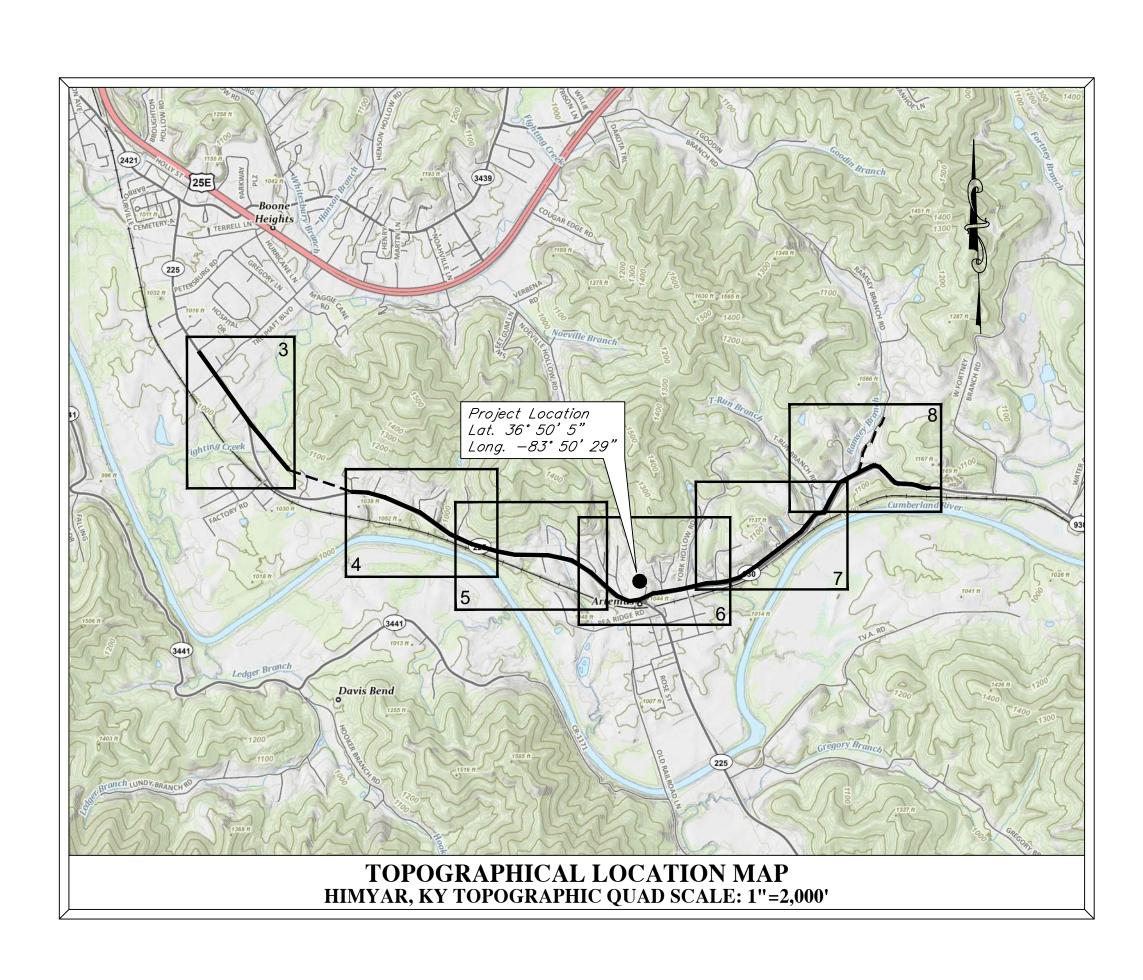
KNOX COUNTY UTILITY COMMISSION BARBOURVILLE CONNECTION-KY 225 KNOX COUNTY, KENTUCKY





INDEX OF SHEETS

DESCRIPTION	SHEET NO.
COVER SHEET	1
GENERAL NOTES	2
BARBOURVILLE CONNECTION - WEST	3
ALTERNATE NO. 1	3-4
BARBOURVILLE CONNECTION - EAST	4-8
KY 225 PUMP STATION SITE PLAN	PS-1
KY 225 PUMP STATION	PS-2
KY 225 PUMP STATION DETAILS	PS-3
STRUCTURAL PLANS AND DETAILS	S1-S5
ELECTRICAL PLANS AND DETAILS	E1-E4
DIRECTIONAL BORE PLAN PROFILE	B1-B3
MISCELLANEOUS DETAILS	D1-D3



- 2. The Contractor shall be responsible for coordinating all construction work with local utility—companies and other concerned parties.
- 3. Existing buried utilities are shown on the drawings in their general location utilizing the best available information. Before construction begins near or through existing utilities (i.e. Gas Co., Telephone Co., etc.) each utility company shall be notified, a request for the exact location of the utility shall be made, and permission and permission to proceed with construction obtained. The utility shall be given at least one week advance notice for location verification. BUD provides a clearinghouse service for member utilities relative to underground utilities location. The Contractor shall contact BUD at telephone no. 1—800—752—6007 or 811.
- 4. Before construction begins through any property, the Contractor shall make himself aware of the exact location of construction through the property and the bounds of the permanent and temporary construction easements.
- 5. The Contractor shall have on hand at the job site 11 1/4°, 22 1/2°, 45° and 90° bends for use where necessary for proper installation.
- 6. Pipe joint deflection shall not exceed 2°. Bending of PVC pipe will not be allowed.
- 7. At some locations, the Contractor may be required to provide extra cover over line. Cost of extra cover is to be included in unit price bid for line installation and no separate payment will be made for such extra cover. All such locations are shown on the plans.
- 8. Connecting new lines to existing lines or to work in other contracts is subsidiary to the contract unless specifically itemized in the Bid Schedule. It includes fittings, sleeves, etc., but does not include gate valves, which are an extra pay item.
- 9. All fittings, thrust restraint and appurtenances to construct the pipelines as shown shall be included in the unit cost for the pipe and are not separate pay items.
- 10. The pipe lengths have been estimated as close as possible. The Contractor shall be responsible for ordering pipe quantities necessary for installation to the limits as shown on the Drawings unless otherwise instructed. Any left—over pipe quantities shall be the property of the Contractor unless other arrangements are made. The Owner shall not be responsible for re—stocking or other charges associated with the left over pipe.
- 11. Ductile iron pipe shall be installed in accordance with Standard AWWA C150/ANSI A21.50 Laying Condition Type 3 unless otherwise noted.
- 12. All driveways that are cut shall be backfilled with KTC #8 or 9-M and shall be included in the unit price for pipe installation.
- 13. All open cut streets and roads and trenches cut in existing pavements shall be backfilled with compacted crushed stone or DGA in accordance with the miscellaneous details drawings.
- 14. Paved driveways shall be free—bored. Free bore unit prices are contained in Bid Schedule. The material in which the free bore is made is unclassified.
- 15. It is the responsibility of the Contractor to comply with all regulations regarding the effect on the environment from the discharge of chlorinated water. See Technical Specification 15103 Subsection 3 for methods of sterilization and for disposing of heavily chlorinated water.
- 16. The time period for pressure testing in this project shall be 6 hours.
- 17. Final cleanup is not a separate pay item in the Bid Schedule which includes seeding and straw mulch along the entire length of the pipeline trench. A power landscape rake shall be used for seedbed preparation. See the Specifications for specific requirements.
- 18. Tracer wire and marking tape shall be installed with the PVC pipe. See Technical Specification 15100, and the miscellaneous details drawings.
- 19. During the process of tapping asbestos concrete mains, the contractor shall conform to OSHA regulations governing the handling of hazardous waste. Pieces of asbestos concrete resulting from the tap shall be double bagged, placed in a rigid container and disposed of in an approved landfill.
- 20. The pipeline shall be "swabbed" prior to pressure testing and sterilization. Pipeline swabbing is not a separate bid item but shall be included in the unit price for pipe. See Technical Specification 15103 of the Specifications.
- 21. Final Cleanup payment is for transmission and distribution pipelines only. It does not include service lines.
- 22. Locations where pipeline is to be installed on state road right—of way are approximately delineated on the drawings. The Contractor, along with the Engineer's Representative, shall determine, precisely, the field locations for transitions between private easements, and state and county road rights—of—way.
- 23. All pipelines installed in the ditchline on state or county rights—of—way shall have 42" minimum cover over top of pipe.
- 24. The pipeline trench width will be strictly enforced. See Technical Specification 15100 for trench width requirements.
- 25. The GENERAL CERTIFICATION NATIONWIDE PERMIT #12 UTILITY LINE BACKFILL AND BEDDING is contained in the Specifications. The Contractor shall read, understand and comply with the requirements and procedures. All crossings of streams that appear as a blue line on a USGS 7.5 minute topographical map shall be accomplished in accordance with: PERMIT #12, UTILITY LINE BACKFILL AND BEDDING. It is the intent of the plans to identify a stream crossing at each blue line stream. Small creek crossings, less than 15 feet measured from top of bank to top of bank, may be accomplished by trenching when the stream is in a no-flow condition. If the stream is in a flow condition, the crossing shall be accomplished by directional boring or other method that complies with the General Certification and is approved by the Engineer. Specific details for stream crossings are contained in the Miscellaneous Details. Bid items for specific stream crossings may be contained in the Bid Schedule with the type of crossing shown on the Plan Sheets. Payment shall be "Each" for directional bores of small stream crossings. All small stream crossings in the project shall be considered the same regardless of width (up to 15 L.F.) or depth. It is the responsibility of the Contractor to determine an average unit price that will be used for payment for each instance a blue line stream is crossed. Stream crossings may be added, for extended lines beyond those shown on the plans, at the same unit price providing the crossings are reasonably similar to those in the initial project. Stream crossings may be deleted, without effecting the unit price, if a line is deleted or shortened. Payment for specific bid item directional bored stream crossings shall be "Lump Sum".
- 26. Rough cleanup is included in the unit price for pipe installation and must be done before payment for pipe will be approved.
- 27. Do not cut fences except where specifically shown and noted.
- 28. The Contractor shall obtain and pay for all grading, storm water, etc. permits, if any required to complete the work. The contractor shall maintain compliance with all conditions, limitations and stipulations of all permits. The contractor shall not commence work, except mobilization, until he has obtained all required permits for said work. The contractor shall supply the owner with copies of all permits within 24 hours of receipt. A KPDES Storm Water Discharge Permit will be required for this project. The contractor shall fill out, sign and submit the Notice of Intent (NOI) and the Notice of Termination (NOT). The Notice to Proceed will not be issued until the Permit has been provided.
- 29. All work shall be provided in accordance with all terms of the General Construction Permit and the Floodplain Construction Permit as issued for the Project by the Kentucky Department for Environmental Protection, Division of Water. The Owner will secure said Construction Permits and deliver a copy of each to the Contractor, to be maintained on—site at all times during construction.

GENERAL NOTES (CONT.)

- 30. All work shall be provided in compliance with all applicable local, state and national building codes.
- 31. All work shall be executed in compliance with the current workplace safety regulations of the U.S. Department of Labor, Occupational Safety and Health Administration (O.S.H.A.).
- 32. The Contractor shall restrict all construction activities to within the limits of the public right—of—way and the private easements and fee parcels unless otherwise approved by the Owner in writing. The Contractor shall be solely liable for any and all Work he performs outside of the boundaries of the public road right—of—way and the private easements and fee parcels provided by the Owner.
- 33. The Contractor is solely responsible for determination of the existence and location of any and all other buried utilities in the vicinity of his Work. Utilities shown on the Project Drawings are purported to be approximate only and not warranted to be complete nor accurately located. Additional buried utility lines, other than as shown on the Project Drawings, may exist in the vicinity of the Project work. The Contractor shall contact local utilities and/or locating service at least 48 hours prior to commencing work on the Project.
- 34. The Contractor shall be responsible for all traffic control measures necessary to the safe execution of his work, including but not limited to flaggers, traffic signage, barricades, construction fencing and nighttime warning lights. Traffic safety provisions shall be employed by the Contractor in accordance with the Standards of the appropriate State and local public highway authorities.
- 35. All excavation and all boring shall be considered unclassified excavation and unclassified boring. No additional payment shall be due and payable to the Contractor for dewatering of pipe trenches/excavations or for excavation and removal of rock or for boring casing through rock.
- 36. All water main fittings shall be ductile iron, mechanical joint compact fittings for water service complying with AWWA Standard C153. Unless otherwise specifically shown or noted, no PVC fitting, other than in—line repair couplings, will be accepted.
- 37. All water main fittings shall be anchored with poured concrete thrust blocks as shown in the miscellaneous details drawings. Wrap fittings in minimum 5—mil plastic (PVC) wrap prior to forming and pouring the block.
- 38. Prior to cutting existing driveways, the Contractor shall notify the property owner/occupant at least 24 hours in advance and shall schedule his Work such to restrict access to not more than 2 hours in one (1) day.
- 39. The Contractor shall repair/replace any and all existing utility lines and equipment damaged by the Contractor's Work, to the satisfaction of the damaged utility and at no additional cost to the Owner.
- 40. The Contractor shall protect all drainage culverts in the vicinity of his work and shall repair or replace all culverts damaged by his Work and at no additional cost to the Owner. All existing culverts may not be shown/noted on the Project Drawings.
- 41. Existing utility lines may be cathodically protected. The installation of all ductile iron pipe, fittings and appurtenances within 100' of cathodically protected utility lines shall comply with AWWA Standard C105 (Polyethylene Encasement), latest revision, and at no additional cost to the Owner. This requirement will be specifically applicable to all new iron pipe located within 100' of the cathodically protected new primary booster station.
- 42. There are no sanitary sewers or drains known to exist in the vicinity of the proposed new water main. If unforeseen sewer or other sanitary facility is encountered, the Engineer shall direct the relocation of the water main to provide separation and/or other protection of the water main in accordance with terms of the Kentucky Department for Environmental Protection, Division of Water Construction Permit. The Contractor shall provide relocation of the water main as directed by the Engineer and the Contract Price adjusted only by/to the number of Bid Item units actually provided.
- 43. No water service shall be activated until the new work has been completed, sterilized, and tested in accordance with the Contract Documents and accepted in writing by the Owner.

ENVIRONMENTAL NOTES

- 1. When crossing all streams, silt barriers, ie. straw bales or silt fences, shall be put in place to prevent sediment runoff into stream. Conventional stream crossings shall be accomplished during low flow periods. Stream banks shall be reseeded with native vegetation beneficial to wildlife immediately following completion of the stream crossing. Disturbed surfaces shall be restored to original contours and excess materials removed to a properly confined area.
- 2. If the removal of any trees greater than (6) inches in diameter at breast height is required, The tree removed shall be accomplished between October 15 and March 31.
- 3. Any excavation by the Contractor that uncovers a historical or archaeological artifact shall be immediately reported to the Owner and Engineer. Construction shall be temporarily halted pending the notification process and further directions after consultation with the State Historic Preservation Ofiicer (SHPO).

HIGHWAY DEPARTMENT NOTES

- 1. Underground utilities installed inside state right of way shall be located within 3—5 feet from the edge of the right of way unless otherwise shown on the plans.
- 2. Underground utilities on state right of way shown more than 5 feet from the edge of the right of way shall be installed with a minimum depth of cover of 42 inches.
- 3. Underground utilities crossing any paved driveway inside state right of way shall be installed by boring unless written permission to open cut is obtained from the property owner.
- 4. Underground utilities shall not be installed in embankment fills or between edge of pavement and ditchline unless specifically noted on permitted plans.
- 5. Fire Hydrants or utility service boxes should be located within 2 feet from the edge of right of way line, or off right of way.
- 6. Contact KTC-DOH District Office prior to beginning work.
- 7. All affected KYTC ditchlines shall remain free of excess silt or erosion and constructed to the normal typical section of the roadway with a minimum depth of 18 inches from the shoulder break point.
- 8. All necessary steps shall be taken to prevent erosion or siltation of the public right of way, adjoining property and waterways.
- 9. All traffic control for construction and maintenance operations will conform to the *Manual on Uniform Traffic Control Devices*. All construction and maintenance operations must be planned with full regard to safety to keep traffic interference to an absolute minimum. Closure of intersecting streets, road approaches or other access points is to be held to a minimum.
- 10. All areas disturbed by utility installation should be kept to a minimum and restoration methods should be in accordance with Kentucky Transportation Cabinet's 2012 Standard Specifications for Road and Bridge Construction.

KNOX COUNTY UTILITY COMMISSIO

BARBOURVILLE CONNECTION - KY 22

KNOX COUNTY, KENTUCKY

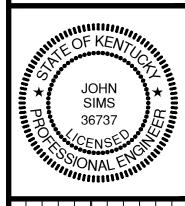
PROJECT NO. **2020132**

SHEET NO.



SHEET NO.





CHECKED BY: JDS
DATE: July 2021
SCALE: 1"=100'
REVISIONS

KENVIRONS Civil & Environmental Engineers



PROJECT NO. **2020132**

SHEET NO.



SHEET NO.

KNOX COUNTY UTILITY COMMISSION

BARBOURVILLE CONNECTION - KY 224

KNOX COUNTY, KENTUCKY

DRAWN BY: JRP
CHECKED BY: KDT
CHECKED BY: JDS
DATE: July 2021
SCALE: 1"=100'
REVISIONS

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DATE: July 8
SCALE: 1"=10
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PROJECT NO. **2020132**

SHEET NO.

KNOX COUNTY UTILITY COMMISSION
BARBOURVILLE CONNECTION - KY 225
KNOX COUNTY, KENTUCKY

DRAWN BY: JRP CHECKED BY: KDT CHECKED BY: JDS DATE: July 2021 SCALE: 1"=100' REVISIONS

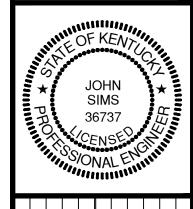
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PROJECT NO. **2020132**

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Civil & Environmental Engineers



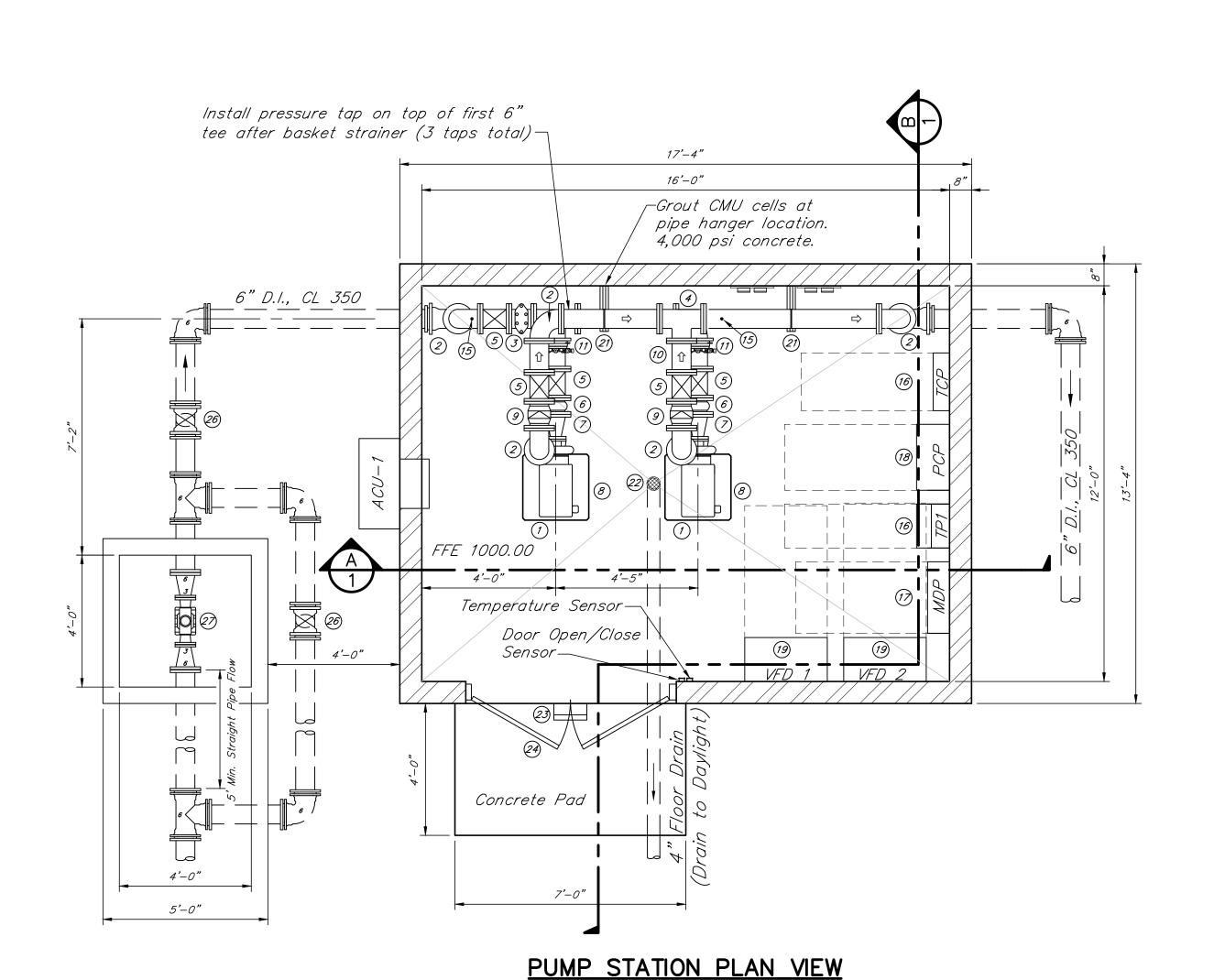
PROJECT NO. 2020132

SHEET NO.



> SHEET NO. PS-1

STATION SHEET NO. PS-2

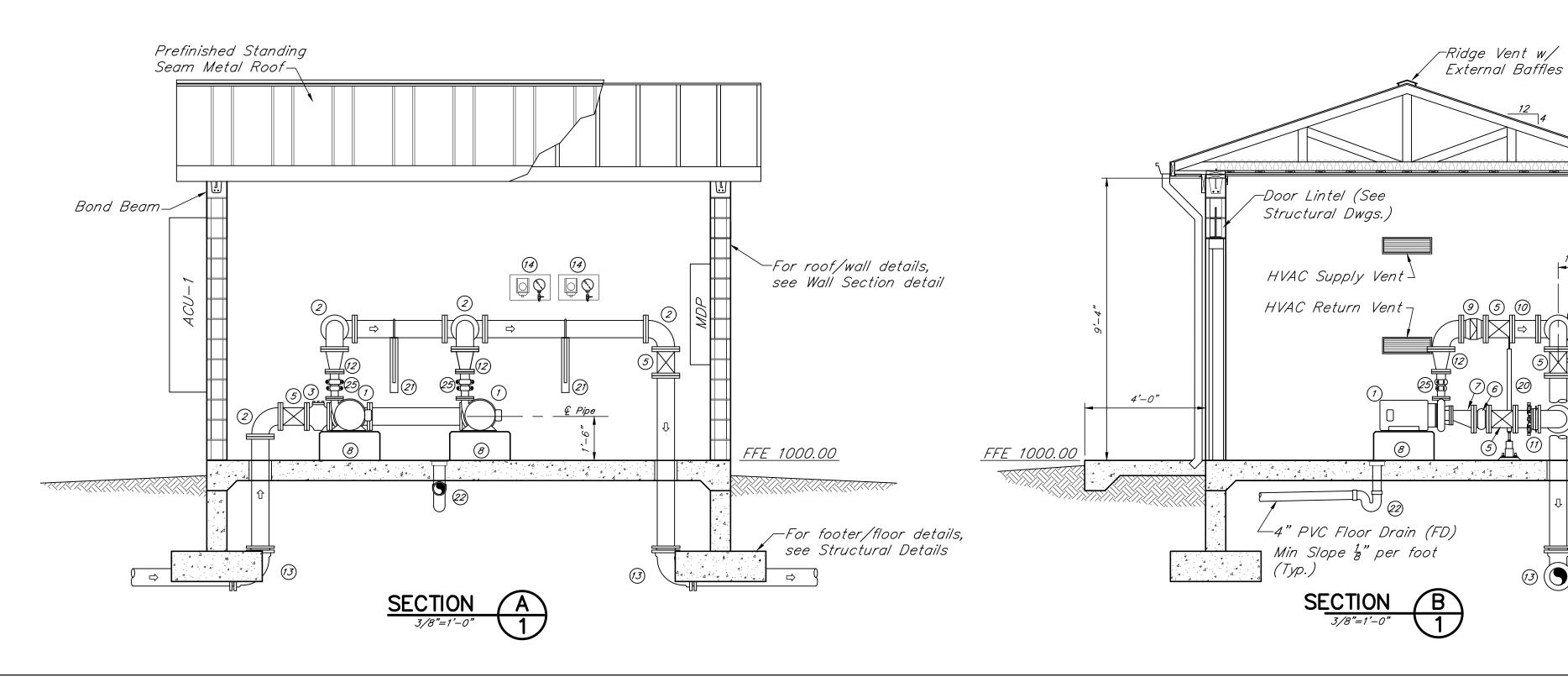


KY 225 PUMP STATION ITEM QTY. DESCRIPTION Pumps: 20 Hp, 480V/3PH/60Hz 300 GPM @ 165' TDH; 3500 RPM 4 6" 90° Elbow 6" Plate Strainer 2 6" Tee 6" Gate Valve 6" Metrasphere Coupling w/ Control Rods (or equal) 2 6"x3" Eccentric Reducer Pump Pedestal (Cast in Place) 6" Globe Style Silent Check Valve 10 2 8" Spool 6" Flanged Coupling Adapter (FCA) 12 2 6"x3" Concentric Reducer 13 2 6" 90° Elbow, MJ 14 3 Pressure Gauge w/ Pressure Transducer 15 | 4 | 1/4" Stop Cock 16 1 Telemetry Panel 17 | 1 | Main Distribution Panel | 18 | 1 | Pump Control Panel 19 2 Variable Frequency Drives 20 2 Pipe Supports Pipe Support Bracket w/ U-bolt 22 1 Floor Drain and 4" PVC Sch. 80 Drain Pipe w/ Trap 23 1 Outdoor Light Fixture w/ Dusk to Dawn Sensor 24 2 36" Insulated Steel Doors 3" Flanged Adapter (No. 41), 3" Flexible Coupling (Style 31), 25 2 3" to 2.5" Reducer (No. 53), 2.5" Flexible Coupling (Style 31), 2.5" Flanged Adapter (No. 41) (Victaulic or Equal) 26 2 6" Mechanical Gate Valve 27 1 3" Neptune MACH 10 Ultrasonic Meter

GENERAL NOTES

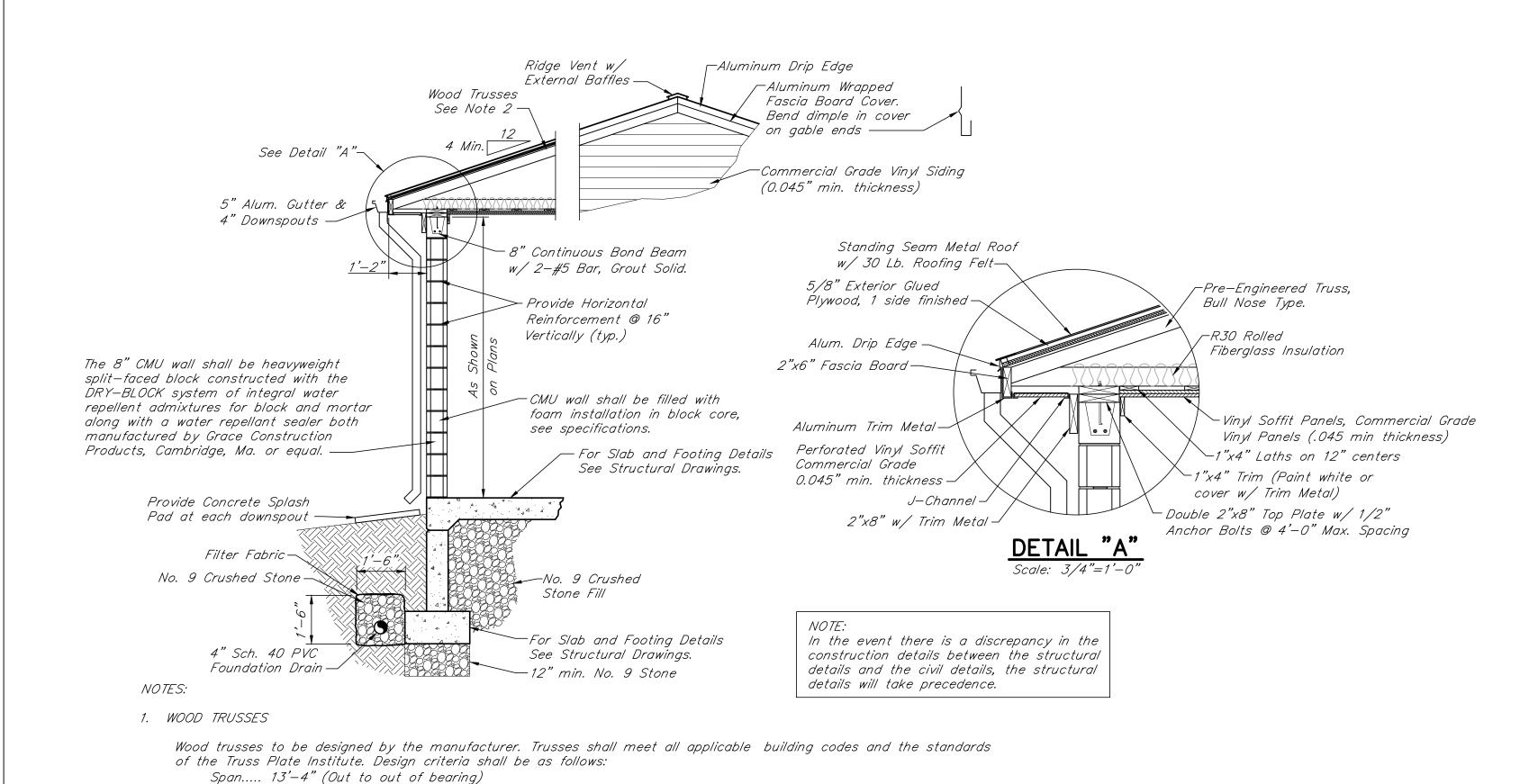
- 1. All yard piping shall be ductile iron, CL 350 pipe. All Ductile Iron Piping shall have restrained gaskets. All M.J. fittings, valves, etc. shall be restrained with EBAA IRON MEGALUG Series 1100 or approved equal.
- 2. The Contractor shall coordinate with the pump Supplier and Engineer regarding the base and other pump dimensions. This coordination is absolutely necessary to assure that the concrete pump pedestals are constructed to the desired dimensions.
- 3. All couplings and flanged coupling adaptors shall be rodded through the adjacent flanges and bolted securely.
- 4. Provide pipe sleeves for all penetrations of walls and floor.
- 5. Pipe drainage from any pump, valve, or device within the pump station shall utilize PVC conduit through the floor slab to the floor drain piping below slab.
- 6. Caulk all control joints, construction joints including slab to wall joint, and frame installations.
- 7. All conduits shall be aluminum. Seal the tubing raceways.
- 8. Use shark bite fittings with all tubing.
- 9. Construct a $\frac{3}{4}$ " chamfer at all construction joints and corners.
- 10. Floor shall be sloped to drain between 1/4" & 1/8" per foot.
- 11. Contractor shall be responsible for interior paint coatings. See Specification Section 09901 "High Performance Coatings" for Schedule.

—Install Concrete Splash Pads at each downspout



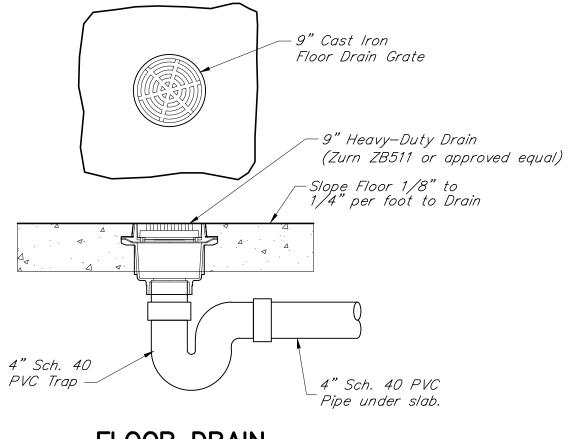
PROJECT NO. 2020132 SHEET NO.

PS-3

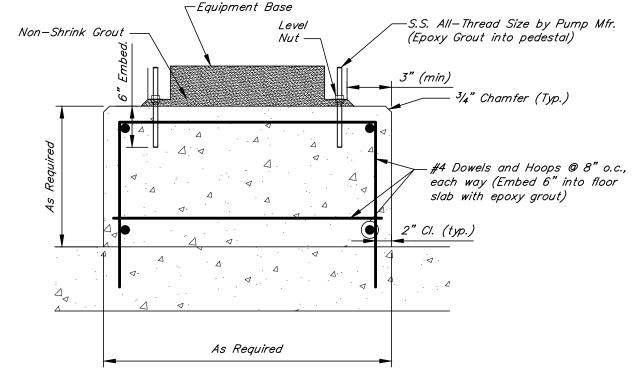


Top live load... 30psf 2. Provide lintels over all openings with lintel block grouted solid with 2-#5 Bars. Length of lintels shall be at least equal to the rough opening plus 8—inch bearing on each side of opening. The lintel block shall have the same face as the wall block.

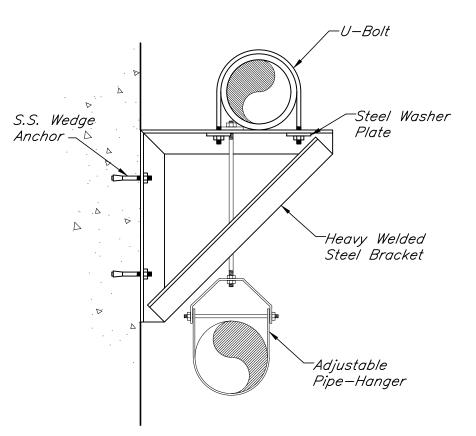
WALL SECTION Scale: 3/8"=1'-0"







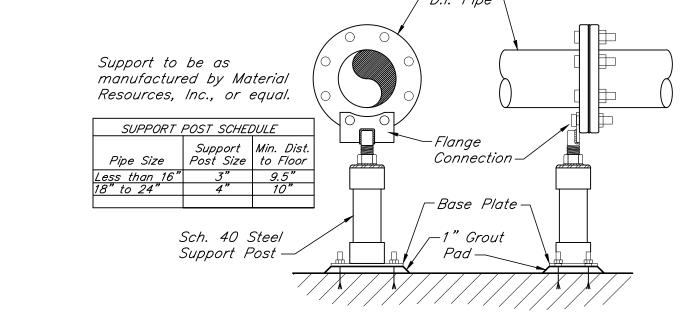
TYPICAL PUMP & EQUIPMENT PEDESTAL CONCRETE SUPPORT REINFORCEMENT

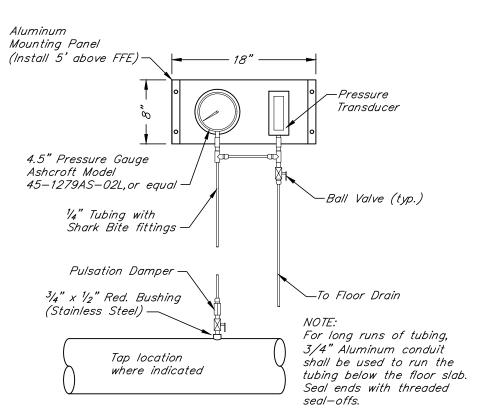


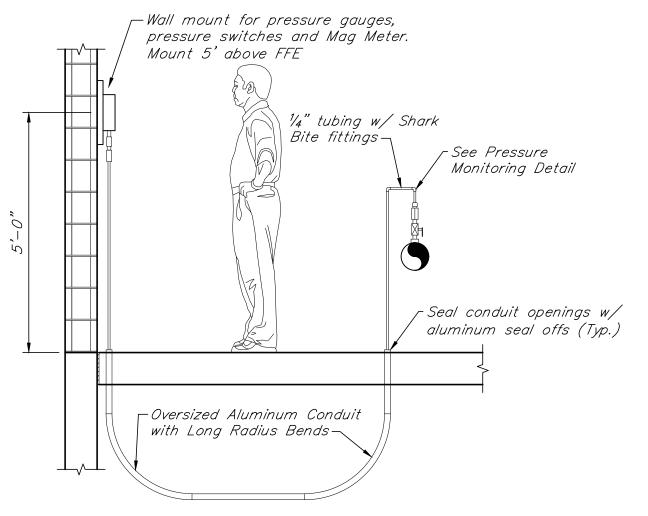
Spacing..... 24" o.c.

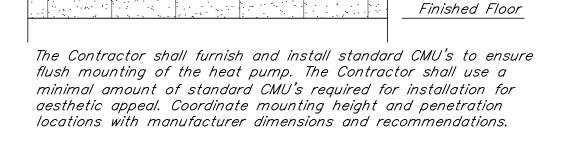
Top dead load... 15psf Bottom dead load... 15psf

Max Deflection... L/240 (where L=span)









CMU DETAIL AT HEAT PUMP

Scale: 1/2"=1'-0"

FLANGED PIPE SUPPORT

PRESSURE MONITORING PANELS

PRESSURE TAP CONNECTION

PUMP STATION DETAILS

Supply Air Opening

Return Air Opening

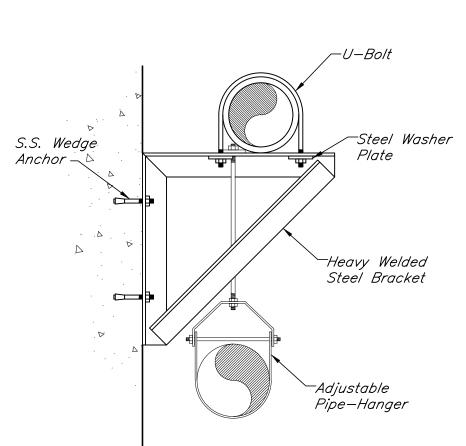
(Interior wall)

(Interior wall)

Heat Pump

Split Faced Block

Standard Block



PIPE BRACKET AND HANGER DETAIL

Mapped short period spectral response acceleration

Mapped 1 second spectral response acceleration

Design short period spectral response acceleration

Design 1 second period spectral response acceleration

MATERIAL STRENGTHS USED IN DESIGN

Seismic site class

Seismic design category

Basic structural system

Seismic response factor

Method of analysis

Seismic coefficient

Seismic importance factor

Seismic force resisting system

(for reference in calculations — see specifications or notes for actual material specifications) Concrete:

Class A (structural)(see specifications)	28	day	f'c	=	4,500	psi
class b (non-struct)(see specifications)	28	day	f'c	=	3,500	psi
Reinforcing bars (ASTM A615 OR A706 GRADE 60)			fy	=	60,000	psi
Welded wire fabric (ASTM A185)			fy	=	65,000	psl
Prestressing strand (ASTM A416 GRADE 270 LO LAX)			fu	=	270,000	psl
Deformed bar anchors (ASTM A496)			fy	=	80,000	psi
Structural steel sections W AND WT (ASTM A992)			fy	=	50,000	psi
Structural steel sections C, L, M, S, HP, MT and ST (ASTM A36)		fy	=	36,000	psi
Structural steel plates bars, and rods u.n.o. (ASTM A36)			fy	=	36,000	psi
Structural steel sections HSS (ASTM A500 GRADE B)			fy	=	46,000	psi
Structural steel pipe (ASTM A53 GRADE B)			fy	=	35,000	psi
Structural bolts (ASTM A325)			fu	=	120,000	psi
Concrete masonry (VARIOUS)			f'm	=	1,500	psi
Soil allowable bearing pressure for foundations			qa	=	3,000	psf
Rock allowable bearing pressure			qa	=	30,000	psf

1. The requirements of these general notes apply unless otherwise noted on plans or in

- 2. All dimensions of existing conditions shall be verified prior to commencing work. Discrepancies between existing conditions or between the drawings and specifications shall be communicated to the structural engineer and architect.
- 3. This structure is designed to be stable and self—supporting only when fully completed. Stability of the structure during construction is the responsibility of the contractor. All necessary temporary bracing required to stabilize and support the structure during all construction phases shall be furnished and installed by the contractor. If required,
- temporary bracing shall be designed by a licensed engineer employed by the contractor. 4. Construction loads imposed on the structural framing shall not exceed the design capacity of the framing at the time such loads are imposed.
- 5. Non—structural elements of the building (architectural finishes, masonry veneer and associated ties, insulation, sheathing, ductwork, piping, etc.) are generally not shown on these structural drawings. Certain non—structural elements that are shown on the structural drawings are shown for reference only. Non—structural elements shall be constructed as shown on the architectural and trade drawings.
- 6. Any material ordered or work performed prior to the engineer's review and approval of the shop drawings is at the contractor's sole risk.

FOUNDATIONS

100 psf

20 psf

15 psf

Ss = 0.347

S1 = 0.101

Sds = 0.352

Sd1 = 0.162

Bearing Wall System

Intermediate Reinforced Masonry Shear Walls

Equivalent Lateral Force Procedure

le = 1.25

Cs = 0.22

R=2

- 1. The foundations have been designed based on assumed bearing capacities.
- 2. Foundation design is based on an allowable bearing capacity of 2,000 psf for native soil
- (undercut as may be required) and controlled fill and 6,000 psf for bedrock. 3. If required, a qualified testing company shall be engaged by the contractor to verify bearing capacities prior to installing foundations.
- 4. All footings shall be supported on undisturbed soil, engineered fill or competent bedrock
- where indicated.
- 5. Fill shall be compacted to 98% of optimum laboratory density in accordance with ASTM D 698 Standard Proctor Method in maximum 8" lifts unless noted otherwise.
- 6. All piers and spread footings are centered on column centerlines and all wall footings are centered under walls unless indicated otherwise.
- 7. Location of existing foundations, if any are shown on drawings, are approximate. exact condition shall be verified at time of construction.
- 8. The structural engineer shall be notified if soft, loose or lower bearing capacity soils or
- rock are encountered. 9. Existing underground utilities in greas of foundation construction shall be located prior to construction of foundations. appropriate measures shall be taken to avoid damage to existing utilities and to ensure adequate foundation bearing around utilities.
- 10. Foundations shall not be placed on mud or muck, soft or loose soil, in standing water or on frozen ground.
- 11. All non-cantilever walls shall be be adequately braced prior to backfill.
- 12. Cantilever retaining walls shall not be backfilled until the concrete has developed 100% of the required 28-day compressive strength for the class of concrete specified.

CAST-IN-PLACE CONCRETE

- 1. All concrete construction shall be performed in accordance with aci 301-10, aci 318-11. ACI 117-10. ACI 308.1-11. and ACI SP-66, the ACI Detailing Manual-2004. Hot and cold weather concrete construction shall be performed in accordance with ACI 305 and ACI 306 as required. Shoring and reshoring of concrete structures shall be performed in accordance with ACI 347. Structural design and removal of concrete formwork, shores and reshores shall be the responsibility of the contractor.
- 2. Shop drawings showing the size, length, quantity, location and mark of all reinforcing bars, supports and accessories shall be submitted for approval prior to fabrication.
- 3. Mix designs and admixture product data shall be submitted for approval prior to ordering
- 4. Concrete properties shall be in accordance with the specifications.
- 5. Reinforcement and accessory properties shall be in accordance with the specifications. 6. Reinforcement compression splices shall be lapped 30 bar diameters of the larger bar.
- 7. Reinforcement tension splices shall be lapped in accordance with the following table:

<u>bar</u>	<u>size</u>	3,000 psi conc. lap length	>=4,000 psi conc. lap length
	#3	17"	15"
	#4	23"	20"
	# 5	28"	24"
	#6	34"	29"
	# 7	49"	43"
	#8	56"	49"
	# 9	69"	60"

add 30% for horizontal top bars with more than 12" of concrete below. add 50% for bar spacing less than two bar diameters. lap length adds are cumulative.

8. Concrete protection for reinforcement shall be in accordance with the following table: clear cover over bars concrete cast against and permanently exposed to earth

concrete east against and permanently exposed to earth	9
concrete exposed to earth or weather	
#6 through #18 bars	2"
#5 bar, W31 or D31 wire and smaller	1 1/2"
concrete not exposed to weather or in contact with ground	
slabs, walls, and joists	

1 1/2"

3/4" #11 bar and smaller 9. The typical details on these drawings contain additional general concrete construction

#14 and #18 bars

- notes and information.
- 10. All concrete shall be reinforced unless noted otherwise.
- 11. supports to adequately position reinforcing bars during construction shall be installed. 12. Foundation dowels of the same size and spacing as vertical steel shall be installed for all
- walls, piers, and columns. 13. All reinforcing at wall and footing corners and intersections shall be continuous by the
- use of bent bars or corner bars unless indicated otherwise. 14. Construction joints shall be positioned so as not to adversely affect the structural performance. Construction joint locations not indicated on the structural drawings shall
- be approved by the structural engineer. 15. Pipe sleeves and inserts shall be installed in concrete work at all penetrations.
- penetrations of beams, joists, columns or structural slabs not indicated on the structural drawings shall be approved by the structural engineer. 16. Only weldable reinforcing bars may be welded.
- 17. Admixtures containing chloride or other corrosive chemicals shall not be used in
- 18. Aggregates shall be free of deleterious or non—durable materials such as cherts. 19. reinforcing shall be adequately tied and supported to hold it in the correct position during construction.
- 20. Concrete shall be consolidated adequately during placement by mechanical vibration in accordance with published practices.
- 21. Unshored slab construction shall be finished level and have the minimum required thickness of concrete at the thinnest section. Beam camber shall be verified prior to placing unshored concrete slabs.
- 22. Plastic chairs shall be used in all concrete that will be exposed to view in the completed structure.
- 23. Exposed concrete corners shall be chamfered minimum $\frac{3}{4}$ ".
- 24. Fill pockets around connections with concrete flush and smooth unless indicated
- 25. Concrete finishes shall be in accordance with the specifications.
- 26. Concrete slab—on—grade flatness and levelness shall be in accordance with the

CONCRETE MASONRY

- 1. Concrete masonry walls shown on the structural drawings are structural walls. concrete masonry walls not shown on the structural drawings are partitions. Refer to architectural drawings for details of partitions unless indicated otherwise on the structural drawings.
- 2. Concrete masonry walls shown on structural drawings shall be constructed in accordance with ACI 530.1 "Specifications for Masonry Structures".
- 3. Installation drawings, product data and material certifications shall be submitted for approval. The submittals shall conform to the specifications.
- 4. Concrete masonry materials shall conform to the requirements of the specifications.
- 5. Minimum compressive strength of concrete masonry (f'm) shall be 1,500 psi determined in
- accordance with the specifications.
- 6. Mortar cement shall be portland-lime cement. Masonry cement shall not be used.
- 7. The typical details on the drawings contain additional general masonry notes and details. 8. Bearing walls shall be anchored at intersections by galvanized steel straps $1 \frac{1}{2}$ " x $\frac{1}{4}$ " x $\frac{2}{4}$ " with 2" bend at 90 degrees each end. Install straps into grouted cores of c.m.u. at 24" maximum vertical spacing. do not install anchors at control joints or where non-bearing partitions abut
- bearing walls. 9. Corners of load bearing concrete masonry walls shall be laid in running bond.
- 10. Provide solid grouted concrete masonry around bearing ends of all beams and joists.
- 11. No openings for trades shall occur in concrete masonry walls within 16 inches of beam bearing
- 12. Pipe sleeves and inserts shall be installed in concrete work at all penetrations.
- 13. Embedded item locations shall be coordinated with the approved shop drawings of the trades. 14. Only weldable reinforcing bars may be welded.
- 15. Concrete masonry is supposed to absorb water from mortar and grout. do not place or grout wet
- concrete masonry units. 16. Webs of masonry units for piers, columns, pilasters, and the starter course shall be mortared.
- webs of masonry units shall also be mortared where required to confine grout. 17. Cells of masonry in piers, columns, pilasters and where otherwise indicated shall align. this may
- require the use of block styles other than stretchers (e.g. square-end block).
- 18. Spaces to be filled with grout shall be kept clean and free from protrusions of masonry or mortar. 19. All cells of below—grade concrete masonry units shall be grouted .
- 20. The maximum grout pour height for each specific type and size of concrete masonry unit shall not exceed the limits specified in ACI 530.1.
- 21. Masonry grouting shall conform to the specifications.
- 22. Vertical control joints are indicated on the civil or architectural drawings.
- 23. Vertical control joints shall be installed between all non-loadbearing partitions and bearing walls.
- 24. Spacing of control joints shall not exceed 24 feet unless noted otherwise. 25. Splice lap lengths for reinforcing shall be in accordance with the following table:

100	Tup Tellytis	101	Telliforcing .
	<u>bar size</u>		<u>lap lengtl</u>
	#3		18"
	#4		25"
	# 5		31"
	#6		57 "

26. Do not embed any non-structural items in structural masonry without written permission from the structural engineer.

STRUCTURAL STEEL

- 1. Detailing, fabrication, and erection of structural steel shall conform to the AISC "Specification for Structural Steel", (ANSI/AISC 360-10), AISC "Code of Standard Practice for Structural Steel Buildings and Bridges", AISC / RCSC "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts" and AWS D1.1 "Structural Welding
- 2. Shop drawings shall be submitted for approval prior to fabrication of structural steel. Shop drawings shall conform to requirements in the specifications.
- 3. Structural steel members shall conform to the following specifications:

member type	specification
wide flange	ASTM A 992
standard beam	ASTM A 36
channel	ASTM A 36
angle	ASTM A 36
plate	ASTM A 36
bar and rod	ASTM A 36
rectangular, square & round tube (hss)	ASTM A 500 Gr B
pipe	ASTM A 53 Gr B
threaded rod	ASTM A 36
anchor rod	ASTM F 1554 Gr 36
common bolts	ASTM A 307 Gr A
high strength bolts (twist off)	ASTM F 1582
high strength bolts (snug tight)	ASTM A 325
direct tension indicating washers	ASTM F 959
hardened washers	ASTM F 436
nuts	ASTM A 563
shear connectors (studs)	ASTM A 108
welding electrode	AWS D1.1 E70XX
<u> </u>	(except as otherwise red

- 4. Grout shall conform to requirements in the specifications.
- 5. The typical details on the drawings contain additional general steel construction notes
- and details. 6. High—strength bolted connections shall be fully pretensioned unless noted as snug tight on the drawings.
- 7. Hardened washers shall be installed under all nuts for fully pretensioned bolts.
- 8. Hardened washers shall be installed over all oversized holes, standard slots and short
- slotted holes. plate washers $\frac{5}{6}$ " thick shall be welded over large holes and long slots. 9. Bolted joints where relative movement is allowed shall have jam nuts to prevent
- 10. Structural steel surface preparation and finishes shall conform to the requirements in the specifications.

PREFABRICATED WOOD TRUSS CONSTRUCTION

- 1. Truss design and manufacture shall conform to the current building code authorized edition of ANSI TPI-1, "National Design Standard for Metal-Plate Connected Wood Truss Construction."
- 2. Truss handling and erection shall conform to the latest edition of BCSI guides. See
- www.sbcindustry.com. 3. Truss layout and truss shop drawings shall be submitted for approval. These drawings shall include:
 - 3.7. a copy of the bcsi jobsite package, which are instructions for safe handling and erection of wood trusses.
 - 3.8. truss layout showing dimensioned location and shipping mark of each truss and
 - locations of all compression web and chord bracing. 3.7. truss configuration, including span, pitch and location of all member intersections.
 - 3.8. species, stress grade, and nominal size of lumber used. 3.9. design loads including point loads and reactions and load combinations used in
 - 3.10. printout of member axial and flexural stresses plus interaction of combined
 - stresses for the controlling load combination. 3.11. printout of truss deflections under service load combinations.
 - 3.12. joint, splice, and truss to truss girder connection design and details.
- 4. Truss shop drawings, and calculations shall be sealed by a professional engineer licensed in the state of Kentucky. 5. Trusses shall be designed for a maximum vertical deflection of 1/480 of the span for 100% live
- load and 1/240 of the span for 100% total load.
- 6. Truss framing members shall be Southern Pine No. 2 or better.
- 7. All connections plates shall be hot-dipped galvanized according to ASTM A 153.
- 8. Trusses shall be spaced at 2'-0" o.c. maximum. Web arrangement shall be manufacturer's standard unless otherwise indicated. See all drawings for openings that may be required in trusses.
- 9. Permanent bracing for individual members of a wood truss shall be shown on the truss design drawings and shall be installed by the building contractor. Permanent bracing shall be installed as indicated on the truss manufacturer's drawings and instructions.
- 10. All bracing that terminates at or is interrupted by structural bearing walls shall be attached
- 11. Lateral brace splices shall be lapped at least two trusses.
- 12. Trusses delivered to the project in more than one piece and all multi-ply trusses shall be connected before installation or according to truss design drawings if indicated otherwise.
- 13. Concentrated loads from construction materials (e.g. roof sheathing bundles) shall not be placed on trusses until all required bracing has been installed and roof sheathing is permanently nailed in
- place. Trusses shall not be overloaded with construction materials. 14. Temporary bracing to prevent lateral movement during erection shall be installed according to the handling and installation guidelines.
- 15. Work points, overhangs and other dimensions not indicated on the structural drawings should be determined from the appropriate drawings. Conflicting dimensions shall be clarified in writing.

ROOF AND WALL PLYWOOD SHEATHING

- 1. All sheathing shall be plywood (not OSB) manufactured in accordance with industry specification PS-1 and shall bear the stamp of either the American Plywood Association (APA) or Timberco inc.
- 2. All sheathing shall be exterior grade. 3. All roof and wall sheathing shall have veneer grade C—C or better.
- 4. Roof sheathing shall have tongue and groove edges and be either APA "Sturd—i—Floor" or TECO
- "Floor Span" with thickness and/or span rating as indicated on the drawings or as required. 5. Wall sheathing shall have plain square edges and be APA "Rated Sheathing" or TECO "Sheathing
- Span" with thickness and/or span rating as indicated on the drawings or as required. 6. All edges of wall sheathing shall be blocked with a 2x wood member and nailed.
- 7. Minimum nailing for roof and wall plywood sheathing shall be 10d common nails at 12" o.c. in the panel interior and 6" o.c. at panel edges and boundaries.

STRUCTURAL WOOD

- 1. All structural wood dimension lumber shall be Southern Pine No. 2 species stress grade and shall bear a stamp by the southern pine inspection bureau (SPIB) indicating this.
- 2. All structural composite lumber (LVLs) shall have the following allowable design stresses:

Fb =	2,750 psi	FcPERP =	750 psi
Fv =	285 psi	E =	2.0 Mpsi
Ft =	1,150 psi	Fc =	2,600 psi

- 3. Submit product data of structural composite lumber for approval prior to ordering.
- 4. Two-ply and three-ply LVLs shall be fastened together with two rows of Simpson SDS25312 screws
- at 12 inches on center on each face. 5. All structural wood construction shall be in conformance with the AF&PA National Design
- Specification for Wood Construction (NDS). 6. All horizontal lumber members shall be fabricated and installed with natural camber (crown)
- 7. Nails shall be common wire nails unless noted otherwise. Nails exposed to weather or in preservative treated wood shall be hot dipped galvanized to ASTM A 153. Wood members shall be nailed as indicated in the wood nailing schedule of the International Building Code if not indicated
- 8. Bolts in wood members shall be ASTM A 307 with factory zinc coating. Holes in wood for bolts shall be χ_{6} oversize. USS flat washers conforming to ASTM F 844 shall be used under bolt heads and nuts against wood. Bolts, nuts and washers exposed to weather or in preservative treated wood shall be hot dipped galvanized to ASTM A 153.
- 9. Connectors indicated as "Simpson" on the drawings shall be manufactured by Simpson Strong—tie,
- 10. Simpson connectors shall be hot—dipped galvanized to ASTM A 123 where indicated or where exposed to weather. Simpson connectors shall be galvanized to ASTM A 653 G180 where in contact with preservative treated wood and not exposed to weather and shall be ASTM A 653 G90
- otherwise or unless indicated otherwise. 11. Product data and a plan and schedule of Simpson connectors showing the model number, quantity, finish and type and number of fasteners for all connections shall be submitted for approval prior to ordering Simpson connectors.
- 12. Simpson anchors shall be installed in accordance with all of the manufacturer's instructions. 13. Preservative treated wood appropriate for the service shall be used where in direct contact with

concrete or masonry or where exposed to weather.

manufacture, fabrication, shipping, storage and construction.

14. Cutting structural lumber members other than as indicated on the structural drawings requires approval of the structural engineer. Notching of lumber will not be permitted. 15. Nominal 1x3 wood crossed bridging with beveled ends or Simpson TB36 steel joist bridging shall be

installed at maximum 8'-0" spacing on all joists with a minimum of one row of bridging on all

joists longer than 10 feet. 16. Structural wood members shall be protected from dirt, moisture, sunlight and damage during



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PROJECT NO. 2020132

SHEET NO. S1

SPECIAL INSPECTION

- 1. Special inspection is required according to section 1704 of the building code.
- 2. Special inspection on this project applies only to the following construction:
 - 2.1. the superstructure (c.m.u. and up) of the filter building addition,
 - 2.2. the chemical feed building addition.
- All other structures shall be inspected according to these notes, but those inspections are not considered "special inspections" as required by the building code because these structures are not primarily for human occupancy and are not in the scope of the building code. The inspector shall keep special inspections and non—"special" inspections reports and tests separate and identifiable for record keeping purposes.
- 3. Special inspections shall be performed for the following work as required in the building
 - 3.1. Contractor's statement of responsibility in accordance with section 1704.4 3.1.1. Contractor shall submit a statement that:
 - 3.1.1.1. acknowledges the requirements stated in this statement of special inspections.
 - 3.1.1.2. acknowledges that control will be exercised over the quality of
 - construction to conform to the approved construction documents. 3.1.1.3. acknowledges that there are organizational procedures in place for
 - exercising control of quality of the construction including: 3.1.1.3.1. appointment of a person within the contractor's organization to exercise control quality of construction
 - 3.1.1.3.2. the persons within the contractor's organization to whom the quality control reports are distributed
 - 3.1.1.3.3. the method and frequency of reporting the quality control results
 - within the contractor's organization. 3.2. Fabricators in accordance with section 1704.2
 - 3.2.1. Submit report of inspector's approval of fabricator's gc plan or fabricator's nationally recognized ac certification.
 - 3.2.2. Submit fabricator's certificate of compliance stating that the work was performed in accordance with the approved construction documents. submitted at the completion of such work.
 - 3.3. Steel construction in accordance with section 1705.2
 - 3.3.1. Submit mill test reports and material certifications for all steel members, fasteners, bolts, nuts, washers, deck, and reinforcement steel for concrete
 - 3.3.2. Submit report of inspection of marking and connection details for all members and connections. verify all steel members and steel deck are installed in the correct locations and are connected in accordance with the construction documents and approved erection drawings.
 - 3.3.3. Submit report of inspection of bolt tensioning for each applicable connection.
 - 3.3.4. Submit report of visual inspection of all field welds.
 - 3.4. Concrete construction in accordance with section 1705.3
 - 3.4.1. Submit material certifications of cement, aggregate, admixtures and reinforcement.
 - 3.4.2. Submit report of compressive strength, slump and air content test results. sample and test concrete at least once per day and once for every additional 100 cubic yards of concrete per day thereafter.
 - 3.4.3. Submit report of inspection of forms, reinforcement, and concrete delivery tickets prior to each placement of concrete.
 - 3.5.4. Submit report of inspection of installation of all wedge and chemical adhesive anchors in concrete.
 - 3.4. Masonry construction in accordance with section 1705.4
 - 3.4.1. Submit material certifications of cement, aggregate, admixtures and reinforcement.
 - 3.4.2. Submit report of test of mortar aggregate ratio and air content and observation of mortar proportioning. test once at beginning of project and once every 5,000 s.f. of wall thereafter.
 - 3.4.3. Submit report of placement of masonry, reinforcement and grout prior to and during each placement of grout.
 - 3.4.4. Submit report of installation of chemical adhesive anchorage in concrete at base of masonry walls. inspect installation of 10% of anchorage installations.
 - 3.5. Wood construction in accordance with section 1705.5
 - 3.5.1. See "Inspection of Fabricators" for inspection of prefabricated wood trusses.
 - 3.5.2. Submit material certifications for wood members, sheathing and fasteners. 3.5.3. Submit report of inspection of connection of roof roof trusses to structure.
 - 3.5.4. Submit report of inspection of all wood framing members and their connections. verify all wood framing members are the correct size and grade and are installed in the correct locations, and are connected in accordance with the construction documents.
 - 3.5.5. Submit report of inspection of nailing of roof sheathing to trusses and structure.
 - 3.6. Soils construction in accordance with section 1705.6
 - 3.6.1. Submit report that soil bearing capacity is adequate according to the
 - geotechnical report prior to each placement of foundation concrete. 3.6.2. Submit report of density and moisture content of controlled fill for each lift under building structure.
 - 3.7. Cast—in—place deep foundations in accordance with section 1705.8
 - 3.7.1. Submit report of continuous observation of all drilling operations including complete and accurate records for each drilled shaft.
 - 3.7.2. Submit report indicating the location, plumbness, diameter, length, concrete volume, embedment into bedrock, and adequate end-bearing strata capacity of each pier.
 - 3.7.3. For concrete, perform tests & inspections as required by the concrete special inspection requirements.
- 4. The type and extent of each test and inspection required for each type of work shall be as indicated in the specifications and/or the building code and the references incorporated therein.
- 5. Inspection reports shall include the:
 - 5.1. name, address, and telephone number of special inspector performing the
 - inspection and making the report. 5.2. dates and locations of samples and tests or inspections, date of report.
 - 5.3. record of temperature and weather conditions at time of sample taking and testing and inspecting.
 - 5.4. description of the work, identification of products, specification section, tests, and inspection methods.
 - 5.5. photographs of the work inspected for that report
 - 5.6. complete test or inspection data.

- 6. Special inspection shall be performed by a qualified inspection and testing agency approved by the building official and the structural engineer.
- 7. Work requiring special inspection shall be inspected by the special inspector for conformance with the approved drawings and specifications. Inspection reports indicating the results of special inspections shall be promptly submitted to the contractor, the civil engineer, the structural engineer.
- 8. The special inspector shall observe activities, actions, and procedures performed before and during execution of the work to guard against defects and deficiencies and substantiate that proposed construction will comply with requirements.
- All special inspections indicating non-conforming work shall be reported immediately to the contractor, the civil engineer and the structural engineer. Impending construction work that would impede economical correction of non-conforming work shall not proceed without written approval. The contractor shall maintain a discrepancy log on the site. log shall list each discrepancy documented by the special inspector, state the date of discovery and special inspector's report number, and room for the special inspector to sign and date when said discrepancy is corrected. Cost of additional retesting that are required due to non-conforming work may be charged to the contractor.
- 10. A final report certifying completion of all required special inspections and correction of any non—conforming work noted in the inspections shall be submitted by the special inspector at the completion of the project, or if not, detailing non-inspected and/or unresolved non-conformances.
- 11. The contractor shall notify the inspector when construction is ready to be inspected. contractor shall give timely and adequate notice to the special inspector.
- 12. The contractor shall provide the special inspector access to plans, shop drawings, and change orders at the jobsite.
- 13. The contractor shall retain at the jobsite all special inspection records submitted by the special inspector and provide these records for review by the engineer and building inspector upon request.

EXPANSION ANCHORS

1. Expansion anchors shall be one of the following products:

Kwik Bolt TZ by HILTI Trubolt+ by ITW Red Head

Strong-bolt by Simpson Strong-tie

- 2. All expansion anchors for the project shall be produced by the same manufacturer unless approved by the structural engineer.
- 3. Expansion anchor product data and a keyed plan showing the location, diameter, length,
- material and finish of each expansion anchor shall be submitted for approval. 4. The expansion anchor manufacturer's installation instructions shall be strictly followed.
- particularly with regard to drilling and cleaning out the hole. 5. If any of the following minimum distances are not indicated or available then verify the detail and field conditions with the structural engineer prior to installing:

<u>c to c distance</u> <u>edge distance</u> <u>anchor dia</u> <u>embed distance</u> <u>mat'l thickness</u> 1/2" 3 1/2" 3 1/2" 5 1/2" 5/8" 3/4"

6. If any of the following conditions are indicated or present then verify acceptability of expansion anchor type, material or finish with the structural engineer prior to installing: cracked concrete or masonry near installation (see edge distance above)

corrosive, chemical or abnormal temperature environment

vibratory or fatigue loading of anchor impact or shock loading of anchor

continuous tension (e.g. hanging loads from ceilings)

CHEMICAL ADHESIVE AND PROPRIETARY ADHESIVE ANCHORS

1. Chemical adhesives and proprietary adhesive anchors shall be produced by one of the following manufacturers:

HILTI, Inc.

ITW Red Head

Simpson Strong-tie

- 2. All chemical adhesives and proprietary adhesive anchors for the project shall be
- produced by the same manufacturer unless approved by the structural engineer. 3. Proprietary adhesive anchors shall be fastened with compatible chemical adhesive from the same manufacturer.
- 4. Chemical adhesive and proprietary adhesive anchor product data and a keyed plan showing the location, type of chemical adhesive and installation conditions of each adhesive anchor shall be submitted for approval. installation conditions are:

dry, damp or wet hole cored hole or hammer drilled hole

standard (per manufacturer) or oversize hole horizontal, vertical or overhead surface

temperature range of installation.

5. The chemical adhesive and proprietary adhesive anchor manufacturer's installation instructions shall be strictly followed, particularly with regard to drilling and cleaning out the hole and the installation conditions.

6. If any of the following minimum distances are not indicated or available then verify the detail and field conditions with the structural engineer prior to installing: <u>c to c distance</u> <u>edge distance</u> <u>embed distance</u> <u>mat'l thickness</u> <u>anchor dia</u>

1/2" 3 1/2" 3 1/2" 5 1/2" 5/8"

7. If any of the following conditions are indicated or present then verify acceptability of chemical adhesive or proprietary adhesive anchor type, material or finish with the structural engineer prior to installing:

corrosive, chemical or abnormal temperature environment vibratory or fatigue loading of anchor impact or shock loading of anchor

continuous tension (e.g. hanging loads from ceilings).

MATERIAL PATTERN LEGEND

3/4"





COMPETENT ROCK

UNDISTURBED SOIL

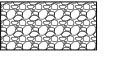
ENGINEERED FILL



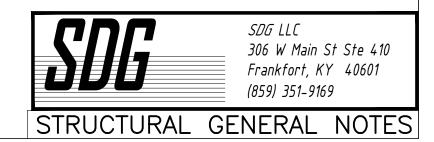
CONCRETE



LEAN CONCRETE FLOWABLE FILL GROUT

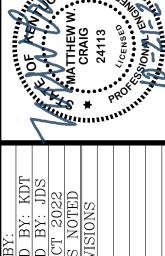


CRUSHED STONE DENSE GRADED AGGREGATE





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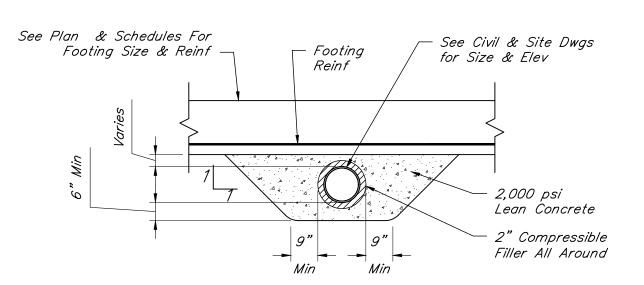






PROJECT NO. 2020132 SHEET NO.

S2



Provide concrete protection around utility line when line is within footing influence zone. See detail above for influence zone definition.

TYPICAL UTILITY LINE BELOW FOOTING

— Bond Beam at Roof & Floor

8" C.M.U.

Opening

Line See Sections for Size and

Reinforcement - Min 2~ #5 at

Masonry Lintel

See Sched Typ

Not to Scale

- Min One Bar Required Each Side

of Control Joint Typ

TYP Bar Spacing

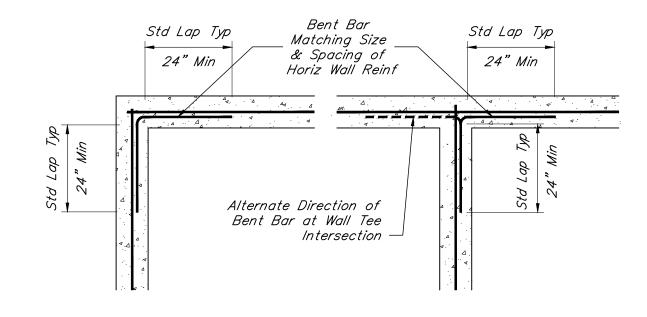
See Plan

Occur at Sides of Openings Do Not Cross Reinf and

Run to Head of Door —

Run Control Joints

Full Height Where They



- 1. Where bar sizes differ, lap for larger size.
- 2. If bend radius creates problems fitting hairpins in wall, provide more smaller hairpins with equal total area to main bars.
- 3. Construction joints shall not occur within 5'-0" of a corner or tee unless indicated otherwise on the drawings.

TYPICAL WALL INTERSECTION REINF

Not to Scale

Not to Scale

Main Bond Beam

Bond Beam Course

NOTES:

Reinforcement See Other Details

Hairpin Bar Matching

Size of Bond Beam Reinf

Lap w/ Main Bond Beam

Horiz Reinforcement Shall Be Continuous Through Joint or Water Stop Provide Tension Lap Splice in Accordance with Standard Lap Key or Other Treatment

NOTES:

1. Maximum Length of Wall Pour = 40'-0".

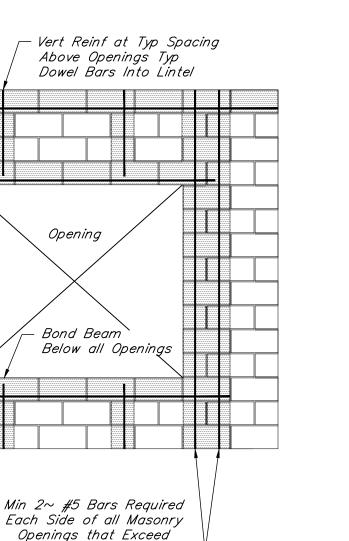
2. Minimum 48 Hours Between Adjacent Pours. 3. See Plans for Additional Joint Locations.

or Tee Intersection.

4. Submit Construction Joint Location Plan For Approval Prior to Construction. 5. Do Not Form Joints Within 5'-0" of a Corner

See Typical Detail

TYPICAL WALL CONSTRUCTION JOINT



∠ MIN 1~ #5 Bar

Each Side of All

Openings —

8" End of

Bond Beam Typ

- 1. Minimum vertical wall reinforcing shall be #5 @ 2'-0" 8. Use lintel block over openings and continue with unless noted otherwise. 2. Vertical wall reinforcing shall be continuous.
- 3. See typical detail for dowels required at base of walls. 9. Control joints shall extend full height of wall and align 4. Center reinforcing bars in grouted cells unless noted otherwise.
- 5. Use bar positioners at minimum 4'-0" spacing to support reinforcing bars.
- 6. Follow specified grouting procedures. 7. Clean mortar from edges of cells so grout can flow smoothly and fill entire cell.
- open-bottom bond beam from edge of opening into wall so that vertical reinforcing at jamb can pass.

48" in Width —

from floor to floor. 10. Where a control joint occurs through a bond beam or lintel bearing, provide $2 \sim \frac{1}{2}$ dowels across joint with grease on one side. Do not continue horizontal reinforcing across control joint.

TYPICAL C.M.U.WALL REINFORCEMENT DETAILS Not to Scale

Additional Reinforcement Backer Rod Adjacent to Control Joint & Sealant See Typ Details Preformed Rubber or P.V.C. Control Joint Gasket (Cont)

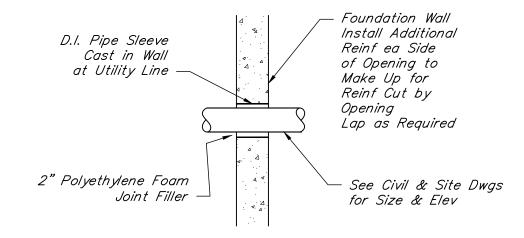
TYPICAL MASY WALL

INTERSECTION DETAIL

1. See architectural drawings for control joint locations.

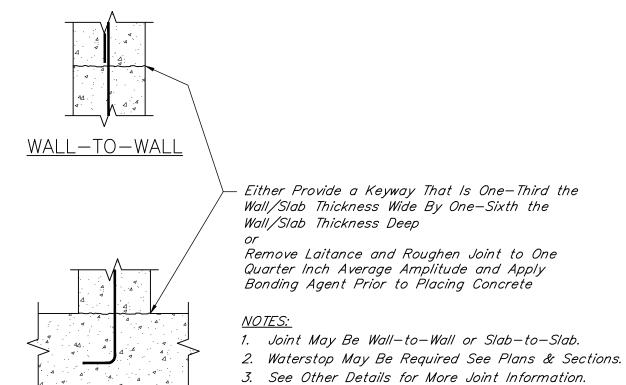
2. Discontinue horizontal joint reinforcing at control joints. 3. Unless otherwise shown or noted, spacing of control joints shall not exceed 24 feet.

> TYPICAL C.M.U. CONTROL JOINT DETAIL Not to Scale



TYPICAL UTILITY LINE THRU FDN WALL

Not to Scale



WALL-TO-SLAB/FDN

Tooth intersecting walls together

in running bond with min 6"

#5 vertical bar centered in

Horizontal joint reinforcement

shall be lapped min 6" at wall

otherwise on plans.

intersection.

intersection.

Cont Vert #5 Bar

at Intersection

Not to Scale

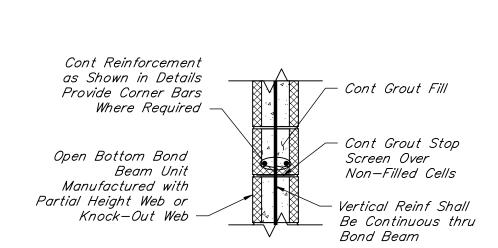
overlap or use masonry strap

every third course unless noted

grouted cell shall be installed at

TYPICL CONSTRUCTION JOINT CONCRETE PREPARATION

Not to Scale



Cont Top Longt'l Reinf

Edge of Opening

Stirrup

Тур

0

w/ Min ³/₄"

Grout Cover

Extend Reinf 16" Past

Min 3/4" Grout Cover Typ

Each Side

Open Bottom Bond Beam

w/ Partial-Height Webs

Full Depth of Lintel and

Each Side in One Pour

Cont Bott Longt'l Reinf

Extend Reinf 16" Past

Min 3/4" Grout Cover Typ

16" Past Edge of Opening

or Knock-Out Webs

Cont Grout Fill

Edge of Opening

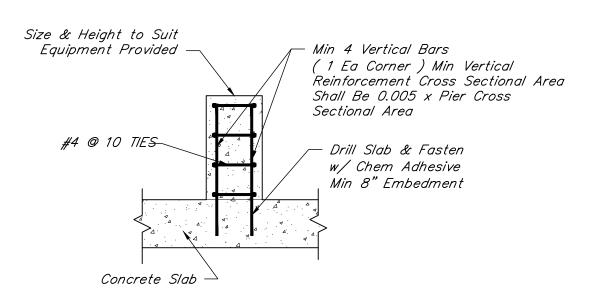
Lintel Block Over Openings

and Open-Bottom Bond Beam

for 16" Past Edge of Openings

Reinforcing shall have 3/4" minimum arout cover to all c.m.u. surfaces.

TYPICAL C.M.U. BOND BEAM DETAIL Not to Scale

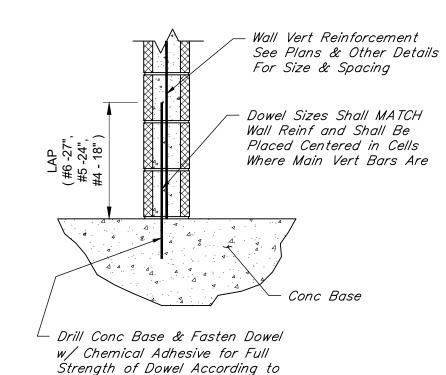


TYPICL CONCRETE EQUIPMENT PEDESTAL Not to Scale

Conc Structure

5/8"ø x 1'-9" Smooth Rod Coated w/ Grease at Every Other Course of C.M.U. Drill Concrete 9" to Accept Dowel Chip Webs of C.M.U. & Grout First Two Cells Adjacent to Wall Full Height

PLAN VIEW



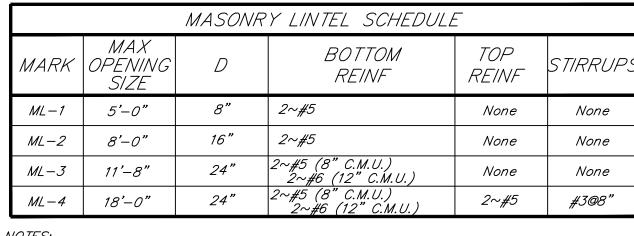
SECTION VIEW

Mfr's Published Installation

Instructions

TYPICAL C.M.U. WALL DOWEL DETAIL

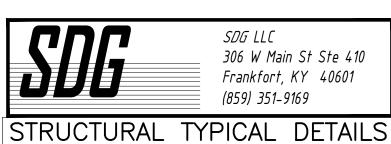
Not to Scale



NOTES:

- 1. Do not use this schedule if concentrated load is applied to the lintel at a height less than half the span above the lintel or if stack bond is specified.
- 2. In lieu of using lintel block on the bottom of lintels which, requires shoring during construction, contractor may use prestressed, precast concrete lintels by "cast-crete" (www.castcrete.com) or approved equal. submit product data and a plan and schedule of lintel locations and sizes for approval for this option.





SI

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ngine **Environmental** 8



PROJECT NO. 2020132

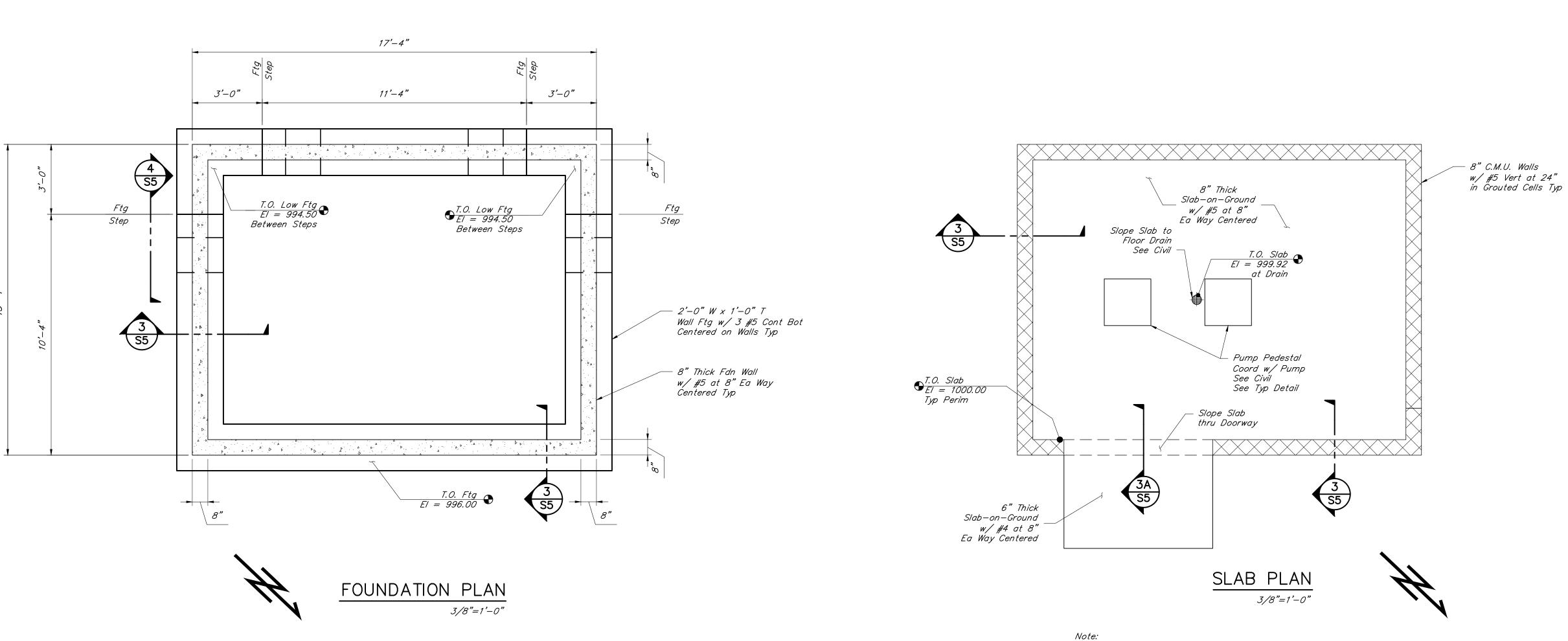
SHEET NO. S3



306 W Main St Ste 410 Frankfort, KY 40601 SHEET NO. KY 225 P.S. – STRUCTURAL PLANS **S4**

SDG LLC

(859) 351-9169



Backfill foundation walls balanced inside and out so that the grade elevation difference on either side of the walls is no more than 24" at any time.

O.F. C.M.U.

1'-11/2" 1'-11/2" Manufactured Metal-Plate-Connected — O.F. C.M.U. O.F. C.M.U. — Wood Trusses at 16" Typical Spacing - ⁵/8" Thick T & G Plywood Roof Sheathing Angled Wall Brace Typ 4X See Section – - Blocking Typ See Sections Use smooth-face c.mu. around hvac unit for flush fit — — Ridge ML−1 High & Low —⁄

 $\frac{1}{S5}$

T.O. C.M.U. EI = 1009.33 €

19'-7"

Backfill foundation walls balanced inside and out so that the

grade elevation difference on either side of the walls is no

more than 24" at any time.

– Fascia Header & Barge Rafter Typ See Sections

ROOF FRAMING PLAN 3/8"=1'-0"

SDG LLC

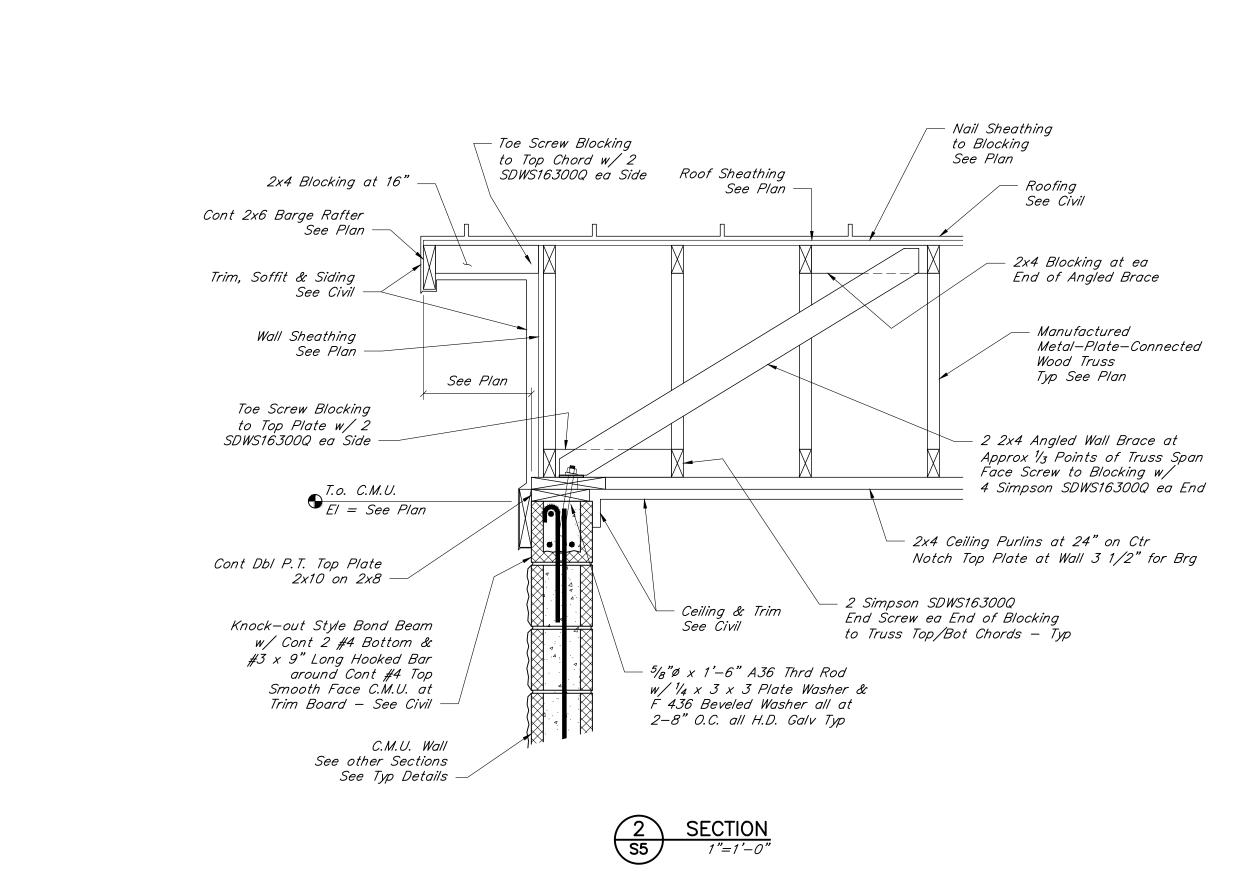
STRUCTURAL DETAILS

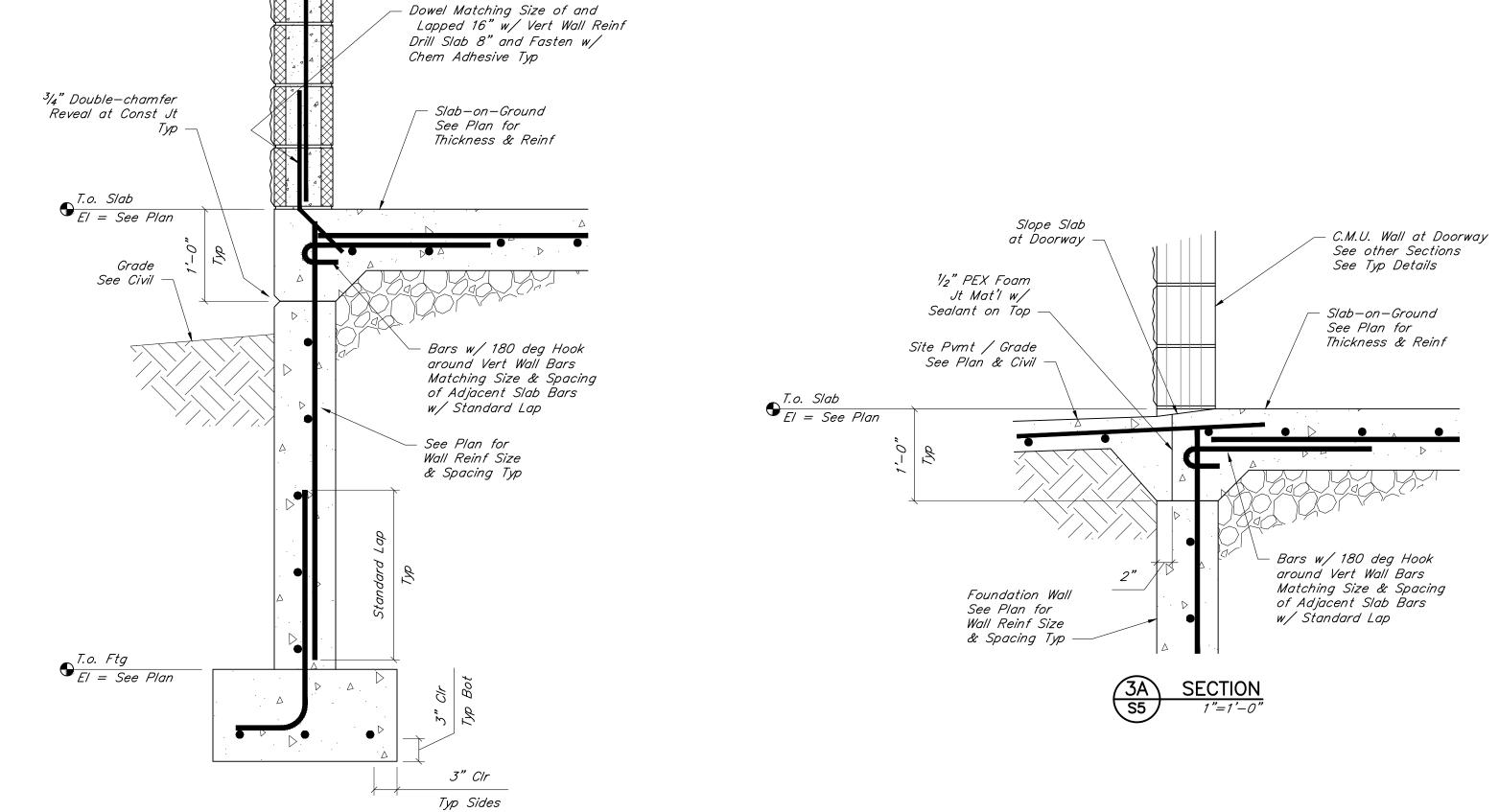
(859) 351-9169

306 W Main St Ste 410 Frankfort, KY 40601

SHEET NO.

S5





Roofing See Civil

 \times

— 2x4 Ceiling Purlins

at 24" on Ctr

Manufactured

Wood Truss

See Plan

Metal-Plate-Connected

2x4 Blocking btwn trusses

Roof Sheathing

2x8 Blocking
every other Truss Space
w/ 2 3"Ø Holes Cntrd
Nail Sheathing to top

Simpson L50

Cntrd on Blocking —

See Plan -

See Civil

Cont Dbl P.T. 2x8 Top Plate —

All H.D. Galv Typ —

2x6 Fascia Header

Gutter, Soffit & Trim

⁵/₈"ø x 1'-6" A36 Thrd Rod

 $w/\frac{1}{4} \times 3 \times 3$ Plate Washer at 2'-8" O.C. in between Trusses

See Plan -

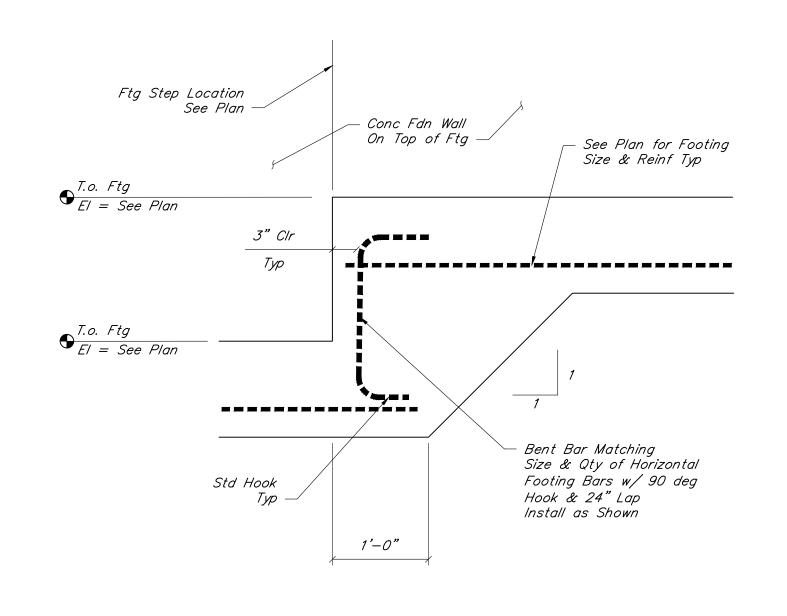
at ea Edge of Sheathing -

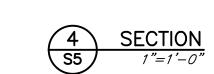
— Simpson H2.5A

- Knock-out Style Bond Beam w/ Cont 2 #4 Bottom & #3 x 9" Long Hooked Bar around Cont #4 Top

Smooth Face C.M.U. at Trim Board — See Civil

- Ceiling & Trim





SHEET	NO.
E-	1

ELE	CTRICAL PLAN SYMBOLS	ELECTA	RICAL DIAGRAM SYMBOLS
 -	ELECTRICAL CIRCUIT: SHORT=PHASE CONDUCTOR; LONG = NEUTRAL, DASHED = EQUIPMENT GROUND EMERGENCY CIRCUIT	****	TRANSFORMER
		 	CAPACITOR
\$3	SWITCH: 3=3 WAY; 4=4 WAY; K=KEY; WP= WEATHERPROOF; M=MOTOR STARTER; PL=PILOT LT	<u></u>	GROUND
*	DUPLEX RECEPTACLE: WP = WEATHERPROOF; GFI = GROUND FAULT; NUMBER = MOUNTING HEIGHT		CURRENT TRANSFORMER
ХФ	SINGLE RECEPTACLE	 	POTENTIAL TRANSFORMER
Ф	208 or 240 VOLT RECEPTACLE	· · · ·	CIRCUIT BREAKER (GENERAL)
	DUPLEX RECEPTACLE, FLUSH FLOORBOX MOUNTED	~~~~	CIRCUIT BREAKER, THERMAL—MAGNETIC
•	SPECIAL PURPOSE RECEPTACLE OUTLET		CIRCUIT BREAKER, MAGNETIC—ONLY
\bigcirc	THERMOSTA T	₩	CIRCUIT BREAKER (DRAWOUT)
M	MOTOR		RELAY CONTACTS (NORMALLY OPENED)
(JUNCTION BOX — SMALL		RELAY CONTACTS (NORMALLY CLOSED)
J	JUNCTION BOX — FLUSH—MOUNTED	7	, ,
_	SAFETY SWITCH — NONFUSED UNLESS NOTED	β	THERMAL OVERLOAD PROTECTION
\boxtimes	OTHERWISE MAGNETIC COMBINATION STARTER — THREE PHASE	+	DOT INDICATES A CONNECTION OF TWO WIRES
	MAGNETIC COMBINATION STARTER — SINGLE PHASE		TERMINALS FOR CONNECTION OF REMOTE WIRING
⊲ × ——∘	TELECOM OUTLET: D = DATA; T = TELEPHONE; C = CABLE; NUMBER = QTY OF CABLES & JACKS CONDUIT TURNED UP		RELAY/CONTACTOR COIL: C = CONTRACTOR; CR = CONTROL RELAY; TR = TIMING RELAY; M = MOTOR HAND-OFF-AUTOMATIC SWITCH
<u>_</u>	CONDUIT TURNED DOWN	X• • · · · ·	Thursday To Tommine Switch
И	WALL MOUNTED SPEAKER OR ALARM HORN	Zx	FULL VOLTAGE NON-REVERSING MOTOR STARTER; X = NEMA SIZE
	PANELBOARD (SURFACE MOUNTED)	X	PILOT LIGHT: $R = RED$; $G = GREEN$; $A = AMBER$;
	PANELBOARD (FLUSH MOUNTED IN WALL)	- X	W = WHITE PILOT LIGHT - PUSH-TO-TEST
-	HEATER—WALL MOUNTED		MOTOR
Ē	EXHAUST FAN/VENTILATOR		FUSED DISCONNECT SWITCH
	SPEAKER GENERAL		
	CLOCK		FLOAT SWITCH
)	EXISTING POWER POLE		TEMPERATURE SWITCH (THERMOSTAT)
•	NEW POWER POLE	•	PRESSURE SWITCH
R —	LIGHTING POLE	0 0	LIMIT SWITCH
	PHOTO CELL	SV OR o-//-o	SOLENOID VALVE COIL
)	MANHOLE	— <u>ETM</u> —	ELAPSED TIME METER
	PULLBOX	-0100 0-	PUSHBUTTONS, N.C. & N.O. RESPECTIVELY
Ð	MUSHROOM HEAD EMERGENCY SWITCH	-0-0-	SELECTOR SWITCH — TWO POSITION
	DUCT SMOKE DETECTOR		TIMER RELAY CONTACT: NORMALLY OPEN — TIMED
)	HEAT DETECTOR	V	OPEN UPON DEENERGIZATION TIMER RELAY CONTACT: NORMALLY CLOSED — TIMED
)	SMOKE DETECTOR		CLOSE UPON DEENERGIZATION
]	FIRE ALARM MANUAL PULL STATION	~~	TIMER RELAY CONTACT: NORMALLY OPEN — TIMED CLOSE UPON ENERGIZATION
	FIRE ALARM HORN/STROBE	- To	TIMER RELAY CONTACT: NORMALLY CLOSED — TIMED OPEN UPON ENERGIZATION
]	FIRE ALARM STROBE	0 0	TRANSFER SWITCH
м	FIRE ALARM ZONE ADDRESSABLE MODULE		GENERA TOR
\smile	SPRINKLER SYSTEM FLOW SWITCH		I

EXTERNAL WIRING

EMERGENCY STOP BUTTON

<u>-0Îo-</u>

ELECTRICAL ABBREVIATIONS

 \Box

 \Box

4

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(C)

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 $+ \downarrow -$

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NEMA XX

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____ TO___

 \odot

DHO OR -D- LIGHTING POLE

SPRINKLER SYSTEM FLOW SWITCH

PASSIVE INFRARED MOTION DETECTOR

INSTRUMENTATION LINE UNDERGROUND

INSTRUMENTATION LINE OVERHEAD

TELEPHONE LINE UNDERGROUND

TELEPHONE LINE OVERHEAD

GROUND ROD

TO THE NEMA RATING INDICATED

ELECTRICAL LINE UNDERGROUND

ELECTRICAL LINE OVERHEAD

ALL WORK IN THE ROOM/AREA SHALL CONFORM

TAMPER SWITCH

KEYNOTE

CALL SWITCH

MAGNETIC DOOR HOLDER

AMPERE

AMPERE FRAME

AMPERE TRIP

BARE COPPER

ABOVE FINISHED FLOOR

AMERICAN WIRE GAUGE

CONDUIT (RACEWAY)

CIRCUIT BREAKER

CIRCUIT

CEILING

CONTROL

DOWN

DEL TA/WYE

DIRECT BURIAL

EMPTY CONDUIT

EQUIPMENT GROUND

ELECTRIC UNIT HEATER

ELECTRIC WATER COOLER

FIRE ALARM CONTROL PANEL

GFCI OR GFI GROUND FAULT CURRENT INTERRUPTING

FULL VOLTAGE, NON-REVERSING

GROUNDING ELECTRODE CONDUCTOR

HAND-OFF-AUTO SELECTOR SWITCH

EXPANSION JOINT

ELE VA TION

ELECTRIC

END-OF-LINE

EMERGENCY

EXISTING

FIRE ALARM

FIBER OPTIC

GROUND

LUMEN

LIGHTS

LIGHTING

LIMIT SWITCH

LOW VOLTAGE

MANUFACTURER

MAIN LUGS ONLY

MEDIUM VOLTAGE

NOT APPLICABLE

NORMALLY OPEN

OVER TEMPERATURE

POLY-VINYL CHLORIDE

SURGE PROTECTION DEVICE

SHIELDED TWISTED INSTRUMENT CABLE

TRANSIENT VOLTAGE SURGE SUPPRESSOR

NOT TO SCALE

OVERHEAD

OVERLOAD POLE

PHASE

PANEL

POWER

SHEET

STA TION

SWITCH

STANDARD

TERMINAL BOX

THERMAL MAGNETIC

VOLTAGE OR VOLTS

TAMPER SWITCH

TELEPHONE

TELE VISION

UNDERGROUND

WEA THERPROOF

TRANSFORMER

UNIT HEATER

RECEPTACLE

SOLID NEUTRAL

STAINLESS STEEL

SINGLE POLE

NON LINEAR

NORMALLY CLOSED

NATIONAL ELECTRICAL CODE

MANHOLE

MOUNTED

MINIMUM

MAIN CIRCUIT BREAKER

MOTOR CIRCUIT PROTECTOR

MOTOR CONTROL CENTER

MAIN DISTRIBUTION PANEL

HORSEPOWER

JUNCTION BOX

KILOVOLT-AMPERES

THOUSAND CIRCULAR MILS

LIGHTING FIXTURE (LUMINAIRE)

KILOWATT-HOUR

EXHAUST FAN

CENTERLINE

CONTROL PANEL

ADJUSTABLE FREQUENCY DRIVE

AUTOMATIC TRANSFER SWITCH

CLOSED CIRCUIT TELEVISION

COPPER OR CONDENSING UNIT

DOUBLE POLE-SINGLE THROW

EQUIPMENT GROUND CONDUCTOR

ELECTRIC WALL HEATER/WATER HEATER

CURRENT TRANSFORMER OR CONSTANT TORQUE

AF

AFF

AFD

AT

ATS

AWG

BC

@ CB

CCTV

CKT

C/L

CLG

CP

CT

CTL

CU

\[\Delta / Y

DB

DN

EC

EF

EG

EGC

EJ

EL

ELEC

EOL

EMERG

EUH

EWC

EWH

EX

FA

FACP

FVNR

GEC

GND

HOA

HP

KVA

KWH

KCMIL

LF

LM

L TG

LTS

LS

LV

MCB

MCP

MCC

MDP

MFR

MH

MIN

MLO

MTD

MV

NA

NC

NEC

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NO

NTS

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PNL

PVC

PWR

SHT

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SP

SPD

SS

STA

STD

STIC

SW

TB

TEL

TM

15

TV

UG

UH

W

WP

W/

XFMR

TVSS

RECEPT

PH OR Ø

J OR JB

FO

DPST

DEVICE	HEIGHT AFF	REMARKS				
RECEPTACLE - LOW	1'-4"	TO BOTTOM OF DEVICE BOX				
LIGHT SWITCH	4'-0"	TO BOTTOM OF DEVICE BOX				
CONTROL STATIONS & PUSH-BUTTONS	4'-0"	TO BOTTOM OF DEVICE BOX				
PANELBOARDS & CONTROL PANELS	6'-6"	TO TOP OF BOX				
SAFETY SWITCH	4'-0"	TO TOP OF BOX				
THERMOSTAT	4'-8"	TO BOTTOM OF DEVICE BOX				
EMERGENCY LIGHT FIXTURES	7'-4"	TO BOTTOM OF DEVICE BOX				

CONDUCTORS	I/O TAG	TYPE	LIMIT	CONTROL	MONITOR	TREND	HISTORIZE	TOTALIZE	AVERAGE	ALARM	REPORT	NOTES
2#14	POWER LOSS ALARM	DI	UNIT		70	0	m	Ш	m	≤	-	NOTES
2#14	DOOR OPEN ALARM	DI			8		1		li le	X		
2#14	HEAT PUMP UNIT ALARM	DI			-					X		
2#14	PUMP 1 CALL-TO-RUN	D0	2	X	15	5						
2#14	PUMP 1 RUNNING STATUS	DI		MANAGE .	Х		X				Х	REPORT # STARTS & RUNTIMES
2#14	PUMP 1 OVERTEMP	DI			19000		, r-cu			Х		
2#14	PUMP1 DRIVE FAULT	DI								X		
2#14	PUMP 1 SUCTION PRESSURE ALARM	DI								Х		
2#18 STIC	PUMP 1 SPEED FEEDBACK	Al	HZ		X	Χ	X					
2#18 STIC	PUMP 1 AMP DRAW	Al	AMP		X	Х	Х		5.	× -		
2#14	PUMP 2 CALL-TO-RUN	DO		X	5							
2#14	PUMP 2 RUNNING STATUS	DI			X		X					REPORT # STARTS & RUNTIMES
2#14	PUMP 2 OVERTEMP	DI								X		
2#14	PUMP 2 DRIVE FAULT	DI								X		
2#14	PUMP 2 SUCTION PRESSURE ALARM	DI								X		
2#18 STIC	PUMP 2 SPEED FEEDBACK	Al	HZ		X	X	X					
2#18 STIC	PUMP 2 AMP DRAW	Al	AMP		X	X	X					
2#18 STIC	FLOWRATE	Al	GPM		X	X	X					
2#18 STIC	FLOW TOTAL PULSE	DI	GAL					X			X	REPORT DAILY & MONTHLY FLOV
2#18 STIC	SUCTION PRESSURE	Al	PSIG		X	X	Χ			X		
2#18 STIC	DISCHARGE PRESSURE	Al	PSIG		X	X	X		10	X		

SCADA I—O TABLE

	WALL MOUNT PACKAGED HEAT PUMP SCHEDULE													
TAG	MODEL	COOL EAT DB/WB	LING OAT DB	TOTAL COOLING MBH	SENSIBLE COOLING MBH	EER ARI-390	HEATING @ 5°F MBH	COP @ 5°F	VOLTAGE / PHASE	OA CFM	CFM	FAN ESP	RPM	ELEC. HEAT KW
HPU-1	BARD W18H	80/67	95	17.5	13.1	11.3	7.0	1.61	230/1ø	20	600	0.1	A/R	4.0

1. REFER TO HEAT PUMP SPECIFICATION FOR ADDITIONAL REQUIREMENTS

2. BASIS OF DESIGN IS BARD 3. PROVIDE MOTORIZED FRESH AIR DAMPER

4. PROVIDE DIGITAL PROGRAMMABLE AUTO-CHANGEOVER THERMOSTAT | 5. PROVIDE CUSTOM—COLOR — OWNER TO SELECT COLOR DURING SUBMITTAL REVIEW

240/120V, 3Ø, 4W LP1 **VOLTAGE: ENCLOSURE:** NEMA 1 MAINS AMPACITY: MOUNTING: SURFACE MAIN C.B. SIZE: 200A KY225 BPS LOCATION: BREAKER SPACES: 42

CIRCUIT DESCRIPTION	AMPS	POLES	BREAKER	NO.	PHASE A AMPS	PHASE B AMPS	PHASE C AMPS	NO.	BREAKER	POLES	AMPS	CIRCUIT DESCRIPTION
SPD		3	30A	1	0			2	80A	3		AFD2 PUMP #2
				3		0		4				
				5			0	6				
AFD1 PUMP #1	61	3	80A	7	61			8	80A	3		SPARE
	61			9		61		10	H. 270. WH. 270. WH. 270. WH. 270. WH. 27			201140
	61			11			61	12				
HEAT PUMP HPU-1	35	2	40A	13	40			14	20A	1	5	NORTH RECEPTACLES
	35			15		35		16				SPACE
FLOWMETER	1	1	15A	17			6	18	20A	1	5	SOUTH RECEPTACLES
SCADA RTU	3	1	15A	19	6			20	15A	1	3	PUMP CONTROL PANEL
SPACE				21		0		22				SPACE
INTERIOR LIGHTING	2	1	20A	23			4	24	20A	1	2	EXTERIOR LIGHTING
SPARE		1	20A	25	1			26	20A	1	1	GENSET ANTI-COND. HEATER
SPACE				27		13		28	30A	2	13	GENSET BLOCK HEATER
GENSET CHARGER	2	1	20A	29			15	30			13	
SPARE		1	20A	31	0			32	20A	1		SPARE
SPACE				33		0		34				SPACE
SPARE		1	20A	35			0	36	20A	1		SPARE
SPARE		1	20A	37	0		200000000000000000000000000000000000000	38	20A	1		SPARE
SPACE				39		0		40				SPACE
SPARE		1	15A	41			0	42	15A	1		SPARE
		TOTAL	AMPS PER P	HACE.	108	109	86					

$II\cap IIT$	FIVTIIDE	SCHEDIII E

1. PROVIDE AN INTEGRAL SURGE PROTECTION DEVICE (SPD) RATED 80KA MIN.

TYPE	MANUFACTURER	CATALOG SERIES	LAMPS	VOL TAGE	MOUNTING	DESCRIPTION DESCRIPTION	SYMBOL
LF-1	HOLOPHANE	EMS LED	4000 LM LED	120V	SURFACE	LINEAR FIXTURE, FIBERGLASS N4X WET LOCATION, CLEAR ACRYLIC DIFFUSER, 6KV/3KA SURGE, 5—YR WARRANTY, MEDIUM DISTRIBUTION, 4000K, 80 CRI	
LF-2	HOLOPHANE	DM30	2X1.9W LED	120V	SURFACE	EMERGENCY FIXTURE, WET LOCATION THERMOPLASTIC HOUSING, NICKEL-CADMIUM BATTERY WITH TEST SWITCH, 5-YR WARRANTY, WITH CAPACITY FOR REMOTE HEADS	
LF-3	HOLOPHANE	CZQRW	2X1.2W LED	9.6 VDC	SURFACE	EMERGENCY EGRESS FIXTURE, WET LOCATION, ALUMINUM POWDER—COAT, COLOR SELECTION BY OWNER, COLD RATED TO —22F, 5—YR WARRANTY, TWIN LAMP	Y
LF-4	HOLOPHANE	HLWPC2	11700 LM LED	120V	WALL	WALLPACK, ALUMINUM POWDER—COAT, WET LOCATION IP—65, COLOR SELECTION BY OWNER, 10KV/10KA SURGE, FULL CUTOFF, 4000K, TYPE 3 MEDIUM DISTRIBUTION, 70 CRI, 5—YEAR WARRANTY, WITH INTEGRAL PHOTOCELL	HX

KNOX COUNTY UTILITY COMMISSION BARBOURVILLE CONNECTION - KY 22

DRAWN BY: JM
CHECKED BY: BLM
CHECKED BY: BLM
DATE: July 2021
SCALE:
REVISIONS

OF KENTUCK

BENJAMIN L. MURPHY 25348

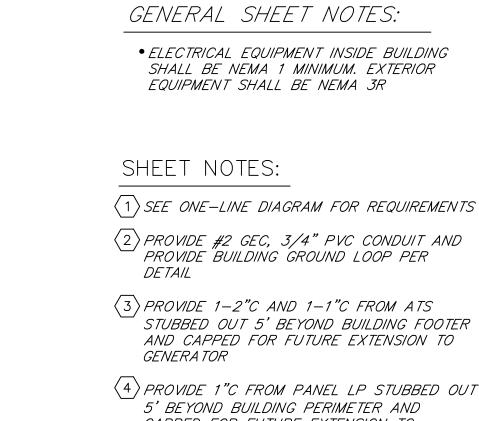
> LINVIKOINS & Environmental Engineer

KI S Civil 8

PROJECT NO. 2020132

SHEET NO. E-2

SHEET NO. E-3



4 PROVIDE 1"C FROM PANEL LP STUBBED OUT 5' BEYOND BUILDING PERIMETER AND CAPPED FOR FUTURE EXTENSION TO GENERA TOR

(5) PROVIDE 3/4"C FROM RTU STUBBED OUT 5" BEYOND BUILDING PERIMETER AND CAPPED FOR FUTURE EXTENSION TO GENERATOR

6 PROVIDE HEAT PUMP UNIT. SEE INSTALLATION DETAIL

(7) PROVIDE BELDEN 3106DB OR EQUAL CABLE 1#14G, 3/4"C

8 PROVIDE 2-2#18 STIC, 1#14G, 3/4"C

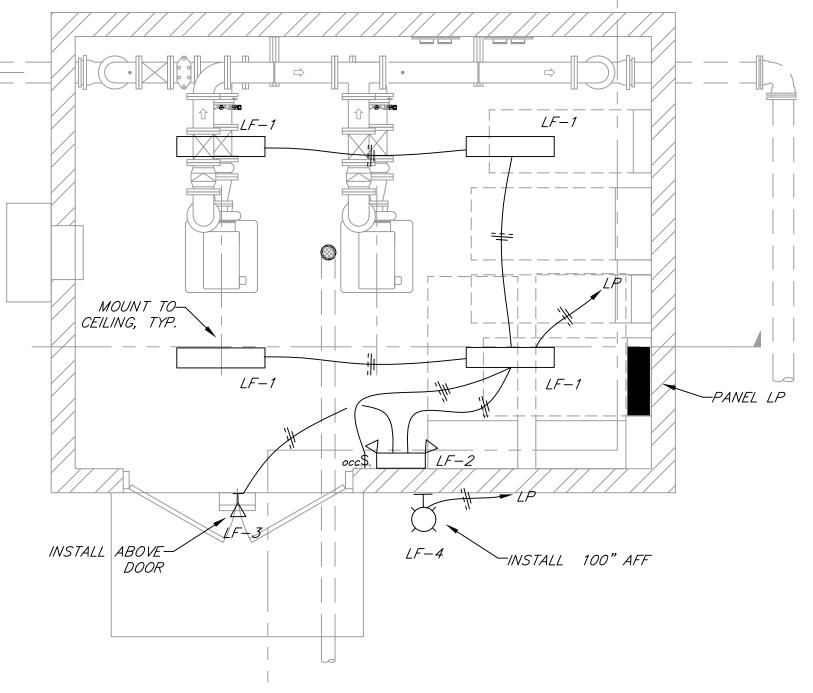
9 PROVIDE THERMOSTAT CABLE, 1#14G, 3/4"C (10) PROVIDE DOOR CONTACT SWITCHES

(11) PROVIDE 10#14, 1#14G, 3/4"C

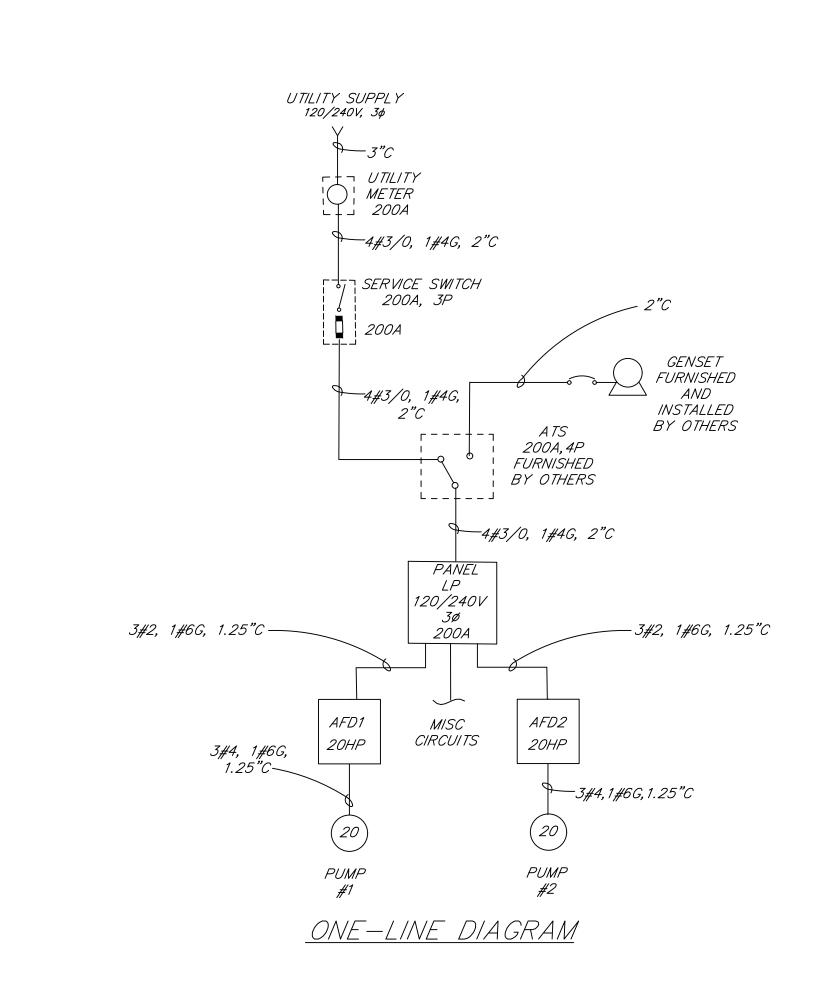
(12) PROVIDE 20#14, 1#14G, 1"C

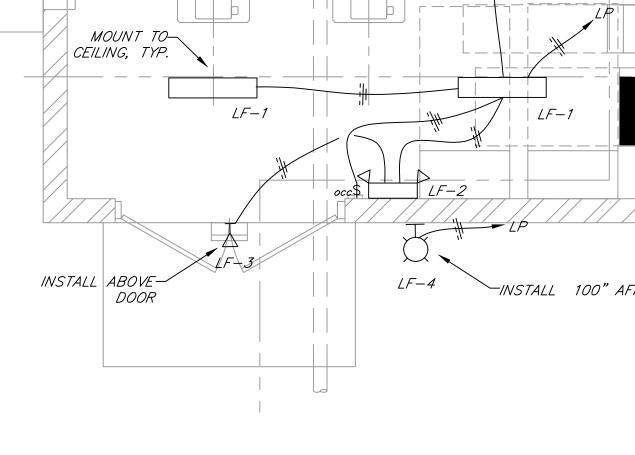
(13) ATS TO BE FURNISHED BY OTHERS AND INSTALLED BY CONTRACTOR

(14) PROVIDE 2#8, 1#10G, 3/4"C



LIGHTING PLAN *3/8"=1'-0"*





—SER VICE SWITCH POWER PLAN

-UL TRASONIC

FLOWMETER

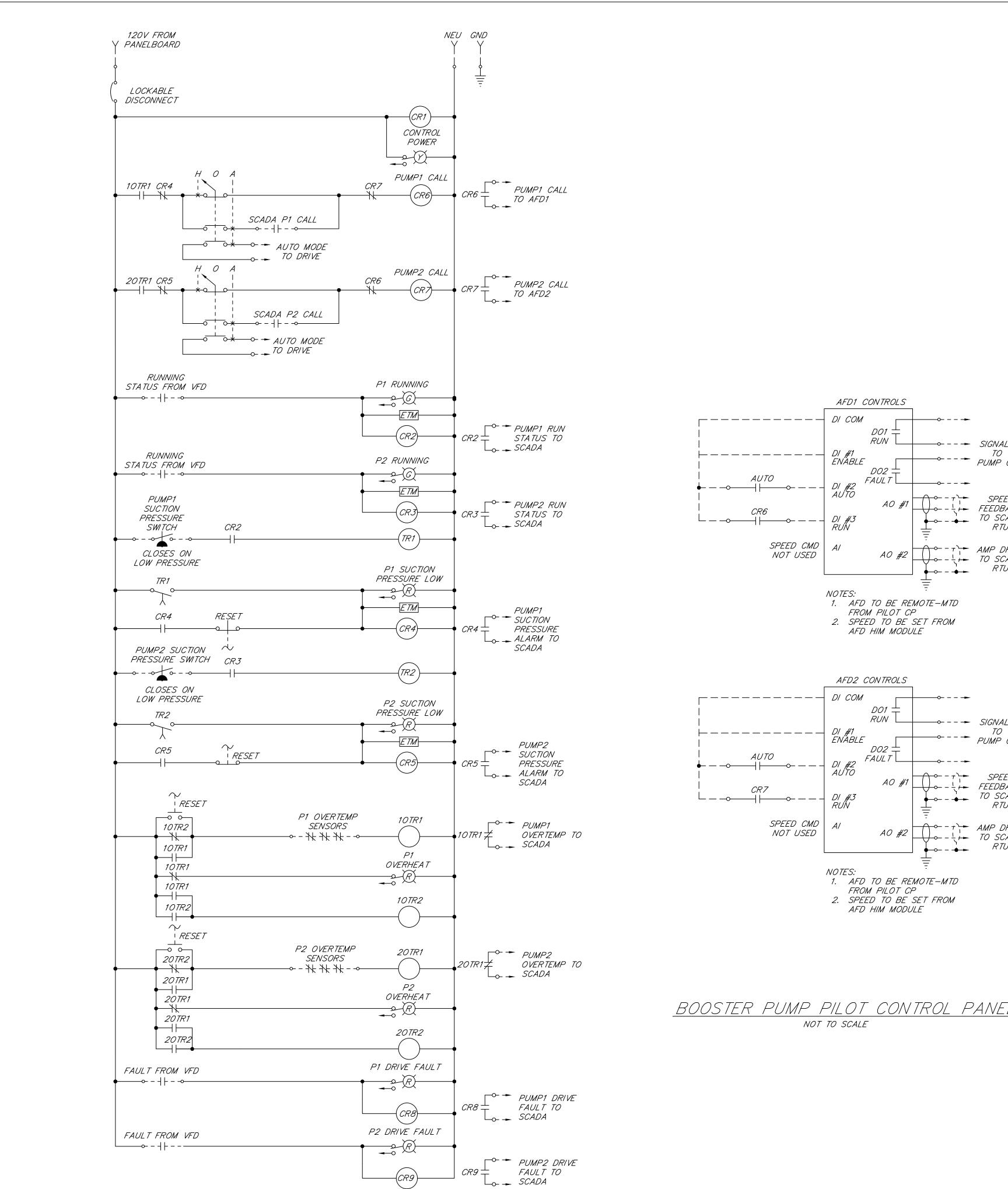
SEE ELECTRICAL

SITE PLAN FOR CONTINUATION

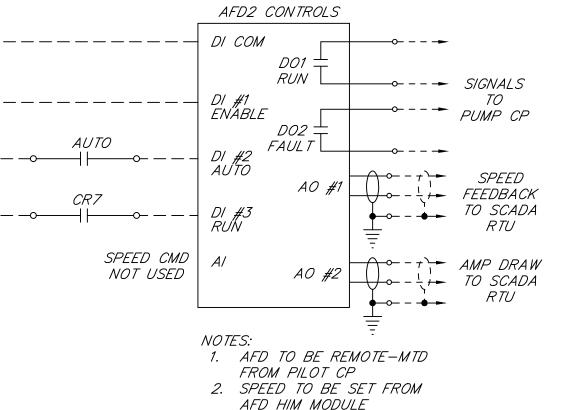
3/8"=1'-0"

-SUCTION AND DISCHARGE PRESSURE TRANSMITTERS

PRESSURE SWITCH, TYP.

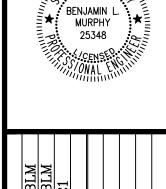


AO #1 SPEED FEEDBACK TO SCADA RTU AO #2 AMP DRAW
TO SCADA RTU



BOOSTER PUMP PILOT CONTROL PANEL

SION X 225 COMMIS ARBO OF KENTUCK



Civil & Environmental Engineers



PROJECT NO. 2020132

ELECTRICAL CONTROL CIRCUITS

Scale: 1"=20'



PROJECT NO. 2020132

SHEET NO. DIRECTIONAL BORE NO.1 B-1

── 960 19+00 18+80

PROJECT NO. 2020132

SHEET NO. B-2

DIRECTIONAL BORE NO. 2

960

13+20

13+40

13+60

13+80

14+00

14+20

14+40

14+60

14+80

15+00

15 + 20

15 + 40

15+60

15+80

16+00

KCUC DIRECTIONAL BORE NO. 2

Scale: 1"=20'

16+60

16+80

17+00

17+20

17+40

17+60

17+80

18+00

18+20

18+40

18+60



SHEET NO. B-3

DIRECTIONAL BORE NO. 3

Horizontal Plane

-A (Bearing Area

V (Volume of gravity block in C.F.)

GRAVITY THRUST BLOCK

1. Thrust restraint table is based on pipeline pressure of 200 psi and earth bearing

2. On large diameter pipes where space limitations or constuction difficulties render

and concrete thrust block size revised at the discretion of the engineer.

3. Concrete shall be 3000 psi minium conforming to KTC Specifications 601.

VERTICAL THRUST BLOCK SCHEDULE

13

29

restrained joint system must be approved by the Engineer.

45° BEND

4. Accessibility to fittings and bolts must be maintained.

| A | V | A

123

17 | 176 | 8

393

484

23 238

29 | 311 |

66 696

July, 2011

5. Wrap fittings in plastic prior to placing concrete.

capacity of 1500psf. During construction, the specific soil type may be evaluated

concrete thrust blocks not feasible or impractical, joint system may be used. This

22 1/2° BEND | 11 1/4° BEND

50

88

137

V | A |

25 | 1

95 2

128 3

167 4

374 8

43

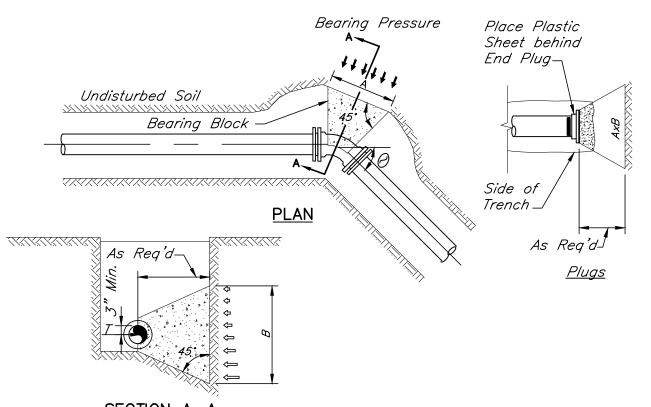
260



SHEET NO.

D-1

MISCELLANEOUS DETAILS



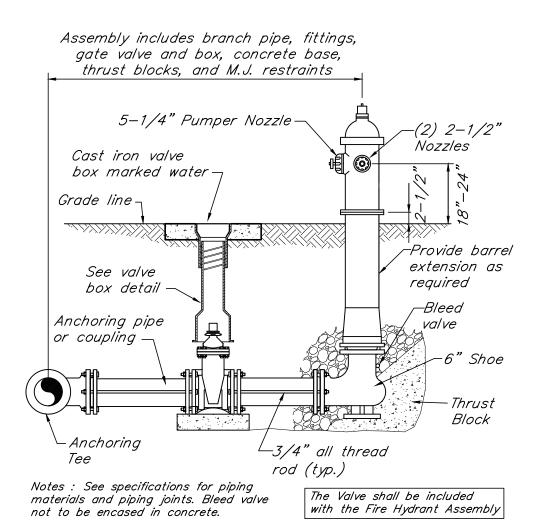
SECTION A-A

NOTES:

- 1. Thrust restraint table is based on pipeline pressure of 200 psi and earthbearing capacity of 1500 psf. During construction, the specific soil type may be evaluated and concrete thrust block size revised at the discretion of the engineer.
- 2. On large diameter pipes where space limitations or constuction difficulties render concrete thrust blocks not feasible or impractical, joint system may be used. This restrained joint system must be approved by the Engineer.
- 3. Concrete shall be 3000 psi minium conforming to KTC Specifications 601.
- 4. Accessibility to fittings and bolts must be maintained.
- 5. Wrap fittings in plastic prior to placing concrete. HORIZONITAL THRUST BLOCK SCHEDULE

	HORIZONTAL THRUST BLOCK SCHEDULE											
PIPE SIZE	90°	BEND	45° l	BEND	22 1/2	* BEND	11 1/4	* BEND	TEE, DEAD END			
(INCHES)	Α	В	Α	В	Α	В	Α	В	Α	В		
3 & 4	3'-3"	1'-8"	2'-4"	1'-2"	1'-8"	1'-0"	1'-0"	1'-0"	2'-8"	1'-4"		
6	4'-8"	2'-4"	3'-5"	1'-8"	2'-6"	1'-3"	1'-6"	1'-0"	3'-10"	2'-0"		
8	6'-0"	3'-0"	4'-5"	2'-3"	3'-2"	1'-7"	2'-3"	1'-2"	5'-0"	2'-6"		
10	7'-6"	3'-9"	5'-5"	2'-9"	3'-10"	2'-0"	2'-9"	1'-5"	6'-3"	3'-2"		
12	8'-10"	4'-5"	6'-6"	3'-3"	4'-8"	2'-4"	3'-4"	1'-8"	7'-5"	3'-9"		
14	10'-3"	5'-2"	7'-6"	3'-9"	5'-4"	2'-8"	3'-10"	2'-0"	8'-8"	4'-4"		
16	11'-8"	5'-10"	8'-7"	4'-4"	6'-1"	3'-0"	4'-4"	2'-2"	9'-9"	4'-11"		
18	13'-0"	6'-6"	9'-7"	4'-9"	6'-10"	3'-5"	4'-10"	2'-5"	11'-0"	5'-6"		
20	14'-5"	7'-3"	10'-7"	5'-4"	7'-7"	3'-9"	5'-4"	2'-8"	12'-2"	6'-1"		
24	17'-3"	8'-8"	12'-8"	6'-4"	9'-0"	4'-6"	6'-5"	3'-3"	14'-6"	7'-3"		

HORIZONTAL THRUST BLOCK



FIRE HYDRANT

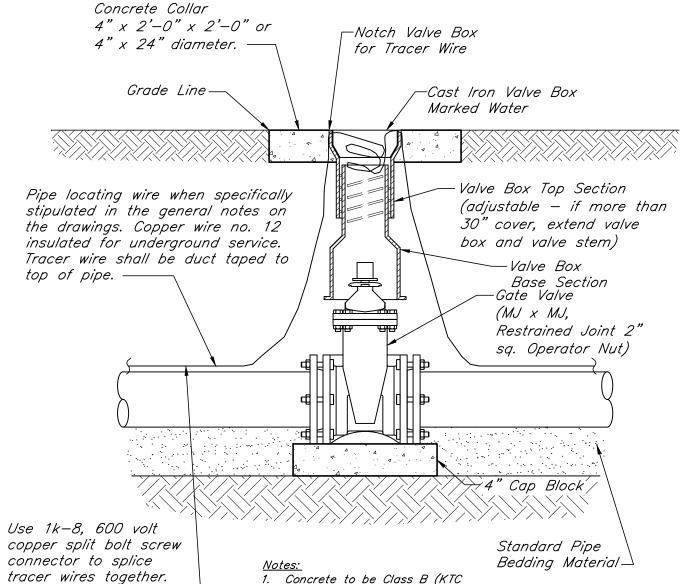
Scale: 1/2"-1'-0"

Dec., 2010

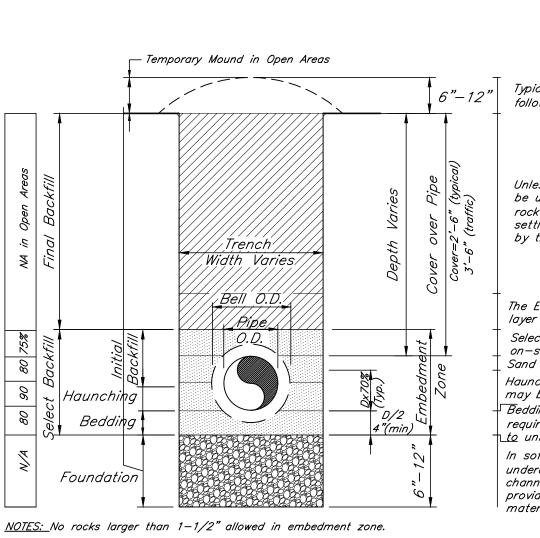
Tape splices with scotch

1. Concrete to be Class B (KTC Spec. 601)
2. See Specifications For Piping seal tape model 2229 — Materials And Piping Joints

─Notch Valve Box for Tracer Wire -Cast Iron Valve Box Marked Water -Valve Box Top Section (adjustable - if more than 30" cover, extend valve box and valve stem) Base Section -Gate Valve $(MJ \times MJ,$ Restrained Joint 2" sq. Operator Nut) →4" Cap Block Standard Pipe



VALVE BOX INSTALLATION



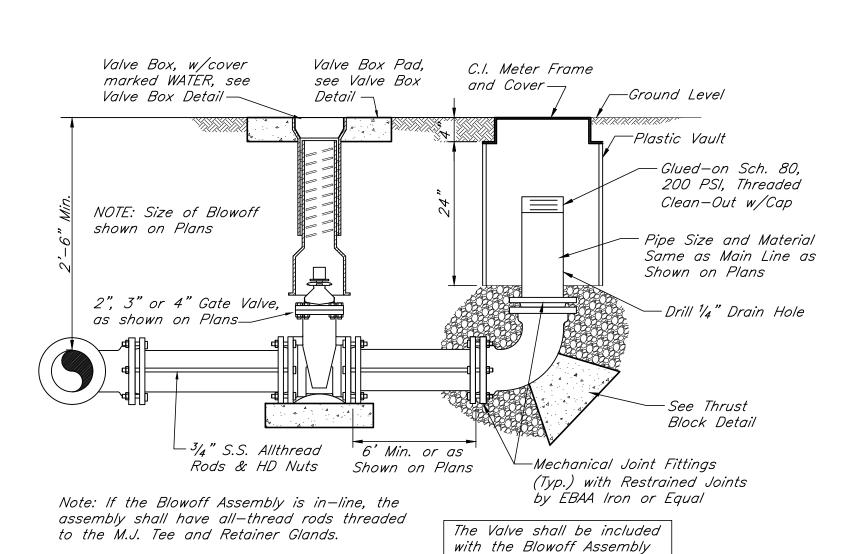
material, etc. which is unyielding.

Trench width should be no wider than necessary for adequate work room and to assure safe working conditions. Nominal outside diameter (O.D.) pipe plus 6" on each side is typically considered minimal, with 8" minimum on each side for gravity sewer installation. For gravity sewer, pipe to be bedded on No. 9 stone and remainder of embedment zone to be backfilled with sand.

TRENCH BACKFILL OPEN AREAS — PLASTIC PIPE

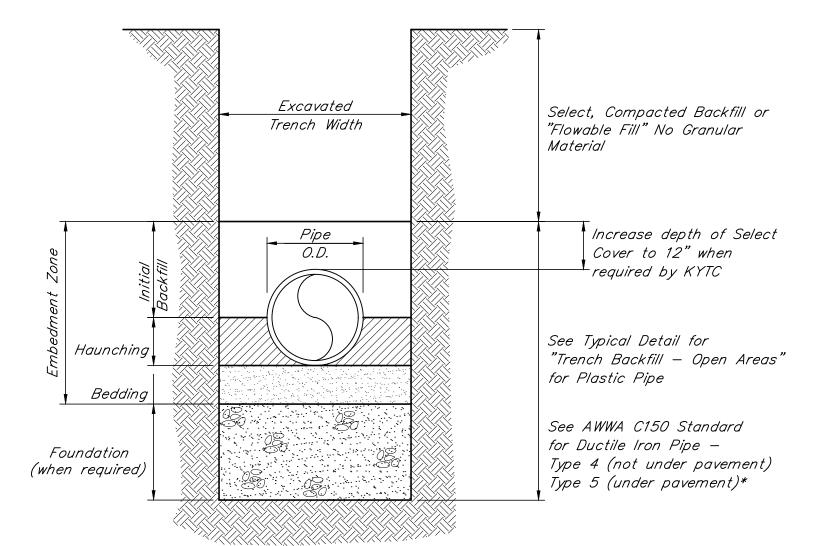
Mar., 2011

Scale: 3/4"=1'-0"



Typically, open areas are final graded, dressed and seeded following two soaking rains...excluding KYTC road ROW's Unless otherwise specified, material excavated from trench may be used for final backfill provided it is relatively free of large rock (>8"), or mixed with sufficient dirt to minimize voids and settlement, and free of other unsuitable materials... as approved by the Engineer The Engineer may require selective placement of an extra buffer layer for extremely rocky backfill to prevent migration Select backfill, lightly compacted (bucket shaping) using suitable on-site material, or dumped sand. Sand or very select material, hand tamped Haunching to be carefully placed — Sand or sandy/clay soil. No. 9's may be required if weak foundation is encountered Bedding to be sand or approved equivalent, (except No. 57's may be required if weak foundation encountered) hand placed and smoothed <u>to uniform grade for support of pipe</u> In soft, wet, muddy or otherwise yielding foundation conditions, undercutting and replacement with No. 2 Stone and/or Class II channel lining, or equivalent, will be required. Objective is to provide a trench bottom free of large stones, clods, frozen

Typical desired densities in open areas are depicted above in the boxes to the left of the figure. In other laying situations, more stringent selection,



(2) No. 6 Rebar

bent as shown

Backfill under Pipe with

NOTES:

3 & 4 | 29

12 248

16 | 439

555

685

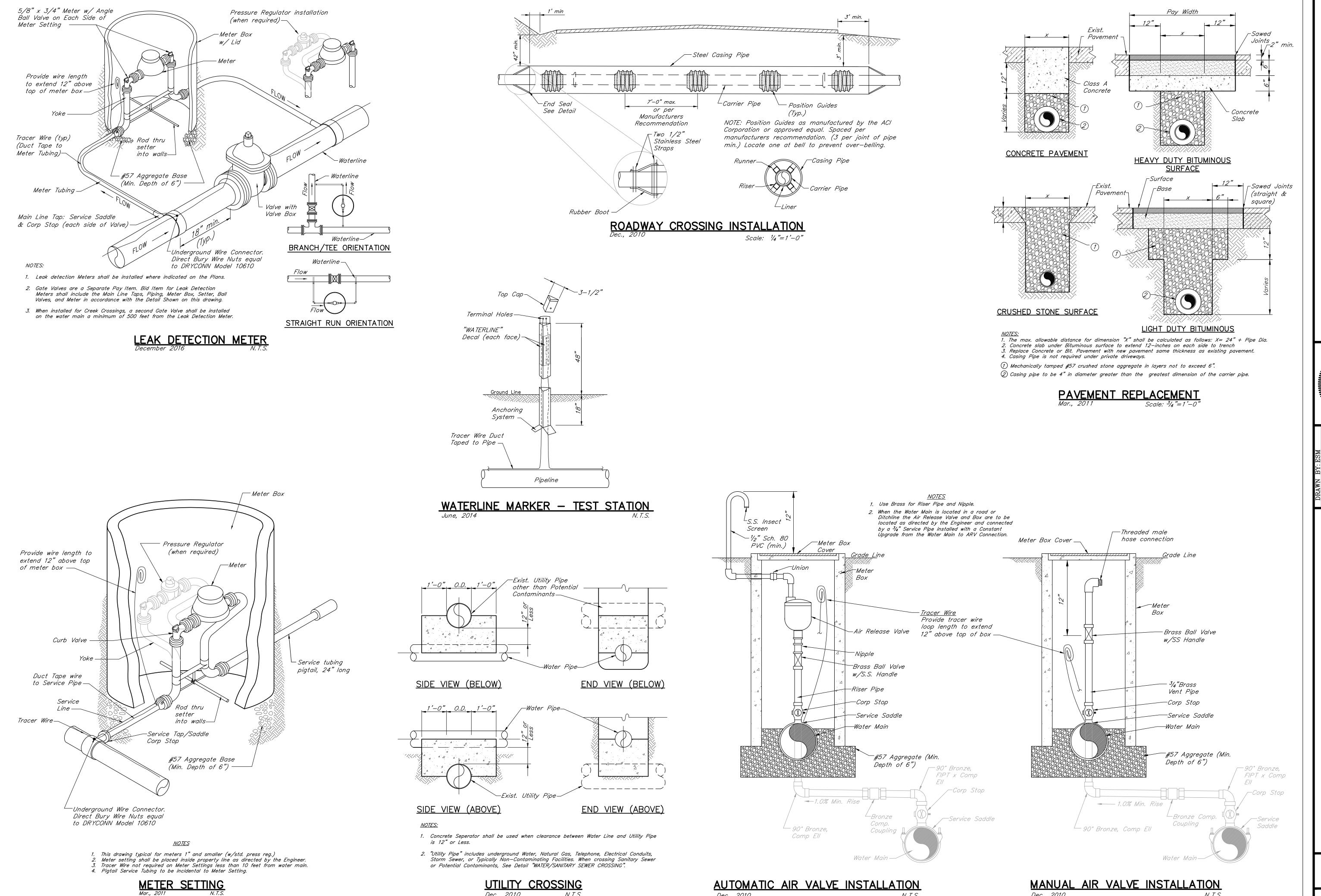
KTC #57 Crushed Stone —

* When "Open-cutting" of State Highway is permitted, pipe laying, encasement requirements, backfill placement, pavement replacement, etc. shall be as required by the encroachment permit issued by the Kentucky Transportation Cabinet (KYTC). By reference, such permit(s) shall become part of the contract. It shall be the CONTRACTOR'S responsibility to maintain a copy of KYTC permit(s) on the job site at all times.

TRENCH BACKFILL ON HIGHWAY ROW

Dec., 2010

N.T.S.



KNOX COUNTY UTILITY COMMISSION OF STREET CONNECTION - FOR COUNTY KENTILEY

DRAWN BY: ESM
CHECKED BY: KDT
CHECKED BY: JDS
DATE: July 2021
SCALE: As Noted
REVISIONS
REVISIONS

ENVIRONS

Renvironmental Engineers

Civil &

PROJECT NO. **2020132**

SHEET NO. **D-2**

MISCELLANEOUS DETAILS

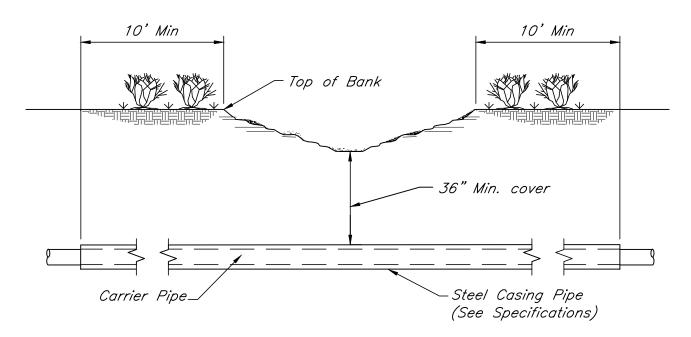
> SHEET NO. D-3

Original Ground

NOTES: This crossing shall be made with appropriate fittings to prevent excess joint deflection. Normally four (4) fittings will be required. The Contractor, at his option, may provide extra approach trench depth to avoid use of bends and thrust blocks. Allowable deflection of pipe may not be exceeded under any situation.

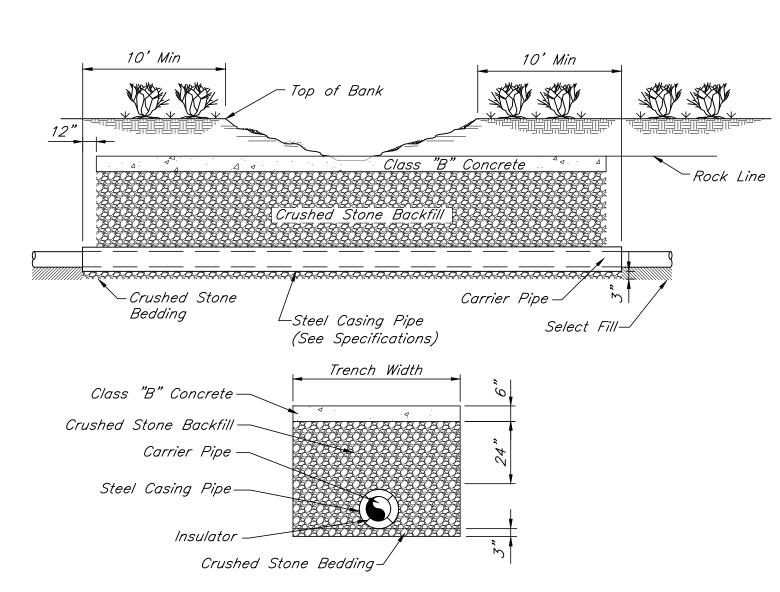
Thrust blocking for vertical bends shall be anchored to pipe in accordance with the Detail for Vertical Thrust Blocks.

NORMAL DITCH CROSSING WITHOUT ENCASEMENT Dec., 2010



Note: This Crossing Shall Be Made With Appropriate fittings to prevent Excess Joint Deflection. Normally Four (4) Fittings will be required. The Contractor, at his option, may provide extra approach Trench Depth to avoid use of Bends. Allowable Deflection of Pipe may not be exceeded under any situation. See typ. Roadway Crossing Installation Detail for Insulator placement.

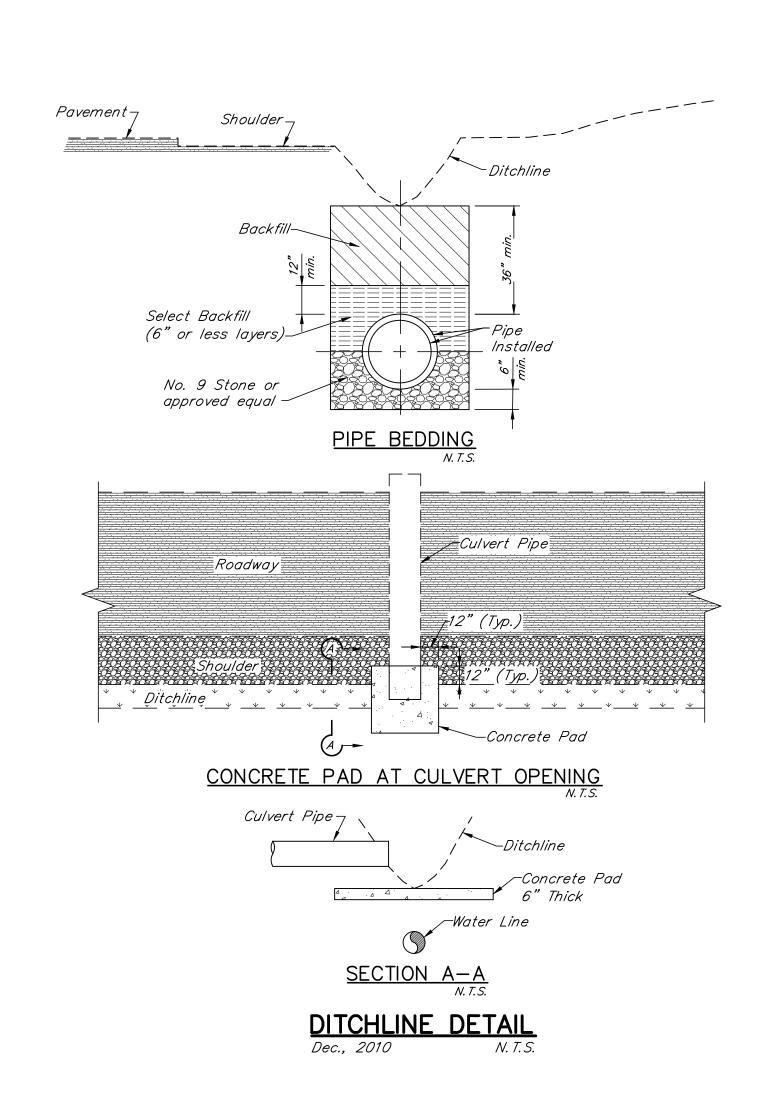
SPECIAL STREAM CROSSING IN EARTH (TYPE A)

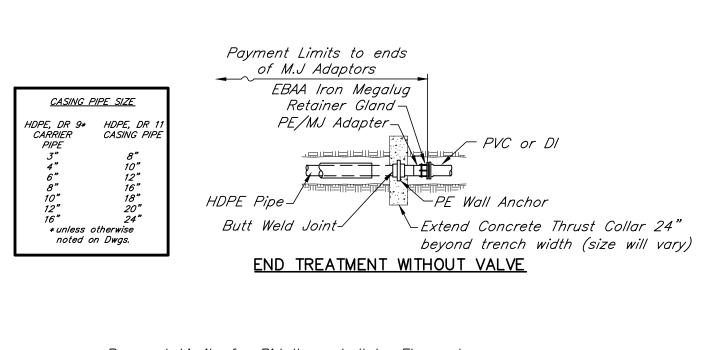


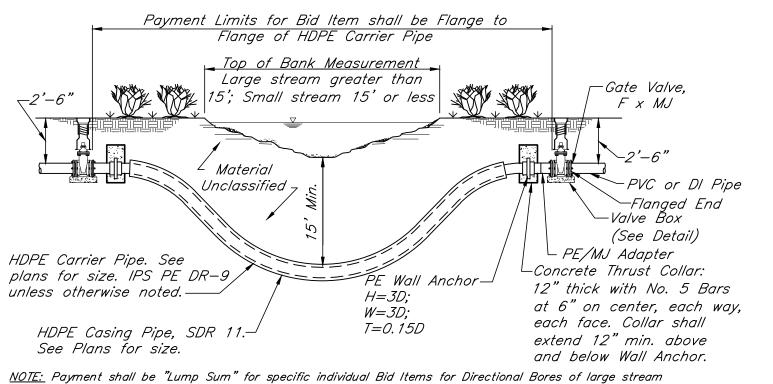
<u>NOTE:</u> This Crossing shall be made with Appropriate Fittings to prevent Excess Joint Deflection. Normally Four (4) Fittings will be Required. The Contractor, at his option, may provide extra Approach Trench Depth to avoid use of Bends. Allowable Deflection of Pipe may not be exceeded under any situation. See Typ. Roadway Crossing Installation Detail for Insulator Placement

STREAM CROSSING IN SOLID ROCK (TYPE B)
Dec., 2010

N.T.S.







NOTE: Payment shall be "Lump Sum" for specific individual Bid Items for Directional Bores of large stream crossings and/or some classified small streams where the physical crossing characteristics differ significantly from the other small streams in the project. Determination of required length is responsibility of Contractor. When a creek crossing test meter is shown on the drawings and it is necessary to tap the HDPE pipe for the meter connection, the tapping saddle specifically manufactured for HDPE pipe shall be

Payment shall be "each" for directional bores of small stream crossings unless contained in an individual specific bid item. All small stream crossings in the project shall be considered the same regardless of width (up to 15 L.F.) or depth. It is the responsibility of the Contractor to determine an average unit price that will be used for payment for each instance a blue line stream is crossed. Stream crossings may be added, for extended lines beyond those shown on the plans, at the same unit price providing the crossings are reasonably similar to those in the initial project. Stream crossings may be deleted, without affecting the unit price, if a line is deleted or shortened.

DIRECTIONAL BORE FOR STREAM CROSSINGS