

## Attachment K Traffic Study

# **Traffic Impact Study**Thoroughbred Solar - Hart County, KY

Prepared for

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#### INTRODUCTION

This traffic impact study has been completed for proposed development of a solar facility known as Thoroughbred Solar (the Project) in Hart County, Kentucky, west of the town of Rowletts, KY. The majority of the development will be located within Rowletts Cave Springs Road, Interstate 65, State Highway 335 and G Wilson Lane. An additional piece of development is enclosed by State Highway 335, Johns Lane, and Maple Grove Lane. The vicinity map (Map 1) displays the location of the Project and study area. The site map (Map 2) illustrates the proposed layout of the Project and its access drives.

The Project will be built on existing farm/ agricultural land. While construction activities will generate additional traffic on local roads, once the Project is generating electricity using solar energy the number of trips to the site will be minimal. This traffic impact study analyzes five roadways in the area that will be impacted by entrances to the Project or the trips generated by Project construction. These roadways include the following:

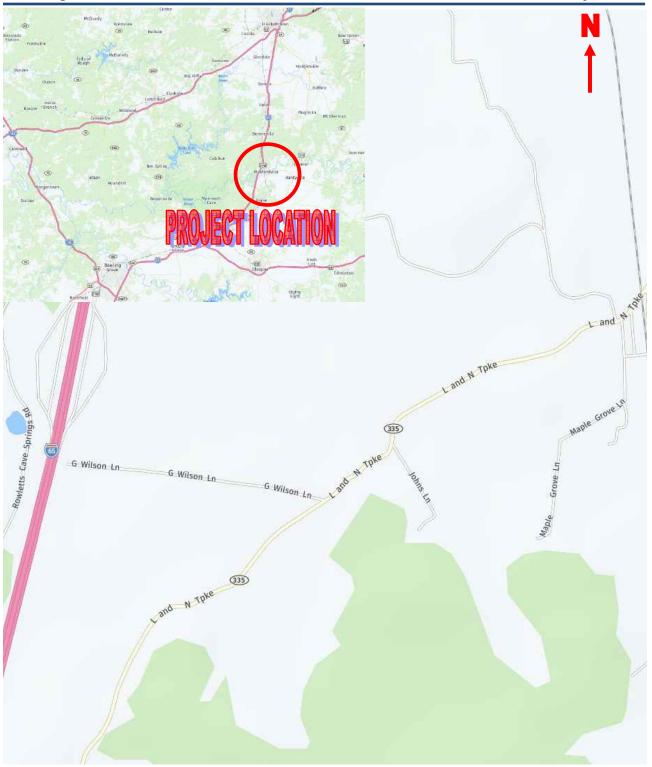
- G Wilson Lane
- Johns Lane
- State Highway 335
- Maple Grove Lane
- Rowletts Cave Springs Road

In the vicinity of the Project, the surrounding area consists of farmland and scattered single family housing, with a greater density of single-family homes in Rowletts.





Johns Lane near the Proposed Substation/Switchyard Entrance (left)
State Highway 335 near the proposed entrance (right)



Map 1. Vicinity Map

#### **EXISTING CONDITIONS**

#### **Regional and Local Access**

The proposed development can be accessed directly from State Highway 335, G Wilson Lane, Rowletts Cave Spring Road, and Johns Lane. State Highway 335 will provide regional access to the proposed development. A brief description of the surrounding roadways follows:

**State Highway 335** – State Highway 335 provides local and regional access to the Project site and generally runs in a southwest to northeast direction in the study area. Lane widths measure approximately 9 feet. In the vicinity of the Project site, this road consists of a single thru lane in each direction. The current speed limit along this roadway is 55 miles per hour (mph). State Highway 335 access Interstate 65 both north and south of the Project site.



**State Highway 335** 

**Rowletts Cave Spring Road** – Rowletts Cave Spring Road provides local access to the Project site and generally runs in an east-west direction in the study area. The roadway measures 17 feet wide without any striping. The current speed limit along this roadway is unposted; per Kentucky law the speed limit defaults to 55 mph.



**Rowletts Cave Spring Road** 



**G Wilson Lane** – G Wilson Lane will provide local access to the Project site and generally runs in an east-west direction in the study area. The roadway measures approximately 17 feet wide without any striping. In the vicinity of the Project the posted speed limit is 30 mph.



**G Wilson Lane** 

**Johns Lane** – Johns Lane will provide local access to the Project site and generally runs in a northwest direction in the study area. The roadway measures approximately 12 feet wide without any striping. The current speed limit along this roadway is unposted; per Kentucky law the speed limit defaults to 55 mph.



**Johns Lane** 

Maple Grove Lane – Maple Grove Lane will not provide direct access to the site but may be impacted by the traffic generated by the site. The roadway measures approximately 12 feet wide without any striping. In the vicinity of the Project site, this road runs in a northeast and a north-south direction. The current speed limit along this roadway is unposted; per Kentucky law the speed limit defaults to 55 mph.



Maple Grove Lane

Interstate 65 – Interstate 65 will provide regional access to the Project site and generally runs in a north-south direction in the study area. The roadway provides three lanes in each direction with lane widths measuring 12 feet, an inside paved shoulder of 16 feet and outside paved shoulder measuring 12 feet with a barrier wall separating the directions of travel. In the vicinity of the Project the posted speed limit is 70 mph.

#### LEVEL OF SERVICE AND DELAY

Level of Service (LOS) was used as the measure of effectiveness for each roadway. According to the Highway Capacity Manual, the level of service is defined in terms of average travel speed, percent time spent following and percent of free-flow speed for two lane highways (See Table 1). The average travel speed (ATS) reflects mobility on a two-way highway. The percent time spent following (PTSF) represents the maneuverability on the highway along with comfort and convenience of travel. The percent free-flow speed (PFFS) represents the ability of the vehicle to travel at or near the posted speed limit. A Level of Service C is desirable

	CLASS	I HIGHWAYS	CLASS II HIGHWAYS	CLASS III HIGHWAYS						
LOS	AVG TRAVEL SPEED (MPH)	PERCENT TIME SPENT FOLLOWING (%)	PERCENT TIME SPENT FOLLOWING (%)	PERCENT FREE- FLOW SPEED (%)						
Α	>55	≤35	≤40	>91.7						
В	>50-55	>35-50	>40-55	>83.3-91.7						
С	>45-50	>45-50 >50-65		>75.0-83.3						
D	>40-45	>65-80	>70-85	>66.7-75.0						
E	≤40	>80	>85	≤66.7						
F	Demand exceeds capacity									

Table 1. Two-Lane Highway Level of Service



#### **Base Traffic Volumes (existing condition)**

Manual traffic counts were taken using traffic tubes for four consecutive days. Traffic counts at G Wilson Lane, Johns Lane, and Maple Grove Lane were taken Saturday, May 7<sup>th</sup>, 2022 through Tuesday, May 10<sup>th</sup>, 2022. Traffic counts at State Highway 335 and Rowletts Cave Springs Road were taken Wednesday, May 12<sup>th</sup>, 2022 through Sunday, May 15<sup>th</sup>, 2022. The traffic tubes were placed in sections of the roadways that will be affected by trips generated for the Project. All traffic volumes can be found in the Appendix.

#### **Background Traffic Volumes**

Construction is estimated to occur in 2023, with the estimated completion date for the Project by the end of 2024. Based on Kentucky Transportation Cabinet (KYTC) count stations along State Highway 335 (050256), the average annual daily traffic (AADT) has been increasing over the past ten years along State Highway 335. KYTC did not have historical traffic data for Rowletts Cave Springs Road, G Wilson Lane, Johns Lane or Maple Grove Lane.

Based on this data, this analysis assumes that the traffic volumes along all roads will increase by two percent over the next ten years. The KYTC count station data for station 050256 can be found in the Appendix.

#### **METHODOLOGY**

Level of Service, average speed, and travel time were measures of effectiveness analyzed using the highway capacity software (HCS2022).

Construction trips were generated for the Project and then distributed to the roadway system based on the existing traffic patterns and engineering judgment. For the analysis, the study uses traffic volumes from the current year as background volumes grown to the completion year, 2024. The design year for this Project was determined to be 2034, ten years after the completion year for the Project. As noted above, based on KYTC traffic counts, all roads were increased by two percent to determine the background traffic for the design year (2034). The assigned volumes from the Project and the background traffic volumes combined to produce the total proposed traffic volumes for existing and build out conditions. HCS2022 was used to analyze the roadway network for existing and proposed conditions in both the current year and build out year (2034). The existing background volumes, LOS, and travel times can be found in the Appendix along with 2022 existing traffic (Fig 1), 2024 background traffic (Fig 1A), 2024 build (Fig 3), 2034 background (Fig 4), and 2034 build (Fig 5) traffic volumes.



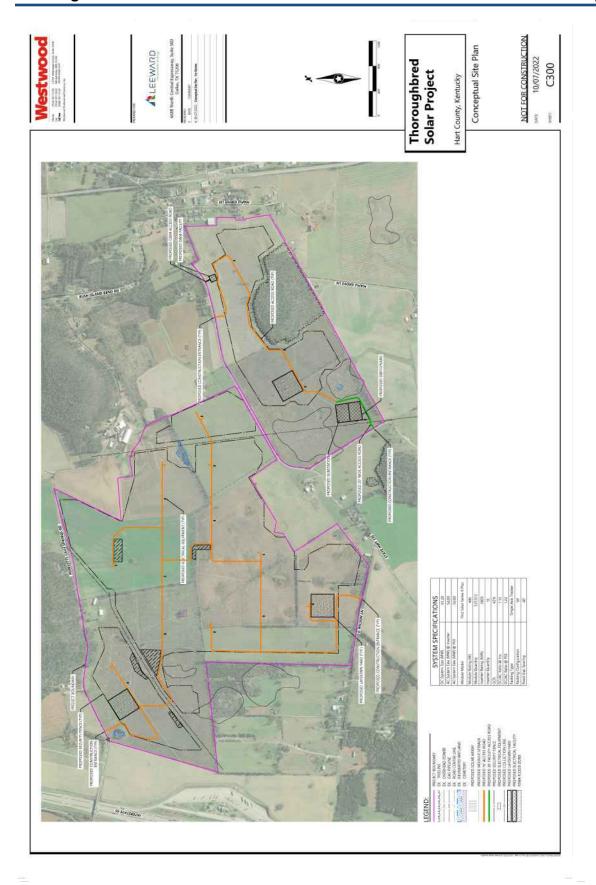
#### TRIP GENERATION AND PROJECTED TRAFFIC VOLUMES

Solar facilities are not included in the *Trip Generation*, 11<sup>th</sup> Edition, a nationally recognized resource of trip generation rates published by the Institute of Transportation Engineers. Therefore, trip estimates were based on information provided by the client and engineering judgement.

#### SITE TRIP GENERATION

The Project will consist of a 50 megawatt solar facility. A solar facility consists of areas of ground-mounted solar panels constructed to generate electricity using solar energy; the electricity will be fed into the existing overhead transmission line that extends across the Project site. The Project will require construction equipment and workers to travel to and from the site throughout the construction phases. The applicant, Thoroughbred Solar, LLC, provided information for each of the different construction phases. It was determined that the one that generated the most trips was 100 vehicles during certain portions of the construction phase of the Project. This value includes construction delivery vehicles and construction worker's vehicles entering and exiting the site. Based upon consideration of 100 vehicles, it was further assumed that 50 vehicles would enter and exit each of the two portions of the Project sites (on either side of State Highway 335) shown on Map 2. Use of the maximum number of trips during construction is expected to result in a maximum-impact scenario, as lesser numbers of deliveries and workers may be present for much of the estimated 12 months of Project construction. Once the Project is operational, worker trips would be extremely limited.





Map 2. Site Map



#### LEVEL OF SERVICE AND DELAY ANALYSIS

All roadway traffic volumes, average vehicle speeds, and LOS information can be found in the Appendix. With background traffic expected to increase as mentioned earlier, the 2034 base traffic volume information will be the focus for comparisons between the projected background traffic and the proposed traffic volumes (full build out). The 2034 No-Build volumes would exist on the roadway system in the absence of the proposed development, and the 2034 Build Volumes are the volumes with the Project's construction traffic included.

The No-Build Scenario analysis assumes that no proposed improvements to the roadway system have been implemented, and without Project construction traffic.

#### INTERSECTION ANALYSIS

#### 2024 No Build Analysis

The HCS analysis reveals that, for 2024 No Build conditions, all roadways operate with a LOS A for both peak hours of the day. Travel times are between one and two and a half minutes per mile of roadway and the average speed is more than 55 mph for all roadways except G Wilson Lane which was calculated to have an average speed of approximately 28 mph.

#### 2024 Build Analysis

The HCS analysis shows that the 2024 Build conditions are similar to the 2024 No Build. All roadways operate with a LOS A for both peak hours of the day. Travel times are between one and two and a half minutes per mile of roadway and the average speed is more than 55 mph for all roadways except G Wilson Lane which continued to have a calculated average speed of approximately 28 mph.

Although major traffic associated with the Project would occur only during the construction phase, because access is on a state route, KYTC standards were followed for this analysis. Therefore, the following scenarios reflect traffic growth over an additional 10-year period.

#### 2034 No Build Analysis

The HCS analysis reveals that all roadways operate similar to the 2024 No Build conditions. All roadways operate with a LOS A for both peak hours of the day. Travel times are between one and two and a half minutes per mile of roadway and the average speed is more than 55 mph for all roadways except G Wilson Lane which has an average speed of approximately 28 mph.

#### **2034 Build Analysis**

The HCS analysis shows that the Build conditions are similar to the 2034 No Build. All roadways operate with a LOS A for both peak hours of the day. Travel times are between one and two and a half minutes per mile of roadway and the average speed is more than 55 mph for all roadways except G Wilson Lane which has an average speed of approximately 28 mph.



		2024 NO BUI	LD		
AM PEAK	Average Speed mph	Percent Followers %	Travel Time to Travel 1 mile, min	Followers Density Foll/min/In	Vehicle LOS
G WILSON LN	27.8	5.1	2.16	0	А
JOHNS LN	56	0	1.07	0	А
MAPLE GROVE LN	55.8	3.7	1.08	0	Α
ROWLETTS CAVE SPRINGS RD	56.3	8.8	1.07	0	Α
STATE HIGHWAY 335	56.4	14.5	1.06	0.1	Α
PM PEAK	Average Speed mph	Percent Followers %	Travel Time to Travel 1 mile, min	Followers Density Foll/min/In	Vehicle LOS
	Speed	Followers	Time to Travel 1	Density	
G WILSON LN	Speed mph	Followers %	Time to Travel 1 mile, min	Density Foll/min/In	LOS
G WILSON LN JOHNS LN	Speed mph	Followers % 7.3	Time to Travel 1 mile, min	Density Foll/min/In	LOS
PM PEAK  G WILSON LN  JOHNS LN  MAPLE GROVE LN  ROWLETTS CAVE SPRINGS RE	Speed mph 27.8	Followers % 7.3 2.1	Time to Travel 1 mile, min 2.16	Density Foll/min/In 0	LOS A A

Table 2. 2024 No Build Summary

AM PEAK Speed mph			2024 BUILD	1		
JOHNS LN   56   0   1.07   0   A	AM PEAK	Speed	Followers	Time to Travel 1	Density	Vehicle LOS
MAPLE GROVE LN         55.8         3.7         108         0         A           ROWLETTS CAVE SPRINGS RD         56.3         8.8         107         0         A           STATE HIGHWAY 335         55.4         26.3         108         0.7         A           Average Speed mph         Percent Followers Followers Mph         Followers Density Foll/min/In mile, min         Vehicl Travel Time to Travel T	G WILSON LN	27.8	24.4	2.16	0.8	Α
ROWLETTS CAVE SPRINGS RE   56.3   8.8   1.07   0   A	JOHNS LN	56	0	1.07	0	А
Average   Percent   Travel   Time to   Followers   Followers   Followers   Followers   Followers   Travel 1   Followers   Followers   Travel 1   Followers   Followers   Travel 1   Followers   Followers   Followers   Followers   Travel 1   Followers   Followers   Followers   Density   Foll/min/In   Indicate   Followers   Foll	MAPLE GROVE LN	55.8	3.7	1.08	0	Α
PM PEAK         Average Speed mph         Percent Followers Wmph         Travel Time to Travel 1 mile, min         Followers Density Foll/min/In         Vehicl LOS           G WILSON LN         27.6         27         2.17         1         A           JOHNS LN         56         2.1         1.07         0         A           MAPLE GROVE LN         55.8         2.8         1.08         0         A           ROWLETTS CAVE SPRINGS RE         56.3         8         1.07         0         A	ROWLETTS CAVE SPRINGS RE	56.3	8.8	1.07	0	А
PM PEAK         Average Speed mph         Percent Followers Wallson LN         Time to Travel 1 mile, min         Followers Density Foll/min/In         Vehicl LOS           G WILSON LN         27.6         27         2.17         1         A           JOHNS LN         56         2.1         1.07         0         A           MAPLE GROVE LN         55.8         2.8         1.08         0         A           ROWLETTS CAVE SPRINGS RE         56.3         8         1.07         0         A	STATE HIGHWAY 335	55.4	26.3	1.08	0.7	Α
G WILSON LN 27.6 27 2.17 1 A  JOHNS LN 56 2.1 1.07 0 A  MAPLE GROVE LN 55.8 2.8 1.08 0 A  ROWLETTS CAVE SPRINGS RE 56.3 8 1.07 0 A	PM PEAK	Speed	Followers	Time to	Density	Vehicle LOS
JOHNS LN         56         2.1         1.07         0         A           MAPLE GROVE LN         55.8         2.8         1.08         0         A           ROWLETTS CAVE SPRINGS RD         56.3         8         1.07         0         A		·				
MAPLE GROVE LN         55.8         2.8         1.08         0         A           ROWLETTS CAVE SPRINGS RE         56.3         8         1.07         0         A	G WILSON LN	27.6	27	2.17	1	A
ROWLETTS CAVE SPRINGS RE         56.3         8         1.07         0         A	JOHNS LN	56	2.1	1.07	0	Α
	MAPLE GROVE LN	55.8	2.8	1.08	0	Α
STATE HIGHWAY 225 550 210 107 0.4 A	ROWLETTS CAVE SPRINGS RE	56.3	8	1.07	0	А
31ATETIIGIIWAT 333 33.9 21.9 1.07 0.4 A	STATE HIGHWAY 335	55.9	21.9	1.07	0.4	Α

Table 3. 2024 Build Summary



		2034 NO BUI	LD		
AM PEAK	Average Speed mph	Percent Followers %	Travel Time to Travel 1 mile, min	Followers Density Foll/min/In	Vehicle LOS
G WILSON LN	27.8	5.9	2.16	0	Α
JOHNS LN	56	0	1.07	0	А
MAPLE GROVE LN	55.8	4.4	1.08	0	А
ROWLETTS CAVE SPRINGS RE	56.3	10.1	1.07	0.1	Α
STATE HIGHWAY 335	56.4	16.7	1.06	0.2	Α
PM PEAK	Average Speed mph	Percent Followers %	Travel Time to Travel 1 mile, min	Followers Density Foll/min/In	Vehicle LOS
	Speed	Followers	Time to Travel 1	Density	
G WILSON LN	Speed mph	Followers %	Time to Travel 1 mile, min	Density Foll/min/ln	LOS
PM PEAK G WILSON LN JOHNS LN MAPLE GROVE LN	Speed mph	Followers % 8	Time to Travel 1 mile, min	Density Foll/min/In	LOS
G WILSON LN JOHNS LN	Speed mph 27.8	Followers % 8 2.1	Time to Travel 1 mile, min 2.16	Density Foll/min/In 0	LOS A A

Table 4. 2034 No Build Summary

		<b>2034 BUILD</b>			
AM PEAK	Average Speed mph	Percent Followers %	Travel Time to Travel 1 mile, min	Followers Density Foll/min/In	Vehicle LOS
G WILSON LN	27.8	24.6	2.16	0.8	Α
JOHNS LN	56	0	1.07	0	Α
MAPLE GROVE LN	55.8	4.4	1.08	0	Α
ROWLETTS CAVE SPRINGS RD	56.3	10.1	1.07	0.1	Α
STATE HIGHWAY 335	55.3	27.8	1.08	0.8	Α
		•	•		
PM PEAK	Average Speed mph	Percent Followers %	Travel Time to Travel 1 mile, min	Followers Density Foll/min/In	Vehicle LOS
G WILSON LN	27.6	27.3	2.18	1	Α
JOHNS LN	56	2.1	1.07	0	А
MAPLE GROVE LN	55.8	3.3	1.08	0	Α
ROWLETTS CAVE SPRINGS RD	56.3	9.3	1.07	0.1	Α
	55.7	23.3	1.08	0.5	Α
STATE HIGHWAY 335	55.7	23.3		0.0	, ,

Table 5. 2034 Build Summary



#### ADDITIONAL STUDY ITEMS

#### **Turn Lane Analysis**

Kentucky Transportation Cabinet's "Warrant Calcs Interactive" spreadsheet was used to determine if turn lanes were warranted along State Highway 335 or G Wilson Lane where the study assumed traffic would be added in association with Project construction & operation. Based on the low volumes along the five roadways analyzed, new turn lanes in the vicinity were not warranted because minimum volume thresholds for the turn lanes were not met. Turn lane warrants for State Highway 335 and G Wilson Lane can be found in the Appendix of this report.

#### **Sight Distance Analysis**

Sight distance triangles were determined utilizing AASHTO's Geometric Design of Highways and Streets, 7th Edition. The amount of recommended sight distances for the roads with access to the Project are summarized in Table 6 below. Figure 6 in the Appendix of this report provides a plan view of the sight triangles. From this figure it is evident that all roadways provide adequate sight distance for passenger cars and trucks entering the roadways from the development. Although this access point did not provide the recommended sight distance for combination trucks it did provide more than adequate sight distance for passenger cars. A 60 mph design speed was assumed for State Highway 335; however, based on roadway geometry it is expected that cars approaching this access point from the east are likely travelling closer to 40 mph, which would provide adequate sight distance for trucks entering the roadway from the development.

REQUIRED SIGHT DISTANCE (FT)											
ROADWAY	RIGHT TURNING LEFT TURNING RIGHT TURNING LEFT TURN CAR SIGHT CAR SIGHT TRUCK SIGHT TRUCK SIG DISTANCE DISTANCE DISTANCE DISTANCE										
G Wilson	335	390	545	600							
Johns Ln*	575	665	930	1020							
Rowletts Cave*	575	665	930	1020							
HWY 335	575	665	930	1020							

<sup>\*</sup>These roads have no posted speed limit; therefore, since the speed limit is assumed to be 55 mph the design speed was assumed to be 60 mph, which is unlikely on these rural roads.

**Table 6. Sight Distance Requirements** 



#### CONCLUSIONS AND RECOMMENDATIONS

The site plan provides multiple access points to the Project during construction and even more access points during operations, which will split the generated traffic and avoid placing too much demand on any one point along the existing roadway network. By spreading the traffic across the network the capacity analysis determined, when comparing the No Build analysis to the Build analysis, the roadways in the study area will continue to operate at a LOS similar to existing conditions. The analysis determined that existing and proposed conditions operated with a LOS A for all roadways in the study area and the average speed for all roadways are near or above the speed limit for all roadways. This analysis was conservative in its approach by assuming that the construction traffic would all enter and exit during each peak hour. Through coordination with the local community the developer will ensure that construction schedule and major deliveries will not occur during peak hours.

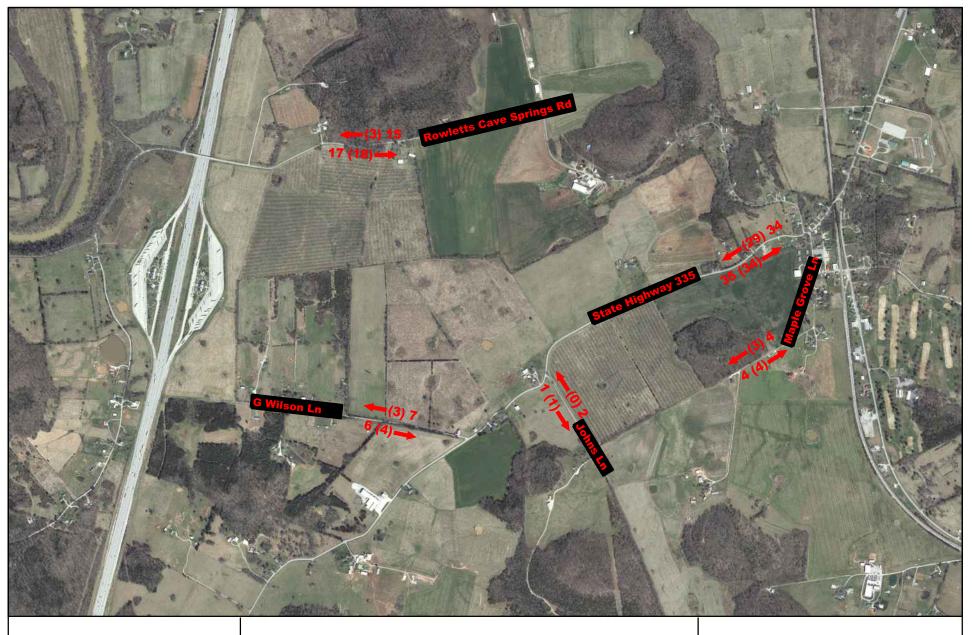
The turn lane analysis determined that no additional turn lanes are warranted for any roadways based on the traffic volumes on the road. The sight distance analysis determined that traffic entering the roadways from the development can do so safely. Some clearing along right of way may be required at these entrances to ensure proper sight distance is provided.

Based on the analyses performed, no changes to the roadway network are recommended within the study area in order for traffic conditions to operate within acceptable conditions.



## **APPENDIX**

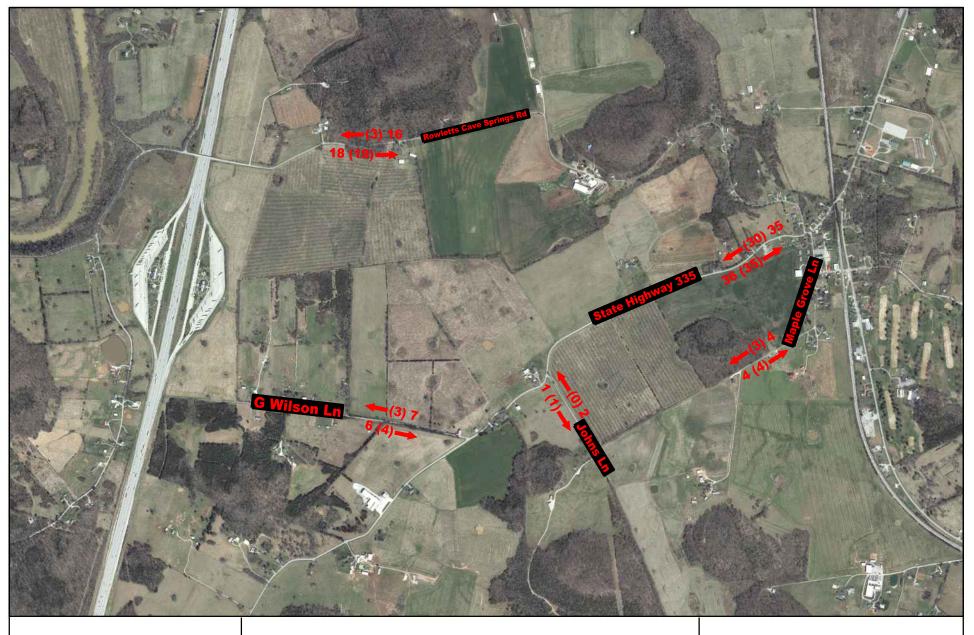




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PROPOSED THOROUGHBRED SOLAR HART COUNTY, KENTUCKY

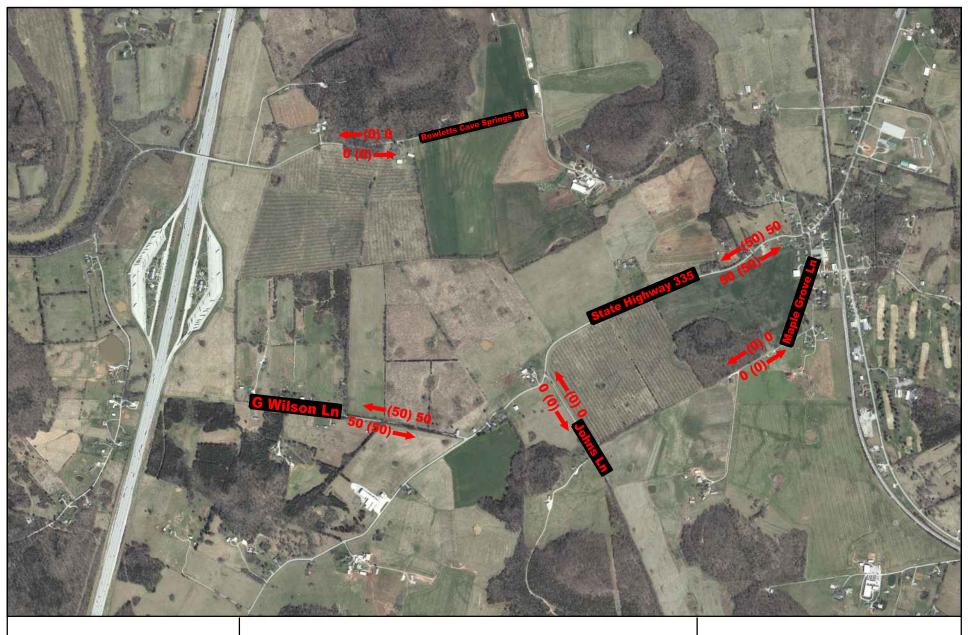
FIGURE 1 2022 EXISTING COUNTS (AM) PM



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PROPOSED THOROUGHBRED SOLAR HART COUNTY, KENTUCKY

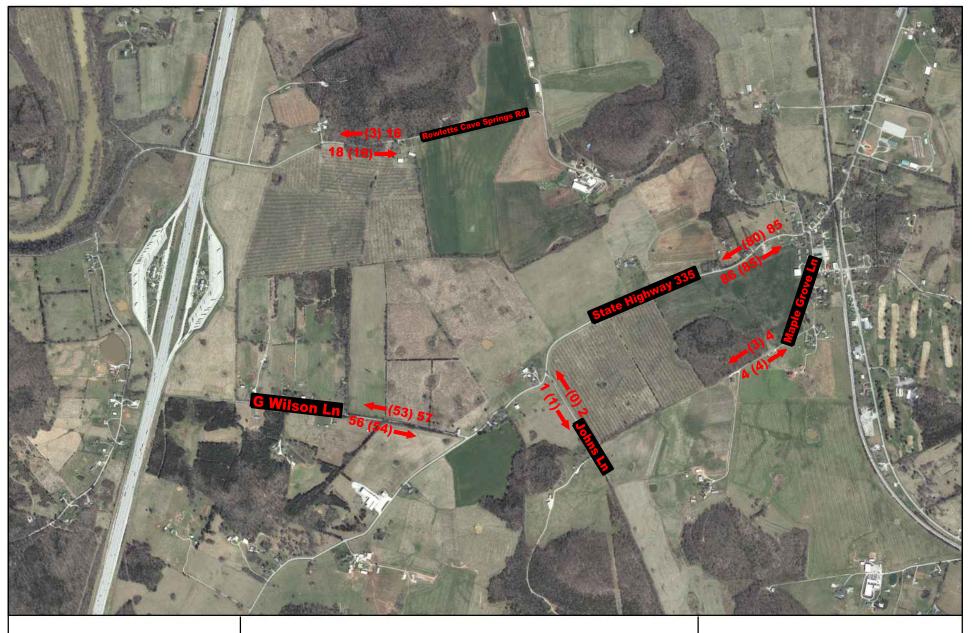
FIGURE 1A 2024 BACKGROUND TRAFFIC (AM) PM



 $\bigwedge^{\downarrow} \bigoplus$ 

PROPOSED THOROUGHBRED SOLAR HART COUNTY, KENTUCKY

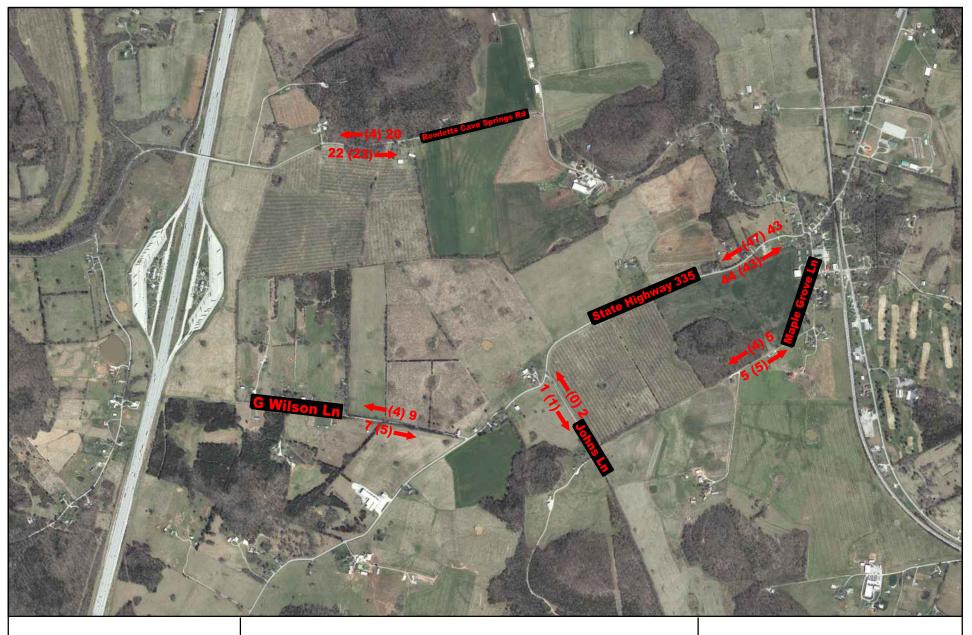
FIGURE 2 2024 TRIPS GENERATED (AM) PM



 $\bigwedge^{\downarrow} \bigvee_{\downarrow}$ 

PROPOSED THOROUGHBRED SOLAR HART COUNTY, KENTUCKY

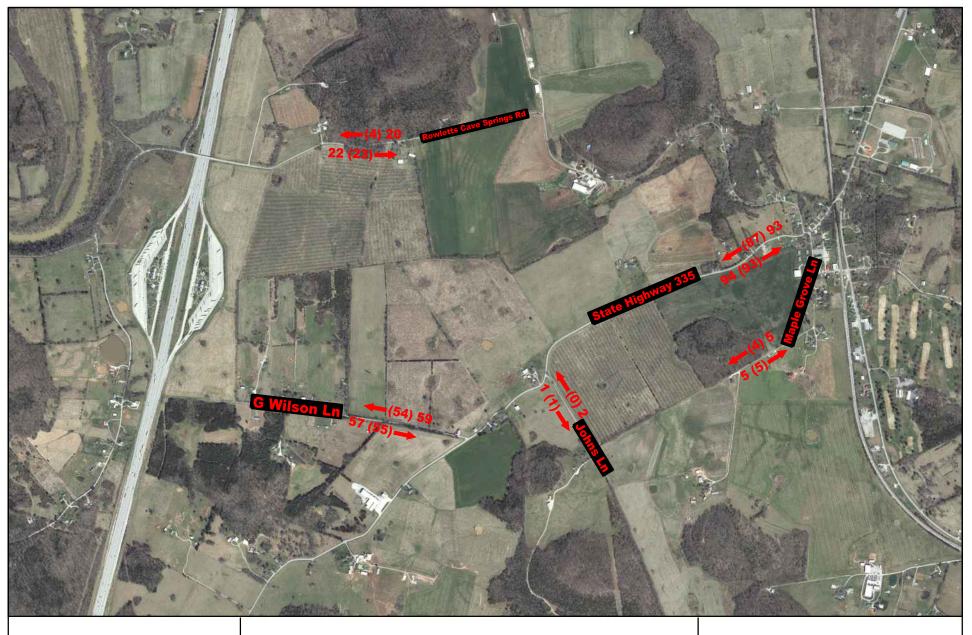
FIGURE 3 2024 BUILD VOLUMES (AM) PM



 $\bigwedge^{\diamondsuit}$ 

PROPOSED THOROUGHBRED SOLAR HART COUNTY, KENTUCKY

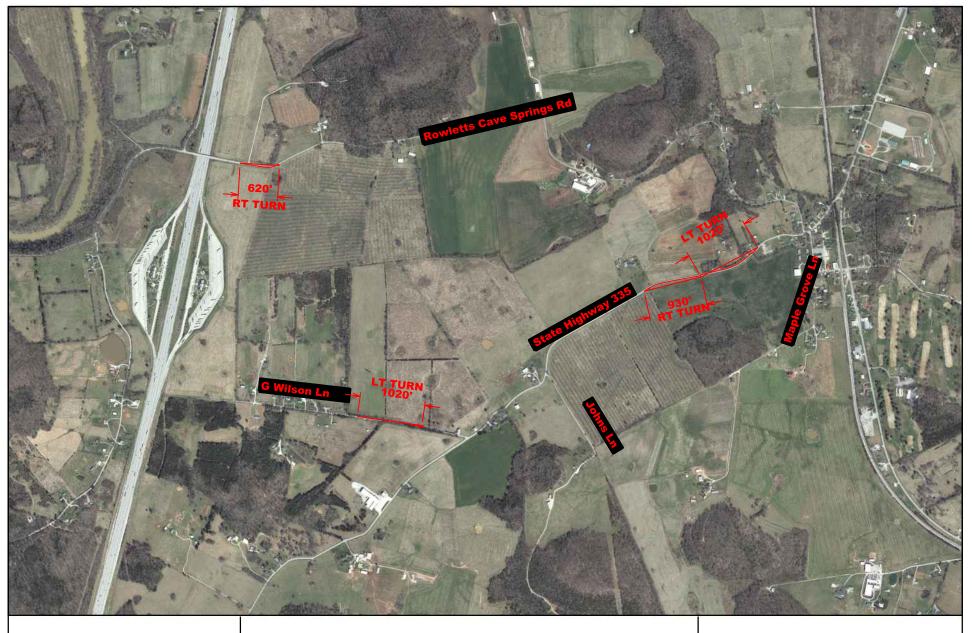
FIGURE 4 2034 NO BUILD VOLUMES (AM) PM



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PROPOSED THOROUGHBRED SOLAR HART COUNTY, KENTUCKY

FIGURE 5 2034 BUILD VOLUMES (AM) PM



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PROPOSED THOROUGHBRED SOLAR HART COUNTY, KENTUCKY

FIGURE 6 SIGHT TRIANGLES



File Name: G Wilson Start Date: 5/6/2022

End Date: 5/11/2022

#### Combined Lanes 5/6/2022 to 5/11/2022

#### Peak Analysis

Classes Excluded From Peaks: None

Date	AM Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor	Pm Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor
5/6/2022	No Volume			, 0141110	1 40001	5:32 PM	11	5:36 PM	5	0.55
5/7/2022	8:25 AM	4	9:06 AM	2	0.50	3:16 PM	11	3:51 PM	4	0.69
5/8/2022	10:57 AM	8	11:20 AM	3	0.67	3:15 PM	12	3:38 PM	4	0.75
5/9/2022	7:53 AM	10	7:53 AM	4	0.63	4:29 PM	16	4:54 PM	7	0.57
5/10/2022	7:49 AM	10	7:49 AM	4	0.63	4:39 PM	15	5:02 PM	7	0.54
5/11/2022	7:48 AM	13	7:48 AM	5	0.65	12:02 PM	5	12:08 PM	3	0.42

#### Classification Statistics

Ciussijicuii	m Similatics												
Unclassed	Motorcycle	esCars &	2 Axle Lo	ngBuses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl
		Trailers			Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi
6	6	281	64	12	33	0	0	3	0	0	0	0	0
1.5%	1.5%	69.4%	15.8%	3.0%	8.1%	0.0%	0.0%	0.7%	0.0%	0.0%	0.0%	0.0%	0.0%

#### Mean, Median, and Mode Averages

Mean: 31.8 Median (50th %): 32.4 Mode: 26.4

AA	۱D	T

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/6/2022	Westbound, None Specified (partial day adjusted)	40		1.00		1.00		40		1.00		40
5/6/2022	Eastbound, None Specified (partial day adjusted)	44		1.00		1.00		44		1.00		44
5/6/2022	Day Total	84						84				84
5/7/2022	Westbound, None Specified	28		1.00		1.00		28		1.00		28
5/7/2022	Eastbound, None Specified	31		1.00		1.00		31		1.00		31
5/7/2022	Day Total	59						59				59
5/8/2022	Westbound, None Specified	30		1.00		1.00		30		1.00		30
5/8/2022	Eastbound, None Specified	29		1.00		1.00		29		1.00		29
5/8/2022	Day Total	59						59				59
5/9/2022	Westbound, None Specified	53		1.00		1.00		53		1.00		53
5/9/2022	Eastbound, None Specified	51		1.00		1.00		51		1.00		51



File Name: G Wilson Start Date: 5/6/2022 End Date: 5/11/2022

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/9/2022	Day Total	104						104				104
5/10/2022	Westbound, None Specified	55		1.00		1.00		55		1.00		55
5/10/2022	Eastbound, None Specified	52		1.00		1.00		52		1.00		52
5/10/2022	Day Total	107						107				107
5/11/2022	Westbound, None Specified (partial day adjusted)	42		1.00		1.00		42		1.00		42
5/11/2022	Eastbound, None Specified (partial day adjusted)	48		1.00		1.00		48		1.00		48
5/11/2022	Day Total	90						90				90
Total		503						503				503
Average		83						83				83



Latitude: 37.232243 Longitude: -85.914994 File Name: G Wilson Date Printed: 5/18/2022 Start Date: 5/7/2022 End Date: 5/10/2022 GPS Accuracy: 0 ft Location Verified: No

12:00 AM 1:00 2:00 3:00 4:00 5:00 6:00	5/2/202 VB * * * * * * * * * * *	22 EB * * *	5/3/20: WB * *	22 EB * *	5/4/20 WB *	22 EB *	5/5/20 WB	22 EB	5/6/20 WB	22 EB	Weekday A WB	Average EB	5/7/202 WB	22 EB	5/8/202 WB	EB
Time W 12:00 AM 1:00 2:00 3:00 4:00 5:00 6:00							WB									
1:00 2:00 3:00 4:00 5:00 6:00	* * * * *	* * *	* * *	*	*	*							***			
1:00 2:00 3:00 4:00 5:00 6:00	* * * *	* * *	*	*	*			*	*	*	*	*	0	0	0	0
2:00 3:00 4:00 5:00 6:00	* * *	*	*			*	*	*	*	*	*	*	0	0	0	0
3:00 4:00 5:00 6:00	* *	*		*	*	*	*	*	*	*	*	*	0	0	0	0
4:00 5:00 6:00	*		*	*	*	*	*	*	*	*	*	*	1	1	0	0
6:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
6:00		*	*	*	*	*	*	*	*	*	*	*	0	1	0	1
	*	*	*	*	*	*	*	*	*	*	*	*	0	0	1	1
7:00	*	*	*	*	*	*	*	*	*	*	*	*	0	2	0	2
8:00	*	*	*	*	*	*	*	*	*	*	*	*	1	0	1	0
9:00	*	*	*	*	*	*	*	*	*	*	*	*	3	0	1	0
10:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
11:00	*	*	*	*	*	*	*	*	*	*	*	*	1	3	4	4
12:00 PM	*	*	*	*	*	*	*	*	*	*	*	*	2	1	2	1
1:00	*	*	*	*	*	*	*	*	*	*	*	*	1	2	3	0
2:00	*	*	*	*	*	*	*	*	*	*	*	*	1	4	0	2
3:00	*	*	*	*	*	*	*	*	*	*	*	*	5	2	3	5
4:00	*	*	*	*	*	*	*	*	*	*	*	*	3	4	3	3
5:00	*	*	*	*	*	*	*	*	*	*	*	*	1	1	3	4
6:00	*	*	*	*	*	*	*	*	*	*	*	*	1	3	2	1
7:00	*	*	*	*	*	*	*	*	*	*	*	*	3	2	2	0
8:00	*	*	*	*	*	*	*	*	*	*	*	*	4	1	4	2
9:00	*	*	*	*	*	*	*	*	*	*	*	*	0	2	1	3
10:00	*	*	*	*	*	*	*	*	*	*	*	*	1	2	0	0
11:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	28	31	30	29
Day	0		0		0		0		0		0		59		59	
AM Peak													9:00	11:00	11:00	11:00
Volume													3	3	4	4
PM Peak													3:00	2:00	8:00	3:00
Volume													5	4	4	5_



Latitude: 37.232243

ADT

ADT: 82

AADT: 82

File Name: G Wilson Date Printed: 5/18/2022 Start Date: 5/7/2022 End Date: 5/10/2022 GPS Accuracy: 0 ft

Longitude: -85.9					A.		ENG	INE	ERI	NG					Location V	erified: No
5/9/2022	5/9/202	22	5/10/202	22	5/11/20	22	5/12/20	)22	5/13/20	22	Weekday Av	/erage	5/14/20	)22	5/15/20	)22
Time	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB
12:00 AM	1	0	1	0	*	*	*	*	*	*	1	0	*	*	*	*
1:00	1	1	1	1	*	*	*	*	*	*	1	1	*	*	*	*
2:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
3:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
4:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
5:00	1	1	1	0	*	*	*	*	*	*	1	0	*	*	*	*
6:00	1	2	1	3	*	*	*	*	*	*	1	2	*	*	*	*
7:00	2	2	2	3	*	*	*	*	*	*	2	2	*	*	*	*
8:00	3	4	3	4	*	*	*	*	*	*	3	4	*	*	*	*
9:00	1	2	0	2	*	*	*	*	*	*	0	2	*	*	*	*
10:00	3	6	2	2	*	*	*	*	*	*	2	4	*	*	*	*
11:00	2	2	2	2	*	*	*	*	*	*	2	2	*	*	*	*
12:00 PM	4	3	2	2	*	*	*	*	*	*	3	2	*	*	*	*
1:00	0	1	1	4	*	*	*	*	*	*	0	2	*	*	*	*
2:00	6	2	7	6	*	*	*	*	*	*	6	4	*	*	*	*
3:00	5	4	3	1	*	*	*	*	*	*	4	2	*	*	*	*
4:00	6	4	2	5	*	*	*	*	*	*	4	4	*	*	*	*
5:00	7	5	10	2	*	*	*	*	*	*	8	4	*	*	*	*
6:00	5	7	3	3	*	*	*	*	*	*	4	5	*	*	*	*
7:00	2	1	3	7	*	*	*	*	*	*	2	4	*	*	*	*
8:00	3	1	5	0	*	*	*	*	*	*	4	0	*	*	*	*
9:00	0	1	5	3	*	*	*	*	*	*	2	2	*	*	*	*
10:00	0	1	1	2	*	*	*	*	*	*	0	2	*	*	*	*
11:00	0	1	0	0	*	*	*	*	*	*	0	0	*	*	*	*
Total	53	51	55	52	0	0	0	0	0	0	50	48	0	0	0	0
Day	104		107		0		0		0		98		0		0	
AM Peak	8:00	10:00	8:00	8:00							8:00	8:00				
Volume	3	6	3	4							3	4				
PM Peak	5:00	6:00	5:00	7:00							5:00	6:00				
Volume	7	7	10	7							8	5				
Comb Total	104		107		0		0		0		98		59		59	



File Name: Johns Lane Start Date: 5/6/2022 End Date: 5/11/2022

#### Combined Lanes 5/6/2022 to 5/11/2022

#### Peak Analysis

Classes Excluded From Peaks: None

Date	AM Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor	Pm Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor
5/6/2022	No Volume					No Volume				
5/7/2022	9:02 AM	2	9:33 AM	1	0.50	3:09 PM	6	3:53 PM	5	0.30
5/8/2022	9:29 AM	2	10:06 AM	1	0.50	4:21 PM	3	4:57 PM	2	0.38
5/9/2022	10:00 AM	2	10:00 AM	1	0.50	3:13 PM	3	3:13 PM	2	0.38
5/10/2022	No Volume					2:22 PM	4	2:49 PM	2	0.50
5/11/2022	No Volume					2:18 PM	2	2:53 PM	1	0.50

#### Classification Statistics

Ciassificano	m Diunsiics												
Unclassed	Motorcycle	esCars &	2 Axle Lor	gBuses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl
		Trailers			Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi
0	1	23	15	0	11	0	0	0	0	0	0	0	0
0.0%	2.0%	46.0%	30.0%	0.0%	22.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

#### Mean, Median, and Mode Averages

Mean: 26.5 Median (50th %): 27.1 Mode: 28.9

AA	DT
----	----

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT	
5/6/2022	Channel 1 (partial day adjusted)	3		1.00		1.00		3		1.00		3	
5/6/2022	Channel 2 (partial day adjusted)	3		1.00		1.00		3		1.00		3	
5/6/2022	Day Total	6						6				6	
5/7/2022	Channel 1	7		1.00		1.00		7		1.00		7	
5/7/2022	Channel 2	6		1.00		1.00		6		1.00		6	
5/7/2022	Day Total	13						13				13	
5/8/2022	Channel 1	5		1.00		1.00		5		1.00		5	
5/8/2022	Channel 2	4		1.00		1.00		4		1.00		4	
5/8/2022	Day Total	9						9				9	
5/9/2022	Channel 1	5		1.00		1.00		5		1.00		5	
5/9/2022	Channel 2	5		1.00		1.00		5		1.00		5	



File Name: Johns Lane Start Date: 5/6/2022 End Date: 5/11/2022

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/9/2022	Day Total	10						10				10
5/10/2022	Channel 1	6		1.00		1.00		6		1.00		6
5/10/2022	Channel 2	6		1.00		1.00		6		1.00		6
5/10/2022	Day Total	12						12				12
5/11/2022	Channel 1 (partial day adjusted)	3		1.00		1.00		3		1.00		3
5/11/2022	Channel 2 (partial day adjusted)	3		1.00		1.00		3		1.00		3
5/11/2022	Day Total	6						6				6
Total		56						56				56
Average		9						9				9



Latitude: 37.231859 Longitude: -85.906143 File Name: Johns Lane Date Printed: 5/18/2022 Start Date: 5/7/2022 End Date: 5/10/2022 GPS Accuracy: 0 ft Location Verified: No

Longitude65.8	100143						DOLLAR VIEW	I 18 Ex	12 12	CN AT					LUCATION VE	enneu. Nu
5/2/2022	5/2/20	)22	5/3/20	)22	5/4/20	22	5/5/20	22	5/6/20	22	Weekday /	Average	5/7/202	22	5/8/20	22
Time	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB
12:00 AM	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
1:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
2:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
3:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
4:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
5:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
6:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
7:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
8:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
9:00	*	*	*	*	*	*	*	*	*	*	*	*	1	0	0	0
10:00	*	*	*	*	*	*	*	*	*	*	*	*	0	1	1	1
11:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	1	0
12:00 PM	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
1:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
2:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
3:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	1
4:00	*	*	*	*	*	*	*	*	*	*	*	*	4	2	0	0
5:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	2	1
6:00	*	*	*	*	*	*	*	*	*	*	*	*	1	1	1	0
7:00	*	*	*	*	*	*	*	*	*	*	*	*	0	1	0	1
8:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
9:00	*	*	*	*	*	*	*	*	*	*	*	*	0	1	0	0
10:00	*	*	*	*	*	*	*	*	*	*	*	*	1	0	0	0
11:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	7	6	5	4
Day	0		0		0		0		0		0		13		9	
AM Peak													9:00	10:00	10:00	10:00
Volume													1	1	1	1
PM Peak													4:00	4:00	5:00	3:00
Volume													4	2	2	11_



File Name: Johns Lane Date Printed: 5/18/2022 Start Date: 5/7/2022 End Date: 5/10/2022 GPS Accuracy: 0 ft Location Verified: No

Latitude: 37.231859 Longitude: -85.906143

12:00 AM	* * * * * * * * * * * * * * * * * * *
12:00 AM	* * * * * * * * * * * * * * * * * * *
1:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	* * * * * * * * * * * * * * * * * * * *
2:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	* * * * * * * * * * * * *
3:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	* * * * * * * * * * * * *
4:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	* * * * * * * * * * * * *
5:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	* * * * * * * * * * *
6:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	*     *     *     *     *     *     *     *     *
7:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	* * * * * * * *
8:00 0 0 0 0 1	* * * * * *
9:00 0 0 0 0 0 0 * * * * * * * * * * * *	* * * *
10:00 0 2 0 0 * * * * * * * * 0 1 * * * * * 11:00 1 0 0 0 * * * * * * * * * * * * * * *	* * *
11:00	* *
12:00 PM	*
1:00	*
	*
2:00 0 0 1 1 1 * * * * * * * 0 0 * * * * *	*
3:00 1 1 2 1 * * * * * * 2 1 * * * *	*
4:00 2 0 0 0 * * * * * * * 1 0 * * * * *	*
5:00 0 1 0 1 * * * * * * 0 1 * * * * *	*
6:00 0 0 0 0 * * * * * * * * 0 0 * * * *	*
7:00 0 0 2 1 * * * * * * 1 0 * * * * *	*
8:00 0 0 0 1 * * * * * * * 0 0 * * * * * *	*
9:00 0 0 0 0 * * * * * * * * 0 0 * * * *	*
10:00 0 0 0 0 * * * * * * * * 0 0 * * * *	*
11:00	*
Total 5 5 6 6 0 0 0 0 0 0 5 3 0 0 0	0
Day 10 12 0 0 0 8 0 0	
AM Peak 11:00 10:00 8:00 10:00	
Volume 1 2 1 1 1 1	
PM Peak 4:00 1:00 3:00 2:00 3:00 3:00 3:00	
Volume 2 1 2 1 2 1 2 1	
Comb Total 10 12 0 0 0 8 13 9	
ADT ADT: 11 AADT: 11	



File Name: KY 335 Start Date: 5/11/2022

End Date: 5/16/2022

#### Combined Lanes 5/11/2022 to 5/16/2022

**Peak Analysis**Classes Excluded From Peaks: None

Date	AM Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor	Pm Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor
5/11/2022	No Volume					3:59 PM	66	3:59 PM	23	0.72
5/12/2022	8:18 AM	64	8:38 AM	28	0.57	3:46 PM	69	4:30 PM	21	0.82
5/13/2022	7:57 AM	67	8:41 AM	28	0.60	3:31 PM	83	3:31 PM	26	0.80
5/14/2022	10:45 AM	54	10:47 AM	20	0.68	3:19 PM	64	4:03 PM	22	0.73
5/15/2022	10:38 AM	38	10:41 AM	14	0.68	3:02 PM	56	3:16 PM	17	0.82
5/16/2022	8:33 AM	71	8:34 AM	28	0.63	12:00 PM	50	12:08 PM	18	0.69

#### Classification Statistics

,	Ciussijicuiio	m Diunsiics												
	Unclassed	Motorcycle	esCars &	2 Axle Lon	gBuses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl
			<b>Trailers</b>			Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi
	7	63	2210	894	39	186	10	0	64	17	0	0	0	0
	0.2%	1.8%	63.3%	25.6%	1.1%	5.3%	0.3%	0.0%	1.8%	0.5%	0.0%	0.0%	0.0%	0.0%

#### Mean, Median, and Mode Averages

Mean: 41.0 Median (50th %): 41.0 Mode: 40.6

#### **AADT**

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/11/2022	Channel 1 (partial day adjusted)	312		1.00		1.00		312		1.00		312
5/11/2022	Channel 2 (partial day adjusted)	335		1.00		1.00		335		1.00		335
5/11/2022	Day Total	647						647				647
5/12/2022	Channel 1	370		1.00		1.00		370		1.00		370
5/12/2022	Channel 2	397		1.00		1.00		397		1.00		397
5/12/2022	Day Total	767						767				767
5/13/2022	Channel 1	446		1.00		1.00		446		1.00		446
5/13/2022	Channel 2	432		1.00		1.00		432		1.00		432
5/13/2022	Day Total	878						878				878
5/14/2022	Channel 1	325		1.00		1.00		325		1.00		325
5/14/2022	Channel 2	324		1.00		1.00		324		1.00		324



File Name: KY 335 Start Date: 5/11/2022 End Date: 5/16/2022

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/14/2022	Day Total	649				•		649				649
5/15/2022	Channel 1	258		1.00		1.00		258		1.00		258
5/15/2022	Channel 2	265		1.00		1.00		265		1.00		265
5/15/2022	Day Total	523						523				523
5/16/2022	Channel 1 (partial day adjusted)	327		1.00		1.00		327		1.00		327
5/16/2022	Channel 2 (partial day adjusted)	343		1.00		1.00		343		1.00		343
5/16/2022	Day Total	670						670				670
Total		4134						4134				4134
Average		689						689				689



File Name: KY 335
Date Printed: 5/18/2022
Start Date: 5/12/2022
End Date: 5/15/2022
GPS Accuracy: 0 ft

Latitude: 0.000000 GPS Accuracy: 0 ft Longitude: 0.000000 Location Verified: No 5/9/2022 5/9/2022 5/10/2022 5/11/2022 5/12/2022 5/13/2022 Weekday Average 5/14/2022 5/15/2022

5/9/2022	2 5/9/2022		5/10/2022 5/11/2022		5/12/202		5/13/202	22	Weekday Average		5/14/2022		5/15/202			
Time	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB
12:00 AM	*	*	*	*	*	*	1	3	2	4	2	4	4	6	2	5
1:00	*	*	*	*	*	*	4	2	1	0	2	1	1	4	4	4
2:00	*	*	*	*	*	*	0	0	0	1	0	0	1	0	3	2
3:00	*	*	*	*	*	*	0	3	4	1	2	2	1	2	2	4
4:00	*	*	*	*	*	*	4	1	4	0	4	0	2	0	3	2
5:00	*	*	*	*	*	*	11	2	8	6	10	4	2	2	2	3
6:00	*	*	*	*	*	*	14	7	10	6	12	6	0	1	1	0
7:00	*	*	*	*	*	*	20	8	25	5	22	6	6	7	7	5
8:00	*	*	*	*	*	*	25	34	29	34	27	34	16	10	8	8
9:00	*	*	*	*	*	*	27	20	19	21	23	20	25	15	8	5
10:00	*	*	*	*	*	*	18	19	22	20	20	20	24	20	19	13
11:00	*	*	*	*	*	*	15	16	41	19	28	18	23	20	13	17
12:00 PM	*	*	*	*	*	*	24	21	32	24	28	22	26	16	12	21
1:00	*	*	*	*	*	*	17	25	22	32	20	28	20	24	29	21
2:00	*	*	*	*	*	*	25	31	25	43	25	37	21	23	23	16
3:00	*	*	*	*	*	*	26	25	36	31	31	28	34	20	31	24
4:00	*	*	*	*	*	*	24	36	34	35	29	36	15	29	16	14
5:00	*	*	*	*	*	*	32	23	23	34	28	28	18	35	9	15
6:00	*	*	*	*	*	*	21	35	26	43	24	39	20	26	13	23
7:00	*	*	*	*	*	*	17	24	27	17	22	20	15	16	14	15
8:00	*	*	*	*	*	*	22	23	15	20	18	22	19	16	14	18
9:00	*	*	*	*	*	*	9	23	16	15	12	19	19	9	9	17
10:00	*	*	*	*	*	*	11	8	16	10	14	9	6	9	13	9
11:00	*	*	*	*	*	*	3	8	9	11	6	10	7	14	3	4
Total	0	0	0	0	0	0	370	397	446	432	409	413	325	324	258	265
Day	0		0		0		767		878		822		649		523	
AM Peak							9:00	8:00	11:00	8:00	11:00	8:00	9:00	10:00	10:00	11:00
Volume							27	34	41	34	28	34	25	20	19	17
PM Peak							5:00	4:00	3:00	2:00	3:00	6:00	3:00	5:00	3:00	3:00
Volume							32	36	36	43	31	39	34	35	31	24
Comb Total	0		0		0		767		878		822		649		523	

ADT

ADT: 704

AADT: 704



File Name: Maple Grove Start Date: 5/6/2022

End Date: 5/11/2022

#### Combined Lanes 5/6/2022 to 5/11/2022

**Peak Analysis**Classes Excluded From Peaks: None

Date	AM Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor	Pm Peak	Hour Volume	Highest Interval Time	Highest Interval Volume	Peak Hour Factor
5/6/2022	No Volume					4:28 PM	4	5:06 PM	3	0.33
5/7/2022	10:59 AM	3	11:24 AM	2	0.38	3:39 PM	14	3:54 PM	5	0.70
5/8/2022	9:02 AM	5	9:41 AM	2	0.63	1:58 PM	11	2:08 PM	6	0.46
5/9/2022	7:42 AM	6	7:56 AM	3	0.50	4:40 PM	8	5:10 PM	5	0.40
5/10/2022	9:16 AM	9	9:39 AM	5	0.45	4:41 PM	8	5:25 PM	3	0.67
5/11/2022	8:08 AM	7	8:32 AM	4	0.44	12:00 PM	4	12:01 PM	4	0.25

Classification Statistics

Ciussijicuii	m Diansiics												
Unclassed	nclassed MotorcyclesCars &		yclesCars & 2 Axle LongBuses		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl
		Trailers			Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi
1	6	85	70	22	42	0	0	0	0	0	0	0	0
0.4%	2.7%	37.6%	31.0%	9.7%	18.6%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

#### Mean, Median, and Mode Averages

29.7 Mean: Median (50th %): 30.7 Mode: 33.1

AA	DT
----	----

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT	
5/6/2022	Eastbound, None Specified (partial day adjusted)	22		1.00		1.00		22		1.00		22	
5/6/2022	Westbound, None Specified (partial day adjusted)	23		1.00		1.00		23		1.00		23	
5/6/2022	Day Total	45						45				45	
5/7/2022	Eastbound, None Specified	19		1.00		1.00		19		1.00		19	
5/7/2022	Westbound, None Specified	24		1.00		1.00		24		1.00		24	
5/7/2022	Day Total	43						43				43	
5/8/2022	Eastbound, None Specified	24		1.00		1.00		24		1.00		24	
5/8/2022	Westbound, None Specified	23		1.00		1.00		23		1.00		23	
5/8/2022	Day Total	47						47				47	
5/9/2022	Eastbound, None Specified	21		1.00		1.00		21		1.00		21	
5/9/2022	Westbound, None Specified	23		1.00		1.00		23		1.00		23	



File Name: Maple Grove Start Date: 5/6/2022 End Date: 5/11/2022

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/9/2022	Day Total	44				-		44				44
5/10/2022	Eastbound, None Specified	28		1.00		1.00		28		1.00		28
5/10/2022	Westbound, None Specified	28		1.00		1.00		28		1.00		28
5/10/2022	Day Total	56						56				56
5/11/2022	Eastbound, None Specified (partial day adjusted)	22		1.00		1.00		22		1.00		22
5/11/2022	Westbound, None Specified (partial day adjusted)	23		1.00		1.00		23		1.00		23
5/11/2022	Day Total	45						45				45
Total		280						280				280
Average		47						47				47



Latitude: 37.235317 Longitude: -85.896803 File Name: Maple Grove Date Printed: 5/18/2022 Start Date: 5/7/2022 End Date: 5/11/2022 GPS Accuracy: 0 ft Location Verified: No

Longitude65.6								1 IN IN		IN AT					Location ve	
5/2/2022	5/2/20	022	5/3/20	)22	5/4/20	22	5/5/20	)22	5/6/20	)22	Weekday	Average	5/7/202	22	5/8/202	2
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
12:00 AM	*	*	*	*	*	*	*	*	*	*	*	*	0	1	0	0
1:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
2:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
3:00	*	*	*	*	*	*	*	*	*	*	*	*	0	1	0	0
4:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
5:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
6:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
7:00	*	*	*	*	*	*	*	*	*	*	*	*	1	0	0	0
8:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	0	0
9:00	*	*	*	*	*	*	*	*	*	*	*	*	1	0	3	1
10:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	1	0
11:00	*	*	*	*	*	*	*	*	*	*	*	*	1	2	0	0
12:00 PM	*	*	*	*	*	*	*	*	*	*	*	*	2	2	0	0
1:00	*	*	*	*	*	*	*	*	*	*	*	*	0	0	1	3
2:00	*	*	*	*	*	*	*	*	*	*	*	*	3	4	5	6
3:00	*	*	*	*	*	*	*	*	*	*	*	*	1	3	0	2
4:00	*	*	*	*	*	*	*	*	*	*	*	*	5	5	1	1
5:00	*	*	*	*	*	*	*	*	*	*	*	*	1	1	2	5
6:00	*	*	*	*	*	*	*	*	*	*	*	*	1	2	1	2
7:00	*	*	*	*	*	*	*	*	*	*	*	*	1	0	1	2
8:00	*	*	*	*	*	*	*	*	*	*	*	*	0	1	1	1
9:00	*	*	*	*	*	*	*	*	*	*	*	*	1	1	1	0
10:00	*	*	*	*	*	*	*	*	*	*	*	*	0	1	7	0
11:00	*	*	*	*	*	*	*	*	*	*	*	*	1	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	19	24	24	23
Day	0		0		0		0		0		0		43		47	
AM Peak													7:00	11:00	9:00	9:00
Volume													11	2	3	1
PM Peak													4:00	4:00	10:00	2:00
Volume													5	5	7	6



Latitude: 37.235317

File Name: Maple Grove Date Printed: 5/18/2022 Start Date: 5/7/2022 End Date: 5/11/2022 GPS Accuracy: 0 ft Location Verified: No

Longitude: -85.89					A .	Vi I	E. IN Cr.			N (1					Location Ve	
5/9/2022	5/9/202	22	5/10/20	22	5/11/20	)22	5/12/2	022	5/13/2	022	Weekday A	verage	5/14/20	22	5/15/20	22
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
12:00 AM	0	0	0	0	0	0	*	*	*	*	0	0	*	*	*	*
1:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
2:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
3:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
4:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
5:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
6:00	0	0	1	0	*	*	*	*	*	*	0	0	*	*	*	*
7:00	3	1	1	1	*	*	*	*	*	*	2	1	*	*	*	*
8:00	2	2	4	3	*	*	*	*	*	*	3	2	*	*	*	*
9:00	0	0	3	4	*	*	*	*	*	*	2	2	*	*	*	*
10:00	0	0	2	1	*	*	*	*	*	*	1	0	*	*	*	*
11:00	3	3	3	4	*	*	*	*	*	*	3	4	*	*	*	*
12:00 PM	2	1	2	0	*	*	*	*	*	*	2	0	*	*	*	*
1:00	0	1	0	0	*	*	*	*	*	*	0	0	*	*	*	*
2:00	3	3	1	2	*	*	*	*	*	*	2	2	*	*	*	*
3:00	2	2	2	0	*	*	*	*	*	*	2	1	*	*	*	*
4:00	2	4	2	4	*	*	*	*	*	*	2	4	*	*	*	*
5:00	3	3	4	4	*	*	*	*	*	*	4	4	*	*	*	*
6:00	1	1	0	1	*	*	*	*	*	*	0	1	*	*	*	*
7:00	0	0	1	1	*	*	*	*	*	*	0	0	*	*	*	*
8:00	0	1	1	3	*	*	*	*	*	*	0	2	*	*	*	*
9:00	0	0	1	0	*	*	*	*	*	*	0	0	*	*	*	*
10:00	0	0	0	0	*	*	*	*	*	*	0	0	*	*	*	*
11:00	0	1	0	0	*	*	*	*	*	*	0	0	*	*	*	*
Total	21	23	28	28	0	0	0	0	0	0	23	23	0	0	0	0
Day	44		56		0		0		0		46		0		0	
AM Peak	7:00	11:00	8:00	9:00							8:00	11:00				
Volume	3	3	4	4							3	4				
PM Peak	2:00	4:00	5:00	4:00							5:00	4:00				
Volume	3	4	4	4							4	4				
Comb Total	44		56		0		0		0		46		43		47	
ADT		ADT: 48	Α	ADT: 48												



File Name: Rowletts Cave Springs Rd Date Printed: 5/18/2022

Start Date: 5/12/2022 End Date: 5/15/2022 GPS Accuracy: 0 ft Location Verified: No

Latitude: 37.243667 Longitude: -85.922928

Longitude65.8								I IN Ex		2N 1.1					Location ve	
5/9/2022	5/9/20	)22	5/10/2	2022	5/11/2	2022	5/12/20		5/13/20	22	Weekday A	verage	5/14/20	)22	5/15/202	
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
12:00 AM	*	*	*	*	*	*	1	1	2	2	2	2	1	1	1	3
1:00	*	*	*	*	*	*	0	0	0	0	0	0	1	0	2	0
2:00	*	*	*	*	*	*	0	1	1	0	0	0	0	0	0	1
3:00	*	*	*	*	*	*	1	0	0	0	0	0	0	0	2	2
4:00	*	*	*	*	*	*	0	0	0	0	0	0	0	0	0	0
5:00	*	*	*	*	*	*	0	2	0	1	0	2	1	0	0	0
6:00	*	*	*	*	*	*	5	4	5	6	5	5	1	3	0	0
7:00	*	*	*	*	*	*	5	4	4	2	4	3	6	0	0	0
8:00	*	*	*	*	*	*	19	5	18	3	18	4	6	4	4	2
9:00	*	*	*	*	*	*	9	7	6	5	8	6	9	4	5	2
10:00	*	*	*	*	*	*	6	4	5	6	6	5	14	8	4	7
11:00	*	*	*	*	*	*	7	6	9	6	8	6	10	8	6	5
12:00 PM	*	*	*	*	*	*	6	7	6	6	6	6	12	10	6	4
1:00	*	*	*	*	*	*	6	4	5	7	6	6	12	19	4	12
2:00	*	*	*	*	*	*	8	5	6	5	7	5	8	9	10	6
3:00	*	*	*	*	*	*	11	11	10	8	10	10	10	10	11	4
4:00	*	*	*	*	*	*	8	14	17	15	12	14	10	6	6	8
5:00	*	*	*	*	*	*	7	18	13	13	10	16	7	10	5	7
6:00	*	*	*	*	*	*	10	8	11	6	10	7	10	7	7	8
7:00	*	*	*	*	*	*	6	5	3	7	4	6	8	7	5	10
8:00	*	*	*	*	*	*	13	8	4	3	8	6	3	6	11	7
9:00	*	*	*	*	*	*	2	7	3	7	2	7	5	6	8	4
10:00	*	*	*	*	*	*	0	11	3	4	2	8	0	1	5	8
11:00	*	*	*	*	*	*	2	6	5	5	4	6	2	2	1	3
Total	0	0	0	0	0	0	132	138	136	117	132	130	136	121	103	103
Day	0		Ċ	)	0		270		253		262		257	·	206	
AM Peak							8:00	9:00	8:00	6:00	8:00	9:00	10:00	10:00	11:00	10:00
Volume							19	7	18	6	18	6	14	8	6	7
PM Peak							8:00	5:00	4:00	4:00	4:00	5:00	12:00 PM	1:00	3:00	1:00
Volume							13	18	17	15	12	16	12	19	11	12
Comb Total	0		C		0	•	270	•	253		262		257	•	206	
ADT		ADT: 246	A	AADT: 246												



File Name: Rowletts Cave Springs Rd Start Date: 5/11/2022

End Date: 5/16/2022

## Combined Lanes 5/11/2022 to 5/16/2022

**Peak Analysis**Classes Excluded From Peaks: None

Date	AM Peak	Hour Volume	Highest Interval	Highest Interval	Peak Hour	Pm Peak	Hour Volume	Highest Interval	Highest Interval	Peak Hour
			Time	Volume	Factor			Time	Volume	Factor
5/11/2022	No Volume					4:47 PM	40	4:55 PM	13	0.77
5/12/2022	8:18 AM	27	8:31 AM	12	0.56	5:13 PM	30	5:13 PM	12	0.63
5/13/2022	8:00 AM	21	8:36 AM	8	0.66	4:25 PM	34	4:39 PM	12	0.71
5/14/2022	10:21 AM	27	10:22 AM	11	0.61	12:46 PM	34	1:24 PM	13	0.65
5/15/2022	10:16 AM	13	10:38 AM	5	0.65	1:43 PM	24	1:44 PM	10	0.60
5/16/2022	8:09 AM	27	8:40 AM	10	0.68	12:00 PM	15	12:02 PM	7	0.54

#### Classification Statistics

Ciussijicuii	on Similarics												
Unclassed	Motorcycle	esCars &	2 Axle Lor	1gBuses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl
		Trailers			Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi
7	23	642	463	9	113	2	0	14	1	8	0	0	0
0.5%	1.8%	50.1%	36.1%	0.7%	8.8%	0.2%	0.0%	1.1%	0.1%	0.6%	0.0%	0.0%	0.0%

### Mean, Median, and Mode Averages

32.8 Mean: Median (50th %): 33.2 Mode: 32.3

#### **AADT**

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/11/2022	Channel 1 (partial day adjusted)	138		1.00		1.00		138		1.00		138
5/11/2022	Channel 2 (partial day adjusted)	127		1.00		1.00		127		1.00		127
5/11/2022	Day Total	265						265				265
5/12/2022	Channel 1	132		1.00		1.00		132		1.00		132
5/12/2022	Channel 2	138		1.00		1.00		138		1.00		138
5/12/2022	Day Total	270						270				270
5/13/2022	Channel 1	136		1.00		1.00		136		1.00		136
5/13/2022	Channel 2	117		1.00		1.00		117		1.00		117
5/13/2022	Day Total	253						253				253
5/14/2022	Channel 1	136		1.00		1.00		136		1.00		136
5/14/2022	Channel 2	121		1.00		1.00		121		1.00		121



File Name: Rowletts Cave Springs Rd Start Date: 5/11/2022 End Date: 5/16/2022

Date	Lane	Volume	X	User	X	Daily	=	ADT	X	Season	=	AADT
5/14/2022	Day Total	257						257				257
5/15/2022	Channel 1	103		1.00		1.00		103		1.00		103
5/15/2022	Channel 2	103		1.00		1.00		103		1.00		103
5/15/2022	Day Total	206						206				206
5/16/2022	Channel 1 (partial day adjusted)	136		1.00		1.00		136		1.00		136
5/16/2022	Channel 2 (partial day adjusted)	132		1.00		1.00		132		1.00		132
5/16/2022	Day Total	268						268				268
Total		1519						1519				1519
Average		253						253				253

## Historical Traffic Volume Summary

## Station Details:

Sta ID:	050256
Sta Type:	Full Coverage
Мар:	<u>Maplt</u>
District:	4
County:	Hart
Route:	050-KY-0335 -000
Route Desc:	L AND N TURNPIKE

Begin MP:	2.5450
Begin Desc:	KY 218 DEPARTURE
End Mp:	6.6270
End Desc:	US 31W
Impact Year:	
Year Added:	

## Newest Count:

AADT:	650
Year:	2020
% Single:	
% Combo:	
K Factor:	10.80
D Factor:	56

#### Definitions:

Sta. ID - Three digit county number + station number

MP - milepoint

Impact Year – year of significant change to traffic pattern within station segment

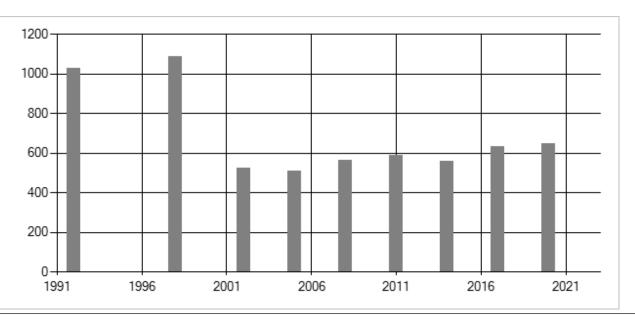
AADT – Annual Average Daily Traffic – the annualized average 24-hour volume of vehicles on a segment of roadway

- % Single single unit truck volume as a percentage of the AADT
- % Combo combination truck volume as a percentage of the AADT

K Factor – peak hour volume as a percentage of the AADT

D Factor – percentage of peak hour volume flowing in the peak direction

Year	AADT	Year	AADT	Year	AADT
2022		2012		2002	525
2021		2011	588	2001	
2020	650	2010		2000	
2019		2009		1999	
2018		2008	566	1998	1090
2017	632	2007		1997	
2016		2006		1996	
2015		2005	511	1995	
2014	561	2004		1994	
2013		2003		1993	



## 2034 AM Build - Left Turn Lane

<u>Input Fields</u>			
Left Turn Volume (vph)	50	Speed Limit (mph)	55
Advancing Volume (vph)	87	No. of through lanes	1
Opposing Volume (vph)	93	Percent Heavy Vehicles (decimal percent)	0.09



2034 AM Build - Right Turn Lane

<u>Input Fields</u>			
Right Turn Volume (vph)	50	Speed Limit (mph)	55
Advancing Volume (vph)	93		



## 2034 PM Build - Left Turn Lane

<u>Input Fields</u>			
Left Turn Volume (vph)	50	Speed Limit (mph)	55
Advancing Volume (vph)	93	No. of through lanes	1
Opposing Volume (vph)	94	Percent Heavy Vehicles (decimal percent)	0.09



## 2034 PM Build - Right Turn Lane

Input Fields			
Right Turn Volume (vph)	50	Speed Limit (mph)	55
Advancing Volume (vph)	94		



## Proposed Thoroughbred Solar

## **G Wilson Ln Access**

## 2034 AM Build - Right Turn Lane

55

## **Input Fields**

Right Turn Volume (vph) 50 Speed Limit (mph) 4



## Proposed Thoroughbred Solar G Wilson Ln Access

## 2034 PM Build - Right Turn Lane

<u>Input Fields</u>			
Right Turn Volume (vph)	50	Speed Limit (mph)	55
Advancing Volume (vph)	59		



		HCS Two-La	ne H	lighway Re	port	
Projec	ct Information					
Analyst		gb	Г	Date		6/3/2022
Agency		pec	1	Analysis Year		2022
Jurisdict	ion		1	Time Analyzed		2024 existing am
Project [	Description	prop solar	ι	Units		U.S. Customary
		Se	egme	ent 1		
Vehicl	e Inputs					
Segmen	t Туре	Passing Constrained	I	Length, ft		5280
Lane Wi	dth, ft	9	9	Shoulder Width, f	t	0
Speed Li	imit, mi/h	55	A	Access Point Dens	sity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	58	(	Opposing Deman	d Flow Rate, veh/h	-
Peak Ho	ur Factor	0.60	1	Total Trucks, %		9.00
Segmen	t Capacity, veh/h	1700	[	Demand/Capacity	(D/C)	0.03
Intern	nediate Results					
Segmen	t Vertical Class	1	F	Free-Flow Speed,	mi/h	56.4
Speed S	lope Coefficient (m)	3.61707	9	Speed Power Coe	fficient (p)	0.41674
PF Slope	e Coefficient (m)	-1.32528	F	PF Power Coefficie	ent (p)	0.75218
In Passin	ng Lane Effective Length?	No	7	Total Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	ç	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	56.4
Vehicl	e Results					
Average	Speed, mi/h	56.4	F	Percent Followers,	, %	14.5
Segmen	t Travel Time, minutes	1.06	F	Follower Density (FD), followers/mi/ln		0.1
Vehicle L	LOS	А				
Facilit	y Results		·			
T	VMT veh-mi/p	VHD veh-h/p			LOS	
1	9	0.00			0.1	А
		December 11CCEN 1		- \/: 2022		Cara areta di 00/03/2022 12/00/

		HCS Two-La	ne F	lighway Re	port		
Proje	ct Information						
Analyst		gb	Ti	Date		6/3/2022	
Agency		pec	,	Analysis Year		2022	
Jurisdict	tion		-	Time Analyzed		2024 existing am	
Project	Description	prop solar	1	Units		U.S. Customary	
		Se	egm	ent 1			
Vehic	le Inputs						
Segmen	nt Type	Passing Constrained	Ti	Length, ft		5280	
Lane Wi	idth, ft	9	:	Shoulder Width, ft	t	0	
Speed L	imit, mi/h	55	,	Access Point Dens	ity, pts/mi	0.0	
Dema	and and Capacity						
Directio	nal Demand Flow Rate, veh/h	29	- (	Opposing Deman	d Flow Rate, veh/h	-	
Peak Ho	our Factor	0.66	-	Total Trucks, %		11.50	
Segmen	nt Capacity, veh/h	1700	1	Demand/Capacity	(D/C)	0.02	
Interr	nediate Results						
Segmen	nt Vertical Class	1	T	Free-Flow Speed,	mi/h	56.3	
Speed S	Slope Coefficient (m)	3.61256	:	Speed Power Coef	fficient (p)	0.41674	
PF Slope	e Coefficient (m)	-1.32560	1	PF Power Coefficie	ent (p)	0.75231	
In Passii	ng Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.0	
%lmpro	vement to Percent Followers	0.0	(	%Improvement to	Speed	0.0	
Subse	egment Data						
# Se	egment Type	Length, ft	Radiu	us, ft	Superelevation, %	Average Speed, mi/h	
1 Ta	angent	5280	1-		-	56.3	
Vehic	le Results					·	
Average	e Speed, mi/h	56.3		Percent Followers,	%	8.8	
Segmen	nt Travel Time, minutes	1.07 Follower Density (FD), followers/mi/ln		0.0			
Vehicle	LOS	А					
Facilit	ty Results						
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS	
1	5	0.00			0.0	A	
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		HCS Two-La	ne H	lighway Re	port	
Projec	ct Information					
Analyst		gb	Г	Date		6/3/2022
Agency		pec	A	Analysis Year		2022
Jurisdict	ion		7	Time Analyzed		2024 existing am
Project [	Description	prop solar	ι	Units		U.S. Customary
		Se	egme	ent 1		
Vehicl	le Inputs					
Segmen	t Туре	Passing Constrained	I	Length, ft		5280
Lane Wi	dth, ft	9	9	Shoulder Width, ft	t	0
Speed Li	imit, mi/h	30	A	Access Point Dens	ity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	6	(	Opposing Demand	d Flow Rate, veh/h	-
Peak Ho	ur Factor	0.63	7	Total Trucks, %		11.80
Segmen	t Capacity, veh/h	1700	[	Demand/Capacity	(D/C)	0.00
Intern	nediate Results					
Segmen	t Vertical Class	1	F	Free-Flow Speed,	mi/h	27.8
Speed S	lope Coefficient (m)	2.06731	9	Speed Power Coef	fficient (p)	0.41674
PF Slope	e Coefficient (m)	-1.34714	F	PF Power Coefficie	ent (p)	0.64082
In Passin	ng Lane Effective Length?	No	7	Total Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	ç	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	27.8
Vehicl	e Results					
Average	Speed, mi/h	27.8	F	Percent Followers,	. %	5.1
Segmen	t Travel Time, minutes	2.16	F	Follower Density (	FD), followers/mi/ln	0.0
Vehicle L	LOS	А				
Facilit	y Results		·			
Т	VMT veh-mi/p	VHD veh-h/p			LOS	
1	1	0.00			0.0	А
		Decembed LICCEN I		. Varaina 2022		C = = = = = = = = = = = = = = = = = = =

		HCS Two-Lai	ne Hi	ighway Re	port					
Project Inf	ormation									
Analyst		gb	D	ate		6/3/2022				
Agency		рес	А	nalysis Year		2022				
Jurisdiction			Ti	ime Analyzed		2024 existing am				
Project Descrip	otion	prop solar	U	Inits		U.S. Customary				
	Segment 1									
Vehicle Inp	outs									
Segment Type		Passing Constrained	Le	ength, ft		5280				
Lane Width, ft		9	SI	houlder Width, ft	t	0				
Speed Limit, m	i/h	55	A	ccess Point Dens	ity, pts/mi	0.0				
Demand a	nd Capacity									
Directional Der	mand Flow Rate, veh/h	0	0	pposing Demand	d Flow Rate, veh/h	-				
Peak Hour Fact	tor	0.50	To	otal Trucks, %		22.00				
Segment Capa	city, veh/h	1700	D	emand/Capacity	(D/C)	0.00				
Intermedia	ate Results									
Segment Vertic	cal Class	1	Fr	ree-Flow Speed,	mi/h	56.0				
Speed Slope C	oefficient (m)	3.59360	Sį	peed Power Coef	fficient (p)	0.41674				
PF Slope Coeff	icient (m)	-1.32691	PI	F Power Coefficie	ent (p)	0.75288				
In Passing Lane	e Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0				
%Improvemen	t to Percent Followers	0.0	%	Improvement to	Speed	0.0				
Subsegme	nt Data									
# Segment	t Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h				
1 Tangent		5280	-		-	56.0				
Vehicle Re	sults	-			-					
Average Speed	l, mi/h	56.0	Pe	ercent Followers,	%	0.0				
Segment Trave	l Time, minutes	1.07	Fo	Follower Density (FD), followers/mi/ln		0.0				
Vehicle LOS		А								
Facility Res	sults									
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS				
1	0	0.00			0.0	A				

		HCS Two-La	ne H	lighway Re	port	
Proje	ct Information					
Analyst		gb	I	Date		6/3/2022
Agency		pec	,	Analysis Year		2022
Jurisdict	tion		-	Time Analyzed		2024 existing am
Project	Description	prop solar	ı	Units		U.S. Customary
		Se	egm	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Constrained	Ī	Length, ft		5280
Lane Wi	idth, ft	9	9	Shoulder Width, ft	t	0
Speed L	imit, mi/h	55	,	Access Point Dens	ity, pts/mi	0.0
Dema	and Capacity		·			
Directio	nal Demand Flow Rate, veh/h	9	7	Opposing Demand	d Flow Rate, veh/h	-
Peak Ho	our Factor	0.45	-	Total Trucks, %		28.30
Segmen	nt Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.01
Interr	nediate Results					
Segmen	nt Vertical Class	1	I	Free-Flow Speed,	mi/h	55.8
Speed S	Slope Coefficient (m)	3.58223	9	Speed Power Coef	fficient (p)	0.41674
PF Slope	e Coefficient (m)	-1.32769	1	PF Power Coefficie	ent (p)	0.75322
In Passir	ng Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	(	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	1-		-	55.8
Vehic	le Results					<u> </u>
Average	e Speed, mi/h	55.8	ı	Percent Followers,	%	3.7
Segmen	nt Travel Time, minutes	1.08	Follower Density (FD), followers/mi/ln		0.0	
Vehicle	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	1	0.00			0.0	A
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		HCS Two-La	ne H	ighway Re	port	
Projec	t Information					
Analyst		gb	Г	Date		6/3/2022
Agency		pec	A	Analysis Year		2022
Jurisdict	ion		Т	Time Analyzed		2024 existing pm
Project [	Description	prop solar	ι	Jnits		U.S. Customary
		Se	egme	ent 1		
Vehicl	e Inputs					
Segmen	t Type	Passing Constrained	L	_ength, ft		5280
Lane Wi	dth, ft	9	5	Shoulder Width, ft	t	0
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	45		Opposing Demand	d Flow Rate, veh/h	-
Peak Ho	ur Factor	0.80	Т	Total Trucks, %		9.00
Segmen	t Capacity, veh/h	1700	[	Demand/Capacity	(D/C)	0.03
Intern	nediate Results					
Segmen	t Vertical Class	1	F	ree-Flow Speed,	mi/h	56.4
Speed S	lope Coefficient (m)	3.61707	5	Speed Power Coef	fficient (p)	0.41674
PF Slope	Coefficient (m)	-1.32528	F	PF Power Coefficie	ent (p)	0.75218
In Passin	ng Lane Effective Length?	No	Т	Total Segment Dei	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	56.4
Vehicl	e Results					
Average	Speed, mi/h	56.4	F	Percent Followers,	%	12.1
Segmen	t Travel Time, minutes	1.06	F	Follower Density (	FD), followers/mi/ln	0.1
Vehicle L	LOS	А				
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			LOS	
1	9	0.00			0.1	А
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		HCS Two-La	ne F	lighway Re	port		
Projec	ct Information						
Analyst		gb		Date		6/3/2022	
Agency		pec		Analysis Year		2022	
Jurisdict	ion			Time Analyzed		2024 existing pm	
Project [	Description	prop solar		Units		U.S. Customary	
		Se	egm	ent 1			
Vehicl	le Inputs						
Segmen	t Type	Passing Constrained		Length, ft		5280	
Lane Wi	dth, ft	9	:	Shoulder Width, ft	t	0	
Speed L	imit, mi/h	55		Access Point Dens	ity, pts/mi	0.0	
Dema	nd and Capacity						
Direction	nal Demand Flow Rate, veh/h	25	- 1	Opposing Demand	d Flow Rate, veh/h	-	
Peak Ho	our Factor	0.71	1	Total Trucks, %		11.50	
Segmen	t Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.01	
Intern	nediate Results						
Segmen	t Vertical Class	1		Free-Flow Speed,	mi/h	56.3	
Speed S	lope Coefficient (m)	3.61256	:	Speed Power Coef	fficient (p)	0.41674	
PF Slope	e Coefficient (m)	-1.32560		PF Power Coefficie	ent (p)	0.75231	
In Passir	ng Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.0	
%lmpro	vement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Subse	egment Data						
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h	
1 Ta	ngent	5280	-		-	56.3	
Vehicl	le Results	•					
Average	Speed, mi/h	56.3		Percent Followers,	. %	8.0	
Segmen	t Travel Time, minutes	1.07 Follower Density (FD), followers/mi/ln		0.0			
Vehicle I	LOS	А					
Facilit	y Results	·					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS	
1	5	0.00			0.0	A	
	2022 Hairragita of Florido All Bioleto	December 1100 EM 1		- \/: 2022		Caracata di 00 (02 (2022 12:11:	

		HCS Two-La	ne H	lighway Re	port	
Proje	ct Information					
Analyst		gb	I	Date		6/3/2022
Agency		pec	,	Analysis Year		2022
Jurisdict	tion		-	Time Analyzed		2024 existing pm
Project	Description	prop solar	ı	Units		U.S. Customary
		Se	egm	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Constrained	Ī	Length, ft		5280
Lane W	idth, ft	9	9	Shoulder Width, ft	t	0
Speed L	imit, mi/h	30	,	Access Point Dens	ity, pts/mi	0.0
Dema	and and Capacity		·			
Directio	nal Demand Flow Rate, veh/h	11	7	Opposing Demand Flow Rate, veh/h		-
Peak Ho	our Factor	0.54	-	Total Trucks, %		11.80
Segmen	nt Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.01
Interr	nediate Results					
Segmen	nt Vertical Class	1	Ī	Free-Flow Speed,	mi/h	27.8
Speed S	Slope Coefficient (m)	2.06731	9	Speed Power Coef	fficient (p)	0.41674
PF Slop	e Coefficient (m)	-1.34714	-1.34714		ent (p)	0.64082
In Passii	ng Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.0
%lmpro	vement to Percent Followers	0.0	(	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	1-		-	27.8
Vehic	le Results					
Average	e Speed, mi/h	27.8	ı	Percent Followers,	%	7.3
Segmer	nt Travel Time, minutes	2.16	ı	Follower Density (	FD), followers/mi/ln	0.0
Vehicle	LOS	A				
Facilit	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	2	0.00			0.0	A
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		HCS Two-La	ne H	lighway Re	port				
Projec	t Information								
Analyst		gb	Г	Date		6/3/2022			
Agency		pec	A	Analysis Year		2022			
Jurisdict	ion		1	Time Analyzed		2024 existing pm			
Project [	Description	prop solar	ι	Jnits		U.S. Customary			
	Segment 1								
Vehicl	e Inputs								
Segmen	t Type	Passing Constrained	L	Length, ft		5280			
Lane Wi	dth, ft	9	5	Shoulder Width, ft	t	0			
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0			
Dema	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	4	(	Opposing Demand	d Flow Rate, veh/h	-			
Peak Ho	ur Factor	0.50	7	Total Trucks, %		22.00			
Segmen	t Capacity, veh/h	1700	Г	Demand/Capacity (D/C)		0.00			
Intern	nediate Results								
Segmen	t Vertical Class	1	F	Free-Flow Speed,	mi/h	56.0			
Speed S	lope Coefficient (m)	3.59360	9	Speed Power Coefficient (p)		0.41674			
PF Slope	Coefficient (m)	-1.32691	F	PF Power Coefficie	ent (p)	0.75288			
In Passin	ng Lane Effective Length?	No	1	Total Segment Dei	nsity, veh/mi/ln	0.0			
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0			
Subse	gment Data								
# Se	gment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	ngent	5280	-		-	56.0			
Vehicl	e Results								
Average	Speed, mi/h	56.0	F	Percent Followers,	%	2.1			
Segmen	t Travel Time, minutes	1.07	F	Follower Density (	FD), followers/mi/ln	0.0			
Vehicle L	LOS	А							
Facilit	y Results	-							
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	1	0.00			0.0	А			
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		HCS Two-La	ne H	lighway Re	port	
Projec	ct Information					
Analyst		gb	Г	Date		6/3/2022
Agency		pec	A	Analysis Year		2022
Jurisdict	ion		1	Time Analyzed		2024 existing pm
Project [	Description	prop solar	ι	Jnits		U.S. Customary
		Se	egme	ent 1		
Vehicl	e Inputs					
Segmen	t Type	Passing Constrained	L	Length, ft		5280
Lane Wi	dth, ft	9	9	Shoulder Width, ft	t	0
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	6	(	Opposing Demand	d Flow Rate, veh/h	-
Peak Ho	ur Factor	0.67	1	Total Trucks, %		28.30
Segmen	t Capacity, veh/h	1700	Г	Demand/Capacity (D/C)		0.00
Intern	nediate Results					
Segmen	t Vertical Class	1	F	Free-Flow Speed,	mi/h	55.8
Speed S	lope Coefficient (m)	3.58223	9	Speed Power Coefficient (p)		0.41674
PF Slope	e Coefficient (m)	-1.32769	F	PF Power Coefficie	ent (p)	0.75322
In Passin	ng Lane Effective Length?	No	1	Total Segment Dei	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	55.8
Vehicl	e Results					
Average	Speed, mi/h	55.8	F	Percent Followers,	%	2.8
Segmen	t Travel Time, minutes	1.08	F	Follower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	1	0.00			0.0	А
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		HCS Two-La	ne Hi	ighway Re	port			
Projec	ct Information							
Analyst		gb	D	ate		6/3/2022		
Agency		pec	А	nalysis Year		2022		
Jurisdict	ion		Ti	ime Analyzed		2024 build am		
Project [	Description	prop solar	U	Inits		U.S. Customary		
Segment 1								
Vehicl	le Inputs							
Segmen	t Type	Passing Constrained	Le	ength, ft		5280		
Lane Wi	dth, ft	9	SI	houlder Width, ft	t	0		
Speed L	imit, mi/h	55	A	ccess Point Dens	ity, pts/mi	0.0		
Dema	nd and Capacity							
Direction	nal Demand Flow Rate, veh/h	142	0	pposing Demand	d Flow Rate, veh/h	-		
Peak Ho	our Factor	0.60		Total Trucks, %		9.00		
Segmen	t Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.08		
Intern	nediate Results							
Segmen	t Vertical Class	1	Fi	ree-Flow Speed,	mi/h	56.4		
Speed S	lope Coefficient (m)	3.61707	Sı	peed Power Coef	fficient (p)	0.41674		
PF Slope	e Coefficient (m)	-1.32528	P	F Power Coefficie	ent (p)	0.75218		
In Passir	ng Lane Effective Length?	No	To	otal Segment Dei	nsity, veh/mi/ln	0.7		
%Impro	vement to Percent Followers	0.0	%	almprovement to	Speed	0.0		
Subse	gment Data							
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	ngent	5280	-		-	55.4		
Vehicl	le Results							
Average	Speed, mi/h	55.4	Pe	ercent Followers,	%	26.3		
Segmen	t Travel Time, minutes	1.08	Fo	ollower Density (	FD), followers/mi/ln	0.7		
Vehicle I	Vehicle LOS A							
Facilit	y Results							
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS		
1	21	0.01			0.7	A		

		HCS Two-La	ne H	lighway Re	port			
Projec	t Information							
Analyst		gb	Г	Date		6/3/2022		
Agency		pec	A	Analysis Year		2022		
Jurisdicti	ion		1	Time Analyzed		2024 build am		
Project D	Description	prop solar	ι	Jnits		U.S. Customary		
Segment 1								
Vehicl	e Inputs							
Segment	t Type	Passing Constrained	L	Length, ft		5280		
Lane Wid	dth, ft	9	5	Shoulder Width, ft	t	0		
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0		
Dema	nd and Capacity							
Direction	nal Demand Flow Rate, veh/h	29	(	Opposing Demand	d Flow Rate, veh/h	-		
Peak Ho	ur Factor	0.66	1	Total Trucks, %		11.50		
Segment	t Capacity, veh/h	1700	[	Demand/Capacity	(D/C)	0.02		
Intern	nediate Results							
Segment	t Vertical Class	1	F	Free-Flow Speed,	mi/h	56.3		
Speed SI	lope Coefficient (m)	3.61256	9	Speed Power Coefficient (p)		0.41674		
PF Slope	Coefficient (m)	-1.32560	F	PF Power Coefficient (p)		0.75231		
In Passin	ng Lane Effective Length?	No	1	Total Segment Dei	nsity, veh/mi/ln	0.0		
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0		
Subse	gment Data							
# Se	gment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h		
1 Tai	ngent	5280	-		-	56.3		
Vehicl	e Results							
Average	Speed, mi/h	56.3	F	Percent Followers,	%	8.8		
Segment	t Travel Time, minutes	1.07	F	Follower Density (	FD), followers/mi/ln	0.0		
Vehicle L	_OS	А						
Facilit	y Results							
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS		
1	5	0.00			0.0	А		
		Decembed LICCEN I		. Varaina 2022		Cara are to de 00 (02 (2022 12:12:0		

		HCS Two-Lai	ne Hi	ighway Re	port			
Project	Information							
Analyst		gb	D	ate		6/3/2022		
Agency		рес	A	nalysis Year		2022		
Jurisdiction	1		Ti	ime Analyzed		2024 build am		
Project Des	scription	prop solar	U	nits		U.S. Customary		
	Segment 1							
Vehicle	Inputs							
Segment T	ype	Passing Constrained	Le	ength, ft		5280		
Lane Width	ı, ft	9	Sł	houlder Width, ft	t	0		
Speed Limi	it, mi/h	30	A	ccess Point Dens	ity, pts/mi	0.0		
Demand	d and Capacity							
Directional	Demand Flow Rate, veh/h	86	0	pposing Demand	d Flow Rate, veh/h	-		
Peak Hour	Factor	0.63		Total Trucks, %		11.80		
Segment C	apacity, veh/h	1700	D	emand/Capacity	(D/C)	0.05		
Interme	ediate Results							
Segment V	ertical Class	1	Fr	ree-Flow Speed,	mi/h	27.8		
Speed Slop	pe Coefficient (m)	2.06731	Sį	Speed Power Coefficient (p)		0.41674		
PF Slope Co	oefficient (m)	-1.34714	PI	F Power Coefficie	ent (p)	0.64082		
In Passing	Lane Effective Length?	No	To	otal Segment Dei	nsity, veh/mi/ln	0.8		
%Improver	ment to Percent Followers	0.0	%	Improvement to	Speed	0.0		
Subsegi	ment Data							
# Segn	nent Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h		
1 Tang	ent	5280	-		-	27.8		
Vehicle	Results				-			
Average Sp	peed, mi/h	27.8	Pe	ercent Followers,	%	24.4		
Segment Ti	ravel Time, minutes	2.16	Fo	ollower Density (	FD), followers/mi/ln	0.8		
Vehicle LOS	ehicle LOS A							
Facility	Results							
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS		
1	14	0.00			0.8	A		

		HCS Two-Lar	ne H	ighway R	eport				
Project Information									
Analyst		gb	С	Date		6/3/2022			
Agency	Ì	pec	Д	Analysis Year		2022			
Jurisdiction			Т	Time Analyzed		2024 build am			
Project Description		prop solar	L	Jnits		U.S. Customary			
	Segment 1								
Vehicle Inputs									
Segment Type		Passing Constrained	L	_ength, ft		5280			
Lane Width, ft		9	S	Shoulder Width,	ft	0			
Speed Limit, mi/h		55		Access Point De	nsity, pts/mi	0.0			
Demand and Capacity						-			
Directional Demand Flow Rate,	veh/h	0	C	Opposing Dema	ind Flow Rate, veh/h	-			
Peak Hour Factor		0.50		Total Trucks, %		22.00			
Segment Capacity, veh/h		1700	С	Demand/Capaci	ty (D/C)	0.00			
Intermediate Results									
Segment Vertical Class		1	F	ree-Flow Speed	d, mi/h	56.0			
Speed Slope Coefficient (m)		3.59360	S	Speed Power Coefficient (p)		0.41674			
PF Slope Coefficient (m)		-1.32691	Р	PF Power Coeffic	cient (p)	0.75288			
In Passing Lane Effective Length	1?	No	Т	Total Segment D	ensity, veh/mi/ln	0.0			
%Improvement to Percent Follo	wers	0.0	9	%Improvement	to Speed	0.0			
Subsegment Data									
# Segment Type		Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tangent		5280	-		-	56.0			
Vehicle Results									
Average Speed, mi/h		56.0	P	Percent Followe	rs, %	0.0			
Segment Travel Time, minutes		1.07	F	ollower Density	/ (FD), followers/mi/ln	0.0			
Vehicle LOS	cle LOS A								
Facility Results									
T VMT veh-mi/p		VHD veh-h/p		Follower Density, followers/		LOS			
1 0		0.00			0.0	A			

		HCS Two-La	ne H	lighway Re	port	
Proje	ct Information					
Analyst		gb	I	Date		6/3/2022
Agency		pec	,	Analysis Year		2022
Jurisdict	tion		-	Time Analyzed		2024 build am
Project	Description	prop solar	ı	Units		U.S. Customary
		Se	egm	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Constrained	- I	Length, ft		5280
Lane Wi	idth, ft	9		Shoulder Width, ft	t	0
Speed L	imit, mi/h	55	,	Access Point Dens	ity, pts/mi	0.0
Dema	and and Capacity		·			
Directio	onal Demand Flow Rate, veh/h	9		Opposing Demand Flow Rate, veh/h		-
Peak Ho	our Factor	0.45	-	Total Trucks, %		28.30
Segmen	nt Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.01
Interr	mediate Results					
Segmer	nt Vertical Class	1	T	Free-Flow Speed,	mi/h	55.8
Speed S	Slope Coefficient (m)	3.58223	9	Speed Power Coef	fficient (p)	0.41674
PF Slope	e Coefficient (m)	-1.32769	PF Power Coefficient (p)			0.75322
In Passii	ng Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.0
%lmpro	ovement to Percent Followers	0.0	(	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.8
Vehic	le Results	•				
Average	e Speed, mi/h	55.8	ı	Percent Followers,	%	3.7
Segmen	nt Travel Time, minutes	1.08	1	Follower Density (	FD), followers/mi/ln	0.0
Vehicle	/ehicle LOS A					
Facilit	ty Results	·				
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	1	0.00			0.0	A
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		HCS Two-La	ne H	lighway Re	port				
Projec	ct Information								
Analyst		gb	Г	Date		6/3/2022			
Agency		pec	A	Analysis Year		2022			
Jurisdict	ion		1	Time Analyzed		2024 build pm			
Project [	Description	prop solar	ι	Units		U.S. Customary			
	Segment 1								
Vehicl	e Inputs								
Segmen	t Type	Passing Constrained	L	Length, ft		5280			
Lane Wi	dth, ft	9	9	Shoulder Width, ft	t	0			
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0			
Dema	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	108	(	Opposing Demand	d Flow Rate, veh/h	-			
Peak Ho	ur Factor	0.80	1	Total Trucks, %		9.00			
Segmen	t Capacity, veh/h	1700	Г	Demand/Capacity (D/C)		0.06			
Intern	nediate Results								
Segmen	t Vertical Class	1	F	Free-Flow Speed,	mi/h	56.4			
Speed S	lope Coefficient (m)	3.61707	9	Speed Power Coef	fficient (p)	0.41674			
PF Slope	e Coefficient (m)	-1.32528	F	PF Power Coefficie	ent (p)	0.75218			
In Passin	ng Lane Effective Length?	No	1	Total Segment De	nsity, veh/mi/ln	0.4			
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0			
Subse	gment Data								
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	ngent	5280	-		-	55.9			
Vehicl	e Results								
Average	Speed, mi/h	55.9	F	Percent Followers,	. %	21.9			
Segmen	t Travel Time, minutes	1.07	F	Follower Density (	FD), followers/mi/ln	0.4			
Vehicle L	LOS	А							
Facilit	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	22	0.00			0.4	А			
		Decembed LICCEN I		. Varai a a 2022		Caracata d. 00 (02 (2022 12.F4.2			

HCS Two-Lane Highway Report								
Project Information								
Analyst	gb	Da	ate		6/3/2022			
Agency	pec	Ar	nalysis Year		2022			
Jurisdiction		Tir	me Analyzed		2024 build pm			
Project Description	prop solar	Ur	nits		U.S. Customary			
Segment 1								
Vehicle Inputs								
Segment Type	Passing Constrained	Le	ength, ft		5280			
Lane Width, ft	9	Sh	noulder Width, ft	t	0			
Speed Limit, mi/h	55	Ac	ccess Point Dens	ity, pts/mi	0.0			
Demand and Capacity								
Directional Demand Flow Rate, veh/h	25	Op	pposing Demand	d Flow Rate, veh/h	-			
Peak Hour Factor	0.71		Total Trucks, %		11.50			
Segment Capacity, veh/h	1700	De	emand/Capacity	(D/C)	0.01			
Intermediate Results								
Segment Vertical Class	1	Fre	ee-Flow Speed,	mi/h	56.3			
Speed Slope Coefficient (m)	3.61256	Sp	Speed Power Coefficient (p)		0.41674			
PF Slope Coefficient (m)	-1.32560	PF	PF Power Coefficient (p)		0.75231			
In Passing Lane Effective Length?	No	То	otal Segment Dei	nsity, veh/mi/ln	0.0			
%Improvement to Percent Followers	0.0	%I	Improvement to	Speed	0.0			
Subsegment Data								
# Segment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h			
1 Tangent	5280	-		-	56.3			
Vehicle Results								
Average Speed, mi/h	56.3	Pe	ercent Followers,	%	8.0			
Segment Travel Time, minutes	1.07	Fo	ollower Density (	FD), followers/mi/ln	0.0			
Vehicle LOS	LOS A							
Facility Results								
T VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1 5	0.00			0.0	Α			

		HCS Two-La	ne Hi	ighway Re	port			
Projec	ct Information							
Analyst		gb	D	ate		6/3/2022		
Agency		pec	А	nalysis Year		2022		
Jurisdict	ion		Ti	ime Analyzed		2024 build pm		
Project [	Description	prop solar	U	Inits		U.S. Customary		
Segment 1								
Vehicl	e Inputs							
Segmen	t Type	Passing Constrained	Le	ength, ft		5280		
Lane Wi	dth, ft	9	SI	houlder Width, ft	t	0		
Speed Li	imit, mi/h	30	А	ccess Point Dens	ity, pts/mi	0.0		
Dema	nd and Capacity					·		
Direction	nal Demand Flow Rate, veh/h	104	0	pposing Deman	d Flow Rate, veh/h	-		
Peak Ho	ur Factor	0.54		Total Trucks, %		11.80		
Segmen	t Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.06		
Intern	nediate Results							
Segmen	t Vertical Class	1	Fr	ree-Flow Speed,	mi/h	27.8		
Speed Sl	lope Coefficient (m)	2.06731	Sį	peed Power Coef	fficient (p)	0.41674		
PF Slope	e Coefficient (m)	-1.34714	PI	F Power Coefficie	ent (p)	0.64082		
In Passin	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	1.0		
%Improv	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0		
Subse	gment Data	-						
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	ngent	5280	-		-	27.6		
Vehicl	le Results	•				'		
Average	Speed, mi/h	27.6	Pe	ercent Followers,	%	27.0		
Segmen	t Travel Time, minutes	2.17	Fo	ollower Density (	FD), followers/mi/ln	1.0		
Vehicle L	LOS	А						
Facilit	y Results	<u> </u>						
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS		
1	14	0.00			1.0	A		

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		HCS Two-La	ne H	lighway Re	port			
Projec	t Information							
Analyst		gb		Date		6/3/2022		
Agency		pec	A	Analysis Year		2022		
Jurisdicti	on		Т	Гime Analyzed		2024 build pm		
Project D	Description (	prop solar	ι	Jnits		U.S. Customary		
Segment 1								
Vehicle	e Inputs							
Segment	: Туре	Passing Constrained	L	_ength, ft		5280		
Lane Wic	dth, ft	9	S	Shoulder Width, ft	t	0		
Speed Li	mit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0		
Demai	nd and Capacity							
Direction	nal Demand Flow Rate, veh/h	4		Opposing Demand	d Flow Rate, veh/h	-		
Peak Hou	ur Factor	0.50	Т	Total Trucks, %		22.00		
Segment	: Capacity, veh/h	1700	С	Demand/Capacity	(D/C)	0.00		
Interm	nediate Results							
Segment	: Vertical Class	1	F	ree-Flow Speed,	mi/h	56.0		
Speed Slo	ope Coefficient (m)	3.59360	S	Speed Power Coefficient (p)		0.41674		
PF Slope	Coefficient (m)	-1.32691	F	PF Power Coefficie	ent (p)	0.75288		
In Passin	g Lane Effective Length?	No	Т	Total Segment Dei	nsity, veh/mi/ln	0.0		
%Improv	rement to Percent Followers	0.0	9	%Improvement to	Speed	0.0		
Subse	gment Data							
# Seg	gment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h		
1 Tar	ngent	5280	-		-	56.0		
Vehicle	e Results							
Average	Speed, mi/h	56.0	F	Percent Followers,	%	2.1		
Segment	Travel Time, minutes	1.07	F	ollower Density (	FD), followers/mi/ln	0.0		
Vehicle L	OS	А						
Facility	y Results							
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS		
1	1	0.00			0.0	А		
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		HCS Two-La	ne Hi	ighway Re	port	
Proje	ct Information					
Analyst		gb	D	Date		6/3/2022
Agency	,	pec	А	nalysis Year		2022
Jurisdic	tion		Ti	ime Analyzed		2024 build pm
Project	Description	prop solar	U	Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segment Type		Passing Constrained		Length, ft		5280
Lane W	idth, ft	9	SI	houlder Width, ft		0
Speed L	Limit, mi/h	55	А	ccess Point Dens	ity, pts/mi	0.0
Dema	and and Capacity					
Directio	onal Demand Flow Rate, veh/h	6		Opposing Demand Flow Rate, veh/h		-
Peak Ho	our Factor	0.67		Total Trucks, %		28.30
Segmer	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.00
Interr	mediate Results					
Segmer	nt Vertical Class	1		Free-Flow Speed, mi/h		55.8
Speed Slope Coefficient (m)		3.58223		Speed Power Coefficient (p)		0.41674
PF Slope Coefficient (m)		-1.32769		PF Power Coefficient (p)		0.75322
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0 %		%Improvement to Speed		0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.8
Vehic	le Results					<u> </u>
Average Speed, mi/h		55.8		Percent Followers, %		2.8
Segment Travel Time, minutes		1.08		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		А				
Facili	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	1	0.00			0.0	A

		HCS Two-La	ne Hi	ighway Re	port			
Project In	formation							
Analyst		gb	D	Date		6/3/2022		
Agency		рес	А	nalysis Year		2022		
Jurisdiction			Ti	ime Analyzed		2034 no build am		
Project Descri	ption	prop solar	U	Inits		U.S. Customary		
	Segment 1							
Vehicle In	puts							
Segment Type	2	Passing Constrained		Length, ft		5280		
Lane Width, ft		9	SI	houlder Width, ft	i e	0		
Speed Limit, n	ni/h	55	А	ccess Point Dens	ity, pts/mi	0.0		
Demand a	and Capacity							
Directional De	emand Flow Rate, veh/h	72		Opposing Demand Flow Rate, veh/h		-		
Peak Hour Fac	ctor	0.60		Total Trucks, %		9.00		
Segment Capa	acity, veh/h	1700		Demand/Capacity (D/C)		0.04		
Intermedi	ate Results							
Segment Vertical Class		1		Free-Flow Speed, mi/h		56.4		
Speed Slope Coefficient (m)		3.61707		Speed Power Coefficient (p)		0.41674		
PF Slope Coefficient (m)		-1.32528		PF Power Coefficient (p)		0.75218		
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.2		
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0		
Subsegme	ent Data							
# Segmen	nt Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h		
1 Tangent		5280	-		-	56.4		
Vehicle Re	esults							
Average Speed, mi/h		56.4		Percent Followers, %		16.7		
Segment Travel Time, minutes		1.06		Follower Density (FD), followers/mi/ln		0.2		
Vehicle LOS		A						
Facility Re	esults							
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS		
1	11	0.00			0.2	A		

		HCS Two-La	ne Hi	ighway Re	port	
Proje	ct Information					
Analyst		gb	D	Date		6/3/2022
Agency		pec	А	nalysis Year		2022
Jurisdict	tion		Ti	ime Analyzed		2034 no build am
Project	Description	prop solar	U	Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segment Type		Passing Constrained		Length, ft		5280
Lane Wi	idth, ft	9	SI	Shoulder Width, ft		0
Speed L	imit, mi/h	55	А	ccess Point Dens	ity, pts/mi	0.0
Dema	and and Capacity					
Directio	nal Demand Flow Rate, veh/h	35		Opposing Demand Flow Rate, veh/h		-
Peak Ho	our Factor	0.66		Total Trucks, %		11.50
Segmen	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.02
Interr	nediate Results					
Segmen	nt Vertical Class	1		Free-Flow Speed, mi/h		56.3
Speed Slope Coefficient (m)		3.61256		Speed Power Coefficient (p)		0.41674
PF Slope Coefficient (m)		-1.32560		PF Power Coefficient (p)		0.75231
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.1
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	56.3
Vehic	le Results					
Average Speed, mi/h		56.3		Percent Followers, %		10.1
Segment Travel Time, minutes		1.07		Follower Density (FD), followers/mi/ln		0.1
Vehicle LOS		А				
Facilit	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	6	0.00			0.1	A

		HCS Two-La	ne Hi	ighway Re	port				
Project I	nformation								
Analyst		gb	D	Date		6/3/2022			
Agency		рес	А	nalysis Year		2022			
Jurisdiction			Ti	ime Analyzed		2034 no build am			
Project Desc	cription	prop solar	U	Units		U.S. Customary			
	Segment 1								
Vehicle I	nputs								
Segment Ty	pe	Passing Constrained		Length, ft		5280			
Lane Width,	ft	9	S	houlder Width, ft	t	0			
Speed Limit	, mi/h	30	А	ccess Point Dens	ity, pts/mi	0.0			
Demand	and Capacity		·						
Directional I	Demand Flow Rate, veh/h	8		Opposing Demand Flow Rate, veh/h		-			
Peak Hour F	actor	0.63		Total Trucks, %		11.80			
Segment Ca	pacity, veh/h	1700		Demand/Capacity (D/C)		0.00			
Intermed	diate Results								
Segment Vertical Class		1		Free-Flow Speed, mi/h		27.8			
Speed Slope Coefficient (m)		2.06731		Speed Power Coefficient (p)		0.41674			
PF Slope Coefficient (m)		-1.34714		PF Power Coefficient (p)		0.64082			
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0			
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0			
Subsegn	nent Data								
# Segm	ent Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tange	nt	5280	-		-	27.8			
Vehicle F	Results					·			
Average Speed, mi/h		27.8		Percent Followers, %		5.9			
Segment Travel Time, minutes		2.16		Follower Density (FD), followers/mi/ln		0.0			
Vehicle LOS		A							
Facility F	Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	1	0.00			0.0	A			

		HCS Two-La	ne H	lighway Re	port				
Projec	ct Information								
Analyst		gb	[	Date		6/3/2022			
Agency		pec	A	Analysis Year		2022			
Jurisdict	ion		Т	Гime Analyzed		2034 no build am			
Project [	Description	prop solar	ι	Jnits		U.S. Customary			
	Segment 1								
Vehicl	e Inputs								
Segmen	t Type	Passing Constrained	L	_ength, ft		5280			
Lane Wi	dth, ft	9	5	Shoulder Width, ft	t	0			
Speed Li	imit, mi/h	55	<i>A</i>	Access Point Dens	iity, pts/mi	0.0			
Dema	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	0	(	Opposing Demand	d Flow Rate, veh/h	-			
Peak Ho	ur Factor	0.50	Т	Total Trucks, %		22.00			
Segmen	t Capacity, veh/h	1700	С	Demand/Capacity	(D/C)	0.00			
Intern	nediate Results								
Segmen	t Vertical Class	1	F	ree-Flow Speed,	mi/h	56.0			
Speed SI	lope Coefficient (m)	3.59360	5	Speed Power Coef	fficient (p)	0.41674			
PF Slope	e Coefficient (m)	-1.32691	F	PF Power Coefficie	ent (p)	0.75288			
In Passin	ng Lane Effective Length?	No	T	Total Segment Dei	nsity, veh/mi/ln	0.0			
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0			
Subse	gment Data								
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tai	ngent	5280	-		-	56.0			
Vehicl	e Results								
Average	Speed, mi/h	56.0	F	Percent Followers,	%	0.0			
Segmen	t Travel Time, minutes	1.07	F	ollower Density (	FD), followers/mi/ln	0.0			
Vehicle L	LOS	А							
Facilit	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	0	0.00			0.0	А			
		Deserved LICCEN I		. Varria - 2022		Cara areta di 00 (03 (3033 14-31)			

		HCS Two-La	ne H	lighway Re	port				
Projec	t Information								
Analyst		gb	[	Date		6/3/2022			
Agency		pec	A	Analysis Year		2022			
Jurisdicti	ion		Т	Time Analyzed		2034 no build am			
Project D	Description	prop solar	ι	Jnits		U.S. Customary			
	Segment 1								
Vehicle	e Inputs								
Segment	t Type	Passing Constrained	L	Length, ft		5280			
Lane Wic	dth, ft	9	5	Shoulder Width, ft	t	0			
Speed Li	mit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0			
Dema	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	11		Opposing Demand	d Flow Rate, veh/h	-			
Peak Hou	ur Factor	0.45	T	Total Trucks, %		28.30			
Segment	t Capacity, veh/h	1700	[	Demand/Capacity	(D/C)	0.01			
Intern	nediate Results								
Segment	t Vertical Class	1	F	Free-Flow Speed,	mi/h	55.8			
Speed SI	ope Coefficient (m)	3.58223	5	Speed Power Coef	fficient (p)	0.41674			
PF Slope	Coefficient (m)	-1.32769	F	PF Power Coefficie	ent (p)	0.75322			
In Passin	g Lane Effective Length?	No	Т	Total Segment Dei	nsity, veh/mi/ln	0.0			
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0			
Subse	gment Data								
# Se	gment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h			
1 Tar	ngent	5280	-		-	55.8			
Vehicle	e Results								
Average	Speed, mi/h	55.8	F	Percent Followers,	%	4.4			
Segment	t Travel Time, minutes	1.08	F	Follower Density (	FD), followers/mi/ln	0.0			
Vehicle L	.OS	А							
Facility	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	1	0.00			0.0	A			
		Decembed LICCEN I		. Vancion 2022		Cara areta di 00/03/2022 14:22:4			

		HCS Two-La	ne H	ighway Re	port				
Projec	t Information								
Analyst		gb	С	Date		6/3/2022			
Agency		pec	Δ	Analysis Year		2022			
Jurisdict	ion		Т	Time Analyzed		2034 no build pm			
Project [	Description	prop solar	l	Jnits		U.S. Customary			
	Segment 1								
Vehicl	e Inputs								
Segmen	t Type	Passing Constrained	L	ength, ft		5280			
Lane Wi	dth, ft	9	S	Shoulder Width, ft	t	0			
Speed Li	imit, mi/h	55	Δ	Access Point Dens	ity, pts/mi	0.0			
Dema	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	55	C	Opposing Demand	d Flow Rate, veh/h	-			
Peak Ho	ur Factor	0.80	Т	Total Trucks, %		9.00			
Segmen	t Capacity, veh/h	1700	С	Demand/Capacity	(D/C)	0.03			
Intern	nediate Results								
Segmen	t Vertical Class	1	F	ree-Flow Speed,	mi/h	56.4			
Speed S	lope Coefficient (m)	3.61707	S	Speed Power Coef	fficient (p)	0.41674			
PF Slope	Coefficient (m)	-1.32528	Р	PF Power Coefficie	ent (p)	0.75218			
In Passin	ng Lane Effective Length?	No	Т	Total Segment De	nsity, veh/mi/ln	0.1			
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0			
Subse	gment Data								
# Se	gment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	ngent	5280	-		-	56.4			
Vehicl	e Results								
Average	Speed, mi/h	56.4	P	Percent Followers,	%	13.9			
Segmen	t Travel Time, minutes	1.06	F	ollower Density (	FD), followers/mi/ln	0.1			
Vehicle L	_OS	А							
Facilit	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	11	0.00			0.1	А			
		Decembed LICCEN I		Varriage 2022		Caracasta di 00 (02 (2022 12:57:2			

		HCS Two-La	ne F	lighway Re	port	
Projec	ct Information					
Analyst		gb	- I	Date		6/3/2022
Agency		pec	,	Analysis Year		2022
Jurisdict	ion		-	Time Analyzed		2034 no build pm
Project [	Description	prop solar	ı	Units		U.S. Customary
		Se	egm	ent 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Constrained	T	Length, ft		5280
Lane Wi	dth, ft	9	:	Shoulder Width, ft	t	0
Speed Li	imit, mi/h	55	,	Access Point Dens	ity, pts/mi	0.0
Dema	nd and Capacity					·
Direction	nal Demand Flow Rate, veh/h	31	- (	Opposing Demand	d Flow Rate, veh/h	-
Peak Ho	ur Factor	0.71	-	Total Trucks, %		11.50
Segmen	t Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.02
Intern	nediate Results					
Segmen	t Vertical Class	1	T	Free-Flow Speed,	mi/h	56.3
Speed S	lope Coefficient (m)	3.61256	:	Speed Power Coef	fficient (p)	0.41674
PF Slope	e Coefficient (m)	-1.32560	1	PF Power Coefficie	ent (p)	0.75231
In Passir	ng Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.1
%lmprov	vement to Percent Followers	0.0	(	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radiu	us, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	1-		-	56.3
Vehicl	le Results					
Average	Speed, mi/h	56.3	1	Percent Followers,	%	9.3
Segmen	t Travel Time, minutes	1.07		Follower Density (	FD), followers/mi/ln	0.1
Vehicle L	LOS	А				
Facilit	y Results	·				
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	6	0.00			0.1	A
	○ 2022 Heit and Heit of Flaciate All Biologo	December 1100 EM 1		- \/: 2022		C

		HCS Two-La	ne H	lighway Re	port	
Projec	ct Information					
Analyst		gb	ı	Date		6/3/2022
Agency		pec	/	Analysis Year		2022
Jurisdict	tion		-	Time Analyzed		2034 no build pm
Project I	Description	prop solar	U	Units		U.S. Customary
		Se	egmo	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Constrained	ı	Length, ft		5280
Lane Wi	idth, ft	9	9	Shoulder Width, ft	t	0
Speed L	imit, mi/h	30	,	Access Point Dens	ity, pts/mi	0.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	13	(	Opposing Demand	d Flow Rate, veh/h	-
Peak Ho	our Factor	0.54	1	Total Trucks, %		11.80
Segmen	nt Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.01
Intern	nediate Results					
Segmen	nt Vertical Class	1	F	Free-Flow Speed,	mi/h	27.8
Speed S	Slope Coefficient (m)	2.06731	9	Speed Power Coef	fficient (p)	0.41674
PF Slope	e Coefficient (m)	-1.34714	ı	PF Power Coefficie	ent (p)	0.64082
In Passir	ng Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	Ç	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	27.8
Vehic	le Results					·
Average	e Speed, mi/h	27.8	F	Percent Followers,	%	8.0
Segmen	nt Travel Time, minutes	2.16	F	Follower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	A				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	2	0.00			0.0	A
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		HCS Two-La	ne Hi	ighway Re	port				
Project In	formation								
Analyst		gb	D	Pate		6/3/2022			
Agency		pec	А	nalysis Year		2022			
Jurisdiction			Т	ime Analyzed		2034 no build pm			
Project Descri	iption	prop solar	U	Jnits		U.S. Customary			
	Segment 1								
Vehicle In	puts								
Segment Type	e	Passing Constrained	L	ength, ft		5280			
Lane Width, f	t	9	S	houlder Width, ft	t	0			
Speed Limit, r	mi/h	55	A	access Point Dens	sity, pts/mi	0.0			
Demand a	and Capacity								
Directional De	emand Flow Rate, veh/h	4	С	Opposing Deman	d Flow Rate, veh/h	-			
Peak Hour Fa	ctor	0.50	0.50 Total Trucks, %			22.00			
Segment Cap	acity, veh/h	1700	D	Demand/Capacity	(D/C)	0.00			
Intermed	iate Results								
Segment Vert	tical Class	1	F	ree-Flow Speed,	mi/h	56.0			
Speed Slope	Coefficient (m)	3.59360	S	Speed Power Coefficient (p)		0.41674			
PF Slope Coe	fficient (m)	-1.32691	Р	F Power Coefficie	ent (p)	0.75288			
In Passing Lar	ne Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0			
%Improveme	nt to Percent Followers	0.0	%	6Improvement to	Speed	0.0			
Subsegm	ent Data								
# Segmer	nt Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tangen	t	5280	-		-	56.0			
Vehicle R	esults								
Average Spee	ed, mi/h	56.0	Р	ercent Followers,	, %	2.1			
Segment Trav	vel Time, minutes	1.07	F	ollower Density (	FD), followers/mi/ln	0.0			
Vehicle LOS		А							
Facility Re	esults								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	1	0.00			0.0	A			

		HCS Two-La	ne Hi	ighway Re	port				
Project II	nformation								
Analyst		gb	D	ate		6/3/2022			
Agency		pec	А	nalysis Year		2022			
Jurisdiction			Ti	ime Analyzed		2034 no build pm			
Project Desc	ription	prop solar	U	Inits		U.S. Customary			
Segment 1									
Vehicle I	nputs								
Segment Typ	ре	Passing Constrained	Le	ength, ft		5280			
Lane Width,	ft	9	SI	houlder Width, ft	t	0			
Speed Limit,	mi/h	55	A	ccess Point Dens	ity, pts/mi	0.0			
Demand	and Capacity								
Directional D	Demand Flow Rate, veh/h	7	0	pposing Demand	d Flow Rate, veh/h	-			
Peak Hour Fa	actor	0.67	To	Total Trucks, %		28.30			
Segment Ca	pacity, veh/h	1700	D	emand/Capacity	(D/C)	0.00			
Intermed	liate Results								
Segment Vei	rtical Class	1	Fı	ree-Flow Speed,	mi/h	55.8			
Speed Slope	Coefficient (m)	3.58223	Sı	Speed Power Coefficient (p)		0.41674			
PF Slope Co	efficient (m)	-1.32769	P	F Power Coefficie	ent (p)	0.75322			
In Passing La	ane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0			
%Improvem	ent to Percent Followers	0.0	%	almprovement to	Speed	0.0			
Subsegm	nent Data								
# Segme	ent Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tanger	nt	5280	-		-	55.8			
Vehicle R	Results								
Average Spe	ed, mi/h	55.8	Pe	ercent Followers,	%	3.3			
Segment Tra	vel Time, minutes	1.08	Fo	ollower Density (	FD), followers/mi/ln	0.0			
Vehicle LOS		А							
Facility R	Results								
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	1	0.00			0.0	A			

		HCS Two-La	ne Hi	ighway Re	port				
Project	Information								
Analyst		gb	D	ate		6/3/2022			
Agency		рес	А	nalysis Year		2022			
Jurisdiction	า		Ti	ime Analyzed		2034 build am			
Project Des	scription	prop solar	U	Inits		U.S. Customary			
Segment 1									
Vehicle	Inputs								
Segment T	ype	Passing Constrained	Le	ength, ft		5280			
Lane Width	n, ft	9	SI	houlder Width, ft	t	0			
Speed Limi	it, mi/h	55	A	ccess Point Dens	ity, pts/mi	0.0			
Deman	d and Capacity								
Directional	Demand Flow Rate, veh/h	155	0	pposing Demand	d Flow Rate, veh/h	-			
Peak Hour	Factor	0.60	0.60 Total Trucks, %			9.00			
Segment C	Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.09			
Interme	ediate Results								
Segment V	ertical Class	1	Fr	ree-Flow Speed,	mi/h	56.4			
Speed Slop	pe Coefficient (m)	3.61707	Sį	Speed Power Coefficient (p)		0.41674			
PF Slope C	oefficient (m)	-1.32528	PI	F Power Coefficie	ent (p)	0.75218			
In Passing	Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.8			
%Improver	ment to Percent Followers	0.0	%	almprovement to	Speed	0.0			
Subseg	ment Data								
# Segn	nent Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tang	ent	5280	-		-	55.3			
Vehicle	Results					·			
Average Sp	peed, mi/h	55.3	Pe	ercent Followers,	%	27.8			
Segment T	ravel Time, minutes	1.08	Fo	ollower Density (	FD), followers/mi/ln	0.8			
Vehicle LO	S	А							
Facility	Results								
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	23	0.01			0.8	A			

		HCS Two-Lar	ne Hi	ighway Re	port				
Project Information	on								
Analyst		gb	D	ate		6/3/2022			
Agency		pec	A	nalysis Year		2022			
Jurisdiction			Ti	ime Analyzed		2034 build am			
Project Description		prop solar	U	Inits		U.S. Customary			
Segment 1									
Vehicle Inputs									
Segment Type		Passing Constrained	Le	ength, ft		5280			
Lane Width, ft		9	Sł	houlder Width, ft	ī	0			
Speed Limit, mi/h		55	A	ccess Point Dens	ity, pts/mi	0.0			
Demand and Capa	city								
Directional Demand Flow	Rate, veh/h	35	0	pposing Demand	d Flow Rate, veh/h	-			
Peak Hour Factor		0.66 Tot		Total Trucks, %		11.50			
Segment Capacity, veh/h		1700	D	emand/Capacity	(D/C)	0.02			
Intermediate Resu	lts								
Segment Vertical Class		1	Fr	ree-Flow Speed,	mi/h	56.3			
Speed Slope Coefficient (r	n)	3.61256	Sį	Speed Power Coefficient (p)		0.41674			
PF Slope Coefficient (m)		-1.32560	PI	F Power Coefficie	ent (p)	0.75231			
In Passing Lane Effective L	ength?	No	To	otal Segment De	nsity, veh/mi/ln	0.1			
%Improvement to Percent	t Followers	0.0	%	Improvement to	Speed	0.0			
Subsegment Data									
# Segment Type		Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tangent		5280	-		-	56.3			
Vehicle Results									
Average Speed, mi/h		56.3	Pe	ercent Followers,	%	10.1			
Segment Travel Time, min	utes	1.07	Fo	ollower Density (	FD), followers/mi/ln	0.1			
Vehicle LOS		A							
Facility Results									
T VN veh-	ЛТ mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1 (	 6	0.00			0.1	A			

		HCS Two-La	ne Hi	ighway Re	port				
Proje	ct Information								
Analyst		gb	D	ate		6/3/2022			
Agency		pec	A	nalysis Year		2022			
Jurisdict	tion		Ti	ime Analyzed		2034 build am			
Project I	Description	prop solar	U	Inits		U.S. Customary			
Segment 1									
Vehic	le Inputs								
Segmen	nt Type	Passing Constrained	Le	ength, ft		5280			
Lane Wi	idth, ft	9	Sł	houlder Width, ft	t	0			
Speed L	imit, mi/h	30	A	ccess Point Dens	ity, pts/mi	0.0			
Dema	and Capacity								
Directio	nal Demand Flow Rate, veh/h	87	0	pposing Demand	d Flow Rate, veh/h	-			
Peak Ho	our Factor	0.63	To	otal Trucks, %		11.80			
Segmen	nt Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.05			
Intern	nediate Results								
Segmen	nt Vertical Class	1	Fr	ree-Flow Speed,	mi/h	27.8			
Speed S	Slope Coefficient (m)	2.06731	Sp	peed Power Coef	fficient (p)	0.41674			
PF Slope	e Coefficient (m)	-1.34714	PI	F Power Coefficie	ent (p)	0.64082			
In Passir	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.8			
%lmpro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0			
Subse	egment Data								
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	27.8			
Vehic	le Results					<u> </u>			
Average	e Speed, mi/h	27.8	Pe	ercent Followers,	%	24.6			
Segmen	nt Travel Time, minutes	2.16	Fo	ollower Density (	FD), followers/mi/ln	0.8			
Vehicle	LOS	DS A							
Facilit	ty Results								
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	14	0.00			0.8	A			

		HCS Two-La	ne H	lighway Re	port				
Projec	ct Information								
Analyst		gb	Г	Date		6/3/2022			
Agency		pec	A	Analysis Year		2022			
Jurisdicti	ion		7	Time Analyzed		2034 build am			
Project D	Description	prop solar	ι	Jnits		U.S. Customary			
	Segment 1								
Vehicl	e Inputs								
Segment	t Туре	Passing Constrained	I	Length, ft		5280			
Lane Wid	dth, ft	9	9	Shoulder Width, ft	t	0			
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0			
Dema	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	0	(	Opposing Demand	d Flow Rate, veh/h	-			
Peak Ho	ur Factor	0.50	7	Total Trucks, %		22.00			
Segment	t Capacity, veh/h	1700	[	Demand/Capacity	(D/C)	0.00			
Intern	nediate Results								
Segment	t Vertical Class	1	F	Free-Flow Speed,	mi/h	56.0			
Speed SI	lope Coefficient (m)	3.59360	9	Speed Power Coef	fficient (p)	0.41674			
PF Slope	e Coefficient (m)	-1.32691	F	PF Power Coefficie	ent (p)	0.75288			
In Passin	ng Lane Effective Length?	No	7	Total Segment Dei	nsity, veh/mi/ln	0.0			
%Improv	vement to Percent Followers	0.0	ç	%Improvement to	Speed	0.0			
Subse	gment Data								
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h			
1 Tai	ngent	5280	-		-	56.0			
Vehicl	e Results								
Average	Speed, mi/h	56.0	F	Percent Followers,	%	0.0			
Segment	t Travel Time, minutes	1.07	F	Follower Density (	FD), followers/mi/ln	0.0			
Vehicle L	LOS	А							
Facilit	y Results		·						
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	0	0.00			0.0	А			
		Deserved LICCEN I		. Varaina 2022		Cara areta di 00 (03 (3033 14-33)			

		HCS Two-La	ne H	lighway Re	port	
Proje	ct Information					
Analyst		gb	I	Date		6/3/2022
Agency		pec	,	Analysis Year		2022
Jurisdict	tion		-	Time Analyzed		2034 build am
Project	Description	prop solar	ı	Units		U.S. Customary
		Se	egm	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Constrained	Ī	Length, ft		5280
Lane Wi	idth, ft	9		Shoulder Width, ft	t	0
Speed L	imit, mi/h	55	,	Access Point Dens	ity, pts/mi	0.0
Dema	and Capacity					·
Directio	nal Demand Flow Rate, veh/h	11		Opposing Demand	d Flow Rate, veh/h	-
Peak Ho	our Factor	0.45	-	Total Trucks, %		28.30
Segmen	nt Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.01
Interr	nediate Results					
Segmen	nt Vertical Class	1	Ti	Free-Flow Speed,	mi/h	55.8
Speed S	Slope Coefficient (m)	3.58223	9	Speed Power Coef	fficient (p)	0.41674
PF Slope	e Coefficient (m)	-1.32769	1	PF Power Coefficie	ent (p)	0.75322
In Passir	ng Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	(	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	1-		-	55.8
Vehic	le Results	•				<u>.</u>
Average	e Speed, mi/h	55.8	ı	Percent Followers,	%	4.4
Segmen	nt Travel Time, minutes	1.08	ı	Follower Density (	FD), followers/mi/ln	0.0
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	1	0.00			0.0	A
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		HCS Two-La	ne H	ighway Re	port				
Projec	t Information								
Analyst		gb	С	Date		6/3/2022			
Agency		pec	A	Analysis Year		2022			
Jurisdict	ion		Т	Time Analyzed		2034 build pm			
Project [	Description	prop solar	ι	Jnits		U.S. Customary			
	Segment 1								
Vehicl	e Inputs								
Segmen	t Type	Passing Constrained L		ength, ft		5280			
Lane Wi	dth, ft	9	S	Shoulder Width, ft	t	0			
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0			
Dema	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	118		Opposing Demand Flow Rate, veh/h		-			
Peak Ho	ur Factor	0.80		Total Trucks, %		9.00			
Segmen	t Capacity, veh/h	1700		Demand/Capacity (D/C)		0.07			
Intern	nediate Results								
Segment Vertical Class		1		Free-Flow Speed, mi/h		56.4			
Speed S	lope Coefficient (m)	3.61707		Speed Power Coefficient (p)		0.41674			
PF Slope	e Coefficient (m)	-1.32528		PF Power Coefficient (p)		0.75218			
In Passin	ng Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.5			
%Improv	vement to Percent Followers	0.0 %Im		%Improvement to Speed		0.0			
Subse	gment Data								
# Se	gment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	ngent	5280	-		-	55.7			
Vehicl	e Results								
Average Speed, mi/h		55.7		Percent Followers, %		23.3			
Segment Travel Time, minutes		1.08		Follower Density (FD), followers/mi/ln		0.5			
Vehicle LOS		A							
Facilit	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	24	0.01			0.5	А			
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		HCS Two-La	ne F	lighway Re	port	
Proje	ct Information					
Analyst		gb		Date		6/3/2022
Agency		pec		Analysis Year		2022
Jurisdict	tion			Time Analyzed		2034 build pm
Project	Description	prop solar		Units		U.S. Customary
		Se	egm	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Constrained		Length, ft		5280
Lane Wi	idth, ft	9	:	Shoulder Width, ft	t	0
Speed L	imit, mi/h	55		Access Point Dens	ity, pts/mi	0.0
Dema	and and Capacity					·
Directio	nal Demand Flow Rate, veh/h	31		Opposing Demand Flow Rate, veh/h		-
Peak Ho	our Factor	0.71		Total Trucks, %		11.50
Segmen	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.02
Interr	nediate Results					
Segment Vertical Class		1		Free-Flow Speed, mi/h		56.3
Speed Slope Coefficient (m)		3.61256	:	Speed Power Coefficient (p)		0.41674
PF Slope	e Coefficient (m)	-1.32560		PF Power Coefficient (p)		0.75231
In Passii	ng Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.1
%lmpro	vement to Percent Followers	0.0 %		%Improvement to Speed		0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	us, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	56.3
Vehic	le Results					·
Average	e Speed, mi/h	56.3		Percent Followers, %		9.3
Segment Travel Time, minutes		1.07		Follower Density (FD), followers/mi/ln		0.1
Vehicle LOS		А				
Facilit	ty Results	<u>'</u>				
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	6	0.00			0.1	A
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		HCS Two-Lai	ne Hi	ighway Re	port				
Project I	nformation								
Analyst		gb	D	Date		6/3/2022			
Agency		рес	A	nalysis Year		2022			
Jurisdiction			Ti	ime Analyzed		2034 build pm			
Project Desc	cription	prop solar	U	Inits		U.S. Customary			
	Segment 1								
Vehicle I	nputs								
Segment Ty	pe	Passing Constrained		Length, ft		5280			
Lane Width,	, ft	9	Sł	houlder Width, ft	t	0			
Speed Limit	, mi/h	30	A	ccess Point Dens	ity, pts/mi	0.0			
Demand	and Capacity								
Directional I	Demand Flow Rate, veh/h	106		Opposing Demand Flow Rate, veh/h		-			
Peak Hour F	actor	0.54		Total Trucks, %		11.80			
Segment Ca	pacity, veh/h	1700		Demand/Capacity (D/C)		0.06			
Interme	diate Results								
Segment Ve	ertical Class	1		Free-Flow Speed, mi/h		27.8			
Speed Slope	e Coefficient (m)	2.06731		Speed Power Coefficient (p)		0.41674			
PF Slope Coefficient (m)		-1.34714		PF Power Coefficient (p)		0.64082			
In Passing L	ane Effective Length?	No		Total Segment Density, veh/mi/ln		1.0			
%Improvem	nent to Percent Followers	0.0 %lr		%Improvement to Speed		0.0			
Subsegn	nent Data								
# Segm	ent Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Tange	ent	5280	-		-	27.6			
Vehicle I	Results								
Average Speed, mi/h		27.6		Percent Followers, %		27.3			
Segment Travel Time, minutes		2.18		Follower Density (FD), followers/mi/ln		1.0			
Vehicle LOS		A							
Facility Results									
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	14	0.00			1.0	A			

		HCS Two-La	ne H	lighway Re	port			
Projec	ct Information							
Analyst		gb	Г	Date		6/3/2022		
Agency		pec	A	Analysis Year		2022		
Jurisdict	ion		1	Time Analyzed		2034 build pm		
Project [	Description	prop solar	ι	Jnits		U.S. Customary		
Segment 1								
Vehicl	le Inputs							
Segmen	t Type	Passing Constrained Le		Length, ft		5280		
Lane Wi	dth, ft	9	9	Shoulder Width, ft	t	0		
Speed Li	imit, mi/h	55	A	Access Point Dens	ity, pts/mi	0.0		
Dema	nd and Capacity							
Direction	nal Demand Flow Rate, veh/h	4		Opposing Demand Flow Rate, veh/h		-		
Peak Ho	ur Factor	0.50		Total Trucks, %		22.00		
Segmen	t Capacity, veh/h	1700		Demand/Capacity (D/C)		0.00		
Intern	nediate Results							
Segmen	t Vertical Class	1		Free-Flow Speed, mi/h		56.0		
Speed S	lope Coefficient (m)	3.59360		Speed Power Coefficient (p)		0.41674		
PF Slope	e Coefficient (m)	-1.32691		PF Power Coefficient (p)		0.75288		
In Passin	ng Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.0		
%Improv	vement to Percent Followers	0.0 %lm		%Improvement to Speed		0.0		
Subse	gment Data							
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	ngent	5280	-		-	56.0		
Vehicl	e Results							
Average Speed, mi/h		56.0		Percent Followers, %		2.1		
Segment Travel Time, minutes		1.07		Follower Density (FD), followers/mi/ln		0.0		
Vehicle LOS		A						
Facilit	y Results							
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS		
1	1	0.00			0.0	А		
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		HCS Two-La	ne H	lighway Re	port	
Projec	ct Information					
Analyst		gb	I	Date		6/3/2022
Agency		pec	,	Analysis Year		2022
Jurisdict	ion		-	Time Analyzed		2034 build pm
Project I	Description	prop solar	ı	Units		U.S. Customary
		Se	egm	ent 1		
Vehic	le Inputs					
Segmen	t Type	Passing Constrained I		Length, ft		5280
Lane Wi	dth, ft	9		Shoulder Width, ft	t	0
Speed L	imit, mi/h	55	,	Access Point Dens	ity, pts/mi	0.0
Dema	nd and Capacity		·			
Directio	nal Demand Flow Rate, veh/h	7		Opposing Demand Flow Rate, veh/h		-
Peak Ho	our Factor	0.67		Total Trucks, %		28.30
Segmen	t Capacity, veh/h	1700		Demand/Capacity (D/C)		0.00
Intern	nediate Results					
Segment Vertical Class		1		Free-Flow Speed, mi/h		55.8
Speed S	lope Coefficient (m)	3.58223		Speed Power Coefficient (p)		0.41674
PF Slope	e Coefficient (m)	-1.32769		PF Power Coefficient (p)		0.75322
In Passir	ng Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.0
%Impro	vement to Percent Followers	0.0 %		%Improvement to Speed		0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	1-		-	55.8
Vehic	le Results	•				<u>.</u>
Average Speed, mi/h		55.8		Percent Followers, %		3.3
Segment Travel Time, minutes		1.08		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		А				
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	1	0.00			0.0	A
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