name)		PHONE	FAX		TICAL STUDY #
Vertical Bridge REIT, LLC		561-406-4015	,		THE STORY IF
ADDRESS (street)		CITY		STATE	ZIP
750 Park of Commerce Dr	rive, Suite 200	Boca Raton		FL	33487
APPLICANT'S REPRESENT			FAX		
Gretchen Blanton		704-472-0374			•
ADDRESS (street)		CITY	· · · · · · · · · · · · · · · · · · ·	STATE	ZIP
750 Park of Commerce Dr	rive, Suite 200	Boca Raton		FL	33487
APPLICATION FOR 🔀	New Construct	ion Alteration	Existing	WORK SCHED	
<b>DURATION</b> Nermar	ient 🔲 Tem	porary (months	days )	Start E	nd
	Building	MARKING/PAINTIN	G/LIGHTING PREFE	RRED	
Antenna Tower		Red Lights & Pai	nt White- med	ium intensity	White- high intensity
Power Line Wate	er Tank	🔀 Dual- red & med	lium intensity white		& high intensity white
Landfill Othe	∍r	Other		<del>_</del>	,
LATITUDE		LONGITUDE		DATUM 🛛	NAD83 NAD27
36°48'09.61"		-88 <sup>0</sup> 47′54.21″		Other	
NEAREST KENTUCKY		<b>NEAREST KENTUCK</b>	Y PUBLIC USE OR M	LITARY AIRPO	RT
City Fancy Farm County G		M25 Mayfield Grave	es County		
SITE ELEVATION (AMSL, fo	eet)	TOTAL STRUCTURE	HEIGHT (AGL, feet)	CURRENT (FA	A aeronautical study #)
431		295		2022-ASO-272	
OVERALL HEIGHT (site ele	vation plus tot	al structure height, j	feet)	PREVIOUS (FA	A aeronautical study #)
726		· · · · · · · · · · · · · · · · · · ·			
DISTANCE (from nearest h	(entucky public	use or Military airpo	ort to structure)	PREVIOUS (K)	' aeronautical study #)
10.48 Nautical Miles		**			
DIRECTION (from nearest	Kentucky publi	ic use or Military airp	oort to structure)		
WNW			· · · · · · · · · · · · · · · · · · ·		
DESCRIPTION OF LOCATION	<b>ON</b> (Attach USC	3S 7.5 minute quadro	angle map or an airp	ort layout drai	ving with the precise site
marked and any certified	survey.)				
See attached					
DESCRIPTION OF PROPOS					
DESCRIPTION OF PROPOS 295' AGL Lattice Cell Towe	· · · -				
295 AGL Lattice Cell Towe	ar ar				
<b></b>					····
FAA Form 7460-1 (Has the	e "Notice of Co	nstruction or Alterat	ion" been filed with	the Federal Avi	ation Administration?)
		termined 01/06/202			
CERTIFICATION (I hereby	certify that all t	the above entries, m	ade by me, are true,	complete, and	correct to the best of
my knowledge and belief.	,				
PENALITIES (Persons failing	ig to comply wi	th KRS 183.861 to 18	83.990 and 602 KAR	050 are liable	for fines and/or
imprisonment as set forth			with FAA regulation		further penalties.)
I 1"	ITLE	SIGNATURE	Alanto 1	DATE	
Gretchen Blanton P	roject Manage	r Gretzbeni	- Junior	06/07/2023	
COMMISSION ACTION		Chairperson,			
		Administrato	or, KAZC	•	
Approved Si	IGNATURE			DATE	•
Disapproved					

**Exhibit H** 

Date: March 23, 2022 POD Job Number: 20-64965

## **GEOTECHNICAL REPORT**

## **EV FANCY FARM**

36° 48′ 09.611674″ N 88° 47′ 54.207499″ W

Kentucky Highway 80, Fancy Farm, KY 42039

Prepared For:



Prepared By:





March 23, 2022

Ms. Jackie Straight Verizon Wireless 2902 Ring Road Elizabethtown, KY 42701

Re: Geotechnical Report – PROPOSED 290' SELF-SUPPORT TOWER w/ 5' LIGHTNING ARRESTOR

Site Name: EV FANCY FARM

Site Address: Kentucky Highway 80, Fancy Farm, Graves County, Kentucky

Coordinates: N36° 48′ 09.611674″, W88° 47′ 54.207499″

POD Project No. 20-64965

Dear Ms. Straight:

Attached is our geotechnical engineering report for the referenced project. This report contains our findings, an engineering interpretation of these findings with respect to the available project characteristics, and recommendations to aid design and construction of the tower and equipment support foundations.

We appreciate the opportunity to be of service to you on this project. If you have any questions regarding this report, please contact our office.

MANAMANAN

Cordially,

Mark Patterson, P.E. Project Engineer

License No.: KY 16300

Copies submitted: (3) Ms. Jackie Straight

EV FANCY FARM March 23, 2022

## **LETTER OF TRANSMITTAL**

## **TABLE OF CONTENTS**

			<u>Page</u>
1.	PUR	POSE AND SCOPE	1
2.	PRO	JECT CHARACTERISTICS	1
3.	SUB	SURFACE CONDITIONS	1
4.	FOU	NDATION DESIGN RECOMMENDATIONS	2
	4.1.	Proposed Tower	2
	4.1.1	1. Drilled Piers	3
	4.1.2	2. Mat Foundation	3
	4.2.	EQUIPMENT PLATFORM	4
	4.3.	EQUIPMENT SLAB	4
	4.4.	EQUIPMENT BUILDING	4
	4.5.	Drainage and Groundwater Considerations	5
5.	GEN	ERAL CONSTRUCTION PROCEDURES AND RECOMMENDATIONS	5
	5.1	Drilled Piers	5
	5.2	FILL COMPACTION	6
	5.3	CONSTRUCTION DEWATERING	6
6	FIEL	D INVESTIGATION	7
7	WAF	RRANTY AND LIMITATIONS OF STUDY	7

## **APPENDIX**

BORING LOCATION PLAN BORING LOGS SOIL SAMPLE CLASSIFICATION

EV FANCY FARM March 23, 2022

Geotechnical Report

PROPOSED 290' SELF-SUPPORT TOWER w/ 5' LIGHTNING ARRESTOR

Site Name: EV FANCY FARM

Kentucky Highway 80, Fancy Farm, Graves County, Kentucky

N36° 48′ 09.611674″, W88° 47′ 54.207499″

1. PURPOSE AND SCOPE

The purpose of this study was to determine the general subsurface conditions at the site of the proposed tower by

drilling three borings and to evaluate this data with respect to foundation concept and design for the proposed

tower and shelter. Also included is an evaluation of the site with respect to potential construction problems and

recommendations dealing with quality control during construction.

2. PROJECT CHARACTERISTICS

Verizon Wireless is proposing to construct a self-support tower and either an equipment shelter, slab, or platform

at N36° 48' 09.611674", W88° 47' 54.207499", Kentucky Highway 80, Fancy Farm, Graves County, Kentucky. The

site is located in a farm field next to a water tower on Kentucky Highway 80 on the northwest side of Fancy Farm.

The proposed lease area will be 10,000 square feet and will be accessed along an existing gravel road from

Kentucky Highway 80 north to the proposed lease area. The elevation at the proposed tower location is about EL

431 and there is about 4-feet of change in elevation across the proposed lease area. The development will also

include a small equipment shelter near the base of the tower. The proposed tower location is shown on the Boring

Location Plan in the Appendix.

3. SUBSURFACE CONDITIONS

The subsurface conditions were explored by drilling three test borings near the base of the proposed tower. The

Geotechnical Soil Test Boring Logs, which are included in the Appendix, describes the materials and conditions

encountered. A sheet defining the terms and symbols used on the boring logs is also included in the Appendix. The

general subsurface conditions disclosed by the test borings are discussed in the following paragraphs.

According to the Kentucky Geological Survey, Kentucky Geologic Map Information Services, the site is underlain by

Loess and the Roxanal Silt with clay, sand, and chert. There is no karst activity in this area.

The borings encountered between 7 and 8 inches of topsoil at the existing ground surface. Below the topsoil, the

borings encountered clayey silt (ML) of low plasticity. The SPT N-values in the silty soil were between 3 to 32 blows per

foot (bpf) that increased with depth generally indicating a soft to hard consistency. Samples with more than 40 percent

1

EV FANCY FARM March 23, 2022

chert fragments were much higher. Between 18.5 and 22 feet, the borings encountered very fine sand (SP) that was

medium to very dense with SPT N-values ranging from 24 to 62. The borings were terminated at the scheduled depths

of 20 and 40 feet in the fine sand.

Observations made at the completion of soil drilling operations indicated the boring to be dry. It must be noted,

however, that short-term water readings in test borings are not necessarily a reliable indication of the actual

groundwater level. Furthermore, it must be emphasized that the groundwater level is not stationary but will fluctuate

seasonally.

Based on the limited subsurface conditions encountered at the site and using Table 1615.1.1 of the 2018 Kentucky

Building Code, the site class is considered "C". Seismic design requirements for telecommunication towers are given in

section 1622 of the code. A detailed seismic study was beyond the scope of this report.

4. FOUNDATION DESIGN RECOMMENDATIONS

The following design recommendations are based on the previously described project information, the subsurface

conditions encountered in our borings, the results of our laboratory testing, empirical correlations for the soil

types encountered, our analyses, and our experience. If there is any change in the project criteria or structure

location, you should retain us to review our recommendations so that we can determine if any modifications are

required. The findings of such a review can then be presented in a supplemental report or addendum.

We recommend that the geotechnical engineer be retained to review the near-final project plans and

specifications, pertaining to the geotechnical aspects of the project, prior to bidding and construction. We

recommend this review to check that our assumptions and evaluations are appropriate based on the current

project information provided to us, and to check that our foundation and earthwork recommendations were

properly interpreted and implemented.

4.1. Proposed Tower

Our findings indicate that the proposed self-support can be supported on drilled piers or on a common mat foundation.

2

Geotechnical Report EV FANCY FARM March 23, 2022

## 4.1.1. Drilled Piers

The following table summarizes the recommended values for use in analyzing lateral and frictional resistance for the various strata encountered at the test boring. It is important to note that these values are estimated based on the standard penetration test results and soil types and were not directly measured. The all values provided are ultimate values and appropriate factors of safety should be used in conjunction with these values. If the piers will bear deeper than about 40 feet, a deeper boring should be drilled to determine the nature of the deeper material.

Depth Below Ground Surface,	0-3	3 - 13	13 - 20	20 - 40
feet				
Ultimate Bearing Pressure (psf)		8,300	11,000	16,500
С	500	1500	2000	0
Undrained Shear Strength, psf				
Ø	0	0	0	32°
Angle of Internal Friction degrees				
Total Unit Weight, pcf	110	120	120	90
Soil Modulus Parameter	30	750	750	90
k, pci				
Passive Soil Pressure,		1,000 +	1,340 +	360 (D <sup>2</sup> )
psf/one foot of depth		40(D-3)	40(D-13)	
Side Friction, psf	100	400	500	2,500

Note: D = Depth below ground surface (in feet) to point at which the passive pressure is calculated.

It is important that the drilled piers be installed by an experienced, competent drilled pier contractor who will be responsible for properly installing the piers in accordance with industry standards nd generally accepted methods, without causing deterioration of the subgrade. The recommendations contained herein relate only to the soil-pier interaction and do not account for the structural design of the piers.

#### 4.1.2. Mat Foundation

The tower could be supported on a common mat foundation bearing on the clay at a minimum of 4 feet can be designed using an allowable soil pressure of 3,500 pounds per square foot may be used. This value may be increased by 30 percent for the maximum edge pressure under transient loads. A friction value of 0.30 may be used between the

EV FANCY FARM March 23, 2022

concrete and the silt soil. The passive pressures given for the drilled pier foundation may be used to resist lateral

forces.

It is important that the mat be designed with an adequate factor of safety with regard to overturning under the

maximum design wind load.

4.2. Equipment Platform

An equipment platform may be supported on shallow piers bearing in the natural clay and designed for a net allowable

soil pressure of 2,500 pounds per square foot. The piers should bear at a depth of at least 30 inches to minimize the

effects of frost action. All existing topsoil or soft natural soil should be removed beneath footings.

4.3. Equipment Slab

A concrete slab supporting the equipment must be supported on at least 6-inch layer of relatively clean granular

material such as gravel or crushed stone containing not more than 10 percent material that passes through a No. 4

sieve. This is to help distribute concentrated loads and equalize moisture conditions beneath the slab. Provided

that a minimum of 6 in. of granular material is placed below the slab, a modulus of subgrade reaction (k30) of 110

lbs/cu.in. can be used for design of the slab. All existing topsoil or soft natural soil should be removed beneath

crushed stone layer.

4.4. Equipment Building

If an equipment building support on a slab is chosen in place of the equipment platform, it may be supported on

shallow spread footings bearing in the natural clay soil and designed for a net allowable soil pressure of 2,500 pounds

per square foot.

The footings should be at least ten inches wide. If the footings bear on soil, they should bear at a depth of at least 30

inches to minimize the effects of frost action. All existing topsoil or soft natural soil should be removed beneath

footings.

The floor slab for the new equipment building can be supported on firm natural soils or on new compacted

structural fill. Floor slabs must be supported on at least 4-inch layer of relatively clean granular material such as

gravel or crushed stone containing not more than 10 percent material that passes through a No. 4 sieve. This is to

help distribute concentrated loads and equalize moisture conditions beneath the slab. Provided that a minimum of

4

Geotechnical Report EV FANCY FARM March 23, 2022

4 in. of granular material is placed below the slab, a modulus of subgrade reaction (k30) of 110 lbs/cu.in. can be used for design of the floor slabs.

#### 4.5. Drainage and Groundwater Considerations

Good site drainage must be provided. Surface run-off water should be drained away from the tower and platform and not allowed to pond. It is recommended that all foundation concrete be placed the same day the excavation is made.

At the time of this investigation, groundwater was not encountered and no special provisions regarding groundwater control are considered necessary for shallow foundations. Any seepage should be able to be pumped with sumps.

#### 5. GENERAL CONSTRUCTION PROCEDURES AND RECOMMENDATIONS

It is possible that variations in subsurface conditions will be encountered during construction. Although only minor variations that can be readily evaluated and adjusted for during construction are anticipated, it is recommended the geotechnical engineer, or a qualified representative be retained to perform continuous inspection and review during construction of the soils-related phases of the work. This will permit correlation between the test boring data and the actual soil conditions encountered during construction.

#### 5.1 Drilled Piers

The following recommendations are recommended for drilled pier construction:

- All piers must be poured the same day drilling is completed so that any shale is not allowed to swell. Clean the foundation bearing area so it is nearly level or suitably benched and is free of ponded water or loose material.
- Make provisions for ground water removal from the drilled shaft excavation. While the borings were dry at completion and significant seepage is not anticipated, the drilled pier contractor should have pumps on hand to remove water in the event seepage into the drilled pier is encountered.
- Specify concrete slumps ranging from 4 to 7 inches for the drilled shaft construction. These slumps are recommended to fill irregularities along the sides and bottom of the drilled hole, displace water as it is placed, and permit placement of reinforcing cages into the fluid concrete.
- Retain the geotechnical engineer to observe foundation excavations after the bottom of the hole is leveled, cleaned of any mud or extraneous material, and dewatered.

Geotechnical Report EV FANCY FARM March 23, 2022

■ Install a temporary protective steel casing to prevent side wall collapse, prevent excessive mud

and water intrusion in the drilled shaft.

The protective steel casing may be extracted as the concrete is placed provided a sufficient head of concrete is maintained inside the steel casing to prevent soil or water intrusion into the newly

placed concrete.

Direct the concrete placement into the drilled hole through a centering chute to reduce side flow

or segregation.

5.2 Fill Compaction

All engineered fill placed adjacent to and above the tower foundation should be compacted to a dry density of at

least 95 percent of the standard Proctor maximum dry density (ASTM D-698). This minimum compaction

requirement should be increased to 98 percent for any fill placed below the tower foundation bearing elevation.

Any fill placed beneath the tower foundation should be limited to well-graded sand and gravel or crushed stone.

The compaction should be accomplished by placing the fill in about 8 inch (or less) loose lifts and mechanically

compacting each lift to at least the specified minimum dry density. Field density tests should be performed on

each lift as necessary to ensure that adequate moisture conditioning and compaction is being achieved.

Compaction by flooding is not considered acceptable. This method will generally not achieve the desired

compaction and the large quantities of water will tend to soften the foundation soils.

5.3 Construction Dewatering

If groundwater is encountered in the shallow foundations, it should be minor and can be handled by conventional

dewatering methods such as pumping from sumps.

If groundwater is encountered in the drilled pier excavations, it may be more difficult since pumping directly from

the excavations could cause a deterioration of the bottom of the excavation. If the pier excavations are not

dewatered, concrete should be placed by the tremie method. If groundwater sits on the bottom of the

6

foundation for longer than an hour, the bottom should be cleaned again before the pier is poured.

EV FANCY FARM March 23, 2022

## **6 FIELD INVESTIGATION**

Three soil test borings were drilled near the base of the proposed tower. Split-spoon samples were obtained by the Standard Penetration Test (SPT) procedure (ASTM D1586) in all test borings. The borings were terminated at the scheduled depths of 20 and 40 feet. The split-spoon samples were inspected and visually classified by a geotechnical engineer. Representative portions of the soil samples were sealed in plastic bags and returned to our laboratory.

The boring logs are included in the Appendix along with a sheet defining the terms and symbols used on the logs and an explanation of the Standard Penetration Test (SPT) procedure. The logs present visual descriptions of the soil strata encountered, Unified System soil classifications, groundwater observations, sampling information, laboratory test results, and other pertinent field data and observations.

#### 7 WARRANTY AND LIMITATIONS OF STUDY

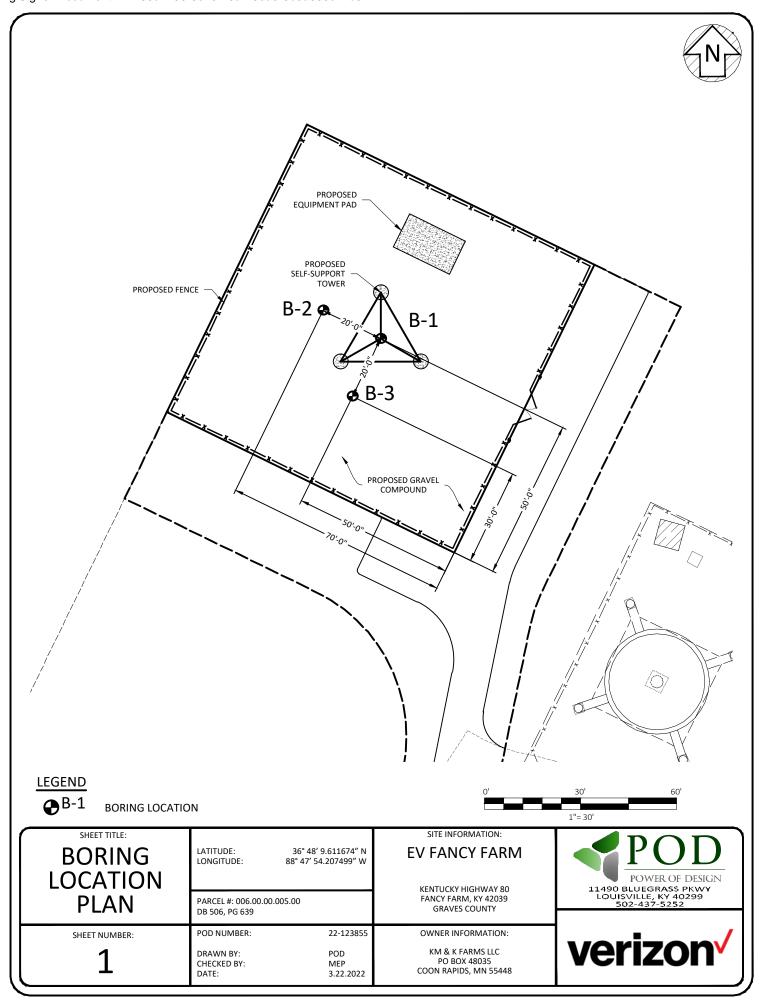
Our professional services have been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties, either express or implied. POD Group is not responsible for the independent conclusions, opinions or recommendations made by others based on the field exploration and laboratory test data presented in this report.

A geotechnical study is inherently limited since the engineering recommendations are developed from information obtained from test borings, which depict subsurface conditions only at the specific locations, times and depths shown on the logs. Soil conditions at other locations may differ from those encountered in the test borings, and the passage of time may cause the soil conditions to change from those described in this report.

The nature and extent of variation and change in the subsurface conditions at the site may not become evident until the course of construction. Construction monitoring by the geotechnical engineer or a representative is therefore considered necessary to verify the subsurface conditions and to check that the soils connected construction phases are properly completed. If significant variations or changes are in evidence, it may then be necessary to reevaluate the recommendations of this report. Furthermore, if the project characteristics are altered significantly from those discussed in this report, if the project information contained in this report is incorrect, or if additional information becomes available, a review must be made by this office to determine if any modification in the recommendations will be required.

## **APPENDIX**

BORING LOCATION PLAN
BORING LOGS
SOIL SAMPLE CLASSIFICATION





# **Boring Log**

Boring: B-1

Page 1 of 1

**Project: EV Fancy Farm** City, State Fancy Farm, KY

H.S.A. Boring Date: **Location: Proposed Tower Center** Method: 10-Mar-22

Section   Commonwealth Drilling   Note: About 7 inches of topsoil were encountered at the ground surface   Commonwealth Drilling   Note: About 7 inches of topsoil were encountered at the ground surface   Commonwealth Drilling   Commonwealth Dri	nside Diameter: 2 1/4" Drill Rig Type					7822 DT (ATV) Hammer Type: Auto												
From   To   Material Description   12.25   15   18   15   18   10   10   10   10   10   10   10	roundwater: DRY							•						· •				
1-2.5   SS   1,	ler: Con	nmonv	vealth Drilling	}	Note: A	Abou	ıt 7 inches	of to	opsoi	l wer	e enc	ountere	d at th	e groun	d surfac	е		
1-2.5   SS   1,			Mater	ial Description			sample Depth ft)	sample Type	-	slows per 5-inch	increment	Recovery (in)	SPT-N value	Sock Quality RQD,%)	Atterberg .imits	Moisture Content (%)	% Fines clay & silt)	Unconfined Compressive
3.0 - stiff, slighty moist, brown and tan mottled							0) (	0,					0,	н )	1		6	
mottled    6-7.5   SS   3, 5, 7   18   12,   17%     12.0   -with fine sand			CLAYEY SILT (N	ИL) - soft, moist, I	brown		1-2.5	SS	1,	1,	2	15	3,			29%		2.
8.5 - 10   SS   5, 7, 8   18   15,   20%     13.5   - very stiff   13.5 - 15   SS   9, 11, 21   18   32,   10%     18.5   - hard with chert fragments   18.5 - 20   SS   25, 24, 14   18   38,   10%     22.0   40.0   SAND (SP) - dense, dry, very fine grained, light gray and tan   23.5 - 25   SS   5, 15, 15   18   30,   7%     28.5   - medium dense   28.5 - 30   SS   9, 12, 15   18   27,   3%     33.5 - 35   SS   8, 11, 13   18   24,   7%     38.5   - very dense   38.5 - 40   SS   11, 25, 37   18   62,   4%		3.0	- stiff, slighty		d tan		3.5 - 5	SS	2,	6,	9	18	15,			23%		3.
12.0 - with fine sand  13.5 - very stiff  13.5-15 SS 9, 11, 21 18 32, 10%  18.5 - hard with chert fragments  18.5-20 SS 25, 24, 14 18 38, 10%  22.0 40.0 SAND (SP) - dense, dry, very fine grained, light gray and tan  23.5-25 SS 5, 15, 15 18 30, 7%  28.5 - medium dense  28.5-30 SS 9, 12, 15 18 27, 3%  38.5 - very dense  38.5-40 SS 11, 25, 37 18 62, 4%							6 - 7.5	SS	3,	5,	7	18	12,			17%		3.
18.5 - hard with chert fragments  18.5-20 SS 25, 24, 14 18 38, 10%  22.0 40.0 SAND (SP) - dense, dry, very fine grained, light gray and tan  23.5-25 SS 5, 15, 15 18 30, 7%  28.5 - medium dense  28.5-30 SS 9, 12, 15 18 27, 3%  33.5-35 SS 8, 11, 13 18 24, 7%  38.5 - very dense  38.5-40 SS 11, 25, 37 18 62, 4%		12.0	- with fine sand				8.5 - 10	SS	5,	7,	8	18	15,			20%		4.
22.0 40.0 SAND (SP) - dense, dry, very fine grained, light gray and tan  23.5-25 SS 5, 15, 15 18 30, 7%  28.5 - medium dense  28.5-30 SS 9, 12, 15 18 27, 3%  33.5-35 SS 8, 11, 13 18 24, 7%  38.5 - very dense  38.5-40 SS 11, 25, 37 18 62, 4%		13.5	- very stiff				13.5-15	SS	9,	11,	21	18	32,			10%		
Section   Sect		18.5	- hard with chert	fragments			18.5-20	SS	25,	24,	14	18	38,			10%		
33.5-35 SS 8, 11, 13 18 24, 7%  38.5 - very dense 38.5-40 SS 11, 25, 37 18 62, 4%	22.0	40.0			grained,		23.5-25	SS	5,	15,	15	18	30,			7%		
38.5 - very dense 38.5-40 SS 11, 25, 37 18 62, 4%		28.5	- medium dense				28.5-30	SS	9,	12,	15	18	27,			3%		
30.0 10 00 12, 12, 13, 07, 120 02, 17, 170							33.5-35	SS	8,	11,	13	18	24,			7%		
		38.5	-	rminated at 40 fe	et		38.5-40	SS	11,	25,	37	18	62,			4%		



# **Boring Log**

**Boring: B-2** 

Page 1 of 1

**Project: EV Fancy Farm** City, State Fancy Farm, KY

H.S.A. Boring Date: Location: 20' NW of Proposed Tower Center Method: 10-Mar-22

	iou.		11.5.A.	borning Dutc.			10 11101								лторс			
Inside Diameter: 2 1/4" Drill Rig Type:				7822 DT (ATV)						Hammer Type: Auto								
-	Groundwater: DRY				Weather: bout 8 inches of topsoil were encountered at the ground surface													
Drille	r: Con	nmonv	vealth Drilling		Note: /	Abou		of to	opsoi	l wer	e enc	ountere	d at th	e groun	d surfac	е		
	From (ft)	To (ft)	Mater	ial Description			Sample Depth (ft)	Sample Type		Blows per 6-inch	increment	Recovery (in)	SPT-N value	Rock Quality (RQD,%)	Atterberg Limits	Moisture Content (%)	% Fines (clay & silt)	Unconfined Compressive Strength, (ksf)
	0.7	18.5	CLAYEY SILT (M	L) - medium stiff, n-tan mottled	moist,		1-2.5								, –			
		3.0	- very stiff	ir tail mottled			3.5 - 5	SS	1, 4,	2,	3 9	15 18	5, 17,			26% 22%		4.0
							6 - 7.5	SS	3,	4,	4	18	8,			23%		1.5
		8.5	- stiff				8.5 - 10			5,	8	18	13,			18%		3.2
		13.5	- hard with chert				13.5-15	SS	17,	27,	40	18	67,			9%		
	18.5	20.0		(SP) - very dense h trace silt and clar minated at 20 fe	ay		18.5-20	SS	15,	22,	23	18	45,			7%		



# **Boring Log**

Boring: B-3

Page 1 of 1

Project: City, State **EV Fancy Farm** Fancy Farm, KY

Boring Date: Method: H.S.A. 10-Mar-22 **Location: 20' SW of Proposed Tower Center** 

	iou:			п.э.А.	boring Date:		10-ivia					<del> </del>		0 SW 01		seu 10v	ver cen	tei
Inside Diameter: 2 1/4" Drill Rig Type:					7822 DT (ATV) Hammer Type: Auto													
Groundwater: DRY											Weath							
Drille	r: Co	mr	nonv	vealth Drilling	g Note	: Abc	ut 8 inche	es of	tops	oil we	re en	counter	ed at t	he grou	nd surfa	ce		
	From (ft)		To (ft)	Mater	rial Description		Sample Depth (ft)	Sample Type		Blows per 6-inch	increment	Recovery (in)	SPT-N value	Rock Quality (RQD,%)	Atterberg Limits	Moisture Content (%)	% Fines (clay & silt)	Unconfined Compressive Strength, (ksf)
	0.7		19.0		IL) - medium stiff, slight oist, brown	<i>'</i>	1-2.5	SS	-	2,	3	14	5,			28%		2.0
			3.0	- very stiff			3.5 - 5	SS	4,	7,	8	18	15,			24%		3.2
			6.0	- medium stiff, m	oist		6 - 7.5	SS	2,	3,	4	16	7,			23%		1.8
							8.5 - 10	SS	5,	6,	8	0	14,					
			13.5	•	t, reddish brown with ve fine sand	ry	13.5-15	SS	4,	8,	12	15	20,			12%		
			18.5	- hard with chert														
	19.0		20.0		: (SP) - very dense, reddi :h trace silt and clay	sh	18.5-20	SS	23,	23,	38	13	61,			8%		
				Boring Te	rminated at 20 feet													

	FINE AND COARSE GRAINED SOIL INFORMATION								
	RAINED SOILS & GRAVELS)		NE GRAINED SO (SILTS & CLAYS		PARTICLE SIZE				
<u>N</u>	Relative Density	<u>N</u>	Consistency	Qu, KSF Estimated	Boulders	Greater than 300 mm (12 in)			
0-4	Very Loose	0-1	Very Soft	0-0.5	Cobbles	75 mm to 300 mm (3 to 12 in)			
5-10	Loose	2-4	Soft	0.5-1	Gravel	4.74 mm to 75 mm (3/16 to 3 in)			
11-20	Firm	5-8	Firm	1-2	Coarse Sand	2 mm to 4.75 mm			
21-30	Very Firm	9-15	Stiff	2-4	Medium Sand	0.425 mm to 2 mm			
31-50	Dense	16-30	Very Stiff	4-8	Fine Sand	0.075 mm to 0.425 mm			
Over 50	Very Dense	Over 31	Hard	8+	Silts & Clays	Less than 0.075 mm			

The **STANDARD PENETRATION TEST** as defined by ASTM D 1586 is a method to obtain a disturbed soil sample for examination and testing and to obtain relative density and consistency information. A standard 1.4-inch I.D./2-inch O.D. split-barrel sampler is driven three 6-inch increments with a 140 lb. hammer falling 30 inches. The hammer can either be of a trip, free-fall design, or actuated by a rope and cathead. The blow counts required to drive the sampler the final two increments are added together and designate the N-value defined in the above tables.

## **ROCK PROPERTIES**

ROCK QUA	LITY DESIGNATION (RQD)		ROCK HARDNESS
Percent RQD	<u>Quality</u>	Very Hard:	Rock can be broken by heavy hammer blows.
0-25	Very Poor	Hard:	Rock cannot be broken by thumb pressure, but can be broken by moderate hammer blows.
25-50	Poor	Moderately	Small pieces can be broken off along sharp edges by considerable
50-75	Fair	Hard:	hard thumb pressure; can be broken with light hammer blows.
75-90	Good	Soft:	Rock is coherent but breaks very easily with thumb pressure at sharp edges and crumbles with firm hand pressure.
90-100	Excellent	Very Soft:	Rock disintegrates or easily compresses when touched; can be hard to very hard soil.

Recovery =	<u>Length of Rock Core Recovered</u> Length of Core Run	X100	63 REC NQ	Core Diameter BQ NQ	Inches 1-7/16 1-7/8
RQD =	Sum of 4 in. and longer Rock Pieces Recovered	X100	43 RQD 00	HQ	2-1/2

Length of Core Run

## **SYMBOLS**

## **KEY TO MATERIAL TYPES**

SOILS				
Gro Sym	oup bols	Typical Names		
GW		Well graded gravel - sand mixture, little or no fines		
GP		Poorly graded gravels or gravel - sand mixture, little or no fines		
GM		Silty gravels, gravel - sand silt mixtures		
GC		Clayey gravels, gravel - sand - clay mixtures		
sw		Well graded sands, gravelly sands, little or no fines		
SP		Poorly graded sands or gravelly sands, little or no fines		
SM		Silty sands, sand - silt mixtures		
SC		Clayey sands, sand - clay mixtures		
ML		Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts		
OL		Organic silts and organic silty clays of low plasticity		
CL		Inorganic clays of low range plasticity, gravelly clays, sandy clays, silty clays, lean clays		
МН		Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts		
СН		Inorganic clays of high range plasticity, fat clays		

ROCKS		
Symbols	Typical Names	
	Limestone or Dolomite	
	Shale	
	Sandstone	

N:	_	OIL PROPERTY SYMBOLS Indard Penetration, BPF				
M:	Мо	Moisture Content, %				
LL:	Liq	Liquid Limit, %				
PI:	Pla	Plasticity Index, %				
Qp:	Po	Pocket Penetrometer Value, TSF				
Qu:	Unconfined Compressive Strength Estimated Qu, TSF					
γ	Dry Unit Weight, PCF					
D: F:	Fin	Fines Content				
SAMPLING SYMBOLS						
	SS	Split Spoon Sample				
	an	Relatively Undisturbed Sample				
	Core 1	Rock Core Sample				

**Exhibit I** 

## **DIRECTIONS FOR SITE**

FROM GRAVES COUNTY COURT CLERK, 101 E SOUTH ST #2, MAYFIELD, KY 42066: HEAD EAST ON E SOUTH ST TOWARD S 6TH ST (190 FT). TURN LEFT AT THE 1ST CROSS STREET ONTO S 6TH ST (361 FT). TURN LEFT ONTO E BROADWAY (2.4 MI). CONTINUE ONTO KY-80 W (7.8 MI). TURN RIGHT ONTO KY-339 N/KY-80 W (249 FT). TURN LEFT ONTO KY-80 W (0.4 MI). SITE WILL BE LOCATED ON RIGHT (NORTH) SIDE OF ROAD.

Prepared by Power of Design Group, LLC – 502-437-5252

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SITE NAME: EV Fancy Farm LOC CODE: 495686 ATTY/DATE: GJ

## LAND LEASE AGREEMENT

This Land Lease Agreement (the "Agreement") made this 20 day of 500 day of 500, 2022, between KM & K Farms, LLC, a Kentucky limited liability company with a mailing address of P.O. Box 65, Fancy Farm, Kentucky 42039, hereinafter designated LESSOR and Kentucky RSA No. 1 Partnership with its principal offices at One Verizon Way, Mail Stop 4AW100, Basking Ridge, New Jersey 07920 (telephone number 866-862-4404), hereinafter designated LESSEE, LESSOR and LESSEE are at times collectively referred to hereinafter as the "Parties" or Individually as the "Party."

## WITNESSETH

In consideration of the mutual covenants contained herein and intending to be legally bound hereby, the Parties hereto agree as follows:

- 1. <u>GRANT.</u> In accordance with this Agreement, LESSOR hereby grants to LESSEE the right to install, maintain and operate communications equipment ("Use") upon the Premises (as hereinafter defined), which are a part of that real property owned, leased or controlled by LESSOR at or near Kentucky Highway 80, Fancy Farm, Kentucky 42039 (the "Property"). The Property is legally described on Exhibit "A" attached hereto and made a part hereof. The Premises are a portion of the Property and are approximately 10,000 square feet and are shown in detail on Exhibit "B" attached hereto and made a part hereof. LESSEE may survey the Premises. Upon completion, the survey shall replace Exhibit "B" in its entirety.
- 2, <u>INITIAL TERM.</u> This Agreement shall be effective as of the date of execution by both Parties ("Effective Date"). The initial term of the Agreement shall be for 5 years beginning on the first day of the month following the Commencement Date (as hereinafter defined). The "Commencement Date" shall be the first day of the month after LESSEE begins installation of LESSEE's communications equipment.
- 3. <u>EXTENSIONS.</u> This Agreement shall automatically be extended for 5 additional 5 year terms unless Lessee terminates it at the end of the then current term by giving LESSOR written notice of the intent to terminate at least 3 months prior to the end of the then current term. The initial term and all extensions shall be collectively referred to herein as the "Term".

## 4. RENTAL,

- a. Rental payments shall begin on the Commencement Date and be due at a total annual rental of in advance, to LESSOR at P.O. BOX 05, Fancy Farm, Kentucky 42039 or to such other person, firm, or place as LESSOR may, from time to time, designate in writing at least 30 days in advance of any rental payment date by notice given in accordance with Paragraph 20 below. LESSOR and LESSEE acknowledge and agree that the initial rental payment shall not be delivered by LESSEE until 60 days after the Commencement Date. Upon agreement of the Parties, LESSEE may pay rent by electronic funds transfer and in such event, LESSOR agrees to provide to LESSEE bank routing information for such purpose upon request of Lessee.
- b. For any party to whom rental payments are to be made, LESSOR or any successor in interest of LESSOR hereby agrees to provide to LESSEE (i) a completed, current version of internal Revenue Service Form W-9, or equivalent; (ii) complete and fully executed state and local withholding

forms if required; and (iii) other documentation to verify LESSOR's or such other party's right to receive rental as is reasonably requested by LESSEE. Rental shall accrue in accordance with this Agreement, but LESSEE shall have no obligation to deliver rental payments until the requested documentation has been received by LESSEE. Upon receipt of the requested documentation, LESSEE shall deliver the accrued rental payments as directed by LESSOR. The annual rental for each five (5) year extension term shall be equal to one hundred ten percent (110%) of the annual rental payable with respect to the immediately preceding five (5) year term.

- c. In consideration for the execution of this Agreement and as a signing bonus contemplating potential delay of the Commencement Date, LESSEE agrees to make a one-time payment to LESSOR in the amount of ("Additional Payment"). The Additional Payment shall be due within sixty (60) days following the date of full execution of this Agreement and shall be due and payable regardless of whether LESSEE commences the Agreement.
- 5. ACCESS. LESSEE shall have the non-exclusive right of ingress and egress from a public right-of-way, 7 days a week, 24 hours a day, over the Property to and from the Premises for the purpose of installation, operation and maintenance of LESSEE's communications equipment over or along a thirty (30) foot wide right-of-way ("Easement"), which is described on Exhibit "A" and shall be depicted on Exhibit "B". LESSEE may use the Easement for the installation, operation and maintenance of wires, cables, conduits and pipes for all necessary electrical, telephone, fiber and other similar support services. In the event it is necessary, LESSOR agrees to grant LESSEE or the provider the right to install such services on, through, over and/or under the Property, provided the location of such services shall be reasonably approved by LESSOR. During the term of this Agreement, LESSEE shall, at LESSEE's cost, promptly repair or replace all physical damage over said Easement proximately caused by the construction, operation, maintenance or use of said facilities by LESSEE. Notwithstanding anything to the contrary, the Premises shall include such additional space sufficient for LESSEE's radio frequency signage and/or barricades as are necessary to ensure LESSEE's compliance with Laws (as defined in Paragraph 27).
- 6. <u>CONDITION OF PROPERTY</u>. LESSOR shall deliver the Premises to LESSEE in a condition ready for LESSEE's Use and clean and free of debris. LESSOR represents and warrants to LESSEE that as of the Effective Date, the Premises (a) in compliance with all Laws; and (b) in compliance with all EH&S Laws (as defined in Paragraph 24).
- 7. IMPROVEMENTS. The communications equipment including, without limitation, the tower structure, antennas, conduits, fencing and other screening, and other improvements shall be at LESSEE's expense and installation shall be at the discretion and option of LESSEE. LESSEE shall have the right to replace, repair, add or otherwise modify its communications equipment, tower structure, antennas, conduits, fencing and other screening, or other improvements or any portion thereof and the frequencies over which the communications equipment operates, whether or not any of the communications equipment, antennas, conduits or other improvements are listed on any exhibit. Any improvements to the roadway comprising the portion of the Easement that shall be used exclusively by LESSEE, its successor, assigns or subtenants, shall be at the LESSEE's expense and shall be at the discretion and option of LESSEE. Notwithstanding the foregoing, nothing contained herein shall prevent the LESSOR or the Fancy Farm Water District from accessing, improving, or maintaining that portion of the Easement which provides access to the water tower depicted on Exhibit "B".

- 8. <u>GOVERNMENT APPROVALS</u>. LESSEE's Use is contingent upon LESSEE obtaining all of the certificates, permits and other approvals (collectively the "Government Approvals") that may be required by any Federal, State or Local authorities (collectively, the "Government Entities") as well as a satisfactory soll boring test, environmental studies, or any other due diligence LESSEE chooses that will permit LESSEE's Use. LESSOR shall cooperate with LESSEE in its effort to obtain such approvals and shall take no action which would adversely affect the status of the Property with respect to LESSEE's Use.
- 9. <u>TERMINATION</u>. LESSEE may, unless otherwise stated, immediately terminate this Agreement upon written notice to LESSOR in the event that (I) any applications for such Government Approvals should be finally rejected; (II) any Government Approval Issued to LESSEE is canceled, expires, lapses or is otherwise withdrawn or terminated by any Government Entity; (III) LESSEE determines that such Government Approvals may not be obtained in a timely manner; (IV) LESSEE determines any structural analysis is unsatisfactory; (V) LESSEE, in its sole discretion, determines the Use of the Premises is obsolete or unnecessary; (VII) with 3 months prior notice to LESSOR, upon the annual anniversary of the Commencement Date; or (VIII) at any time before the Commencement Date for any reason or no reason in LESSEE's sole discretion.

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- 10, INDEMNIFICATION. Subject to Paragraph 11, each Party shall indemnify and hold the other harmless against any claim of liability or loss from personal injury or property damage resulting from or arising out of the negligence or willful misconduct of the indemnifying Party, its employees, contractors or agents, except to the extent such claims or damages may be due to or caused by the negligence or willful misconduct of the other Party, or its employees, contractors or agents. The indemnified Party will provide the indemnifying Party with prompt, written notice of any claim covered by this indemnification: provided that any failure of the indemnified Party to provide any such notice, or to provide it promptly, shall not relieve the indemnifying Party from its indemnification obligation in respect of such claim, expect to the extent the indemnifying Party can establish actual prejudice and direct damages as a result thereof. The indemnified Party will cooperate appropriately with the indemnifying Party in connection with the indemnifying Party's defense of such claim. The indemnifying Party shall defend any indemnified Party, at the indemnissed Party's request, against any claim with counsel reasonably satisfactory to the Indemnified Party. The indemnifying Party shall not settle or compromise any such claim or consent to the entry of any judgment without the prior written consent of each indemnified Party and without an unconditional release of all claims by each claimant or plaintiff in favor of each indemnified Party.
- 11. INSURANCE. The Parties agree that at their own cost and expense, each will maintain commercial general liability insurance with limits not less than \$2,000,000 for injury to or death of one or more persons in any one occurrence and \$2,000,000 for damage or destruction in any one occurrence. The Parties agree to include the other Party as an additional insured. The Parties hereby waive and release any and all rights of action for negligence against the other which may hereafter arise on account of damage to the Premises or the Property, resulting from any fire, or other casualty which is insurable under "Causes of Loss Special Form" property damage insurance or for the kind covered by standard fire insurance policies with extended coverage, regardless of whether or not, or in what amounts, such insurance is now or hereafter carried by the Parties, even if any such fire or other casualty shall have been caused by the fault or negligence of the other Party. These waivers and releases shall apply between the Parties and they shall also apply to any claims under or through either Party as a result of any asserted right of subrogation. All such policies of insurance obtained by either Party concerning the Premises or the Property shall waive the insurer's right of subrogation against the other Party.

12. <u>LIMITATION OF LIABILITY</u>. Except for indemnification pursuant to Paragraphs 10 and 24, a violation of Paragraph 29, or a violation of law, neither Party shall be liable to the other, or any of their respective agents, representatives, or employees for any lost revenue, lost profits, loss of technology, rights or services, incidental, punitive, indirect, special or consequential damages, loss of data, or interruption or loss of use of service, even if advised of the possibility of such damages, whether under theory of contract, tort (including negligence), strict liability or otherwise.

## 13. INTERFERENCE.

- a. LESSEE agrees that LESSEE will not cause interference that is measurable in accordance with industry standards to LESSOR's equipment. LESSOR agrees that LESSOR and other occupants of the Property will not cause interference that is measurable in accordance with industry standards to the then existing equipment of LESSEE.
- b. Without limiting any other rights or remedies, if interference occurs and continues for a period in excess of 48 hours following notice to the interfering party via telephone to LESSEE'S Network Operations Center (at (800) 852-2671/(800) 621-2622) or to LESSOR (at 763-248-2538), the interfering party shall or shall require any other user to reduce power or cease operations of the interfering equipment until the interference is cured.
- c. The Parties acknowledge that there will not be an adequate remedy at law for noncompliance with the provisions of this Paragraph and therefore the Parties shall have the right to equitable remedies such as, without limitation, injunctive relief and specific performance.
- 14. <u>REMOVAL AT END OF TERM.</u> Upon expiration or within 90 days of earlier termination, LESSEE shall remove LESSEE's Communications Equipment (except footings) and restore the Premises to its original condition, reasonable wear and tear and casualty damage excepted. LESSOR agrees and acknowledges that the communications equipment shall remain the personal property of LESSEE and LESSEE shall have the right to remove the same at any time during the Term, whether or not said items are considered fixtures and attachments to real property under applicable laws. If such time for removal causes LESSEE to remain on the Premises after termination of the Agreement, LESSEE shall pay rent at the then existing monthly rate or on the existing monthly pro-rata basis if based upon a longer payment term, until the removal of the communications equipment is completed.
- 15. HOLDOVER. If upon expiration of the Term the Parties are negotiating a new lease or a lease extension, then this Agreement shall continue during such negotiations on a month to month basis at the rental in effect as of the date of the expiration of the Term. In the event that the Parties are not in the process of negotiating a new lease or lease extension and LESSEE holds over after the expiration or earlier termination of the Term, then LESSEE shall pay rent at the then existing monthly rate or on the existing monthly pro-rate basis if based upon a longer payment term, until the removal of the communications equipment is completed rental.
- 16. <u>RIGHT OF FIRST REFUSAL</u>. If at any time after the Effective Date, LESSOR receives an offer or letter of Intent from any person or entity that is in the business of owning, managing or operating communications facilities or is in the business of acquiring landlord interests in agreements relating to communications facilities, to purchase fee title, an easement, a lease, a license, or any other interest in the Premises or any portion thereof or to acquire any interest in this Agreement, or an option for any of the foregoing, LESSOR shall provide written notice to LESSEE of said offer ("LESSOR's Notice"), LESSOR's Notice shall include the prospective buyer's name, the purchase price being offered, any other

consideration being offered, the other terms and conditions of the offer, a description of the portion of and interest in the Premises and/or this Agreement which will be conveyed in the proposed transaction, and a copy of any letters of Intent or form agreements presented to LESSOR by the third party offeror, LESSEE shall have the right of first refusal to meet any bona fide offer of sale or transfer on the terms and conditions of such offer or by effectuating a transaction with substantially equivalent financial terms, if LESSEE falls to provide written notice to LESSOR that LESSEE intends to meet such bona fige offer within thirty (30) days after receipt of LESSOR's Notice, LESSOR may proceed with the proposed transaction in accordance with the terms and conditions of such third party offer, in which event this Agreement shall continue in full force and effect and the right of first refusal described in this Paragraph shall survive any such conveyance to a third party. If LESSEE provides LESSOR with notice of LESSEE's intention to meet the third party offer within thirty (30) days after receipt of LESSOR's Notice, then if LESSOR's Notice describes a transaction involving greater space than the Premises, LESSEE may elect to proceed with a transaction covering only the Premises and the purchase price shall be pro-rated on a square footage basis. Further, LESSOR acknowledges and agrees that If LESSEE exercises this right of first refusal, LESSEE may require a reasonable period of time to conduct due diligence and effectuate the closing of a transaction on substantially equivalent financial terms of the third-party offer. LESSEE may elect to amend this Agreement to effectuate the proposed financial terms of the third party offer rather than acquiring fee simple title or an easement interest in the Premises. For purposes of this Paragraph, any transfer, bequest or devise of LESSOR's interest in the Property as a result of the death of LESSOR, whether by will or intestate succession, or any conveyance to LESSOR's family members by direct conveyance or by conveyance to a trust for the benefit of family members shall not be considered a sale for which LESSEE has any right of first refusal.

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- 17. <u>RIGHTS UPON SALE</u>. Should LESSOR, at any time during the Term, decide (i) to sell or otherwise transfer all or any part of the Property, or (ii) to grant to a third party by easement or other legal instrument an interest in and to any portion of the Premises, such sale, transfer, or grant of an easement or interest therein shall be under and subject to this Agreement and any such purchaser or transferee shall recognize LESSEE's rights hereunder. In the event that LESSOR completes any such sale, transfer, or grant described in this Paragraph without executing an assignment of the Agreement whereby the third party agrees in writing to assume all obligations of LESSOR under this Agreement, then LESSOR shall not be released from its obligations to LESSEE under this Agreement, and LESSEE shall have the right to look to LESSOR and the third party for the full performance of the Agreement.
- 18. <u>LESSOR'S TITLE.</u> LESSOR covenants that LESSEE, on paying the rent and performing the covenants herein, shall peaceably and quietly have, hold and enjoy the Premises. LESSOR represents and warrants to LESSEE as of the Effective Date and covenants during the Term that LESSOR has full authority to enter into and execute this Agreement and that there are no liens, judgments, covenants, easement, restrictions or other impediments of title that will adversely affect LESSEE's Use.
- 19. <u>ASSIGNMENT</u>. Subject to the terms of Paragraph 16, without any approval or consent of the other Party, this Agreement may be sold, assigned or transferred by either Party to any other third party. LESSEE may sublet the Premises in LESSEE's sole discretion.
- 20. NOTICES. Except for notices permitted via telephone in accordance with Paragraph 13, all notices hereunder must be in writing and shall be deemed validly given if sent by certified mail, return receipt requested or by commercial courier, provided the courier's regular business is delivery service and provided further that it guarantees delivery to the addressee by the end of the next business day following

the courier's receipt from the sender, addressed as follows (or any other address that the Party to be notified may have designated to the sender by like notice):

LESSOR: KM & K Farms, LLC

Attn: Keith Hayden 352 Carrico Road Fancy Farm, KY 42039

LESSEE: Kentucky RSA No. 1 Partnership

180 Washington Valley Road Bedminster, New Jersey 07921 Attention: Network Real Estate

Notice shall be effective upon actual receipt or refusal as shown on the receipt obtained pursuant to the foregoing.

- SUBORDINATION AND NON-DISTURBANCE. Within 15 days of the Effective Date, LESSOR shall obtain a Non-Disturbance Agreement, as defined below, from its existing mortgagee(s), ground lessors and master lessors, if any, of the Property. At LESSOR's option, this Agreement shall be subordinate to any future master lease, ground lease, mortgage, deed of trust or other security interest (a "Mortgage") by LESSOR which from time to time may encumber all or part of the Property; provided, however, as a condition precedent to LESSEE being required to subordinate its interest in this Agreement to any future Mortgage covering the Property, LESSOR shall obtain for LESSEE's benefit a non-disturbance and attornment agreement for LESSEE's benefit in the form reasonably satisfactory to LESSEE, and containing the terms described below (the "Non-Disturbance Agreement"), and shall recognize LESSEE's rights under this Agreement. The Non-Disturbance Agreement shall include the encumbering party's ("Lender's") agreement that, if Lender or its successor-in-interest or any purchaser of Lender's or its successor's interest (a "Purchaser") acquires an ownership interest in the Property, Lender or such successor-in-interest or Purchaser will honor all of the terms of the Agreement. Such Non-Disturbance Agreement must be binding on all of Lender's participants in the subject loan (if any) and on all successors and assigns of Lender and/or its participants and on all Purchasers. In return for such Non-Disturbance Agreement, LESSEE will execute an agreement for Lender's benefit in which LESSEE (1) confirms that the Agreement is subordinate to the Mortgage or other real property interest in favor of Lender, (2) agrees to attorn to Lender if Lender becomes the owner of the Property and (3) agrees to accept a cure by Lender of any of LESSOR's defaults, provided such cure is completed within the deadline applicable to LESSOR. In the event LESSOR defaults in the payment and/or other performance of any mortgage or other real property interest encumbering the Property, LESSEE may, at its sole option and without obligation, cure or correct LESSOR's default and upon doing so, LESSEE shall be subrogated to any and all rights, titles, liens and equities of the holders of such mortgage or other real property interest and LESSEE shall be entitled to deduct and setoff against all rents that may otherwise become due under this Agreement the sums paid by LESSEE to cure or correct such defaults.
- 22. <u>DEFAULT</u>. It is a "Default" if (i) either Party falls to comply with this Agreement and does not remedy the fallure within 30 days after written notice by the other Party or, if the fallure cannot reasonably be remedied in such time, if the failing Party does not commence a remedy within the allotted 30 days and diligently pursue the cure to completion within 90 days after the initial written notice, or (ii) LESSOR falls to comply with this Agreement and the failure interferes with LESSEE's Use and LESSOR does

not remedy the failure within 5 days after written notice from LESSEE or, if the failure cannot reasonably be remedied in such time, if LESSOR does not commence a remedy within the allotted 5 days and diligently pursue the cure to completion within 15 days after the initial written notice. The cure periods set forth in this Paragraph 22 do not extend the period of time in which either Party has to cure interference pursuant to Paragraph 13 of this Agreement.

- 23. <u>REMEDIES</u>. In the event of a Default, without limiting the non-defaulting Party in the exercise of any right or remedy which the non-defaulting Party may have by reason of such default, the non-defaulting Party may terminate this Agreement and/or pursue any remedy now or hereafter available to the non-defaulting Party under the Laws or judicial decisions of the state in which the Property is located. Further, upon a Default, the non-defaulting Party may at its option (but without obligation to do so), perform the defaulting Party's duty or obligation. The costs and expenses of any such performance by the non-defaulting Party shall be due and payable by the defaulting Party upon invoice therefor. If LESSEE undertakes any such performance on LESSOR's behalf and LESSOR does not pay LESSEE the full undisputed amount within 30 days of its receipt of an invoice setting forth the amount due, LESSEE may offset the full undisputed amount due against all fees due and owing to LESSOR under this Agreement until the full undisputed amount is fully relimbursed to LESSEE.
- 24. ENVIRONMENTAL. LESSEE shall conduct its business in compliance with all applicable laws governing the protection of the environment or employee health and safety ("EH&S Laws"). LESSEE shall indemnify and hold harmless LESSOR from claims to the extent resulting from LESSEE's violation of any applicable EH&S Laws or to the extent that LESSEE causes a release of any regulated substance to the environment. LESSOR shall indemnify and hold harmless LESSEE from all claims resulting from the violation of any applicable EH&S Laws or a release of any regulated substance to the environment except to the extent resulting from the activities of LESSEE. The Parties recognize that LESSEE is only leasing a small portion of LESSOR's property and that LESSEE shall not be responsible for any environmental condition or issue except to the extent resulting from LESSEE's specific activities and responsibilities. In the event that LESSEE encounters any hazardous substances that do not result from its activities, LESSEE may relocate its facilities to avoid such hazardous substances to a mutually agreeable location or, if LESSEE desires to remove at its own cost all or some the hazardous substances or materials (such as soil) containing those hazardous substances, LESSOR agrees to sign any necessary waste manifest associated with the removal, transportation and/or disposal of such substances.
- 25. <u>CASUALTY</u>. If a fire or other casualty damages the Property or the Premises and Impairs LESSEE's Use, rent shall abate until LESSEE'S Use is restored. If LESSEE's Use is not restored within 45 days, LESSEE may terminate this Agreement.
- 26. <u>CONDEMNATION</u>. If a condemnation of any portion of the Property or Premises Impairs LESSEE's Use, Lessee may terminate this Agreement. LESSEE may on its own behalf make a claim in any condemnation proceeding involving the Premises for losses related to LESSEE's communications equipment, relocation costs and, specifically excluding loss of LESSEC's leasehold interest, any other damages LESSEE may incur as a result of any such condemnation. LESSOR shall be entitled to receive any such condemnation proceeds to which LESSOR is entitled to receive as fee simple owner of the Property or Premises.
- 27. <u>APPLICABLE LAWS</u>. During the Term, LESSOR shall maintain the Property in compliance with all applicable laws, EH&S Laws, rules, regulations, ordinances, directives, covenants, easements, consent decrees, zoning and land use regulations, and restrictions of record, permits, building codes, and

the requirements of any applicable fire insurance underwriter or rating bureau, now in effect or which may hereafter come into effect (including, without limitation, the Americans with Disabilities Act and laws regulating hazardous substances) (collectively "Laws"). LESSEE shall, in respect to the condition of the Premises and at LESSEE's sole cost and expense, comply with (I) all Laws relating solely to LCSSEE's specific and unique nature of use of the Premises; and (II) all building codes requiring modifications to the Premises due to the improvements being made by LESSEE in the Premises. It shall be LESSOR's obligation to comply with all Laws relating to the Property, without regard to specific use (including, without limitation, modifications required to enable LESSEE to obtain all necessary building permits).

## 28, TAXES.

- a. LESSOR shall involce and LESSEE shall pay any applicable transaction tax (including sales, use, gross receipts, or excise tax) imposed on LESSEE and required to be collected by LESSOR based on any service, rental space, or equipment provided by LESSOR to LESSEE. LESSEE shall pay all personal property taxes, fees, assessments, or other taxes and charges imposed by any Government Entity that are imposed on LESSEE and required to be paid by LESSEE that are directly attributable to LESSEE's equipment or LESSEE's use and occupancy of the Premises. Payment shall be made by LESSEE within 60 days after presentation of a receipted bill and/or assessment notice which is the basis for such taxes or charges. LESSOR shall pay all ad valorem, personal property, real estate, sales and use taxes, fees, assessments or other taxes or charges that are attributable to LESSOR's Property or any portion thereof imposed by any Government Entity.
- b. LESSEE shall have the right, at its sole option and at its sole cost and expense, to appeal, challenge or seek modification of any tax assessment or billing for which LESSEE is wholly or partly responsible for payment. LESSOR shall reasonably cooperate with LESSEE at LESSEE's expense in filling, prosecuting and perfecting any appeal or challenge to taxes as set forth in the preceding sentence, including but not limited to, executing any consent, appeal or other similar document. In the event that as a result of any appeal or challenge by LESSEE, there is a reduction, credit or repayment received by LESSOR for any taxes previously paid by LESSEE, LESSOR agrees to promptly reimburse to LESSEE the amount of said reduction, credit or repayment. In the event that LESSEE does not have the standing rights to pursue a good faith and reasonable dispute of any taxes under this paragraph, LESSOR will pursue such dispute at LESSEE's sole cost and expense upon written request of LESSEE.
- 29. <u>NON-DISCLOSURE</u>. The Parties agree this Agreement and any Information exchanged between the Parties regarding the Agreement are confidential. The Parties agree not to provide copies of this Agreement or any other confidential information to any third party without the prior written consent of the other or as required by law, except the Parties shall be permitted to disclose information regarding this Agreement to their respective legal counsel, accountants, and other professional advisors and agents who shall be advised of this nondisclosure provision. If a disclosure is required by law, prior to disclosure, the Party shall notify the other Party and cooperate to take lawful steps to resist, narrow, or eliminate the need for that disclosure.
- 30. MOST FAVORED LESSEE. LESSOR represents and warrants that the rent, benefits and terms and conditions granted to LESSEE by LESSOR hereunder are now and shall be, during the Term, no less favorable than the rent, benefits and terms and conditions for substantially the same or similar tenancies or licenses granted by LESSOR to other parties. If at any time during the Term LESSOR shall offer more favorable rent, benefits or terms and conditions for substantially the same or similar tenancies or licenses as those granted hereunder, then LESSOR shall, within 30 days after the effective date of such

offering, notify LESSEE of such fact and offer LESSEE the more favorable offering. If LESSEE chooses, the Parties shall then enter into an amendment that shall be effective retroactively to the effective date of the more favorable offering, and shall provide the same rent, benefits or terms and conditions to LESSEE. LESSEE shall have the right to decline to accept the offering. LESSOR's compliance with this requirement shall be subject, at LESSEE's option, to independent verification.

31. MISCELLANEOUS. This Agreement contains all agreements, promises and understandings between LESSOR and LESSEE regarding this transaction, and no oral agreement, promises or understandings shall be binding upon either LESSOR or LESSEE in any dispute, controversy or proceeding. This Agreement may not be amended or varied except in a writing signed by all Parties. This Agreement shall extend to and bind the heirs, personal representatives, successors and assigns hereto. The failure of either Party to insist upon strict performance of any of the terms or conditions of this Agreement or to exercise any of its rights hereunder shall not waive such rights, and such Party shall have the right to enforce such rights at any time. The performance of this Agreement shall be governed, interpreted, construed and regulated by the laws of the state in which the Premises is located without reference to its choice of law rules. Except as expressly set forth in this Agreement, nothing in this Agreement shall grant, suggest or imply any authority for one Party to use the name, trademarks, service marks or trade names of the other for any purpose whatsoever. LESSOR agrees to execute a Memorandum of this Agreement, which LESSEE may record with the appropriate recording officer. The provisions of the Agreement relating to indemnification from one Party to the other Party shall survive any termination or expiration of this Agreement. Time is of the essence with respect to all provisions, covenants, terms, and conditions of this Agreement.

IN WITNESS WHEREOF, the Parties hereto have set their hands and affixed their respective seals the day and year first above written.

and year first above written.	
	LESSOR:
On in land	KM & K Farms, LLC  By: W. K. C.
WITNESS	Name: 6.16. Hayotal  Date: 12/3/2026
Danice C.Miller	Date:/2/3/2626 LESSEE:
	Kentucky RSA No. 1 Partnership d/b/a Verizon Wireless; By: Cellco Partnership d/b/a Verizon Wireless, Its General Partner By: Name:  Bd Maher  Director - Network Field Engineering
Ibigail Ball	Date: 9 70 1

## EXHIBIT "A"

## **DESCRIPTION OF PROPERTY AND PREMISES**

Description of Property:

ALL THAT PARCEL OF LAND IN THE IN THE COUNTY OF GRAVES AND COMMONWBALTH OF KENTUCKY AS MORE FULLY DESCRIBED IN DEED DATED DECEMBER 12, 2016 AND RECORDED IN THE GRAVES COUNTY CLERK'S OFFICE IN DEED BOOK 506, PAGE 639

## LEGAL DESCRIPTIONS

## Proposed lease area

THE FOLLOWING IS A DESCRIPTION OF THE PROPOSED LEASE AREA TO BE LEASED FROM THE PROPERTY CONVEYED TO XINS & I. FARMAS LLC AS RECORDED BY DEED BOOK 506, PAGE 639 OF RECORD HE THE OFFICE OF THE CLERK OF GRAVES COUNTY, RENTUCKY, PARCEL ID: 006.03 03.035.00, WHICH IS MORE PARTICULARLY DESCRIBED AS FOLLOWS.

BEARING DATUM USED HEREIN IS BASED UPON RENTUCKY STATE PLANE COORDINATE SYSTEM, SINGLE ZONE, MAD 81, FROM A REAL TIME KHIEMATIC GLOBAL POSITIONING SYSTEM OBSTRYATION USING THE YENTUCKY TRAILSPORTATION CABINET REAL TIME GPS HETWORK COMPETTED DICTURE 14: 2020

COMMERCING AT A FOUND S/B" REBAR WITH A YELLOW CAP STAMPED 'RT CARTER PLS' AT THE SOUTHWEST CORNER OF THE PARCEL CONSTRUCT TO JASON S & KAYLA HERISON AS DESCRIBED IN DEED BOOK 4-17, PAGE 52, PARCEL ID: 006.00.00.007 CO, AND BEIRG IN THE HORTH RIGHT OF WAY LINE OF KENTUCKY HIGHWAY 60; FOR REFERENCE, SAID COMMENCEMENT POINT IS 564'00Y 14"E 904 95" FROM A FOUND 17.7" REBAR WITH NO CAP IN THE SOUTHWAY 60; FOR REFERENCE, SAID COMMENCEMENT POINT IS 564'00Y 14"E 904 95" FROM A FOUND 17.7" REBAR WITH NO CAP IN THE SOUTHWAY 60 THE HORTH RIGHT OF WAY LINE OF SAID LENTUCKY HIGHWAY 60, THERICE HIGHWOY 14" W 275 99" TO A POINT ON THE HORTH RIGHT OF WAY LINE OF SAID KENTUCKY HIGHWAY 60, THERICE HIGHWOY 16" AND LINE HORTH RIGHT OF WAY LINE OF SAID KENTUCKY HIGHWAY 60, THERICE HIGHWOY 16" SOUTH LINE OF SAID KAN & KFARMS LIC, THERICE LEAVING SAID LINE HIS-159"/46" & 234,23" TO A SET 17.1" REBAR, 18" LONG, CAPPED "PATTERSON PLS 1136", HERCAFTER REFERRED TO AS A "SET 1PC", AT THE SOUTHWEST CORNER OF THE PROPOSED LEASE AREA AND BEING THE TRUE POINT OF BEGINNING HIGHWEST CORNER OF THE PROPOSED LEASE AREA AND BEING THE TRUE POINT OF BEGINNING CONTAINING 10,000,000 SQUARE FEET AS PER SURVEY BY WARK E. PATTERSON, PLS 113136 DATED JUHE 24, 2020.

## PROPOSED 30' / VARIABLE WIDTH ACCESS & UTILITY EASEMENT

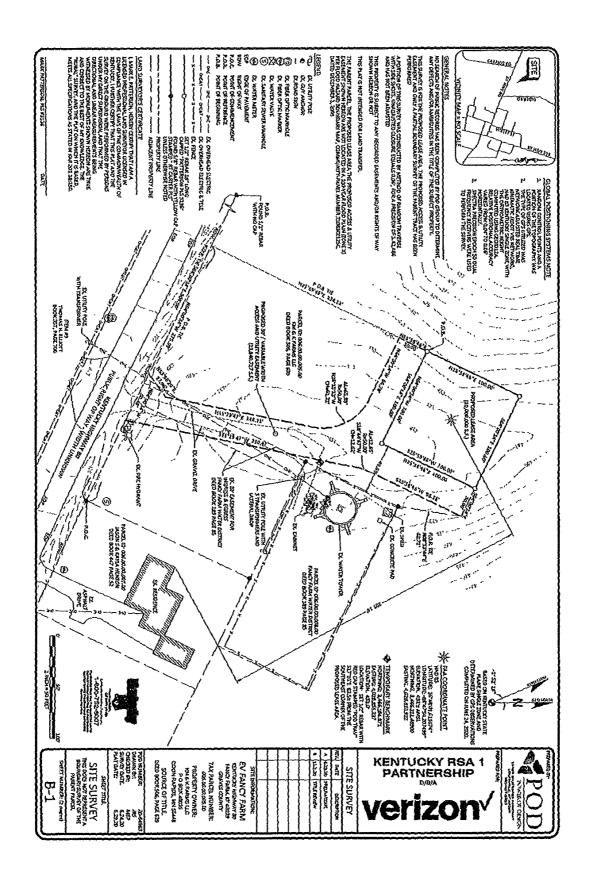
THE FOLLOWING IS A DESCRIPTION OF THE PROPOSED BY / VARIABLE WINDTH ACCESS MID UTILITY EASEMENT TO BE GRANTED FROM THE PROPERTY CONVEYED TO KIM & K FARMS LIC AS RECORDED IN GEED BOOK SU6, PAGE 639 OF RECORD IN THE OFFICE OF THE CLERK OF GRAYES COUNTY, KENTUCKY, PARCEL ID. 806 63 00.003,00, WHICH IS MORE PARTICULARLY DESCRIBED AS FOLLOWS:

ECARING DATUM USED HEREIN IS BASED UPON KENTUCKY STATE PLANE COORDINATE SYSTEM, SINGLE ZONE, NAD 83, FROM A REAL TIME HINEMATIC GLOBAL POSITIONING SYSTEM OBSERVATION USING THE LENTUCKY TRANSPORTATION CABINET REAL TIME GPS HETWORK COMPLETED ON WHIC 14, 2010.

## EXHIBIT "B"

## SURVEY OF THE PREMISES

(see attached)



THIS SURVEY DOES NOT CONSTITUTE A TITLE SEARCH BY PODO GROUP, LLC AND AS SUCH WE ARE NOT RESPONSIBLE FOR THE INVESTIGATION OR INDEPENDENT SAME FOR EAST INVESTIGATION OR SECRETARY CONFERENCE OF SECRETARY INVESTIGATION OF SECRETARY CONFERENCE THE PROPERTY LINESCORDED BY DESCRIPTION OF SECRETARY INVESTIGATION OF SECRETARY OF SECRETARY INVESTIGATION OF SECRETARY INVESTIGATION OF SECRETARY CONFESSION OF SECRETARY INVESTIGATION OF SECRETARY OF SECRETARY INVESTIGATION OF SECRETARY OF

## SCHEDULE B SECTION II (EXCEPTIONS)

- DESCTE, LINKS, DILLINGUAVEZS, ACCESSE CLANS GROTHER MATTEKS, E ANY, CESATE), RETA JAPSANINS IN THE PUBLIC RECORDS OB ATTACHING SERVENUENT TO THE EFFECTIVE DATE AND FROM TOTA CHAT THE APPROCEDUA INCLUDES ACCUMENTED HE OF RECORD HE STATE OR INTEREST OR MONTEMEET THEREON COVERED BY THIS COMMATINESSET (POWER OF DESIGN GROUP, LLC., DID NOT DAMBINE OR ADDRESS THE ITEM.)
- TAXES OR SPECIAL ASSESSABITS WHICH ARE NOT SHOWN AS DISTING UBBS BY THE PUBLIC RECORDS. (NOT A LAND SURVEYING MATTER, THEREFORE POWER OF DESIGN GROUP, LIC., DID NOT EXAMINE OR ADDRESS THIS ITEM,)
- ANY BUCROACHMENT, BUCHMBRANCE, VIGLATION, VARIATION, OR ADVERSE CHOLMSTANCE AFFECTING THE TITLE THAT WOULD BE DUCLOSED BY AN ACQUANTE AND CONPELTE LAND SURVEY OF THE LIVEL (NO ENCOCHOMENTS WISE OBSERVED ON THE AREA OF THE PREMISES, OR ESSENBEIT POWERS OF BESSEN BOOK DROOMS ALL THEFORE, THE SHOULD NOT BE CONSTRUED AS NO BUCHDACHMENTS EXST.)
- RIGHTS OR DAING OF PARTIES IN POSSESSION NOT SHOWN BY THE PUBLIC RECEIRDS. (FIGHTS OR CLAIMS ARE NOT A LAND SURVEYING MATTER. THEREFORE POWER OF DESIGN GROUP, LLC, DID NOT EXAMINE ORADOREST THE TIENA!
- ANY LIEN OR RIGHT TO A LIEN, FOR SERVICES, LAGOR, OR NATERIAL HERETOFORE OR HEREATER FURNISHED, RAPOSED BY LAW AND NOT SHOWN BY THE PUBLIC RECORDS, (INOT A LAND SURVEYING MATTER, THEREFORE POWER OF DESIGN GROUP, LLC, DID NOT EXAMINE OR ADDRESS THIS HEIM.)
- Excenditior claims of excenents, not shown by the public records, ipower of dexian group, i.e., did not damne or address that teal)

TAXES, OR SPECIAL ASSESSMENTS, IF ANY, NOT SHOWN AS DESTING LENS BY THE PUBLIC RECORDS.

PARCEL ID # 005,00.00,005.00
COMMENTS, PARENT PARCEL
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2019 GTY & COUNTY ANN

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(NOTA LAMO SUPERING AMOTER, THESEFORE POWER OF DESMI GROUP, LLC, DO NOT DUANNE OR ADDRESS THE CITAL)

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JENS/JUDGEMENTS

NONE OF RECORD.

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  AND BEFORE AND RECORDED IN PROOF, 150, PABLE CTAT, DRAWET COUNTING RECORDED, [DASHMENT IN DEED AS RECORDED IN BOOK 250,
  PAGE 477 DOES NOT AFFECT THE PARENT PANCEL ON THE ACCESS & UTILITY ENCOMENT.]
- SUBJECT OF ASSANDAT FOR POWEST UNES AND WATER METERS IN DEED PERM LAMES AL FLUOTT TRUSTEE OF THE LAMES AL FLUOTT DECLARATION OF TRUST TO THOMAS H, ELLIOTT DATED LIDE/SUBSE AND RECORDED DID/12/2058 IN BOOK 157, DAKE TOA, DANNES COLUMT BECORDE, DESEMBNT AS RECORDED IN BOOK 2577, PAGE TOD DOES NOT AFFECT THE PARASTY PARKEZ, ORTHE ACCESS & UTILITY EXSENDENT AND IS SHOWN HERSON).
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- 11. SUBECTTO DED OF EASHERT FROM ALGERE GOATED, AND WITE, MAIT VERWON GOATEN TO FANCY FRAM WATER & SEMPE DISTRICT DATED 6/20/12956 AND RECORDED 6/30/12959 IN BOOK 355, PAGE DO GRAMES COUNTY RECORDED, (BUSDMENT AS RECORDED NA BOOK 356, PAGE DO GRAMES COUNTY RECORDED, (BUSDMENT AS RECORDED NA BOOK 356, PAGE DO BOOK 356
- 12. SHRED TO ANY MATTERS AS MAY RE SHOWN ON PAIT OF QUALL HOLLOW SHROWS DEPONS ON PRIVES RECORDED 1272/2016 IN PLAY FROME P. PAGE 75. GRAVES COLUMN PRECENTS. (THIS YORDS THE SAGRANGH SHOWN ON PAIR AND THE ACT THIS YORD HOLD ESS. PAGE 639) (PANTIES AS SHOWN IN PAUT IN DOOR ESS. PAGE 639) (PANTIES AS SHOWN IN PAUT IN DOOR ESS. PAGE 639) (PANTIES AS SHOWN IN PAUT IN DOOR ESS. PAGE 639) (PANTIES AS SHOWN IN PAUT IN DOOR ESS. PAGE 639) (PANTIES AS SHOWN IN PAUT IN DOOR ESS. PAGE 639) (PANTIES AS SHOWN IN PAUT IN DOOR ESS. PAGE 639) (PANTIES AS SHOWN IN PAUT IN PAUT OF THE ACTES AS THE PAGE 639).

## LEGAL DESCRIPTIONS

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FOLLOWS.

POD NWRGO DOWN

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# PROPOSED 30" / VARIABLE WIDTH ACCESS & UTILITY EASEMENT

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KENTUCKY RSA PARTNERSHIP

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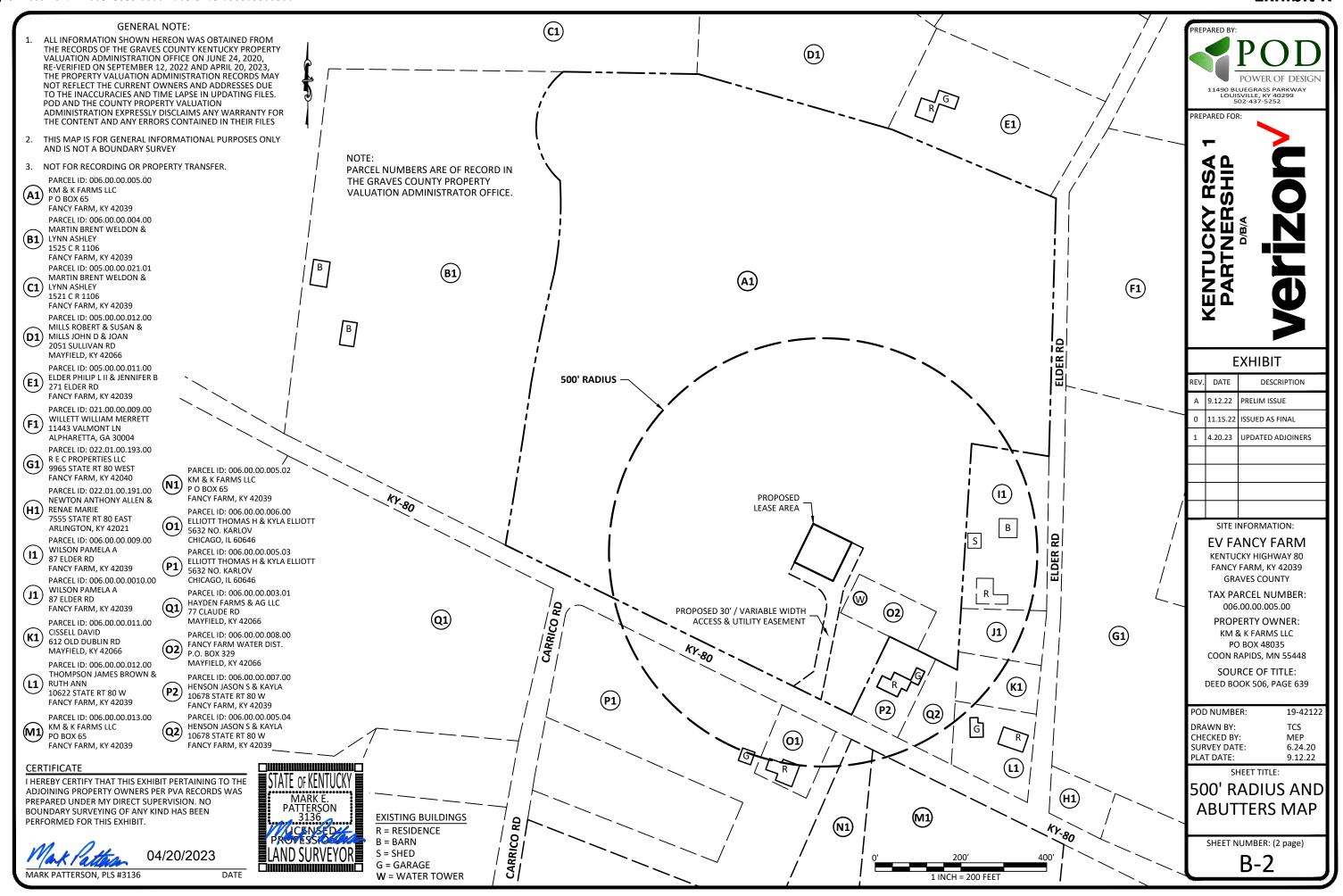
PROPERTY OWNER
NA 2 K FARMS LLC
P O BOX ARDS
COON RUPIDS, MM SS448
SOURCE OF TITLE
DEED BOOK 506, PAGE 639 EV FANCY FARM
XENTUCKY HORMAY 20
FANCY FARM, XY 42029
GRAVES COUNTY MUNINATURE OFFERS TAX PARCEL NUMBER: DATE DESCRIPTION SITE SURVEY

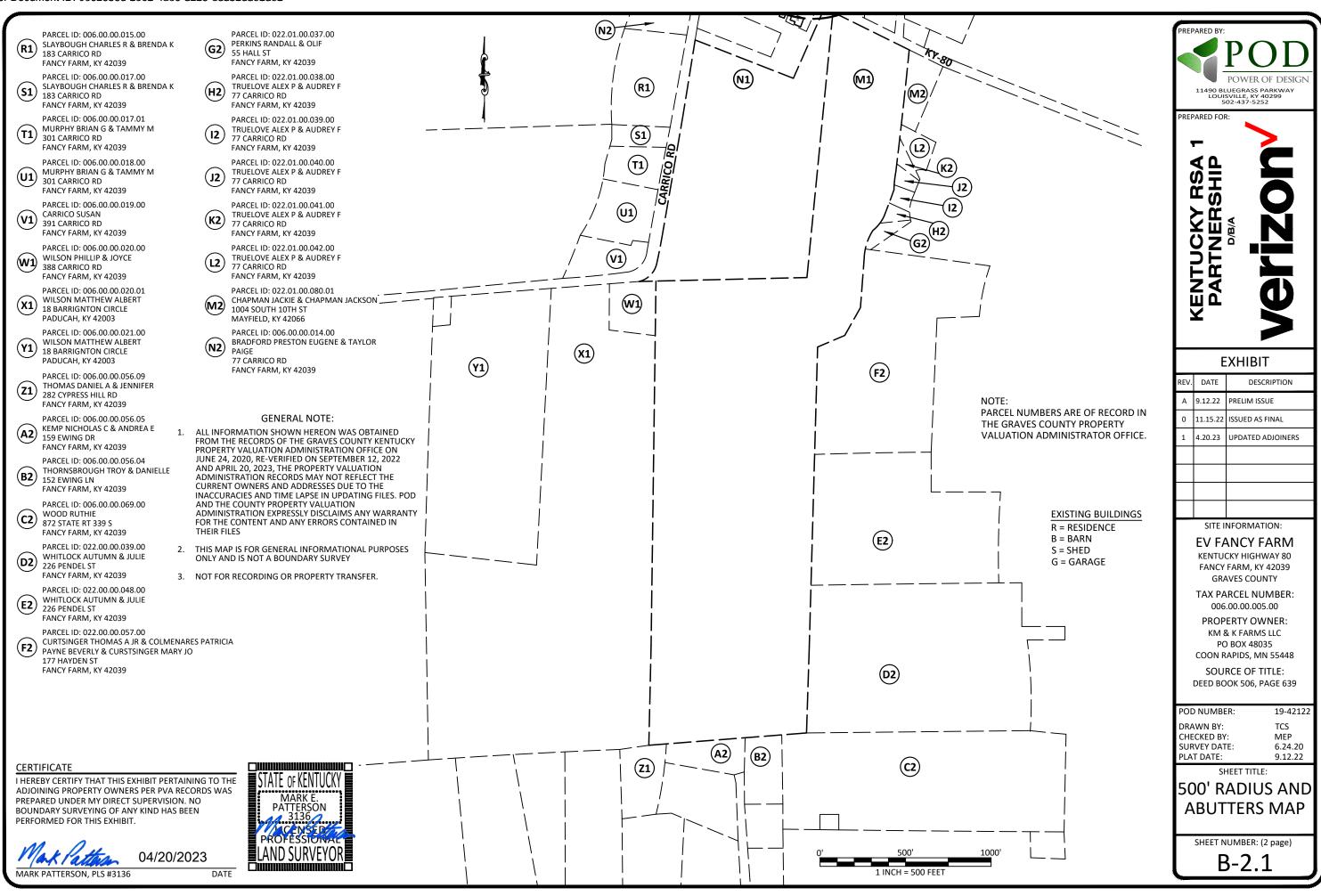
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SITE SURVEY
THE DOES NOT REPRESENT A
BOUNDARY SURVEY OF THE
PARENT PARCEL

HEET NUMBER (2 pages)

B-1.1





## **Exhibit L**

## **EV Fancy Farm Notification List**

KM & FARMS LLC PO BOX 65 FANCY FARM, KY 42039

MARTIN BRENT WELDON & LYNN ASHLEY 1521 C R 1106 FANCY FARM, KY 42039

MARTIN BRENT WELDON & LYNN ASHLEY 1521 C R 1106 FANCY FARM, KY 42039

MILLS ROBERT & SUSAN & MILLS JOHN D & JOAN 2051 SULLIVAN RD MAYFIELD, KY 42066

ELDER PHILIP L II & JENNIFER B 271 ELDER RD FANCY FARM, KY 42039

WILLETT WILLIAM MERRETT 11443 VALMONT LN ALPHARETTA, GA 30004

R E C PROPERTIES LLC 9965 STATE RT 80 WEST FANCY FARM, KY 42040

NEWTON ANTHONY ALLEN & RENAE MARIE 7555 STATE RT 80 EAST ARLINGTON, KY 42021

WILSON PAMELA A 87 ELDER RD FANCY FARM, KY 42039

CISSELL DAVID 612 OLD DUBLIN RD MAYFIELD, KY 42066

THOMPSON JAMES BROWN & RUTH ANN

10622 STATE RT 80 W FANCY FARM, KY 42039

ELLIOTT THOMAS H c/o KYLA ELLIOTT 5632 NO. KARLOV CHICAGO, IL 60646

HAYDEN FARMS & AG LLC 77 CLAUDE RD MAYFIELD, KY 42066

FANCY FARM WATER DISTRICT PO BOX 329 MAYFIELD, KY 42066

HENSON JASON S & KAYLA 10678 STATE RT 80 W FANCY FARM, KY 42039

SLAYBOUGH CHARLES R & BRENDA K 183 CARRICO RD FANCY FARM, KY 42039

MURPHY BRIAN G & TAMMY M 301 CARRICO RD FANCY FARM, KY 42039

CARRICO SUSAN 391 CARRICO RD FANCY FARM, KY 42039

WILSON PHILLIP & JOYCE 388 CARRICO RD FANCY FARM, KY 42039

WILSON MATTHEW ALBERT 18 BARRIGNTON CIRCLE PADUCAH, KY 42003

THOMAS DANIEL A & JENNIFER 282 CYPRESS HILL RD FANCY FARM, KY KEMP NICHOLAS C & ANDREA E 159 EWING DR FANCY FARM, KY 42039

THORNSBROUGH TROY & DANIELLE 152 EWING LN FANCY FARM, KY 42039

WOOD RUTHIE 872 STATE RT 339 S FANCY FARM, KY 42039

WHITLOCK AUTUMN & JULIE 226 PENDEL ST FANCY FARM, KY 42039

CURTSINGER THOMAS A JR & COLMENARES PATRICIA PAYNE BEVERLY & CURSTSINGER MARY JO 177 HAYDEN ST FANCY FARM, KY 42039

PERKINS, RANDALL & OLIF 55 HALL ST FANCY FARM, KY 42039

TRUELOVE ALEX P & AUDREY F 77 CARRICO RD FANCY FARM, KY 42039

CHAPMAN JACKIE & CHAPMAN JACKSON 1004 SOUTH 10TH ST MAYFIELD, KY 42066

BRADFORD PRESTON EUGENE & TAYLOR PAIGE 77 CARRICO RD FANCY FARM, KY 42039

## **Exhibit M**



www.clarkquinnlaw.com

Russell L. Brown Attorney at Law rbrown@clarkquinnlaw.com 320 N. Meridian St., Ste. 1100 Indianapolis, IN 46204 (317) 637-1321 main (317) 687-2344 fax

April 21, 2023

## Notice of Proposed Construction of Wireless Communications Facility Site Name: Fancy Farm

Cellco Partnership, d/b/a Verizon Wireless and VB BTS II, LLC / Vertical Bridge is filing an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located on Kentucky Highway 80, Fancy Farm, Kentucky 42039, (North Latitude: (36° 48' 09.61, West Longitude 88° 47' 54.21"). The proposed facility will include a 290-foot tall antenna tower, plus a 5-foot lightning arrestor and related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

This notice is being sent to you because the County Property Valuation Administrator's records indicate that you may own property that is within a 500' radius of the proposed tower site or contiguous to the property on which the tower is to be constructed. You have a right to submit testimony to the Kentucky Public Service Commission ("PSC"), either in writing or to request intervention in the PSC's proceedings on the application. You may contact the PSC for additional information concerning this matter at: Kentucky Public Service Commission, Executive Director, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2023-00133 in any correspondence sent in connection with this matter.

We have attached a map showing the site location for the proposed tower. Applicant's radio frequency engineers assisted in selecting the proposed site for the facility, and they have determined it is the proper location and elevation needed to provide quality service to wireless customers in the area. Please feel free to contact us at 317-637-1321 if you have any comments or questions about this proposal.

Sincerely,

Russell L. Brown

Attorney for Applicant

RLB/jdj enclosure

## Site Location Map







9589 0710 5270 0167 3920 45



MARTIN BRENT WELDON & LYNN ASHLEY 1525 C R 1106 FANCY FARM, KY 42039



## **CERTIFIED MAIL**



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KM & FARMS LLC PO BOX 65 FANCY FARM, KY 42039



## **CERTIFIED MAIL**



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MARTIN BRENT WELDON & LYNN ASHLEY 1521 C R 1106 FANCY FARM, KY 42039





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## **CERTIFIED MAIL**



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ELDER PHILIP L II & JENNIFER B 271 ELDER RD FANCY FARM, KY 42039

## Clark Quinn 1rk, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL**



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R E C PROPERTIES LLC 9965 STATE RT 80 WEST FANCY FARM, KY 42040

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WILSON PAMELA A 87 ELDER RD FANCY FARM, KY 42039





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CISSELL DAVID 612 OLD DUBLIN RD MAYFIELD, KY 42066

## Clark Quinn ark, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL®**



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THOMPSON JAMES BROWN & RUTH ANN 10622 STATE RT 80 W FANCY FARM, KY 42039

## Clark Quinn ark, Quinn, Moses, Scott & Grahn, LLP

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APR 21 2023

ELLIOTT THOMAS H & KYLA ELLIOTT 5632 NO. KARLOV CHICAGO, IL 60646



## **CERTIFIED MAIL**



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APR 21 2023

HAYDEN FARMS & AG LLC 77 CLAUDE RD MAYFIELD, KY 42066



Russell L. Brown Attorney at Law rbrown@clarkquinnlaw.com 320 N. Meridian St., Ste. 1100 Indianapolis, IN 46204 (317) 637-1321 main (317) 687-2344 fax

June 6, 2023

Notice of Proposed Construction of Wireless Communications Facility Site Name: Fancy Farm

Cellco Partnership, d/b/a Verizon Wireless and VB BTS II, LLC / Vertical Bridge is filing an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located on Kentucky Highway 80, Fancy Farm, Kentucky 42039, (North Latitude: (36° 48' 09.61, West Longitude 88° 47' 54.21"). The proposed facility will include a 290-foot tall antenna tower, plus a 5-foot lightning arrestor and related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

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Sincerely,

Russell L. Brown

Attorney for Applicant

RLB/mnw enclosure

Site Location Map







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FIRST-CLASS

MILLS ROBERT & SUSAN & MILLS JOHN D & JOAN 2051 SULLIVAN RD MAYFIELD, KY 42066

## Clark Quinn 1rk, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL**



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FIRST-CLASS

WILLETT WILLIAM MERRETT 11443 VALMONT LN ALPHARETTA, GA 30004

## ClarkQuinn ark, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL**



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ELLIOTT THOMAS H c/o KYLA ELLIOTT 5632 NO. KARLOV CHICAGO, IL 60646



9589 0710 5270 0167 3921 13

FIRST-CLASS

FANCY FARM WATER DISTRICT PO BOX 329 MAYFIELD, KY 42066

## Clark Quinn ark, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL**



9589 0710 5270 0167 3921 20

FIRST-CLASS

HENSON JASON S & KAYLA 10678 STATE RT 80 W FANCY FARM, KY 42039

## Clark Quinn lark, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL**



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SLAYBOUGH CHARLES R & BRENDA K 183 CARRICO RD FANCY FARM, KY 42039





FIRST-CLASS

MURPHY BRIAN G & TAMMY M 301 CARRICO RD FANCY FARM, KY 42039

## Clark Quinn ark, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL**



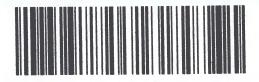
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CARRICO SUSAN 391 CARRICO RD FANCY FARM, KY 42039

## Clark Quinn lark, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL**



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WILSON PHILLIP & JOYCE 388 CARRICO RD FANCY FARM, KY 42039



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FIRST-CLASS

US POSTAGE MIPITNEY BOWES

WILSON MATTHEW ALBERT **18 BARRIGNTON CIRCLE** PADUCAH, KY 42003

## Slark Quinn ark, Quinn, Moses, Scott & Grahn, LLP

## **CERTIFIED MAIL®**



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FIRST-CLASS

THOMAS DANIEL A & JENNIFER 282 CYPRESS HILL RD **FANCY FARM, KY** 

## ark, Quinn, Moses, Scott & Grahn, LLP

## FIED MAIL



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KEMP NICHOLAS C & ANDREA E 159 EWING DR FANCY FARM, KY 42039





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Clark Quinn

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## **CERTIFIED MAIL**

FANCY FARM, KY 42039



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WOOD RUTHIE 872 STATE RT 339 S FANCY FARM, KY 42039

Clark Quinn
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## **CERTIFIED MAIL**



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www.clarkquinnlaw.com

Russell L. Brown Attorney at Law rbrown@clarkquinnlaw.com 320 N. Meridian St., Ste. 1100 Indianapolis, IN 46204 (317) 637-1321 main (317) 687-2344 fax

April 21, 2023

Via Certified Mail, Return Receipt Requested

Hon. Jesse Perry Graves County Judge/Executive 101 East South Street Mayfield, KY 42066

RE: Notice of Proposal to Construct Wireless Communications Facility Kentucky Public Service Commission Docket No. 2023-00133 Site Name: Fancy Farm

Dear Judge Perry:

Cellco Partnership, d/b/a Verizon Wireless and VB BTS II, LLC / Vertical Bridge is filing an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located on the north side of Kentucky Highway 80, Fancy Farm, Kentucky 42039, (North Latitude: (36° 48' 09.61, West Longitude 88° 47' 54.21"). The proposed facility will include a 290-foot tall antenna tower, plus a 5-foot lightning arrestor and related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

You have a right to submit comments to the PSC or to request intervention in the PSC's proceedings on the application. You may contact the PSC at: Executive Director, Public Service Commission, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2023-00133 in any correspondence sent in connection with this matter.

We have attached a map showing the site location for the proposed tower. Verizon Wireless' radio frequency engineers assisted in selecting the proposed site for the facility, and they have determined it is the proper location and elevation needed to provide quality service to wireless customers in the area. Please feel free to contact us with any comments or questions you may have.

Sincerely,

Russell L. Brown

Attorney for Applicant

## Site Location Map





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Hon. Jesse Perry Graves County Judge/Executive 101 East South Street Mayfield, KY 42066

SENDER: COMPLETE THIS SECTION	COMPLETE THIS SECTION ON DELIVERY
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# SITE NAME: Fancy Farm NOTICE SIGNS

The signs are at least (2) feet by four (4) feet in size, of durable material, with the text printed in black letters at least one (1) inch in height against a white background, except for the word "tower," which is at least four (4) inches in height.

Cellco Partnership, d/b/a Verizon Wireless proposes to construct a telecommunications **tower** on this site. If you have questions, please contact Clark, Quinn, Moses, Scott & Grahn, LLP, 320 N. Meridian Street, Indianapolis, IN 46204; 317-637-1321, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2023-00133 in your correspondence.

Cellco Partnership, d/b/a Verizon Wireless proposes to construct a telecommunications **tower** on this site. If you have questions, please contact Clark, Quinn, Moses, Scott & Grahn, LLP, 320 N. Meridian Street, Indianapolis, IN 46204; 317-637-1321, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2023-00133 in your correspondence.



VIA EMAIL: classifieds@messenger-inquirer.com

**Exhibit P** 

Robert B. Scott
Charles R. Grahn
Frank D. Otte\*
John "Bart" Herriman
William W. Gooden\*\*
Michael P. Maxwell
Russell L. Brown\*\*
Jennifer F. Perry
Keith L. Beall
N. Davey Neal
Travis W. Cohron
Maggie L. Sadler
Kristin A. McIlwain
Olivia A. Hess

Land Use Consultant Elizabeth Bentz Williams, AICP

> \*Also admitted in Montana †Also admitted in Kentucky

Registered Civil Mediator

Benton, KY 42025

Mayfield Messenger

86A Commerce Blvd.

RE: Legal Notice Advertisement

Site Name: Fancy Farm

To Whom It May Concern,

Please publish the following legal notice advertisement in the next available edition of the Mayfield Messenger Publication:

#### NOTICE

Cellco Partnership, d/b/a Verizon Wireless is filing an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located on the north side of Kentucky Highway 80, Fancy Farm, Kentucky 42039, (North Latitude: (36° 48' 09.61, West Longitude 88° 47' 54.21"). The proposed facility will include a 290-foot tall antenna tower, plus a 5-foot lightning arrestor and related ground facilities. You have a right to submit comments to the PSC or to request intervention in the PSC's proceedings on the application. You may contact the PSC at: Executive Director, Public Service Commission, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2023-00133 in any correspondence sent in connection with this matter.

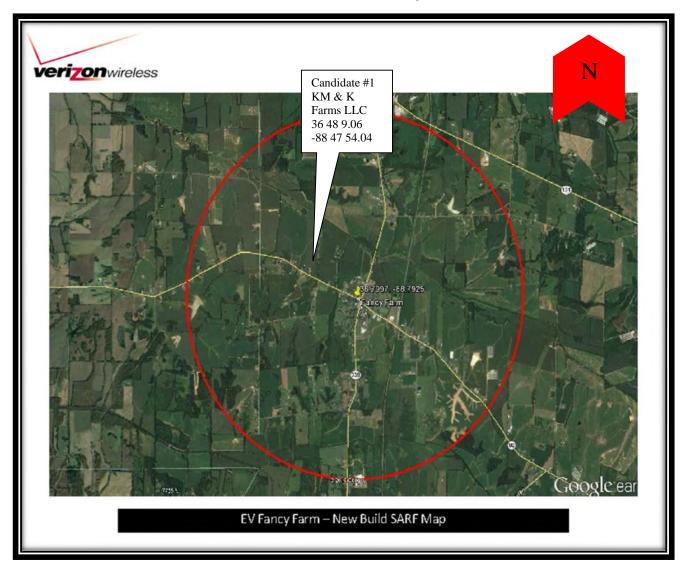
After this advertisement has been published, please forward a tearsheet copy, affidavit of publication, and invoice to Clark, Quinn, Moses, Scott & Grahn, LLC, 320 N. Meridian Street, Indianapolis, IN 46204 or by email to ebw@clarkquinnlaw.com. Please call me on my cell with any questions at 317-902-2187 if you have any questions. Thank you for your assistance.

Sincerely,

Thinksh But William

Elizabeth Bentz Williams, AICP

Search Area Map
MK & K Farms LLC - EV Fancy Farm





Tuesday, September 20, 2022

RE: Proposed Verizon Wireless Communications Facility

Site Name: EV Fancy Farm.

Type of Tower: 290' self-support Tower.

Location: HIGHWAY 80 FANCY FARM, KY 42039.

#### To Whom It May Concern:

As a radio frequency engineer for Verizon Wireless, I am providing this letter to state the need for a Verizon Wireless site called **EV Fancy Farm**.

The EV Fancy Farm site is proposed with the below objectives:

- 1. Offload 4G traffic from busy site to the northwest.
- 2. Offload 4G traffic from busy site to the northeast.
- 3. Improve 4G throughput to existing heavy data users.
- Improve 4G network reliability by increasing the amount of time our customers operate on 4G instead of 3G.

Currently the area is experiencing high demand for wireless high-speed data. Growth forecasts have triggered the need for an additional site in the area. The tower is needed to provide all Verizon customers in the area with the best experience on their 4G wireless devices.

Raw Land – Design plans for a new tower would provide tower height of **290'**. The new structure height was decided upon to best cover the offload area and interact with the existing Verizon sites. If we are limited to building a structure less than the proposed height, another tower would be needed in the vicinity in the near future. In addition, building a structure that is too short can cause existing taller sites to shoot over the proposed site and building a site that is too tall can cause the proposed site to shoot over existing sites. Both situations create a poor experience from a user perspective. The new structure will be placed near the center of the area with high traffic demand and offload the surrounding sites greatly. The new tower design meets stated objectives.

Verizon Wireless cares about the communities as well as the environment and prefers to collocate on existing structures when available. It can be noticed from any map that Verizon Wireless is currently collocated on many existing structures in the area. We prefer collocation due to reduced construction costs, faster deployment, and environment protection. However, Verizon Wireless was unable to find a suitable structure within the center of demand area to collocate the proposed **EV Fancy Farm** site.



Verizon Wireless design engineers establish search area criteria in order to effectively meet coverage objectives as well as offload existing Verizon cell sites. When met, the criterion also reduces the need for a new site to cover the area in the immediate future. Each cellular site covers a limited area, depending on site configuration and the surrounding terrain. Cell sites are built in an interconnected network; which means each cell site must be located so that their respective coverage areas are contiguous. This provides uninterrupted communications throughout the coverage area.

Since collocation is generally the most cost-effective means for prompt deployment of new facilities, Verizon Wireless makes every effort to investigate the feasibility for using existing towers or other tall structures for collocation when designing a new site or system expansion. However, collocation on an existing tower or tall structure is not always feasible due to location of existing cell sites. Cell sites are placed in a way so they provide smooth hand off to each other and are placed at some distance from each other to eliminate too much overlap. Too much overlap may result in a waste of resources and raise a system capacity overload concern.

This cell site has been designed, and shall be constructed and operated in a manner that satisfies regulations and requirements of all applicable governmental agencies that have been charged with regulating tower specifications, operation, construction, and placement, including the FAA and FCC.

Sincerely,

Michael Fahim.

RF Engineer, Verizon Wireless

Michael



STATE OF INDIANA

COUNTY OF HAMILTON

Subscribed and sworn to before me this 20th day of 4FFT., 2022.

**Notary Public** 

THOMAS E KROH Notary Public - Seal Hamilton County - State of Indiana Commission Number NP0732024 My Commission Expires Feb 28, 2029 Printed THOMAS E. KROH

County of Residence HAM CTON

My Commission expires:

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### **Exhibit Ra**



Tuesday, September 20, 2022

**RE: GRAVES County Zoning Plots** 

Site Name: EV Fancy Farm.

To Whom It May Concern:

This map is not a guarantee of coverage and may contain areas with no service. This map reflects a depiction of predicted and approximate wireless coverage of the network and is intended to provide a relative comparison of coverage. The depictions of coverage do not guarantee service availability as there are many factors that can influence coverage and service availability. These factors vary from location to location and change over time. The coverage areas may include locations with limited or no coverage. Even within a coverage area shown, there are many factors, including but not limited to, usage volumes, service, outage, and customer's equipment, and terrain, proximity to buildings, foliage, and weather that may impact service.

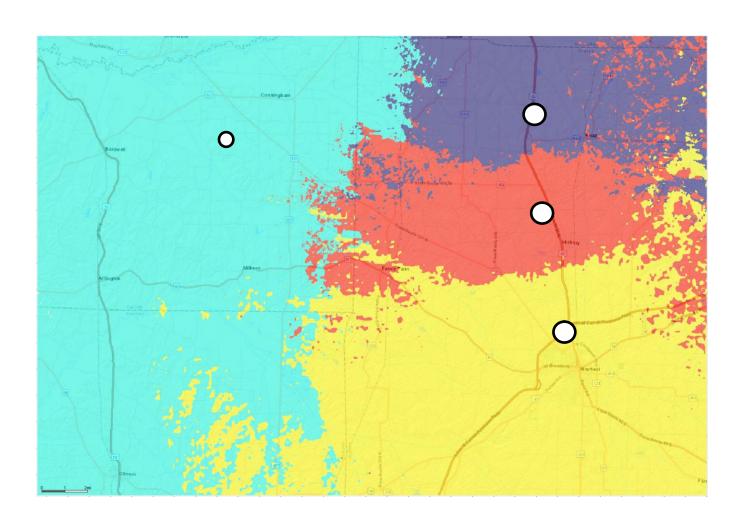
The proposed site is needed to offload capacity from existing sites. This map reflects the predicted coverage area that will be offloaded from existing sites and transferred to the proposed site.

Michael Fahim.

RF Engineer, Verizon Wireless



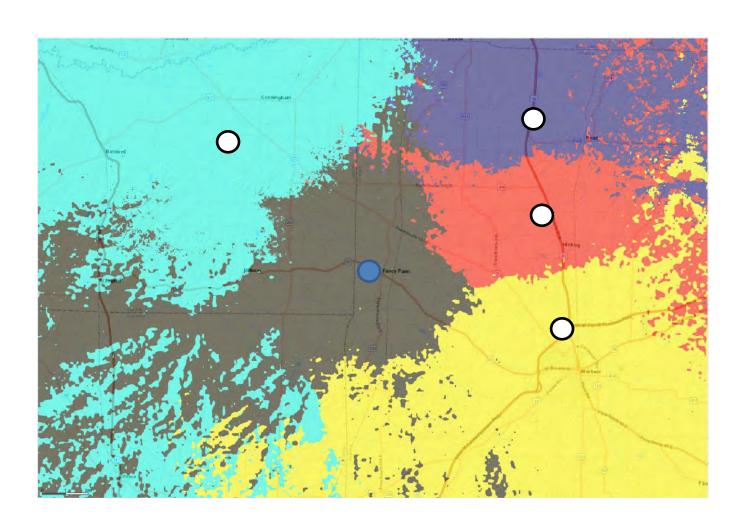
## **EV Fancy Farm Pre**



Legend:	
Existing Verizon Sites	0
Proposed Verizon Site	
Future Verizon Site	0
County Border	



## **EV Fancy Farm Post**



Legend:	
Existing Verizon Sites	0
Proposed Verizon Site	
Future Verizon Site	0
County Border	

# verizon /



## **Exhibit S List and Identity and Qualifications of Professionals**

Mark E. Patterson Professional Land Surveyor Kentucky License 3136 Power of Design Group, LLC 11490 Bluegrass Parkway Louisville, KY 40299

Mark E. Patterson Professional Engineer Kentucky License 16300 Power of Design Group, LLC 11490 Bluegrass Parkway Louisville, KY 40299

Nathan Andrew Ross Professional Engineer Kentucky License 35794 Valmont 1545 Pidco Dr. Plymouth, IN 46563

Vince Caprino Construction Manager Verizon Wireless 2421 Holloway Road Louisville, KY 40299

Michael Fahim RF Engineer Verizon Wireless 2421 Holloway Road Louisville, KY 40299 STATE OF INDIANA )
) SS:
COUNTY OF MARION )

### AFFIDAVIT OF CERTIFICATION COMMONWEALTH OF KENTUCKY PUBLIC SERVICE COMMISSION

I Russell L. Brown, attorney for Cellco Partnership, d/b/a Verizon Wireless do hereby certify that as the person supervising the preparation of this application that the all statements and information contained herein are true and accurate to the best of that person's knowledge, information, and belief formed after a reasonable inquiry for all information within this application.

Russell L. Brown

Attorney, for Cellco Partnership, d/b/a Verizon Wireless

STATE OF INDIANA, COUNTY OF MARION, SS:

Subscribed and sworn to before me this 6th day of June 202

(Notary Public

Printed Name of Notary: Elizabeth Bentz Williams

My commission expires: November 18, 2028

My County of Residence: Marion

Commission #: <u>0639620</u>