COMMONWEALTH OF KENTUCKY BEFORE THE PUBLIC SERVICE COMMISSION

In the Matter of:

THE APPLICATION OF)CELLCO PARTNERSHIP D/B/A VERIZON WIRELESS)FOR ISSUANCE OF A CERTIFICATE OF PUBLIC)CONVENIENCE AND NECESSITY TO CONSTRUCT)A WIRELESS COMMUNICATIONS FACILITY)IN THE COMMONWEALTH OF KENTUCKY)IN THE COUNTY OF BALLARD)

CASE NO. 2022-0016

SITE NAME: BARLOW

* * * * * * *

APPLICATION FOR CERTIFICATE OF PUBLIC CONVENIENCE AND NECESSITY FOR CONSTRUCTION OF A WIRELESS COMMUNICATIONS FACILITY

Cellco Partnership, d/b/a Verizon Wireless ("Applicant"), by counsel, pursuant to (i) KRS §§278.020, 278.040, 278.650, 278.665, and other statutory authority, and the rules and regulations applicable thereto, and (ii) the Telecommunications Act of 1996, respectfully submits this Application requesting issuance of a Certificate of Public Convenience and Necessity ("CPCN") from the Kentucky Public Service Commission ("PSC") to construct, maintain, and operate a Wireless Communications Facility ("WCF") to serve the customers of the Applicant with wireless communications services.

In support of this Application, Applicant respectfully provides and states the following

information:

1. The complete name and address of the Applicant: Cellco Partnership, d/b/a Verizon Wireless, having a local address of 2421 Holloway Road, Louisville, KY 40299.

2. Applicant is a Delaware general partnership and a copy of the Amended Certificate of Assumed Name is on file with the Secretary of State of Commonwealth of Kentucky is included as part of **Exhibit A**.

3. Applicant proposes construction of an antenna tower for communications services, which is to be located in an area outside the jurisdiction of a planning commission, and Applicant submits this application to the PSC for a certificate of public convenience and necessity pursuant to KRS §§ 278.020(1), 278.040, 278.650, 278.665, and other statutory authority.

4. The Applicant operates on frequencies licensed by the Federal Communications Commission ("FCC") pursuant to applicable FCC requirements. A copy of the Applicant's FCC licenses to provide wireless services are attached to this Application or described as part of **Exhibit B**, and the facility will be constructed and operated in accordance with applicable FCC regulations.

5. The public convenience and necessity require the construction of the proposed WCF. The construction of the WCF will bring or improve the Applicant's services to an area currently not served or not adequately served by the Applicant by increasing coverage or capacity and thereby enhancing the public's access to innovative and competitive wireless communications services. A statement from Applicant's RF Design Engineer outlining said need is attached as **Exhibit Q** along with Propagation Maps attached as **Exhibit Qa**. The WCF is an integral link in the Applicant's network design that must be in place to provide adequate coverage to the service area.

6. To address the above-described service needs, Applicant proposes to construct a WCF at 2244 Steve Denton Road, Barlow KY 42024 (37° 06' 42.15"North latitude, 89° 02'

44.58" West longitude), on a parcel of land located entirely within the county referenced in the caption of this application. The property on which the WCF will be located is owned by Myatt Family Trust pursuant to a Deed recorded at Deed Book 93, Page 150 in the office of the County Clerk. The proposed WCF will consist of a 285-foot tall tower, with an approximately 5-foot tall lightning arrestor attached at the top, for a total height of 290-feet. The WCF will also include concrete foundations and a shelter or cabinets to accommodate the placement of the Applicant's radio electronics equipment and appurtenant equipment. The Applicant's equipment cabinet or shelter will be approved for use in the Commonwealth of Kentucky by the relevant building inspector. The WCF compound will be fenced and all access gate(s) will be secured. A description of the manner in which the proposed WCF will be constructed is attached as **Exhibit**

C and Exhibit D.

7. A list of utilities, corporations, or persons with whom the proposed WCF is likely to compete along with a map showing the proposed location as well as the identified like facilities is attached as **Exhibit E**.

8. The site development plan and a vertical profile sketch of the WCF signed and sealed by a professional engineer registered in Kentucky depicting the tower height, as well as a proposed configuration for the antennas of the Applicant has also been included as part of **Exhibit C**.

9. Foundation design plans signed and sealed by a professional engineer registered in Kentucky and a description of the standards according to which the tower was designed are included as part of **Exhibit D**.

10. Applicant has considered the likely effects of the installation of the proposed WCF on nearby land uses and values and has concluded that there is no more suitable location

reasonably available from which adequate services can be provided, and that there are no reasonably available opportunities to co-locate Applicant's antennas on an existing structure. When suitable towers or structures exist, Applicant attempts to co-locate on existing structures such as communications towers or other structures capable of supporting Applicant's facilities; however, no other suitable or available co-location site was found to be located in the vicinity of the site.

11. A copy of the Determination of No Hazard to Air Navigation issued by the Federal Aviation Administration ("FAA") is attached as **Exhibit F**.

12. A copy of the Kentucky Airport Zoning Commission ("KAZC") Approval to construct the tower is attached as Exhibit G.

13. A geotechnical engineering report was performed at the WCF site by Alt & Witzig Engineering, Inc. Indianapolis, IN, dated August 30, 2018, and is attached as **Exhibit H**. The name and address of the geotechnical engineering firm and the professional engineer registered in Kentucky who prepared the report are included as part of **Exhibit H**.

14. Clear directions to the proposed WCF site from the County seat are attached as **Exhibit I**. The name and telephone number of the preparer of **Exhibit I** are included as part of this exhibit.

15. Applicant, pursuant to a written agreement, has acquired the right to use the WCF site and associated property rights. A copy of the agreement or an abbreviated agreement recorded with the County Clerk is attached as **Exhibit J**.

16. Personnel directly responsible for the design and construction of the proposed WCF are well qualified and experienced. The tower and foundation drawings for the proposed tower submitted as part of **Exhibit D** bear the signature and stamp of a professional engineer

registered in the Commonwealth of Kentucky. All tower designs meet or exceed the minimum requirements of applicable laws and regulations.

17. The Construction Manager for the proposed facility is Vince Caprino and the identity and qualifications of each person directly responsible for design and construction of the proposed tower are contained in **Exhibits C & D**.

18. As noted on the Survey attached as part of **Exhibit C**, the surveyor has determined that the tower site and access easement are not within any flood hazard area per Flood Hazard Boundary Map, Community Panel Number 21007C0085C, Dated July 7, 2014. Also find a letter from the surveyor regarding the Flood Data, attached as **Exhibit Ca**.

19. **Exhibit C** includes a map drawn to an appropriate scale that shows the location of the proposed tower and identifies every owner of real estate within 500 feet of the proposed tower (according to the records maintained by the County Property Valuation Administrator). Every structure and every easement within 500 feet of the proposed tower or within 200 feet of the access road including intersection with the public street system is illustrated in **Exhibit C**.

20. Applicant has notified every person who, according to the records of the County Property Valuation Administrator, owns property which is within 500 feet of the proposed tower or contiguous to the site property, by certified mail, return receipt requested, of the proposed construction. Each notified property owner has been provided with a map of the location of the proposed construction, the PSC docket number for this application, the address of the PSC, and will be informed of his or her right to request intervention. A list of the notified property owners and a copy of the form of the notice to be sent by certified mail to each landowner are attached as **Exhibit K** and **Exhibit L**, respectively.

21. Applicant has notified the applicable County Judge/Executive by certified mail, return receipt requested, of the proposed construction. This notice included the PSC docket number under which the application will be processed and informed the County Judge/Executive of his/her right to request intervention. A copy of this notice is attached as **Exhibit M**.

22. Notice signs meeting the requirements prescribed by 807 KAR 5:063, Section 1(2) that measure at least 2 feet in height and 4 feet in width and that contain all required language in letters of required height, have been posted, one in a visible location on the proposed site and one on the nearest public road. Such signs shall remain posted for at least two weeks after filing of the Application, and a copy of the posted text is attached as **Exhibit N**. A legal notice advertisement regarding the location of the proposed facility has been published in a newspaper of general circulation in the county in which the WCF is proposed to be located. A copy of the newspaper legal notice advertisement is attached as **Exhibit O**.

23. The general area where the proposed facility is to be located is undeveloped and removed a significant distance from any residential structures. The nearest residential structure is 284.38 feet from the proposed tower site.

24. The process that was used by the Applicant's radio frequency engineers in selecting the site for the proposed WCF was consistent with the general process used for selecting all other existing and proposed WCF facilities within the proposed network design area. Applicant's radio frequency engineers have conducted studies and tests in order to develop a highly efficient network that is designed to handle voice and data traffic in the service area. The engineers determined an optimum area for the placement of the proposed facility in terms of elevation and location to provide the best quality service to customers in the service area. A radio frequency design search area prepared in reference to these radio frequency studies was

considered by the Applicant when searching for sites for its antennas that would provide the coverage deemed necessary by the Applicant. A map of the area in which the tower is proposed to be located which is drawn to scale and clearly depicts the necessary search area within which the site should be located pursuant to radio frequency requirements is attached as **Exhibit P**.

25. The tower must be located at the proposed location and proposed height to provide necessary service to wireless communications users in the subject area, as set out and documented in the RF Design Engineers' Statement of Need and Propagation Maps attached as **Exhibit Q**. The proposed tower will expand and improve voice and data service for Verizon Wireless customers.

26. All Exhibits to this Application are hereby incorporated by reference as if fully set out as part of the Application.

27. All responses and requests associated with this Application may be directed to:

Russell L. Brown Clark, Quinn, Moses, Scott & Grahn, LLP 320 North Meridian Street, Suite 1100 Indianapolis, IN 46204 Phone: (317) 637-1321 FAX: (317) 687-2344 Email: rbrown@clarkquinnlaw.com WHEREFORE, Applicant respectfully request that the PSC accept the foregoing Application for filing, and having met the requirements of KRS §§278.020(1), 278.650, and 278 .665 and all applicable rules and regulations of the PSC, grant a Certificate of Public Convenience and Necessity to construct and operate the WCF at the location set forth herein.

Respectfully submitted,

Russell L. Brown Clark, Quinn, Moses, Scott & Grahn, LLP 320 North Meridian Street, Suite 1100 Indianapolis, IN 46204 Phone: (317) 637-1321 / FAX: (317) 687-2344 Email: rbrown@clarkquinnlaw.com Attorney for Cellco Partnership d/b/a Verizon Wireless

LIST OF EXHIBITS

А	Applicant Entity
В	FCC License Documentation
_	Site Development Plan:
C	500' Vicinity Map Legal Descriptions
	Flood Plain Certification Site Plan
	Vertical Tower Profile
Ca	Letter from Surveyor regarding Flood Data
D	Tower and Foundation Design
E	Competing Utilities, Corporations, or Persons List And Map of Like Facilities in Vicinity
F	FAA
G	KAZC Approval
Н	Geotechnical Report
Ι	Directions to WCF Site
J	Copy of Real Estate Agreement
K	Notification Listing
L	Copy of Property Owner Notification
М	Copy of County Judge/Executive Notice
Ν	Copy of Posted Notices
0	Copy of Newspaper Legal Notice Advertisement
Р	Copy of Radio Frequency Design Search Area
Q	Copy of RF Design Engineer Statement of Need
Qa	Propagation Maps

COMMONWEALTH OF KENTUCKY TREY GRAYSON SECRETARY OF STATE



Secretary of State Received and Filed 08/27/2008 12:05:00 PM Fae Receipt: \$20.00

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CERTIFICATE OF ASSUMED NAME

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COMMONWEALTH OF KENTUCKY ELAINE N. WALKER, SECRETARY OF STATE

Division of Business Filings Business Filings DD Dov 74s	Amended Certil	ficate of Assumed	Name	AAN	
Frankfort, KY 40802 (502) 564-3490 www.soit Ky.gov					
Pursuant to the provisions of KR purpose, submits the following st	s 365, the undersigned e alement: rizon Wireless	pplies to amand the certi	Scale of assumed	name and, for Ital	
(The num	e must be identical to the use	the on record with the Decrei	uy of State.)		
2. The certificate of assumed na	nie was filed with the Sec	relary of State or:	6/21/2008		
9. The manual advisor office and	danan Af an 3 be				
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One venzon way		asking Ridge	NJ	07920	
Street Address or Fost Office Box Nun	upeta C	illy	Ettily	Zip	
4. The principal office address is	hereby changed to:				
Sinel Address of Post Office Box Num	nburu - C	aty.	(Lab)	- 26	
5. This application will be effectiv	ve upon filing, unless a de	alayed effective date and	for time is provided	1. The effective date	
or the delayed effective date can	not be prior to the dete th	e application is filed. The	e date and/or time	(Delayed alfective date and/or tion)	
8. The changes in the Identity of	the partners are as follow	e: See Addendu	m for curren	it partners	
 					
declare under penalty of perjury	under the laws of Kenius GTE Wireless Incor	by that the forgoing is in pointed	to and correct.		
same ascharger	Jans & Schanker	A	sistani Secretary	1/21/2012	
Signature of Applicant	Frinted Hamp	ji	Ve	Date	

(04/11)

Addendum

The full name of the Partnership is Celleo Partnership, a Delaware general partnership composed of the following partners:

General Partners of Cellco Partnership	Address				
Bell Atlantic Mobile Systems LLC	One Verizon Way Basking Klidge, NJ 07920				
GTE Wireless Incorporated	One Vorizon Way Basking Ridge, NJ 07920				
PCS Nucleus, L.P.	Denver Place South Tower 999-18 th Street, Suite 1750 Denver, CO 80202				
IV ParinerCo, LLC	Denver Place South Tower 999-18 th Street, Suite 1750 Denver, CO 80202				

REFERENCE COPY

This is not an official FCC license. It is a record of public information contained in the FCC's licensing database on the date that this reference copy was generated. In cases where FCC rules require the presentation, posting, or display of an FCC license, this document may not be used in place of an official FCC license.

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KENTUCKY RSA 5055 NORTH POIN	NO. 1 PARTNERSHIP NT PKWY, NP2NE NE'	TWORK ENGINEER	ING	CL - (Cellular
ALPHARETTA, G	A 30022			Market Numer CMA443	Channel Block B
FCC Registration Num	her (FRN): 000183670	10		Sub-Marke	t Designator 0
Market Name Kentucky 1 - Fulton					
Grant Date 08-30-2011	Effective Date 11-02-2016	Expiration Dat 10-01-2021	e Fiv	e Yr Build-Out Date	Print Date
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Transmitting ERP (watts) Antenna: 4	0.360	0.200	0.200	0.350	18.200	89.130	66.070	2.630
Maximum Transmitting ERP	in Watts: 140.820							
Azimuth(from true north) 0	45	90	135	180	225	270	315
Antenna Height AA1 (meters Transmitting EDD (watts)) 75.100	73.400	74.100	70.100	102.600	100.900	74.700	81.300
	100.000	38.020	0.200	0.380	0.200	0.200	1.260	42.660
Location Latitude	Longitude	G	round Ele	vation	Structure Hgt	t to Tip	Antenna St	ructure
Loouton Luttude	Longitude	(1	neters)		(meters)	ľ	Registratio	n No.
14 37-05-47.2 N	088-42-35.2 W		04.2		63.4		1200593	
Address: (Paducah West) 44	415 Merredith Rd.							
City: Paducah County: N	ICCRACKEN Sta	te: KY	Construct	ion Dead	dline: 07-08-20)14		
			—					
Antenna: 4								
Maximum Transmitting ERP	in Watts: 140.820							
Azimuth(from true north) 0	45	90	135	180	225	270	315
Antenna Height AAT (meters	59.900	55.900	65.200	50.700	38.200	34.700	42.800	64.600
Antenna: 5	24.580	50.820	50.310	19.100	0.840	0.330	0.330	1.370
Maximum Transmitting ERP	in Watts: 140.820							
Azimuth(from true north	0	45	90	135	180	225	270	315
Transmitting ERP (watts)	9 59.900	55.900	65.200	50.700	38.200	34.700	42.800	64.600
Antenna: 6	0.440	0.440	12.210	/0.3/0	112.800	57.980	3.400	0.440
Maximum Transmitting ERP	in Watts: 140.820						•=•	~ ~ ~
Azimutn(from true north Antenna Height AAT (meters	1) () 1) 59.900	45 55 900	90 (5.200	135	180	225	270	315
Transmitting ERP (watts)	20.830	0.780	65.200 0.440	50.700 0.440	2.790	54.700 42.940	42.800	04.000 89.900
	_0.000		~					~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~



Call Sign: KNKQ306	1	File Number:			P	rint Date	:	
Location Latitude 15 36-46-54.2 N	Longitude 088-03-28.1	Gi (n W 19	round Elev neters) 99.0	ration Str (m 12	ructure Hg neters) 6.5	t to Tip	Antenna St Registratio 1205551	ructure n No.
Address: 14664 Canton R City: Golden Pond Cou	oad ntv: TRIGG Sta	ite: KY – Coi	nstruction	Deadline:	05-19-2006			
					05 17 2000			
Antenna: 2 Maximum Transmitting ER Azimuth(from true nor Antenna Height AAT (meter Transmitting ERP (watts)	P in Watts: 140.82 th) 0 rs) 165.0 96.61	0 45 00 178.000 0 96.610	90 160.400 96.610	135 174.500 96.610	180 170.600 96.610	225 167.000 96.610	270 177.000 96.610	315 183.900 96.610
Location Latitude	Longitude	G	round Elev peters)	ation St	ructure Hg (eters)	t to Tip	Antenna St Registratio	ructure n No.
16 36-34-03.0 N	089-10-30.9	W 10)9.4	91	.4		1282534	II 110.
Address: (Hickman site) H	Holley Street							
City: Hickman County:	FULTON State	e: KY Cons	struction D	eadline: 05	5-28-2014			
Antenna: 1 Maximum Transmitting ER Azimuth(from true nor Antenna Height AAT (meter Transmitting ERP (watts)	CP in Watts: 140.82 th) 0 rs) 105.5 141.7	0 45 00 102.800 00 118.910	90 96.700 1.140	135 89.300 0.580	180 75.700 0.580	225 68.400 0.580	270 107.900 0.580	315 107.300 4.050
Antenna: 2 Maximum Transmitting ER Azimuth(from true non Antenna Height AAT (mete Transmitting ERP (watts) Antenna: 3	and the second	0 45 102.800 4.050	90 96.700 141.730	135 89.300 118.910	180 75.700 1.140	225 68.400 0.580	270 107.900 0.580	315 107.300 0.580
Maximum Transmitting ER Azimuth(from true nor Antenna Height AAT (mete Transmitting ERP (watts)	RP in Watts: 140.82 th) 0 rs) 105.5 0.460	0 45 00 102.800 0.460	90 96.700 0.460	135 89.300 0.460	180 75.700 0.460	225 68.400 7.710	270 107.900 45.610	315 107.300 24.600
Location Latitude	Longitude	G	round Elev	ation St	ructure Hg	t to Tip	Antenna St	ructure
17 37-10-55 4 N	088-56-437	(n w 10	neters)	(II) 00	leters)		Registratio	n No.
Address: (Monkey's Eyeb City: Kevil County: BA	row) 4625 Odgen (ALLARD State:	Colvin Circle KY Constr	ruction Dea	dline: 10-	24-2014		1252015	
					-			
Antenna: 1 Maximum Transmitting ER Azimuth(from true nor Antenna Height AAT (mete Transmitting ERP (watts) Antenna: 2	RP in Watts: 140.82 th) 0 rs) 85.90 7.080	0 45 0 83.500 125.890	90 90.600 478.630	135 69.600 112.200	180 74.300 4.570	225 84.600 1.580	270 86.500 1.000	315 83.200 1.000
Maximum Transmitting ER Azimuth(from true non Antenna Height AAT (mete Transmitting ERP (watts)	Image: Watts: 140.82 th) 0 rs) 85.90 1.000	0 45 0 83.500 1.410	90 90.600 12.020	135 69.600 213.800	180 74.300 446.680	225 84.600 64.570	270 86.500 2.820	315 83.200 1.000

.....

Call Sign: KNKQ306	File Number:				Print Date:				
Location Latitude	Longitude		Ground Elevation (meters)		Structure Hgt to Tip (meters)		Antenna Structure Registration No.		
17 37-10-55.4 N	088-56-43.7 W	102.7			99.1		1252613		
Address: (Monkey's Eyebrow) 4625 Odgen Colv	in Circle							
City: Kevil County: BALL	ARD State: KY	Const	ruction De	adline:	10-24-2014				
Antenna: 4 Maximum Transmitting ERP in Azimuth(from true north) Antenna Height AAT (meters) Transmitting ERP (watts)	Watts: 140,820 0 85.900 2.000	45 83.500 2.000	90 90.600 2.000	135 69.60 2.000	180 0 74.300 2.000	225 84.600 398.110	270 86.500 549.540	315 83.200 4.900	
Control Points:									
Control Pt. No. 3									
Address: 500 W. Dove Rd.		6							
City: Southlake County: T	ARRANT State	TX T	[[] Telephone	Numbe	r: (800)264-66	20			

Waivers/Conditions:

License renewal granted on a conditional basis, subject to the outcome of FCC proceeding WT Docket No. 10-112 (see FCC 10-86, paras. 113 and 126).



GPD GROUP, INC 520 South Main Street, Suite 253: Akron, OH 4431 330.572.2100 Fax: 330.572.2102

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2	DESCRIPTION				
	PROJECT INFORMATION, SITE MAPS, SHEET INDEX		SCR 0% R VCE LOUIS		
	SURVEY PLAN		DE3 STAT		
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	TOWER ELEVATION		De Reise		
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	(REFERENCE ONLY)	2.5			
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	OVP & INTEGRATED LOAD CENTER H-FRAME ELEVATIONS AND DETAILS		ISSUED FOR:		-
	EQUIPMENT CABINET ELEVATIONS		REVIEW		-
	EQUIPMENT PAD LIGHTING PLAN				_
			PERMIT		
			CONSTRUCTION	ON	-
	GROUNDING SITE PLAN		DECODD		_
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	GROUNDING DETAILS		PROJECT MANAGE		7
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SURVEYOR CERTIFICATE

TO: PIKE LEGAL GROUP AMERICAN TITLE COMPANY, INC FILE NO. CT-566 COMMITMENT DATE: NOVEMBER 27, 2017 @8:00 AM SITE NAME: EV BARLOW "STEVE DENTON ROAD" GRANTEE: THE MYATT FAMILY TRUST DATE: NOVEMBER 16, 2016 GRANTOR: CHARLES MYATT AND DEENA MYATT, HUSBAND & WIFE, AND CHARLES MYATT AS EXECUTOR OF THE ESTATE OF JIMMY MYATT BOOK/PAGE: 112, 227

SCHEDULE B-SECTION II

I CERTIFY THAT THIS PLAT AND SURVEY WERE MADE UNDER MY SUPERVISION, AND THAT THE ANGULAR AND LINEAR MEASUREMENTS, AS WHITNESSED BY MONUMENTS SHOWN HEREON, ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

THIS SURVEY AND PLAT MEETS OR EXCEEDS THE MINIMUM STANDARDS OF THE GOVERNING AUTHORITIES.

SURVEYOR STATEMENT-MY COMMENTS ARE BASED SOLELY ON THE TITLE DOCUMENT THAT HAVE BEEN SUPPIED TO ME BY THE TITLE COMPANY. SINCE THE TITLE DOCUMENTS ARE FURNISHED FOR THE PARENT TRACT, OUR TOPOGRAPHIC SURVEY IS OF A PORTION OF THAT TRACT. MY COMMENTS ARE RESTRICTED TO EXCLUSIONS THAT I CAN DETERMINE AFFECT ONLY OUR PORTION OF THE PARENT TRACT, NO BOUNDARY SURVEY WAS PERFORMED ON THE PARENT TRACT, THUS IT IS NOT POSSIBLE TO DETERMINE WITH CERTAINTY EXCLUSIONS REFERENCING THE PARENT TRACT.

SCHEDULE "B" SECTION II EXCEPTIONS

ITEM 1 -NOT A SURVEYOR RELATED ITEM. ITEM 2 -NOT A SURVEYOR RELATED ITEM. ITEM 3 -NOT A SURVEYOR RELATED ITEM.

Relel - will

END OF SCHEDULE B-II RALPH M. WALLEM INDIANA LAND SURVEYOR EFFECTIVE DATE: NOVEMBER 10, 2015





LEASE AREA DESCRIPTION

A PART OF A 69 ACRE PARCEL OF LAND OWNED BY THE MYATT FAMILY TRUST AS RECORDED IN DEED BOOK 112, PAGE 227, AND LYING SOUTHWEST OF THE INTERSECTION OF SALIE CIRCLE ROAD AND STEVE DENTON ROAD, BALLARD COUNTY, KENTUCKY.

COMMENCING AT AN IRON PIN FOUND AT THE INTERSECTION OF SAID SALIE CIRCLE ROAD AND STEVE DENTON ROAD, SAID POINT ALSO BEING THE NORTHEAST CORNER OF A PARCEL OF LAND OWNED BY DAVID L. JONES AS RECORDED IN DEED BOOK 93, PAGE 150, THENCE ALONG THE CENTERLINE OF SALIE CIRCLE ROAD NORTH 68 DEGREES 26 MINUTES 09 SECONDS WEST 172.37 FEET; THENCE SOUTH 71 DEGREES 49 MINUTES 20 SECONDS WEST 78.70 FEET; THENCE NORTH 76 DEGREES 06 MINUTES 17 SECONDS WEST 128.76 FEET; THENCE SOUTH 32 DEGREES 16 MINUTES 40 SECONDS WEST 132.35 FEET; THENCE NORTH 57 DEGREES 43 MINUTES 20 SECONDS WEST 30.00 FEET TO THE SOUTHERNMOST LEASE CORNER AND BEING THE TRUE PLACE OF BEGINNING OF THIS LEASE AREA DESCRIPTION; THENCE NORTH 57 DEGREES 43 MINUTES 20 SECONDS WEST 100.00 FEET; THENCE NORTH 32 DEGREES 16 MINUTES 40 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 20 SECONDS EAST 100.00 FEET; THENCE SOUTH 32 DEGREES 16 MINUTES 40 SECONDS WEST 100.00 FEET TO THE TRUE PLACE OF BEGINNING AND CONTAINING 10,000 SQUARE FEET, (0.23 ACRES), MORE OR LESS.

30' ACCESS & UTILITY EASEMENT DESCRIPTION

A PART OF A 59 ACRE PARCEL OF LAND OWNED BY THE MYATT FAMILY TRUST AS RECORDED IN DEED BOOK 112, PAGE 227, AND LYING SOUTHWEST OF THE INTERSECTION OF SALIE CIRCLE ROAD AND STEVE DENTON ROAD, BALLARD COUNTY, KENTUCKY.

COMMENCING AT AN IRON PIN FOUND AT THE INTERSECTION OF SAID SALIE CIRCLE ROAD AND STEVE DENTON ROAD, SAID POINT ALSO BEING THE NORTHEAST CORNER OF A PARCEL OF LAND OWNED BY DAVID L. JONES AS RECORDED IN DEED BOOK 93, PAGE 150, THENCE ALONG THE CENTERLINE OF SALIE CIRCLE ROAD NORTH 68 DECREES 26 MINUTES 09 SECONDS WEST 172.37 FEET TO THE TRUE PLACE OF BEGINNING OF THIS ACCESS AND EASEMENT DESCRIPTION; THENCE SOUTH 71 DEGREES 49 MINUTES 20 SECONDS WEST 78,70 FEET; THENCE NORTH 76 DEGREES 06 MINUTES 17 SECONDS WEST 128.76 FEET; THENCE SOUTH 32 DEGREES 16 MINUTES 40 SECONDS WEST 132.35 FEET; THENCE NORTH 57 DEGREES 43 MINUTES 20 SECONDS WEST 30.00 FEET TO THE SOUTHERNMOST LEASE CORNER: THENCE NORTH 32 DEGREES 16 MINUTES 40 SECONDS EAST 143.46 FEET; THENCE SOUTH 76 DEGREES D6 MINUTES 17 SECONDS EAST 141.78 FEET; THENCE NORTH OO DEGREES OD MINUTES OD SECONDS EAST 23.37 FEET; THENCE SOUTH 68 DEGREES 26 MINUTES 09 SECONDS EAST 81.66 FEET TO THE TRUE PLACE OF BEGINNING AND CONTAINING 10,105.1 SQUARE FEET, (0.23 ACRES), MORE OR LESS.

VERIZON LEASE AREA DESCRIPTION

A PART OF A 69 ACRE PARCEL OF LAND OWNED BY THE MYATT FAMILY TRUST AS RECORDED IN DEED BOOK 112, PAGE 227, AND LYING SOUTHWEST OF THE INTERSECTION OF SALIE CIRCLE ROAD AND STEVE DENTON ROAD, BALLARD COUNTY, KENTUCKY.

COMMENCING AT AN IRON PIN FOUND AT THE INTERSECTION OF SAID SALIE CIRCLE ROAD AND STEVE DENTON ROAD, SAID POINT ALSO BEING THE NORTHEAST CORNER OF A PARCEL OF LAND OWNED BY DAVID L. JONES AS RECORDED IN DEED BOOK 93, PAGE 150, THENCE ALONG THE CENTERLINE OF SALIE CIRCLE ROAD NORTH 68 DEGREES 26 MINUTES 09 SECONDS WEST 172.37 FEET; THENCE SOUTH 71 DEGREES 49 MINUTES 20 SECONDS WEST 78.70 FEET; THENCE NORTH 76 DEGREES 06 MINUTES 17 SECONDS WEST 128.76 FEET; THENCE SOUTH 32 DEGREES 16 MINUTES 40 SECONDS WEST 132.35 FEET; THENCE NORTH 57 DEGREES 43 MINUTES 2D SECONDS WEST 30.00 FEET TO THE SOUTHERNMOST LEASE CORNER; THENCE NORTH 37 DEGREES 57 MINUTES 19 SECONDS WEST 31.94 FEET TO THE TRUE PLACE OF BEGINNING OF THIS VERIZON LEASE AREA; THENCE NORTH 56 DEGREES 46 MINUTES 12 SECONDS WEST 30,00 FEET; THENCE NORTH 33 DEGREES 13 MINUTES 48 SECONDS EAST 12.00 FEET; THENCE SOUTH 56 DEGREES 46 MINUTES 12 SECONDS EAST 30.00 FEET; THENCE SOUTH 33 DEGREES 13 MINUTES 48 SECONDS WEST 12.00 FEET AND CONTAINING 360 SQUARE FEET.



NOTE: THIS DRAWING DOES NOT REPRESENT A BOUNDARY SURVEY.

PARENT PARCEL DESCRIPTION

TRACT NO 1: BEGINNING AT A STAKE IN THE CORNER OF THE HULDA CRICE TRACT WITH DOUBLE WHITE OAK, SOUTH 41 DEG. WEST 13 LINKS, BLACK OAK NORTH 18/2 DEG. EAST 30 LINKS: THENCE NORTH 71 DEG. WEST 58 POLES TO A STAKE IN TERRELL'S LINE; THENCE NORTH 181/2 DEG. EAST WITH TERRELL LINE 127 POLS TO A STAKE IN THE CENTER OF THE TERRELL LANDING ROAD, SASSAFRAS SOUTH 46 DEG. WEST 1 POLE AND LARGE WHITE OAK NORTH 18 DEG. EAST 2 POLES AND B7 LINKS; THENCE SOUTH 84 DEG. EAST WITH THE CENTER OF SAID ROAD 84 POLES TO A STAKE IN HULDA CRICE'S LINE; THENCE SOUTH 28 DEG. WEST 134 POLES TO THE BEGINNING, CONTAINING 56 ACRES AND 34 POLES, MORE OR LESS, SUBJECT TO LEGAL HIGHWAYS.

EXCEPTION NO. 1: LESS AND EXCEPT HOWEVER, A ONE ACRE TRACT OF LAND CONVEYED BY WILLIAM B. WATSON TO JIMMIE MYATT BY DEED DATED JANUARY 27, 1960, DEED BOOK 64, PAGE 250 AND DESCRIBED AS FOLLOWS: BEGINNING AT A STAKE AT THE SOUTHEAST CORNER OF A 56 ACRE TRACT OF LAND, SAID STAKE BEING THE CENTER OF AND AT A TURN OF A COUNTY GRAVEL ROAD KNOWN AS STEVE DENTON ROAD; THENCE NORTH 71 DEG. WEST 210 FEET TO A STAKE IN THE CENTER OF SAID ROAD; THENCE NORTH 181/2 DEG. WEST 210 FEET WITH THE CENTER OF STEVE DENTON ROAD TO THE POINT OF BEGINNING, CONTAINING ONE (1) ACRE, AND BEING ONE ACRE OUT OF THE SOUTHEAST CORNER OF THE 56 ACRE TRACT.

EXCEPTION NO. 2; LESS AND EXCEPT HOWEVER, A ONE AND ONE-HALF AGRE TRACT OF LAND CONVEYED BY BILLIE EVELYN EWING AND HUSBAND, BILL N. EWING TO DAVID JONES BY DEED DATED APRIL 19, 1984 OF RECORD IN MICROFILM CABINET 1. DRAWER 9, CARD NO.___ ____AND DESCRIBED AS FOLLOWS: BEGINNING AT A POINT IN THE CENTER OF THE TERRELL LANDING COUNTY ROAD, SAID POINT BEING THE CENTER OF THE DENTON COUNTY ROAD SOUTH 30 DEG. WEST 413 FEET TO A NEW CORNER IN THE HENRY ADAMS TRACT OF LAND; THENCE ON A NEW LINE WITH THE ADAMS LAND NORTH 80 DEG. WEST 160 FEET; THENCE NORTH 30 DEG. EAST 413 FEET TO A POINT IN THE CENTER OF TERRELL LANDING COUNTY GRAVEL ROAD; THENCE WITH THE CENTER OF SAID ROAD SOUTH 80 DEC. EAST 160 FEET TO THE POINT OF BEGINNING AND CONTAINING ONE AND ONE HALF ACRES, MORE OR LESS, ACCORDING TO A SURVEY MADE APRIL 13, 1983 BY B. ALLIE, SURVEYOR, WICKLIFFE, KENTUCKY.

TRACT NO. 2: BEGINNING AT A BLACK OAK ON THE BANK OF A RAVINE, IN TERRELL'S LINE; THENCE NORTH 18 DEG. EAST 5% POLES TO A STAKE IN THE CENTER OF TERRELL LANDING ROAD, SASSAFRAS AND WHITE OAK POINTERS; THENCE NORTH 65 DEG. WEST 33 POLES TO A STAKE IN THE RAVINE; THENCE SOUTH 82 DEG. WEST 20 POLES TO THE INTERSECTION OF THE TERRELL LINE; THENCE WITH TERREL'S LINE IN A SOUTHERN DIRECTION TO THE BEGINNING AND CONTAINING 2% ACRES, MORE OR LESS, SUBJECT TO LEGAL HIGHWAYS.

TRACT NO. 3: BEGINNING AT A STAKE IN THE NORTHEAST CORNER OF THE ORIGINAL 191% ACRE TRACT OF LAND OWNED BY J.H. CRICE; THENCE SOUTH 20 DEG, WEST 132% POLES; THENCE NORTH 71 DEG. WEST 27.17 POLES TO A STAKE, A NEW CORNER; THENCE NORTH 20 DEG. EAST 132% POLES TO A STAKE IN THE NORTH LINE OF THE ORIGINAL J.H. CRICE TRACT OF LAND; THENCE SOUTH 72 DEG. EAST 27.17 POLES TO THE BEGINNING, CONTAINING 22% ACRES, MORE OR LESS, BY ACTUAL SURVEY MADE BY D.W. JUDD, MCCRACKEN COUNTY SURVEYOR, IN YEAR 1938, BUT SUBJECT TO ALL LEGAL HIGHWAYS.

LESS AND EXCEPT HOWEVER, A TWO ACRE TRACT OF LAND CONVEYED BY JESSIE C. HOWLE AND HUSBAND, A.K. HOWLE TO JESSIE ROSS BY DEED DATED MAY 7, 1945, DEED BOOK 51, PAGE 212, AND DESCRIBED AS FOLLOWS: BEGINNING AT A STAKE IN THE ROAD AT THE SOUTHEAST CORNER OF THE 221/2 ACRE TRACT; THENCE NORTH 71 DEG. WEST 450 FEET TO THE SOUTHWEST CORNER OF THE 22% ACRE TRACT; THENCE NORTH 20 DEG. EAST APPROXIMATELY 195 FEET TO A STAKE, A NEW CORNER, THENCE SOUTH 71 DEG. EAST 450 FEET TO A STAKE, THE SOUTHWEST CORNER OF THE 56 AGRE TRACT; THENCE SOUTH 20 DEG. WEST APPROXIMATELY 195 FEET TO THE PLACE OF BEGINNING, CONTAINING TWO (2) ACRES, MORE OR LESS, SUBJECT TO ALL LEGAL HIGHWAYS.

BEING IN ALL RESPECTS THE SAME PROPERTY CONVEYED TO JIMMIE MYATT AND WIFE, ETHEL MYATT, S ONE-HALF (1/2) INTEREST, AND CHARLES MYATT AND WIFE, DEENA MYATT, A ONE-HALF (1/2) INTEREST, BY DEED DATED APRIL 19, 1984, RECORDED IN CABINET 1, DRAWER 9, CARD 16, 1128 BALLARD COUNTY CLERK'S OFFICE, ETHEL MYATT PREDECEASED JIMMIE MYATT, UPON HER DEATH THIER. ONE-HALF (%) IN THE PROPERTY FULLY VESTED IN JIMMIE MYATT PURSUANT TO THE SURVIVORSHIP CLAUSE STATED IN THE AFORESAID DEED, JIMMIE MYATT IS NOW DESEASED. PURSUANT TO THE TERMS OF THE LAST WILL AND TESTAMENT OF JIMMIE MYATT RECORDED IN WILL BOOK 6, PAGE 71, CHARLES MYATT IS APPOINTED AS EXECUTOR WITH THE POWER TO TRANSFER REAL ESTATE,

TO HAVE AND TO HOLD THE ABOVE PREMISES. TOGETHER WITH ALL APPURTENANCES THEREUNTO BELONGING, TO THE GRANTEE, ITS SUCCESSORS AND ASSIGNS FOREVER.

PARENT PARCEL DEED DESCRIPTION FURNISHED TO BENCHMARK SERVICES, INC. NO BOUNDARY SURVEY WAS PERFORMED TO CREATE THIS LEASE/ACCESS AREA.

verizon Wireless 1961 NORTHPOINT BLVD. SUITE 130 HIX50N, TN 37343 BENCHMARK SERVICES, INC. Consulting Engineers Land Surveyory 338 North Main Street (810) 663.3049 benchmarlaßnew twobic.tam PROJECT MUMBER: 20161506655 SITE NAME: EV BARLOW SITE ADDRESS: 2557 STEVE DENTON RD BARLOW, KY 42024 LEASE AREA: 10000 SQ. FT. PROPERTY OWNER: MYATT FAMILY TRUST APLES MYATT & DISING MYATT, TRUSTER 2244 STEVE DENTON ROAD BAFILOW, KENTUCKY 42024 TAX PARCEL ID: 24-30 COUNTY: BALLARD COUNTY SOURCE OF TITLE: DEED BK 112, PG 227 LATITUDE: 37º 06' 42.146' N LONGITUDE: 89° 02' 44.583' W CHKD BY: DATE: DWG BY: GVW RMW 12.22.17 NO. REVISION/ASSUE DATE: ACID TITLE ACID VIRIEDNI LIIAAM ACID PARINT PARINE 1. 8.28.18 2. 1000 20M NOTE 1.15.19 INTE: SURVEY PLAN SHEET: 3 OF 3

					2		3	4
	1	KEVI	SION LOG					
		<u>REV</u> * A B C	MM/DD/YY 10/31/18 01/22/19 01/24/19 02/08/19	SHEET NUMBER & NAME ISSUED FOR REVIEW ADDED DISTANCE TO HOUSE CALLED OUT ALL BUILDINGS ON OVERALL PLANS REVISED TITLE OF SHEET C-1B TO "TOWER DISTANCE REVISED SHEET INDEX TO REFLECT SHEET C-1B TITLE FINAL CONSTRUCTION DRAWINGS	TO PROPERTY LINES AND RESIDENTIAL STRUC CHANGE	ICTURES"		
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GENERAL SITE CONSTRUCTION NOTES

1. SCHEDULE:

THE CONSTRUCTION OF THE SITE REQUIRES A CERTAIN SEQUENCE OF EVENTS TO MINIMIZE CONSTRUCTION TIME REQUIRED UNTIL AVAILABILITY OF CELLULAR TELEPHONE SERVICE. VERIFY SCHEDULE DURING BID WALK.

2. TOWER OWNER REPRESENTATIVE:

- VERIZON WIRELESS 2421 HOLLOWAY ROAD LOUISVILLE, KY 40299 CONTACT: AMY HARPER MOBILE: 502-520-330 E-MAIL: amy.harper@verizonwireless.com
- MYATT FAMILY TRUST 2244 STEVE DENTON ROAD, BARLOW, KY 42024 CONTACT: JIMMIE H. MYATT PHONE: 270-334-3616 E-MAIL: TBD

PROPERTY OWNER REPRESENTATIVE:

3. ANTENNA INSTALLATION:

THE VERIZON WIRELESS CONTRACTOR INSTALLING THE PLATFORM SHALL ENSURE THAT THE PLATFORM IS ALIGNED BASED ON THE VERIZON WIRELESS RF PLAN. DURING THIS WORK, THE GENERAL CONTRACTOR SHALL BE LIMITED TO THE WORK WHICH CAN BE PERFORMED OUTSIDE THE VICINITY OF THE TOWER.

4. ACCESS DRIVEWAY

THE GENERAL CONTRACTOR SHALL CONSTRUCT THE ACCESS DRIVE PER THE ALIGNMENT AS SHOWN ON THESE DRAWINGS. IF ANY AREA OF CONSTRUCTION REQUIRES ADDITIONAL FILL OF AGGREGATE STONE THAN SHOWN ON THE DRAWINGS, THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER FOR APPROVAL OF THE SIZE, TYPE AND QUANTITY OF STONE/FILL NECESSARY AND SHALL NOT BE USED UNTIL APPROVAL BY THE ENGINEER. THE GENERAL CONTRACTOR SHALL ALSO OBTAIN DRIVE PERMIT FOR NEW ACCESS DRIVE.

5. PREFABRICATED EQUIPMENT CANOPY AND GENERATOR:

THIS VERIZON WIRELESS CONTRACTOR SHALL PROVIDE ALL LABOR, EQUIPMENT AND MATERIALS FOR THE PROPER LIFTING, TRANSPORTING AND ASSEMBLY OF THE PREFABRICATED EQUIPMENT CANOPY FROM THE TRANSPORT TRUCK BED TO THE FINAL POSITION ON THE CONCRETE FOUNDATION. THE GENERATOR SHALL BE LIFTED INTO PLACE BY USING TWO SPREADER BAR ASSEMBLIES. EACH SPREADER BAR SHALL BE A MINIMUM 3' WIDE (RATED TO CARRY 4 TONS). THE GENERATOR WIGHT IS 3,000# WITH TANK PEDESTAL VERIZON WIRELESS CONTRACTOR SHALL ANCHOR THE GENERATOR BASE TO THE CONCRETE SLAB USING ANCHOR BOLTS. EXTREME CAUTION SHALL BE TAKEN IN THE INSTALLATION OF THE PLATFORM TO AVOID CONTACT WITH EXISTING OVERHEAD UTILITY LINES.

THE VERIZON WIRELESS CONTRACTOR IS RESPONSIBLE FOR ATTACHING, SECURING OR ASSEMBLING ANY ACCESSORY OR LOOSE ITEMS THAT ARE SHIPPED WITH THE PREFABRICATED EQUIPMENT CANOPY AND GENERATOR AND SHALL INCLUDE THIS WORK IN THE PLATFORM INSTALLATION PORTION OF THE BID.

6. UTILITIES:

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE EXCAVATION AND PROPER BACKFILLING OF TRENCHES AND SUPPLY CONDUIT REQUIRED FOR UNDERGROUND TELEPHONE & ELECTRICAL UTILITIES. ALL TRENCHING SHALL BE COMPACTED TO 95% MAXIMUM DRY DENSITY IN ACCORDANCE WIASTM D-1557.

THE CONTRACTOR SHALL RUN AN ELECTRICAL TRENCH WITH 2 1/2" CONDUIT AND DETECTABLE PULL TAPE FROM THE NEW RADIO EQUIPMENT PAD AND END AT THE NEW METER BOARD STRUCTURE. THE CONTRACTOR SHALL THEN RUN (2) TWO 4" CONDUIT WITH PULL TAPE FROM THE METER BOARD TO THE NEW PAD MOUNTED TRANSFORMER LOCATION AND STUB UP 6" ABOVE GRADE. THE CONTRACTOR WILL THEN RUN (1) 2 1/2" UNDERGROUND CONDUIT FROM THE NEW TRANSFORMER TO THE NEW UTILITY POLE, CONTACT TOM DILLWORTH @ 270-442-7321, WITH JACKSON PURCHASE ENERGY BEFORE BEGINNING CONSTRUCTION TO VERIFY LOCATION OF CONDUIT AND TRANSFORMER. CONTRACTOR WHILL AND FAME TO THE NEW UTILITY POLE, CONTACT TOM DILLWORTH @ 270-442-7321, WITH JACKSON PURCHASE ENERGY BEFORE BEGINNING CONSTRUCTION TO VERIFY LOCATION OF CONDUIT AND TRANSFORMER. CONTRACT TOM DILLWORTH @ 270-442-7321, WITH JACKSON PURCHASE ENERGY BEFORE BEGINNING CONSTRUCTION TO VERIFY LOCATION OF CONDUIT AND TRANSFORMER. CONTRACT TOM DILLWORTH @ 270-442-7321, WITH JACKSON PURCHASE ENERGY TO SETUP AN INSPECTION OF THE LECTRIC PROVIDER TO HAVE TARMINUM DEPTH OF 36". CONTRACTOR SHALL CONTACT LECTRIC PROVIDER TO HAVE TRENCH AND CONDUIT INSPECTED BEFORE THE BOARD STRUCTURE. THE CONTRACTOR SHALL CONTACT JACKSON PURCHASE ENERGY TO SETUP AN INSPECTION OF THE TRENCHES AFTER INSPECTION AND SETTLEMENT. CONTACT SHALL CONTACT DECTRIC PROVIDER THALE TO FOUR WEEKS PRIOR TO CONSTRUCTION FOR NEW SERVICE AND COORDINATION OF ACCESS TO SITE. THE CONTRACTOR SHALL CONTACT JACKSON PURCHASE ENERGY TO SETUE. THE CONTRACT CONTACT SUBCE AND COORDINATION OF ACCESS TO SITE. THE CONTRACTOR SHALL CONTACT JACKSON PURCHASE ENERGY TO SETLECTRIC PROVIDER THREE TO FOUR WEEKS PRIOR TO CONSTRUCTION FOR NEW SERVICE AND COORDINATION OF ACCESS TO SITE. THE CONTRACTOR SHALL CONTACT JACKSON PURCHASE ENERGY FOR A COPY OF THEIR SPECIFICATIONS BEFORE CONSTRUCTION

THE ELECTRICAL CONTRACTOR SHALL RUN ELECTRICAL CONDUCTORS FROM NEW METER CENTER TO NEW VZW INTEGRATED LOAD CENTER WITHIN TOWER OWNER INSTALLED 2 1/2" CONDUIT. CONTACT JACKSON PURCHASE ENERGY TO HAVE NEW METER INSTALLED.

THE VERIZON WIRELESS GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE ORDERING AND COORDINATING THE DELIVERY OF A FIBER OPTIC SERVICE LINE TO THE RADIO EQUIPMENT ENCLOSURE. COORDINATE WITH THE FIBER PROVIDER AND THE VERIZON WIRELESS FACILITY ENGINEER.

1. THE CONTRACTOR SHALL RUN IN THE FIBER OPTIC TRENCH (1) "VERIZON WIRELESS ONLY" 4" PVC CONDUIT WITH PULL TAPE AND TRACER WIRE FROM THE NEW RADIO EQUIPMENT PAD TO THE NEW HAND HOLE OUTSIDE COMPOUND WITHIN EASEMENT, VERIZON WIRELESS GENERAL CONTRACTOR WILL COORDINATE WITH FIBER PROVIDER (TBD BY ZW) TO VERIFY EXACT LOCATION OF CONDUIT AND HAND HOLE BEFORE CONSTRUCTION STARTS. WILL THEN COORDINATE WITH FIBER PROVIDER THE FIBER PROVIDER WILL PROVIDE SERVICE TO THE TELCCONTRACTOR FOR THIS INSTALLATION. THE FIBER PROVIDER WILL PROVIDE SERVICE TO THE TELCCONTRACTOR FOR THIS INSTALLATION.

ACK IN VERIZON WIRELESS EQUIPMENT CABINET. 2. THE CONTRACTOR SHALL RUN IN THE FIBER OPTIC TRENCH (1) "VERIZON WIRELESS ONLY" 4" PVC CONDUIT WITH PULL TAPE FROM THE NEW HAND HOLE LOCATED OUTSIDE COMPOUND TO THE FUTURE FIBER PEDESTAL LOCATION AND STUB-UP. THE CONTRACTOR SHALL RUN 4" PVC CONDUIT WITH PULL TAPE FROM "VERIZON WIRELESS ONLY" HAND HOLE OUTSIDE COMPOUND TO NEW 36*x60" HAND HOLE AT RIGHT OF WAY. THE CONTRACTOR SHALL RUNATE WITH THE VERIZON WIRELESS CONSTRUCTION MANAGER AND THE FIBER SERVICE PROVIDER ON THE FUTURE FIBER PEDESTAL LOCATION. THE CONTRACTOR SHALL PROPERLY BACKFILL THE TRENCHES AFTER SETTLEMENT. CONTACT FIBER PROVIDER (TBD BY VERIZON WIRELESS) FOR APPOINTMENT TO VISIT THE SITE, FOUR TO SIX WEEKS PRIOR TO CONSTRUCTION FOR NEW SERVICE.

ELECTRIC SERVICE PROVIDED BY:

JACKSON PURCHASE ENERGY ADDRESS: 2900 IRVIN COBB ROAD PADUCAH, KY 42002 CONTACT: TOM DILLWORTH PHONE: 270-556-0112 EMAIL: tom.dillworth@JPEnergy.com

FIBER OPTIC SERVICE PROVIDED BY

FIBER PROVIDER TO BE DETERMINED BY VERIZON WIRELESS

Z. SITE GRADING

A UNIFORMLY GRADE AREA TO BE SMOOTH SURFACE FREE FROM IRREGULAR SURFACE CHANGES. COMPLY WITH COMPACTION REQUIREMENTS AND GRADE TO CROSS SECTION TOPO LINES AND ELEVATIONS INDICATED.

1. COMPOUND SURFACE GRADES ARE TO BE SLOPED TO DIRECT WATER AWAY FROM EQUIPMENT PAD AND TOWER TO PREVENT STANDING AND PONDING WATER.

2. COMPOUND SURFACE SHALL BE COMPACTED TO A 95% MAXIMUM DRY DENSITY TO ALLOW PROPER STERILIZATION FOR ACCESS TO ALL CUSTOMERS DENSITY TESTING MAY BE REQUIRED AT VERIZON WIRELESS' DISCRETION DUE TO QUESTIONABLE COMPACTION OF FINISH SURFACE GRADE OR SUB-GRADE.

3. DITCHES/SWALES AROUND THE COMPOUND AREA AND ALONG ACCESS ROAD SHALL BE CONSTRUCTED SO TO PROVIDE A SMOOTH TRANSITION BETWEEN EXISTING GRADES/SLOPE AND NEW PROPOSED GRADES.

4. SITE GRADING AND DRAINAGE SHOULD BE CONSTRUCTED TO PREVENT WATER FROM ENTERING THE COMPOUND SURFACE OR THE ACCESS ROAD SUB-GRADE.

B. <u>MOISTURE CONTROL</u> - UNIFORMLY MOISTEN OR AERATE SUB-GRADE AND EACH SUBSEQUENT FILL OR BACK FILL LAYER BEFORE COMPACTION TO WITHIN 90% OF OPTIMUM MOISTURE CONTENT. DO NOT PLACE BACK FILL OR FILL MATERIAL ON SURFACES THAT ARE MUDDY, FROZEN OR CONTAIN FROST, SNOW OR ICE.

C. <u>STOCKPILING MATERIAL (TOP SOIL OR FILL DIRT)</u> - SHOULD BE PLACED IN AN AREA THAT CAN BE CONTROLLED TO PREVENT WATER, SNOW, OR ICE FROM EFFECTING MOISTURE CONTENT. STOCKPILES MAY HAVE TO BE COVERED TO PREVENT ADDITIONAL MOISTURE FROM ACCUMULATING SO ACCEPTABLE FILL CAN BE USED.

D. <u>DE-WATERING</u> - PREVENT SURFACE WATER AND SUBSURFACE OR GROUND WATER FROM ENTERING EXCAVATIONS, FROM PONDING ON PREPARED SUB-GRADE, AND FROM FLOODING PROJECT OR BUILD AREA.

E. <u>EROSION CONTROL</u> - MEASURES SHALL BE MAINTAINED THROUGHOUT THE DURATION OF THE PROJECT.

ALL INLETS, DRAINS, PIPES, SWELLS, AND ROADS SHALL BE KEPT CLEAN AND FREE OF DIRT AND SILT.

F. GEOTEXTILE FABRIC - AFTER PLACEMENT AND COMPACTION OF FILL WITHIN THE WORK AREA AND BEFORE THE PLACEMENT OF LIMESTONE AGGREGATE, (SEE SITE AREA SURFACING DETAIL ON D-1), THE ENTIRE DISTURBED WORK AREA SHALL BE COVERED WITH A GEOTEXTILE FABRIC. THIS FABRIC SHALL BE "TENCATE (MIRAFI 500X) WOVEN ENGINEERING FABRIC" INSTALLED ACCORDING TO THE MANUFACTURERS RECOMMENDATIONS.

8. TRASH REMOVAL & SANITATION:

THE GENERAL CONTRACTOR AND VERIZON WIRELESS GENERAL CONTRACTOR SHALL REMOVE ALL TRASH AS CREATED BY HIMSELF AND HIS SUBCONTRACTORS. TRASH SHALL BE REMOVED FROM THE SITE IN A TIMELY FASHION TO A LEGAL DISPOSAL AREA. THE GENERAL CONTRACTOR SHALL ALSO REMOVE ALL TRASH CREATED BY OTHER CONTRACTORS INCLUDING CABLE REELS, CARDBOARD BOXES AND PACKING. NO BURNING OR BURYING OF TRASH IS PERMITTED.

THE GENERAL CONTRACTOR SHALL PROVIDE AND MAINTAIN A PORTABLE TOILET FOR THE DURATION OF THE CONSTRUCTION PROJECT.

9. TOWER:

A. THE GENERAL CONTRACTOR SHALL VERIFY THE EQUIPMENT PAD FOUNDATION IS LOCATED CORRECTLY WITH RESPECT TO THE TOWER FOUNDATION. THE CONTRACTOR MUST NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES PRIOR TO POURING CONCRETE.

B. TOWER & FOUNDATION DESIGN ARE BY OTHERS FOR TOWER OWNER. PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL VERIFY IN WRITING FROM TOWER OWNER THAT THE TOWER IS STRUCTURALLY SUFFICIENT TO SUPPORT ALL LOADINGS AS OUTLINED IN THESE DOCUMENTS. TOWER AND FOUNDATION DESIGN SHALL BE PERFORMED BY A LICENSED ENGINEER.

C. THE GENERAL CONTRACTOR SHALL VERIFY THE TOP OF FOUNDATION MATCHES THE FAA APPROVAL LETTER.

10. EXCAVATION OF UTILITIES:

A FIELD VERIFY THE LOCATION OF ANY EXISTING UNDERGROUND UTILITIES PRIOR TO EXCAVATING IN THE VICINITY OF THE SITE. ALL EXCAVATIONS SHALL BE MADE BY HAND OVER OR UNDER OR IMMEDIATELY ADJACENT TO ANY EXISTING UTILITIES & GROUNDING.

B. ALL UTILITY COMPANIES SHALL BE NOTIFIED AT LEAST 48 HOURS PRIOR TO ANY CONSTRUCTION ON THIS PROJECT. CONTACT UNDERGROUND UTILITY PROTECTION SERVICE BEFORE YOU DIG AT 1-800-382-5544 OR 811.

C. EXISTING UTILITIES ARE SHOWN FROM THE SURVEY AND ARE NOT NECESSARILY TOMPLETE AND ACCURATE. THE CONTRACTOR SHALL BE RESPONSIBLE TO LOCATE, EXPOSE AND DETERMINE IF CONFLICTS EXIST WITH THE NEW IMPROVEMENTS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER (A&E FIRM) IN ORDER TO RESOLVE ANY CONFLICTS.

11. CONTRACTOR'S LICENSE

THE GENERAL CONTRACTOR, VERIZON WIRELESS GENERAL CONTRACTOR AND ALL OF THEIR SUBCONTRACTORS THAT DO ANY WORK ON THIS PROJECT SHALL BE CURRENTLY LICENSED TO PERFORM WORK IN THE LOCATION OF THIS SITE. PROOF OF LICENSES SHALL BE SUPPLIED TO VERIZON WIRELESS PRIOR TO THE COMMENCEMENT OF ANY WORK.

12. SEEDING

ALL DISTURBED AREAS SHALL BE REPAIRED AND SEEDED BY THE GENERAL CONTRACTOR, UNLESS OTHERWISE NOTED. SEED DISTURBED AREAS W/4 POUNDS/1000 SQ. FT. - 60% KENTUCKY BLUEGRASS, 18% CREEPING RED FESCUE, 22% ANNUAL RYEGRASS.

13. TRAFFIC CONTROL:

THE CONTRACTOR SHALL FURNISH, INSTALL AND MAINTAIN ALL REQUIRED TRAFFIC CONTROL DEVICES IN ACCORDANCE WITH THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES OR GOVERNING LOCAL ACENCIES.

14. CONSTRUCTION STAKING:

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION STAKING NEEDED TO COMPLETE ALL THE CONSTRUCTION SHOWN HEREON. CONTACT DESIGN ENGINEER TO SCHEDULE CONSTRUCTION STAKING.

15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO EXISTING PROPERTY RESULTING FROM THE CONSTRUCTION ACTIVITIES, INCLUDING BUT NOT LIMITED TO PAVEMENT, FINISHED GRADES, LANDSCAPING, BUILDINGS, SURVEY MARKERS, FIELD TILES, CULVERTS, ETC.

16. IN THE EVENT ANY DISCREPANCIES OR ERRORS ARE FOUND ON THESE PLANS OR ANY CONFLICT OR PROBLEMS ARE ENCOUNTERED DURING CONSTRUCTION, THE GENERAL CONTRACTOR OR VERZON WIRELESS GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER BEFORE PROCEEDING WITH THE WORK. NO ADDITIONAL COMPENSATION WILL BE PAID TO THE GENERAL CONTRACTOR OR VERIZON WIRELESS GENERAL CONTRACTOR FOR WORK HAVING TO BE REDONE FOR GRADE OR GEOMETRIC DISCREPANCIES IF NOTICE TO THE ENGINEER HAS NOT BEEN PROVIDED. THE ENGINEER RESERVES THE RIGHT TO MAKE MINOR ADJUSTMENTS AS NECESSARY TO ACCOMPLISH THE INTENT OF THESE PLANS.

17. ALL SITE WORK AND CONSTRUCTION SHALL CONFORM TO ANY AND ALL APPLICABLE CODES AND VERIZON WIRELESS STANDARDS AND SPECIFICATIONS.

18. ALL ELEVATIONS AND TOPOGRAPHIC INFORMATION WAS TAKEN FROM A SURVEY SUPPLIED TO GPD GROUP, INC. BY BENCHMARK SERVICES. GPD GROUP, INC. HAS NOT VERIFIED THIS INFORMATION AND DOES NOT WARRANT ANY INFORMATION SUPPLIED BY OTHERS.

19. THE GENERAL CONTRACTOR SHALL MAINTAIN A COMPLETE AS-BUILT SET OF PLANS AND CONDITIONS, AND SUBMIT SAME TO THE ENGINEER WITHIN 7 DAYS OF COMPLETION OF CONSTRUCTION. AS-BUILT PLANS SUBMITTED TO THE ENGINEER SHALL INCLUDE A DRAWING WITH DIMENSIONS SHOWING THE LOCATION OF THE UNDERGROUND UTILITIES, GROUNDING GRID, EQUIPMENT PAD, TOWER FOUNDATION, TOWER PLATFORM ORIENTATION, AND FENCE WITHIN THE LEASE AREA OR PROPERTY AND BE CERTIFIED BY A LICENSED PROFESSIONAL SURVEYOR.

20. THE GENERAL CONTRACTOR AND VERIZON WIRELESS GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFICATION OF ALL GOVERNING AGENCIES THAT REQUIRE SITE INSPECTION OF THE WORK AND/OR SIMPLY NOTIFICATION.

21. THE GENERAL CONTRACTOR AND VERIZON WIRELESS GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFICATION AND COORDINATION OF ALL TESTING AGENCIES THAT REQUIRE SITE INSPECTION OR TESTING OF THE WORK AS DIRECTED IN THESE PLANS, GOVERNING AGENCIES AND ALL APPLICABLE CODES.

22. PRIOR TO THE SUBMISSION OF BIDS, THE GENERAL CONTRACTOR AND HIS SUBCONTRACTORS SHALL VENTY ALL DETAILS AND SCHEDULES ON THE DRAWINGS AND SPECIFICATIONS PROVIDED BY THE OWNER, FOR MEANING OF ABBREVIATIONS AND ADDITIONAL REQUIREMENTS AND INFORMATION. CHECK ALL CONSTRUCTION DOCUMENTS TO INCLUDE, BUT NOT LIMITED TO, GEDTECHNICAL REPORT, STRUCTURAL ANALYSIS, TOWER, MECHANICAL AND ELECTRICAL DRAWINGS, FOR SCALE, SPACE LIMITATIONS, DOOR SWINGS, ADJACENT CARRIER EQUIPMENT COORDINATION AND ADDITIONAL INFORMATION, ETC. REPORT ANY DISCREPANCIES, CONFLICTS, ETC. TO THE OWNER EFORE SUBMITTING BIDS.

24. WORK SITE SAFETY:

A CONSTRUCTION WORK PRESENTS UNIOUE THREATS TO HEALTH AND SAFETY. THE CONTRACTOR AND VERIZON WIRELESS CONTRACTOR ARE RESPONSIBLE TO EDUCATE THEIR WORK FORCE OF THESE DANGERS AND LIMIT THEIR EXPOSURE TO HAZARDS. THIS EDUCATION SHALL INCLUDE BUT NOT BE LIMITED TO APPLICABLE TRAINING COURSES AND CERTIFICATIONS, PROPER PERSONAL PROTECTIVE EQUIPMENT USAGE, DAILY TAILGATE MEETINGS AND ANY OTHER PREVENTATIVE MEASURES WHICH MAY BE REASONABLY EXPECTED. THE CONTRACTOR, VERIZON WIRELESS CONTRACTOR AND ALL SUB-CONTRACTOR, SHALL BE RESPONSIBLE FOR THE SAFETY OF THE WORK AREA, ADJACENT AREAS AND ANY PROPERTY OCCUPANTS WHO MAY BE AFFECTED BY THE WORK UNDER CONTRACT. ALL CONTRACTORS SHALL REVIEW ALL LANDOWNER, PRIME CONTRACTOR, CARRIER, OSHA, AND LOCAL SAFETY GUIDELINES AND AT ALL TIMES SHALL CONFORM TO THE MOST RESTRICTIVE OF THESE STANDARDS TO ENSURE A SAFE WORKPLACE.

NONVERVEX.
B. TOWER WORK PRESENTS ADDITIONAL THREATS TO HEALTH AND SAFETY. ALL TOWER WORKERS WORKING ON A TOWER MUST BE ADEQUATELY TRAINED AND MONITORED TO ENSURE THAT SAFE WORK PRACTICES ARE LEARNED AND FOLLOWED. AS REQUIRED BY OSHA, WHEN WORKING ON EXISTING COMMUNICATION TOWERS, EMPLOYEES MUST BE PROVIDED WITH APPROPRIATE FALL PROTECTION, TRAINED TO USE THIS FALL PROTECTION PROPERLY, AND THE USE OF FALL PROTECTION, MUST BE CONSISTENTLY SUPERVISED AND ENFORCED BY THE CONTRACTOR.

C. ALL SAFETY EQUIPMENT SHALL BE INSPECTED ACCORDING TO ALL OSHA AND INDUSTRY SCHEDULED INTERVALS AND ALL INSPECTIONS SHALL BE DOCUMENTED PER APPLICABLE CODES AND STANDARDS.

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- HIS SHEET

2. EXCAVATE A 4"x4" TRENCH UPSLOPE

LONG THE LINE OF STAKES.

4. BACKFILL AND COMPACT THE

C-7

EXISTING GRADE

EROSION CONTROL MUST BE

MAINTAINED ON ALL SLOPES &

DITCH LINES UNTIL VEGETATION HAS BEEN WELL ESTABLISHED

2' SILT FENCE

EXCAVATION SOIL

1. SET THE STAKES

3. STAPLE FILTER MATERIAL TO STAKES AND EXTEND IT INTO THE TRENCH.

ELEVATIO

ACCESS ROAD OR

COMPOUND

SURFACE_

POINTS A SHOULD BE HIGHER THAN POINT B

EROSION CONTROL BLANKETS OR 6"

TO 8" CLASS #2 CHANNEL LINE

STONE OR RIP RAP

DRAINAGE

2 TO 1

SLOPE

NOTES

DITCHLINE

3'-0"

FLOWLINE

WHEN FACED WITH SLOPES LESS THAN 2 TO 1 SLOPES, ALL SLOPES

SHOULD BE DOZER TRACKED PRIOR TO SEEDING. ALL SLOPES SHOULD HAVE EROSION CONTROL BLANKETS OR RIP RAP

EMBEDDED ON SLOPES SURFACES TO REDUCE EROSION.

2. ALL FLOWLINES MUST BE INSTALLED BELOW SUB-GRADE OF

DRAINAGE DITCHLINE DETAIL (SWALE)

COMPOUND. (AT MINIMUM OF 6" BELOW)

DRAINAGE

SILT FENCE DETAI

SCALE: N.T.

- .IAN RMANENT SEEDING ORMANT SEEDINGS TEMPORARY SEEDINGS SODDING MULCHING
- A. KENTUCKY BLUEGRASS 90 LBS./AC/MIXED PERENNIAL RYEGRASS 30 LBS./ACRE

B. KENTUCKY BLUEGRASS 135 LBS./AC/MIXED PERENNIAL RYEGRASS 45 LBS./AC. 2 TON STRAW MULCH PER ACRE

- THE ENGINEER.
- WITHIN 10 DAYS.

- SHOWN ON PLANS.

- WILL BE USED ON THIS SITE: SILT BARRIERS 2. SILT FENCE
 - 3. CONSTRUCTION ENTRANCE

- STAKE AND/OR FLAG LIMITS OF CLEARING
- PROCEDURES SHALL BE DISCUSSED
- CLEARING & GRUBBING, AS NECESSARY, FOR INSTALLATION OF PERIMETER CONTROLS.
- INSTALL SILT FENCE PERIMETER CONTROLS AS SHOWN ON PLANS
- PROMPTLY CLEARING & GRUBBING THE REMAINING SITE AS NECESSARY
- ACTIVITIES
- COMPLETELY STABILIZED
- REMOVE SEDIMENT CONTROLS 11



NOTES:

- STONE SIZE TWO INCH STONE SHALL BE USED, OR RECYCLED CONCRETE EQUIVALENT.
- 2. THE CONSTRUCTION ENTRANCE SHALL COINCIDE WITH THE PROPOSED DRIVE AS SHOWN ON THE PLAN. PAVEMENT THICKNESS - STONE LAYER SHALL BE AT LEAST 6" THICK.
- DRIVEWAY WIDTH THE ENTRANCE SHALL BE AT LEAST 10' WIDE BUT NOT LESS THAN FULL WIDTH AT POINTS WHERE INGRESS/EGRESS OCCURS.
- BEDDING A GEOTEXTILE SHALL BE PLACED OVER THE ENTIRE AREA 5. PRIOR TO PLACING STONE. IT SHALL HAVE A GRAB TENSILE STRENGTH OF AT LEAST 200 LBS. AND A MULLEN BURST STRENGTH OF AT LEAST 190 LBS.
- CULVERT A PIPE OR CULVERT SHALL BE CONSTRUCTED UNDER THE ENTRANCE IF NEEDED TO PREVENT SURFACE WATER FLOWING ACROSS THE ENTRANCE FROM BEING DIRECTED OUT ONTO PAVED SURFACES
- WATER BAR A WATER BAR SHALL BE CONSTRUCTED AS PART OF THE CONSTRUCTION ENTRANCE IF NEEDED TO PREVENT SURFACE RUNOFF FROM FLOWING THE LENGTH OF THE CONSTRUCTION ENTRANCE AND OUT ONTO PAVED SURFACES
- MAINTENANCE TOP DRESSING OF ADDITIONAL STONE SHALL BE APPLIED AS CONDITIONS DEMAND, MUD SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC ROADS, OR ANY SURFACE WHERE RUNOFF OS NOT CHECKED BY SEDIMENT CONTROLS, SHALL BE REMOVED IMMEDIATELY, REMOVAL SHALL BE ACCOMPLISHED BY SCRAPING OR SWEEPING
- SWEEPING. CONSTRUCTIONS ENTRANCES SHALL NOT BE RELIED UPON TO REMOVE MUD FROM VEHICLES AND PREVENT OFF SITE TRACKING. VEHICLES THAT ENTER AND LEAVE THE CONSTRUCTION SHALL BE RESTRICTED FROM MUDDY AREAS

TEMPORARY STABILIZED CONSTRUCTION ENTRANCE C-7

SCALE: N.T.

EROSION & SEDIMENT CONTROL NOTES

ALL WORK SPECIFIED AS A/AN DOT ITEM SHALL BE GOVERNED BY THE STATE OF INDIANA OF DEPARTMENT OF TRANSPORTATION CONSTRUCTION AND MATERIAL SPECIFICATION HANDBOOK. IT IS CONTRACTORS RESPONSIBILITY TO POSSESS AND BE FAMILIAR WITH APPLICABLE SECTIONS. THIS CONTRACT DRAWING SHALL BE MADE AVAILABLE ON SITE AT ALL TIMES AND PRESENTED UPON REQUEST. IF UNFORESEEN STORM WATER POLLUTION PREVENTION IS ENCOUNTERED, ADDITIONAL STORM WATER POLLUTION PREVENTION (SWPP) MEASURES MAY BE REQUESTED BY THE OWNER. COUNTY ENGINEER, PROJECT ENGINEER OR SOIL CONSERVATION SERVICE REPRESENTATIVE AT ANYTIME. SUCH REQUESTS SHALL BE IMPLEMENTED IMMEDIATELY AT CONTRACTOR'S EXPENSE. 3. ALL STORM WATER POLLUTION PREVENTION ITEMS SHALL BE INSTALLED AS SHOWN OR NOTED ON

 PLANT TEMPORARY SEEDING AND MULCHING IN ALL AREAS THAT SHALL BE INACTIVE FOR 7 DAYS OR MORE. ALL DISTURBED AND ERODED EARTH SHALL BE REGRADED AND SEEDED WITHIN 14 DAYS WITH SEEDING, AS DEFINED ABOVE AND AS SHOWN ON THE TABLE BELOW. TO ESTABLISH STABILITY AND PROVIDE SEDIMENT CONTROL, WHERE POSSIBLE. TEMPORARY SEEDING GROWTH SHALL NOT BE MOWED UNTIL IT HAS GONE TO SEED FOR 1 YEAR.

EB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.
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C. SPRING OATS 100 LBS./ACRE

D. WHEAT OR CEREAL RYE 150 LBS./ACRE E. SOD (NURSERY CROWN KENTUCKY BLUEGRASS) 150 LBS /ACRE

F. STRAW MULCH 2 TONS PER ACRE

* IRRIGATION NEEDED DURING JUNE, JULY & SEPTEMBER

IRRIGATION NEEDED FOR 2-3 WEEKS AFTER SODDING

5. PERMANENT VEGETATION SHALL BE INSTALLED WITHIN 10 DAYS AT THE COMPLETION OF ANY PLANTING AND SOD

 AT SUCH TIME ROUGH GRADING OF THE SITE IS COMPLETE AND DRAINAGE DIVERTS TO INLETS. INLET SEDIMENT FILTERS SHALL BE INSTALLED AT ALL INLET STRUCTURES TO KEEP PIPIN SYSTEMS FREE OF SILTATION.

SILT BARRIERS SHALL BE INSTALLED AROUND ALL EXISTING OR NEW STORM INLETS, CATCH BASI AND YARD DRAINS. INSTALL ROCK CHECK DAMS FOR HEADWALL INLETS FOR STORM WATER POLLUTION PREVENTION.

 STORM WATER POLLUTION PREVENTION MEASURES SHALL BE INSTALLED OR TOPSOIL STOCKPIL AND OTHER TEMPORARILY DISTURBED AREAS AS SHOWN ON THESE PLANS AND AS DIRECTED B

CONTRACTOR SHALL INSPECT ALL SWPP MEASURES DAILY AND REPAIR AS NECESSARY TO PREVENT EROSION. SILTATION SHALL BE REMOVED FROM AREAS WHERE FAILURES HAVE OCCURRED AND CORRECTIVE ACTION TAKEN WITHIN 24 HOURS TO MAINTAIN ALL SWPP. 10. SILT BARRIERS, CONSTRUCTION ENTRANCES, AND SILT FENCES SHALL REMAIN IN PLACE UNTIL A GOOD STAND OF GRASS HAS BEEN OBTAINED AND/OR PAVING OPERATIONS ARE COMPLETE CONTRACTOR SHALL KEEP SILT FROM ENTERING ANY STORM DRAINAGE SYSTEM. ONCE SITE HAS BEEN COMPLETELY STABILIZED, ANY SILT IN PIPES AND DRAINAGE SWALES SHALL BE REMOVED

11. TEMPORARY SEDIMENTATION AND STORM WATER POLLUTION PREVENTION MEASURES MUST BE INSPECTED AND LOGGED BY THE CONTRACTOR FOR INSPECTION, LOGGING SHALL BE WEEKLY AND AFTER RAIN STORMS.

12. UTILITY COMPANIES MUST COMPLY WITH ALL STORM WATER POLLUTION PREVENTION MEASURES AS DEFINED ON THE STORM WATER PREVENTION PLANS, DETAILS AND NOTES. THE TOTAL AREA OF DISTURBANCE FOR THIS PROJECT IS APPROXIMATELY 0.96 ACRES 14. ALL EXISTING WATER COURSES WITHIN THE PROJECT LIMITS SHALL BE TEMPORARILY PROTECTED DURING LAND CLEARING AND GRADING OPERATIONS. SOLIS WITHIN 50 FEET OF SAID WATER COURSES SHALL BE STABILIZED WITHIN 2 DAYS OF THE INITIAL CLEARING / GRADING OPERATION AS

ALL DISTURBED AREAS SHALL BE STABILIZED WITHIN 7 DAYS OF FINAL GRADING. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN ALL SEDIMENTATION AND STORM WATER POLLUTION PREVENTION ITEMS AT ALL TIMES.

17. ALL STORM WATER POLLUTION PREVENTION PRACTICES WILL BE INSTALLED BEFORE ANY OTHER EARTH MOVING OCCURS. 18. THE FOLLOWING STORM WATER POLLUTION PREVENTION AND SEDIMENT CONTROL MEASURES

4. EROSION CONTROL SEED BLANKETS - SPEC FOR TEMPORARY EROSION CONTROL BLANKETS ON SLOPES/DITCHES

CONSTRUCTION SEQUENCE

DURING PRECONSTRUCTION MEETING ALL EROSION & SEDIMENT CONTROL FACILITIES &

INSTALL CONSTRUCTION ENTRANCE, IF CONDITIONS ARE SUCH THAT MUD IS COLLECTION ON VEHICLE TIRES, THE TIRES MUST BE CLEANED BEFORE THE VEHICLES ENTER THE PUBLIC ROADWAY, THE SITE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT THE TRACKING OR FLOW OF MUD INTO THE PUBLIC RIGHT-OF-WAY. ALL MATERIALS SPILLED, DROPPED, WASHED OR TRACKED FROM VEHICLES ONTO THE ROADWAY MUST BE REMOVED

BEGIN FILLING & GRADING AS REQUIRED TO REACH SUBGRADE. CONSTRUCT AND MAINTAIN TEMPORARY DRAINAGE SWALES DURING FILLING AND GRADING

CONSTRUCT SITE WORK INCLUDING STORM DRAINAGE FACILITIES. MAINTAIN EROSION & SEDIMENTATION CONTROL MEASURES UNTIL THE SITE HAS BEEN








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SCALE: N. T. S.







70 PSF ACTUAL MATERIAL WEIGHTS 120 MPH

INTERNATIONAL BUILDING CODE (IBC) 2012 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES ASCE7-10 OSHA REGULATIONS

ASTM A53 TYPE E OR S GRADE B, OR ASTM A501.

THE SUBSURFACE REPORT WAS PROVIDED BY ALT & WITZIG ENGINEERING, INC.,

ALL CONTRACTORS SHALL EXERCISE GREAT CARE DURING EXCAVATION. ALL CONTRACTORS SHALL PREDETERMINE UTILITY LOCATIONS AND NOTIFY THE ENGINEER

THE SUBSURFACE REPORT IS NOT TO BE CONSIDERED AS A COMPLETE RECORD OF THE EXISTING CONDITIONS AT THE SITE. THE CONTRACTOR SHALL VERIFY ALL EXISTING SITE CONDITIONS, INCLUDING SUBSURFACE CONDITIONS. THE CONTRACTOR SHALL OBTAIN PERMISSION FROM THE OWNER PRIOR TO SITE ENTRY FOR THE PURPOSE OF CONDUCTING SOIL

FOUNDATION SUBGRADES SHALL BE HAND TRIMMED AND COMPACTED. ALL BACKFILL TO BE COMPACTED TO 95% MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D-1557

CONCRETE CYLINDERS SHALL BE TAKEN AND TESTED BY A QUALIFIED CONCRETE TESTING COMPANY. THE GENERAL CONTRACTOR SHALL PROVIDE ONE (1) CYLINDER SAMPLE TAKEN PER TRUCK OR FOUR (4) TOTAL FOR BUILDING FOUNDATION AND THE CONCRETE MUST MEET A 4" SLUMP. THE GENERAL CONTRACTOR SHALL PROVIDE THREE (3) COPIES OF TESTING RESULTS TO VERIZON WIRLESS AND TOWER OWNER CONSTRUCTION MANAGERS. FAILURE TO PROVIDE WRITTEN DOCUMENTATION WILL RESULT IN A DEDUCTION FROM THE CONTRACT. THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR ANY ADDITIONAL COSTS IN TESTING DUE TO DELAYS BY THE GENERAL CONTRACTOR OR HIS SUBCONTRACTORS. USE AIR ENTRAINMENT IN

> - LATEST EDITION OF ACI-318 AS ADOPTED BY THE STATE OF KENTUCKY - BLDG. FOUNDATION F'c = 4000 PSI @ 28 DAYS 2"- 4" SLUMP - REINFORCING ASTM A615, GRADE 60 - ANCHOR BOLTS ASTM F1554 A36 - FOOTINGS

BOTTOM/SIDES 3"

REINFORCING EMBEDMENT AND LAP SPLICES (INCHES) FOR 4000 PSI CONCRETE

ANCHORAGE SPLICE (OTHER) ANCHORAGE (TOP) SPLICE (TOP)

ALL CONCRETE CONSTRUCTION SHALL CONFORM TO ACI*96, "STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE" AND ACI 305, 306 AND 307 UNLESS NOTED OTHERWISE

2. ALL DETAILING, FABRICATION AND PLACING OF CONCRETE SHALL CONFORM TO ACI 318-11.

3. ALL CONCRETE EXPOSED TO WEATHER SHALL CONTAIN 6% (±1%) AIR ENTRAINMENT.

4. PROVIDE CORNER BARS AT ALL LOCATIONS WHERE REINFORCEMENT CHANGES DIRECTION.



1-800-752-6007 KENTUCKY STATE LAW, IT IS AGAINST THE LAW TO EXCAVATE WITHOUT NOTIFYING THE INDERGROUND LOCATION SERVICE TWO (2) VORKING DAYS BEFORE COMMENCING WORK.

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	1		KENTUCKY RSA No. 1 PARTNERSHIP	d/b/a VERIZON WIRELESS	Verizon wireless	220 E. OGTH STREET	05545 VA0031 STD-4V6011		
				ES ES				ANNUN A MUNICIPAL	
FV BARI OW		BARLOW, KY 42024				DETAILS AND NOTES		19	
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1/2" DIA, HILTI-Z-R STAINLESS STEEL ANCHOR (MIN. 4" EMBEDMENT) -TYP. OF 4 PER BEARING PAD

> ADHESIVE (OR ENGINEER APPROVED EQUIVALENT) INJECTED INTO HOLE FULL DEPTH PRIOR TO INSTALLATION OF ANCHOR

> > S-2

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GENERAL ELECTRICAL NOTES

UNLESS NOTED AS VERIZON WIRELESS GENERAL CONTRACTOR OR GC, ALL WORK ITEMS ON ELECTRICAL "E" & GROUNDING "G" SHEETS SHALL BE PERFORMED BY THE GENERAL CONTRACTOR'S ELECTRICAL CONTRACTOR. ALL WORK SHOWN AS NEW UNLESS NOTED EXISTING.

2. THE GENERAL CONTRACTOR AND VERIZON WIRELESS ELECTRICAL CONTRACTORS SHALL BE LICENSED TO PERFORM WORK IN THE COUNTY OF THIS PROJECT AND SHALL BE RESPONSIBLE FOR OBTAINING AN ELECTRICAL PERMIT FROM THE COUNTY

 UNDERGROUND CONDUIT SHALL BE SCH. 40 PVC PLASTIC DUCT WITH ALL BENDS MINIMUM 24" RADIUS 90° SWEEP SCH. 80 UNLESS OTHERWISE NOTED ON DRAWINGS. ELECTRICAL AND FIBER UTILITY LATERAL CONDUITS SHALL BE MINIMUM 36" RADIUS 90° SWEEP SCH. 80. ALL PVC SCH80 PIPING AND FITTINGS SHALL USE BELL END WHERE FEASIBLE

4. ALL WORK SHALL BE IN ACCORDANCE WITH N.F.P.A. AND N.E.C. CODES, NEMA STANDARDS AND ALL LOCAL CODES.

PRIOR TO TRENCHING AND FOR COORDINATING OF THE FIBER AND ELECTRICAL SERVICES, SEE SHEET C-6 GENERAL NOTE #6 FOR UTILITY CONTACT NAMES & PHONE NUMBERS

ABOVE GRADE RISER CONDUIT SHALL BE RIGID SCH. 80 PVC WITH MATCHING FITTINGS UNLESS NOTED OTHERWISE.

ALL WIRE SHALL BE COPPER, 600V THHN-2/THWN-2 OR XHHW-2 90°C UNLESS

ALL ELECTRICAL CONTRACTORS SHALL ABIDE BY ALL GUIDELINES REQUIRED BY TOWER OWNER AND VERIZON WIRELESS. INCLUDING BUT NOT LIMITED TO. a) STAGE CONSTRUCTION AND REPORT WHEN ON AND OFF THE SITE. b) PROVIDE LOG BOOK RECORDS OF PERSONNEL ON TOWER. c) PROVIDE R.F. MONITORING DEVICES TO TOWER PERSONNEL, d) VERIZON WIRELESS TO INSPECT ALL TRENCHING BEFORE BACKFILLING.

THE ELECTRICAL CONTRACTOR SHALL INSTALL ALL UNDERGROUND CONDUITS SHOWN ON PLANS UNLESS NOTED OTHERWISE, ALL UNDERGROUND CONDUIT VITALL DE STUBBEU-UP ANU CAPPED AT THE LOCATION INDICATED ON PLAN VZW CONTRACTOR SHALL COMPLETE ALL CONDUIT TERMINATIONS TO VZW ENCLOSURES. SHALL BE STUBBED-UP AND CAPPED AT THE LOCATION INDICATED ON PLANS.

10 THE VZW ELECTRICAL CONTRACTOR SHALL VERIEY ELECTRICAL SERVICE REQUIRED - 120/240V, SINGLE PHASE, 200 AMP BEFORE ROUGH IN.

GENERAL CONTRACTOR AND VZW ELECTRICAL CONTRACTORS SHALL PROVIDE FIRE WATCH DURING ALL WELDING OPERATIONS. TWO (2) HAND HELD 30 LB. FIRE EXTINGUISHERS & ADEQUATE WATER SUPPLY SHALL BE APPROVED BY THE CLIENT AND THE TOWNSHIP WATER AUTHORITY.

12. SEALALL PENETRATIONS WITH SILICONE SEALANT

8.

17

13. ELECTRICAL WORK PRESENTS SPECIFIC THREATS TO THE HEALTH AND SAFETY OF WORKERS ON SITE. SPECIFICALLY ELECTROCUTIONS ARE THE FOURTH LEADING CAUSE OF DEATH ON CONSTRUCTION SITES. ALL ELECTRICAL WORKERS LEADING CAUSE OF DEATH ON CONSTRUCTION STES. ALL ELECTRICAL WORKERS SHALL HAVE CURRENT CERTIFICATIONS WHICH SATISFY ALL TRAINING REQUIREMENTS FOR THE ELECTRICAL WORK THEY ARE PERFORMING PER OSHA STANDARDS. ALL ELECTRICAL WORKERS SHALL ADHERE TO ALL SAFETY RULES AND REGULATIONS FOR WORKER AND PUBLIC SAFETY ALL WORK SHALL BE THE VOLTAGES PRESENT FOR EACH TASK. THE CONTRACTOR SHALL REVIEW ALL LANDOWNER, PRIME CONTRACTOR, CARRIER, OSHA, NEPA 70E, AND LOCAL SAFETY GUIDELINES AND AT ALL TIMES SHALL CONFORM TO THE MOST RESTRICTIVE OF THESE STANDARDS TO ENSURE A SAFE WORKPLACE.

14. CONNECT NEUTRAL TERMINAL IN SERVICE DISCONNECTING DEVICE TO GROUNDING ELECTRODE. (2/0 AWG TINED STRANDED COPPER GROUNDING ELECTRODE CONDUCTOR IN 1° PVC CONDUIT). CONNECT FIRST GROUNDING ELECTRODE TO SECOND GROUNDING ELECTRODE WITH 2/0 AWG TINNED STRANDED COPPER CONDUCTOR EXOTHERMICALLY WELDED TO ELECTRODES.

15. VZW E.C. SHALL VERIFY SUFFICIENT CAPACITY EXISTS AT EXISTING METER CENTER, NOTIFY ENGINEER IMMEDIATELY IF SERVICE IS NOT ADEQUATE.

16. REFER TO ELECTRICAL SITE PLAN FOR CONDUIT AND WIRE REQUIREMENTS. ALL ELECTRICAL EQUIPMENT PLACARDS MUST BE ENGRAVED PLASTIC LABELS ONLY NO P-TOUCH LABELS ARE TO BE USED. PLACARDS SHALL BE 1.1/2" x 5" IN SIZE AND PERMANENTLY AFFIXED TO THE ENCLOSURES EASILY VISIBLE LOCATIONS.

18. VERIZON WIRELESS ELECTRICAL PLACARD REQUIREMENTS: a) AVAILABLE FAULT CURRENT RATING FROM UTILITY PROVIDER WITH DATE. b) VERIZON WIRELESS SERVICE DISCONNECT AT METER BASE. C) VERIZON MAIN BREAKER AT INTEGRATED LOAD CENTER 'ILC'

C) VENCON WIND REARACK AT INTEGRATED LODD CENTER LCC. d) GENERATOR NEUTRAL CONDUCTOR BONDED TO SERVICE GROUNDING ELECTRODE IN THIS ENCLOSURE AT METER CENTER AND INTEGRATED LOAD CENTER "ILC" NEUTRAL BAR. e) VERIZON WIRELESS GENERATOR

f) VERIZON WIRELESS FIBER ONLY AT ALL VERIZON WIRELESS FIBER HAND HOLES FASTENED TO INSIDE WALL OF HAND HOLE.

19. ALL EQUIPMENT SHALL BE GROUNDED PER LATEST EDITION OF NEC AND AS INDICATED ON GROUNDING PLAN

20. ELECTRICAL EQUIPMENT SHALL BE MIN 3'-0" FROM ANY STRUCTURE AND AS REQUIRED BY LOCAL UTILITY COMPANIES

21. 2 AWG BARE SOLID TINNED COPPER WIRE LEADS FROM NEW H-FRAME STRUCTURE TO BE BONDED TO EXISTING GROUNDING SYSTEM (TYP) 22. ALL METALLIC CABINETS & ENCLOSURES MUST BE GROUNDED WITH 2 AWG

23. ALL ABOVE GRADE GROUND VZW CONNECTIONS SHALL BE IN 1/2" FLEXIBLE PVC CONDUIT w/ANTI-THEFT COMPOUND. ALL OTHER LEADS SHALL BE IN 1/2" PVC CONDUIT

24. ALL HARDWARE USED MUST BE GALVANIZED OR STAINLESS STEEL, NO ZINC OR OTHER MATERIAL IS TO BE USED.

25. VERIZON WIRELESS FURNISH & INSTALL ALL INNER CONNECTING CONDUITS BETWEEN CABINETS AND LAND ALL AC POWER TO VZW EQUIPMENT.

26. ALL CONDUIT RUNS SHALL HAVE A CONTINUOUS SLOPE DOWNWARD AND AWAY FROM THE METERBOARD SO THAT WATER WILL NOT FLOW FROM THE EQUIPMENT. TRENCHES SHALL BE EXCAVATED ALONG STRAIGHT LINES BEFORE CONDUIT ARE INSTALLED SO THE ELEVATION CAN BE ADJUSTED, IF NECESSARY, TO AVOID UNSEEN OBSTACLES.

27. PRIOR TO THE SUBMISSION OF BIDS, THE GENERAL CONTRACTOR AND HIS SUBCONTRACTORS SHALL VERIFY ALL DETAILS AND SCHEDULES ON THE DRAWINGS AND SPECIFICATIONS PROVIDED BY THE OWNER. FOR MEANING OF ABBREVIATIONS AND ADDITIONAL REQUIREMENTS AND INFORMATION, CHECK STRUCTURAL AND OTHER MECHANICAL AND ELECTRICAL DRAWINGS FOR SCALE SPACE LIMITATIONS DOOR SWINGS ADJACENT CARRIER FOURPMENT COORDINATION AND ADDITIONAL INFORMATION FTC. REPORT ANY DISCREPANCIES, CONFLICTS, ETC. TO THE OWNER BEFORE SUBMITTING BIDS





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NOTE: FIBER OPTIC INNERDUCTS SHALL TERMINATE INSIDE CHARLES CABINET ONLY









EQUIPMENT PAD LIGHTING PLAN	
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 -WIRED TO LIGHT CIRCUIT
 -PROVIDE MANUAL OVERRIDE
 -(1) WATTSTOPPER (P/N EW-200-120-W)

- (2) 12 AWG IN 1/2" EMT CONDUIT (TYP.)

_ 120VAC LED FLOODLIGHT MOUNTED TO CANOPY FRAME. AIM LIGHT TOWARDS EQUIPMENT. FIXTURES SHALL BE CONTROLLED WITH LOCALLY MOUNTED 120 VAC MOTION DETECTOR -(6) LITHONIA LIGHTING (P/N OFL1-LED-P1-40K-MVOLT-THK-DDBXD)









GROUNDING KEYED NOTES

- FURN. & INST. 1 2/0 AWG INSULATED STRANDED COPPER GND. WIRE FOR BATTERY BAY GROUND (BBG) TO (EER). CONNECTION TO (EER) SHALL BE CADWELD. CONNECT THE 1 - 2/0 AWG INSULATED STRANDED COPPER GND. CONNECTION TO THE GROUND BAR IN THE BATTERY BAY SHALL BE MECHANICAL. ROUTE LEAD IN LIQUIDTIGHT FLEXIBLE NONMETALLIC CONDUIT W/ANTI-THEFT COMPOUND.
- 2 CEPSG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND. WIRE FOR CEPSG POST TO (EER). FURNISH & INSTALL THE THEFT DETERRENT GROUND POST ASSEMBLY PART #TDSGAPAC14 FOR THE COAX ENTRY PROTECTION SYSTEM (AS MFG'D. BY ERICO GROUNDING) TO THE POST. CONNECT THE 2 AWG BARE SOLID TINNED COPPER TO THE POST. THE CONNECTION TO POST SHALL BE CADWELD. REFER TO THE GENERAL INSTALLATION GUIDE AS SUPPLIED WITH THEFT DETERRENT POST MOUNT GROUND ASSEMBLY, SEE DETAIL 1 ON SHEET G4 FOR DETAILS. CONNECTION (EER) SHALL BE CADWELD.
- CG: FURN. & INST. 2 2 AWG BARE SOLID TINNED COPPER GND. WIRE FOR CABINET GROUND (CG) TO (EER). CONNECTION (EER) SHALL BE CADWELD, FURN. & INST. TWO HOLE LONG BARREL LUGS ON EXISTING 2 AWG BARE TINNED COPPER LEADS. VZW E.C. SHALL FURNISH & INSTALL DRAGON TOOTH WASHERS AND #8 STAINLESS STEEL FASTENERS ON ALL EQUIPMENT CABINET EXTERIOR GROUND LOCATIONS (TYP. 2 PER CABINET). REMOVE ALL SURFACE PAINT AND USE ANTIOXIDENT COMPOUND BETWEEN METAL AND WASHER. ALL LEADS SHALL BE DRESSED TO REDUCE TRIP HAZARDS. DETAIL 3 ON SHEET G-4 FOR DETAILS.
- CSG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND WIRE FOR ICE BRIDGE CHANNEL TO (EER). CONNECTION (EER) AND ICE BRIDGE CHANNEL SHALL BE CADWELD.
- FURN. & INST. 2 AWG BARE SOLID TINNED COPPER BURIED EXTERIOR ELECTRODE GROUND RING (EER) AROUND RADIO EQUIPMENT PAD AND TOWER BURIED AT 36° OR 6° BELOW FROST LINE (WHICHEVER IS GREATER). MAINTAIN 24° FROM EQUIPMENT PAD AND TOWER FOUNDATIONS MINIMUM.
- FG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND. WIRE FOR FIBER ENTRANCE GROUND (FEG) TO (TGE). RUN INTO THE POWER PLANT CABINET AND MAKE A MECHANICAL CONNECTION TO THE FIBER SERVICE GROUND POINT. ROUTE LEAD IN GALV. FLEXIBLE METALLIC CONDUIT CRIMPED TO LEAD AT 6" O.C. CONNECTION TO (EER) SHALL BE CADWELD.
- FG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND WIRE FROM NEW FENCING TO GROUND RING (EER). CONNECTION TO FENCING SHALL BE AT THE BASE OF FENCE POST USING CADVELD AND WEAVE GND. WIRE THRU FABRIC AND ATTACH TO TOP RAIL WITH CADVELD. CONNECTION TO (EER) SHALL BE CADVELD.
- B FTWGL: FURN. & INST. FUTURE TOWER WAVEGUIDE GROUND BAR LEADS. CONTRACTOR SHALL COIL UP TEN (10') OF 2 AWG SOLID TINNED COPPER GROUND (TYP.)
- GGG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND. WIRE FOR THE GENERATOR ENCLOSURE TO (EER), CONNECTION TO GENERATOR ENCLOSURE SHALL BE MECHANICAL. ROUTE LEAD IN FLEXIBLE NONMETALLIC CONDUIT WANTI-THEFT COMPOUND. "GENERATOR SERVICE GROUND WHERE REQUIRED BY JURISDICTION HAVING AUTHORITY"
- GG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND WIRE AT EACH GATE POST AND PROVIDE GROUND WIRE TO BONDING JUMPER FROM GATE POST TO FENCE POST.
- GRE: FURN. & INST. GROUND RODS TO 36" BELOW FINISH GRADE (OR 6" BELOW FROST LINE, WHICHEVER IS GREATER) AT A MINIMUM SPACING OF 10'-0" AT RADIO EQUIPMENT PAD GROUND SYSTEM. CONNECTION TO (EER) SHALL BE CADWELD.
- BSG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND. WIRE FROM ICE BRIDGE SUPPORT POST TO (EER). CONNECTION TO (EER) & ICE BRIDGE SUPPORT POST SHALL BE CADWELD. SEE SHEET S-1 FOR ICE BRIDGE POST LOCATIONS.
- T3 PEG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND. WIRE FOR THE 'ILC' GROUND BAR TO (EER). CONNECT 1 2 AWG BARE SOLID TINNED COPPER GND WIRE FOR THE POWER ENTRANCE GROUND (PEG). CONNECTION OF THE WIRE TO THE INTEGRATED LOAD CENTER GROUND BAR SHALL BE MECHANICAL. ROUTE LEAD IN FLEXIBLE NONMETALLIC CONDUIT WANTI-THEFT COMPOUND. CONNECTION (EER) SHALL BE CADWELD.
- PCSG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND. WIRE FOR THE PAD CANOPY SUPPORT BASE TO (EER). CONNECT SOLID TINNED COPPER ONE HOLE GROUND TAB. WITH AN EXOTHERMIC CONNECTION. THE CONNECTION OF THE GROUND WIRE AND LUG TO THE CANOPY SUPPORT BASE SHALL BE EMECHANICAL. SEE DETAIL 3 ON SHEET G-3. ROUTE LEAD IN FLEXIBLE NONMETALLIC CONDUIT WANTI-THEFT COMPOUND. CONNECTION (FER) SHALL BE CADWELD.
- TBS: FURN. & INST. 1-2 AWG BARE SOLID TINNED COPPER GND. WIRE FROM TOWER LEG BASE PLATE TO (EER). CONNECTION TO TOWER LEG BASE PLATE SHALL BE CADWELD OR MECHANICAL TO LEG AND (EER) SHALL BE CADWELD.
- TWG: FURN. & INST. THEFT DETERRENT GROUND ASSEMBLY KIT PART #TDSGABC14 FOR THE TOWER WAVEGUIDE GROUND (TWG) DIRECTLY TO THE TOWER (SEE SITE SPECIFIC GROUNDING PLAN OF DESIGN DRAWINGS).
- UG: FURN. & INST. 1 2 AWG BARE SOLID TINNED COPPER GND. WIRE (UFER GROUND) FOR PAD FOUNDATION REINFORCEMENT STEEL CONNECTION TO (EER) SHALL BE CADWELD. PROVIDE HEAT SHRINK TUBING OR ELECTRICAL TAPE PROTECTION FOR CONDUCTOR AT TRANSITION BETWEEN CONCRETE AND SOLL.
- OGA:
 VZW E.C. SHALL FURN. & INST. THE THEFT DETERRENT GROUND ASSEMBLY KIT PART #TDSGAPC14 FOR THE OVP GROUND (OGL)

 DIRECTLY TO THE OVP H-FRAME POST. SEE DETAIL 4 ON SHEET G-4.
- OGL:
 FURN. & INST. 1 6 AWG GREEN INSULATED STRANDED COPPER GND. WIRE FROM OVP GROUND POINT TO OVP GROUND ASSEMBLY (OGA). CONNECTION TO GROUND BAR SHALL BE MECHANICAL.
- CONNECTION (EER) SHALL BE CADWELD.

GROUNDING NOTES

- ALL UNDERGROUND CONNECTIONS ON THE LIGHTNING PROTECTION SYSTEM SHALL BE EXOTHERMIC WELDED USING THE CADWELD PROCESS. THE VERIZON WIRELESS E.C. SHALL FURN. & INST. ALL THESE CONNECTIONS, INCLUDING WELD METALS, MOLDS AND TOOLS. THE VERIZON WIRELESS E.C. SHALL FURN. & INST. 5/8" x 10" COPPER CLAD STEEL (COPPER JACKET 0.0012" MIN.) GROUND RODS, DRIVEN VERTICAL TO 36" BELOW FIN. GRADE (OR 6" BELOW FROSTLINE, WHICHEVER IS GREATER) @ 10-0". O.C. MINIMUM.
- 2. THE E.C. SHALL FURN. & INST. 2 AWG BARE SOLID TINNED COPPER GND. WIRE AT A DEPTH OF 36" BELOW FIN. GRADE FOR THE TOWER AND EQUIPMENT PAD (EER) AND ICE BRIDGE (CSG).
- 3. ALL EXTERIOR GND. CONNECTIONS SHALL BE EXOTHERMIC CADWELD (U.N.O).
- 4. UPON COMPLETION OF THE EQUIPMENT PAD GROUNDING RING AND BEFORE BONDING TO THE TOWER GROUND RING, THE VERIZON WIRELESS E.C. SHALL MEGGER TEST THIS GROUNDING FIELD. THE REQUIRED RESISTANCE LEVEL IS 5 OHMS OR LESS. THE VERIZON WIRELESS E.C. SHALL NOTIFY THE ENGINEER IF THESE REQUIREMERTS ARE NOT ACHIEVED. THE VERIZON WIRELESS E.C. SHALL SUBMIT PRICING TO VERIZON WIRELESS FOR THE INSTALLATION OF ADDITIONAL GROUND RODS REQUIRED FOR PROPER RESISTANCE. UPON APPROVAL FROM VERIZON WIRELESS, THE VERIZON WIRELESS E.C. SHALL INSTALL ADDITIONAL GROUND RODS AS REQUIRED. AFTER PASSING TEST, THE VERIZON WIRELESS E.C. SHALL BOND THE EQUIPMENT PAD RING TO THE TOWER RING. THE VERIZON WIRELESS E.C. SHALL NOTIFY VERIZON WIRELESS CONSTRUCTION MANAGER 48 HOURS PRIOR TO BACKFILLING TRENCHES, POURING CONCRETE FOR FOUNDATIONS, TO INSPECT BONDS AND INSPECT ANY/ALL BREAKS AND REPAIRS TO THE GROUND RING.
- 5. GROUND SYSTEM SHALL BE VISUALLY INSPECTED BY A VERIZON WIRELESS CONSTRUCTION ENGINEER BEFORE BACKFILLING IF REQUESTED.
- 6. NO SHARP 90° BENDS SHALL BE USED. A LONG SWINGING RADIUS BEND REQUIRED.
- 7. ALL EQUIPMENT PAD AND EQUIPMENT GROUNDING SHALL BE IN ACCORDANCE WITH VERIZON WIRELESS SPECIFICATIONS.
- ALL ABOVE GROUND BARE COPPER CONDUCTORS BELOW 10' ABOVE GRADE SHALL BE INSTALLED IN FLEXIBLE PVC CONDUIT. CONDUIT SHALL BE FILLED WITH THEFT DETERRENT COMPOUND (ELECTRIC MOTION COMPANY ANTI-THEFT COMPOUND EM-5101).
- 9. BARE COPPER CONDUCTORS SHALL NOT BE INSTALLED WHERE THEY MAY BE IN CONTACT WITH GALVANIZED METALS. THE CONDUCTORS SHALL BE INSULATED OR ENCLOSED IN PVC CONDUIT, PLASTIC SEALTIGHT OR INSTALLED WITH STANDOFF SUCH THAT NO CONTACT BETWEEN DISSIMILAR METALS MAY TAKE PLACE.
- 10. CONNECTION OF COPPER CONDUCTORS TO GALVANIZED METAL OR ALUMINUM SHALL BE AVOIDED. BRASS OR STAINLESS STEEL LUGS OR BARS SHALL BE USED FOR THESE CONNECTIONS.
- 11. ALL CRIMP LUG CONNECTIONS TO ALL GROUND BARS SHALL BE LUBRICATED WITH A CORROSION INHIBITOR ("OXY-GREASE") OR APPROVED EQUAL.
- 12. GROUND ASSEMBLIES SHALL BE THEFT DETERRENT DESIGN AS MANUFACTURED BY ERICO INC. AND FURNISHED BY VERIZON WIRELESS. PROVIDE TWO (2) LUG HOLES PER VERIZON WIRELESS STANDARDS. UNLESS NOTED OTHERWISE.

LEGEND						
BBG	BATTERY BAY GROUND					
CEPSG	COAX ENTRY PROTECTION SYSTEM GROUND					
CG	CABINET GROUND					
CSG	CABLE SUPPORT GROUND					
EER	BURIED EXTERIOR ELECTRODE GROUND RING					
EGC	EQUIPMENT GROUNDING CONDUCTOR (NEC DESIGNATION)					
FEG	FIBER ENTRANCE GROUND					
FG	FENCE GROUND					
FTWGL	FUTURE TOWER WAVEGUIDE GROUND LEAD					
GEC	GROUNDING ELECTRODE CONDUCTOR (NEC DESIGNATION)					
GEG	GENERATOR ENCLOSURE GROUND					
GES	GROUNDING ELECTRODE SYSTEM (NEC DESIGNATION)					
GG	GATE GROUND					
GRE						
000						
OGA						
OUGL						
PEG	POWER ENTRANCE GROUND					
PCSG	PAD CANOPY SUPPORT GROUND DOWN LEAD					
TBG	TOWER BASE GROUND					
TWG	TOWER WAVEGUIDE GROUND ASSEMBLY					
UG	UFER GROUND (PIER FOUNDATION)					
	CABLE TO CABLE CADWELD CONNECTION					
á	CABLE TO GND, ROD CADWELD CONNECTION					
×	TEST WELL					
8	INSPECTION PORT CABLE TO CABLE CADWELD CONNECTION					
	THE ECHICIT STILLE IS SADEL ONDWEED CONNECTION					





2 AWG SOLID GROUND

CONDUCTOR (TYP)













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GROUND METER BASE ENCLOSURE WITH A 2/0 AWG BARE SOLID

TINNED COPPER (GEC) TO GROUND RING IN 1" PVC CONDUIT

2 AWG SOLID GROUND

CONDUCTOR (TYP)-











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		/		KENTLICKY RSA No. 1 PARTNERSHIP	d/b/a VERIZON WIRELESS	Vertical wireless	200 E. 90TH STREET	04284 VMVIII SIDUMVIM	
EV. DATE DESCRIPTION	A 10/31/18 ISSUED FOR 90% REVIEW	B 01/22/19 ADDED DISTANCE TO HOUSE	C 01/24/19 ADDED BUILDING CALL OUTS AND REVISED SHEET TITLE	0 02/08/19 FINAL CONSTRUCTION DRAWINGS					
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	EV BARLOW 2557 STEVE DENTON ROAD BARLOW, KY 42024 ANTENNA PLAN AND DETAILS (REFERENCE ONLY)								
	ISSUED FOR: REVIEW PERMIT CONSTRUCTION RECORD PROJECT MANAGER DESIGNER TTP								
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VERIZON WIREL	ESS
SITE NAME: EV	BARLOW - B
	VVVV OTEN

SITE ADDRESS: XXXX STEVE DENTON RD BARLOW, KY 42024 SITE EMIS#: 616190973 SITE COORDINATES: 37°06'42.15" N, 89°02'44.58" W





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Designed Appurtenance Loading

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Elev	Description	Tx-Line	Elev	Description	Truline
275	3V-Boom - 12ft Face - 3ft Standoff		245	(6) JAHH-65C-R38	(12) 1 5/8"
275	(1) RCMDC-6627-PF-48		230	3V-Boom - 10tt Face - 3ft Standoff	(12) 100
275	(3) 4449 B13 + 85		230	(6) JAHH-65C-R3B	(12) 1 5/8°
275	(3) 8843 82 + B66A		215	3V-Boom - 10ft Face - 3ft Standoff	(12) 1 00
275	(3) CBC78T-DS-43		215	(6) JAHH-85C-R38	(12) 1 5/8"
275	(6) JAHH-65C-R38	(2) 1 1/4"	200	3V-Boom - 10ft Face - 3lt Standoff	(12) 1 44
260	Leg Dish Mount		200	(6) JAHH-65C-R3B	(12) 1 5/8"
260	(1) 8' Solid Dish W/ Radoms	(1) 1 5/5	185	Lea Dish Mount	1.47 1.40
245	3V-Boom - 10it Face - 3ft Standoff		185	(1) 6' Solid Dish W/ Radome	(1) 1 5/8"

	Sabre Communications Corporation
Sabre Industries /	P.O. Box 658
Towers and Poles	Sioux City, IA 51102-0658 Phone (712) 255-6601 Fare (712) 255-6601
Information contained havain is the sole goo	perty of Sabre Communications Corporation, peretitation a
Inde secret as defined by lows Code Ch. 9 or part for any purpose wheteover without	50 and shall not be reproduced, copied or used in whele See prior william consent of Sabra Communications

Dama 1

2

DESIGN APPURTENANCE

SCALE: N.T.S.

	Description; Date:	280' S3R 12/13/2018	By NM	
1	LOAD	ING	(2)	
			TW-1	

	19-5171-TJH			
mer.	VERIZON WIRELE	ss		
lame:	Barlow, KY 232179			
iption;	280" S3R			
	12/13/2018	By	NM	

19-5171-TJH			
VERIZON WIRELE	\$\$		
Barlow, KY 232179			
280' S3R			
12/13/2018	By	NM	
	19-5171-TJH VERIZON WIRELE Barlow, KY 232179 2807 S3R 12/13/2018	19-5171-TJH VERIZON WIRELESS Barlow, KY 232179 2807 S3R 12/13/2018 By:	19-5171-TJH VERIZON WIRELESS Barlow, KY 232179 2807 S3R 12/13/2018 By: NM

Dema :

	3	20 Se 30.5	D outh 72.2	C RSA No. 1 PARTNERSHIP	NERIZON WIRELESS		Suit OH 0.57		C. 31 11 02
DESCRIPTION	ISSUED FOR 90% REVIEW	ADDED DISTANCE TO HOUSE	ADDED BUILDING CALL OUTS AND REVISED SHEET TITLE	FINAL CONSTRUCTION DRAWINGS	d/b/a		R		
REV. DATE	REV. DATE REV. DATE R 0/12/19 K 0 0/22/19 KI 0 02/08/19 FI								
	EV BARLOW 2557 STEVE DENTON ROAD BARLOW, KY 42024						(REFERENCE ONLY)		
	SSU REV PER CON REC			R: TION IGER		DE	ESIGN	 ier	





BENCHMARK SERVICES, INC.

Consulting Engineers & Land Surveyors 318 NORTH MAIN STREET HUNTINGBURG, INDIANA 47542 (812) 683-3049

January 15, 2019

TO: Whom it may concern,

RE: Verizon Site Name: EV Barlow Flood Data Ballard County, KY

Parcel Owner: Myatt Family Trust Charles Myatt & Deena Myatt, Trustees 2557 Steve Denton Road Barlow, KY 42024

Proposed Center of Tower: Latitude 37° 06' 42.145" and the Longitude of 89° 02' 44.583" and a ground elevation of 363.85' AMSL.

A small portion on the Northwest Parent Parcel Property is located in Zone "AE", The proposed Lease Area, the proposed Access & Utility Easements and the proposed Center of Tower are not located in a 100-year flood plain (Zone X) per Flood Hazard Boundary Map, Community Panel No 21007C0085C, dated of July, 7, 2014.

Please see the attached Map 21007C0085C.

Wall

RALPH M. WALLEM BENCHMARK SERVICES, INC. Consulting Engineers & Land Surveyors DATE: 1.15.2019 PROFESSIONAL LAND SURVEYOR NO. 2195



National Flood Hazard Layer FIRMette

37°6'56.47"N



Legend

SE



E FIS REPORT FOR D	ETAILED LEG	END AND INDEX MAP FOR FIRM PANEL LAYOUT
		Without Base Flood Elevation (BFE)
		Zone A, V, A99 With BEE or Depth Zone AF, AO, AH, VF, AP
PECIAL FLOOD		Partulatory Electrony
AZARD AREAS		Regulatory Floodway
		0.2% Annual Chance Flood Hazard Areas
		of 1% annual chance flood with average
		depth less than one foot or with drainage
		areas of less than one square mile Zone X
		Future Conditions 1% Annual
		Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to
IER AREAS OF		Levee, see Notes, zone A
JOOD HAZARD		Area with Flood Risk due to Levee 20he b
	NO SCREEN	Area of Minimal Flood Hazard Zone ¥
		Effective LOMBs
UTHER AREAS		Area of Undetermined Flood Hazard Zone D
GENERAL		Channel, Culvert, or Storm Sewer
STRUCTURES		Levee, Dike, or Floodwall
	(B) 20.2	Cross Sections with 1% Annual Chance
	17.5	Water Surface Elevation
	(a)	Coastal Transect
	~~~~ 513 ~~~~	Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
OTHER		Profile Baseline
FEATURES	<u> </u>	Hydrographic Feature
		Digital Data Available
	<u> </u>	No Digital Data Available
MAP PANELS		Unmapped V
•	The nit	n displayed on the man is an anomyimate
<b>Y</b>	point s	elected by the user and does not represent
	an aut	horitative property location.
This map com	plies with F	EMA's standards for the use of
digital flood m	aps if it is i	not void as described below.
ine pasemap	snown com	ipnes with FEMA's basemap

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 1/15/2019 at 12:38:07 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



Structural Design Report 280' S3R Series SD Self-Supporting Tower Site: Barlow, KY Site Number: 232179

Prepared for: VERIZON WIRELESS by: Sabre Towers & Poles TM

Job Number: 19-5171-TJH

December 13, 2018

Tower Profile	1-2
Foundation Design Summary (Option 1)	3
Foundation Design Summary (Option 2)	4
Maximum Leg Loads	5
Maximum Diagonal Loads	6
Maximum Foundation Loads	7
Calculations	8-26



		в								280'
1.75 S.R.		NONE							1000	
e,		8							_	260'
2.0 S.		NONE							1150	
ين		В					5			240'
2.5 S.	X 1/8	NONE							155	
.R.	L 2 X 2	В				/8"		5	0	220'
2.75 S						(1) 5,		28 @	185	
							2		8	200'
Ľ.							6.7		205	
3.0 S									5	180'
							8.5		217	
S.R.	X 3/16			ų	μ		5.		0	160'
3.25	L 2 X 2		NONE	ĨON	ION		10.2		279	4.401
	16						2		51	140'
S.R.	1/2 X 3/						1		35(	4201
3.75	1/2 X 2	ONE					75'		53	120
	L2	z					13.		36	100
							ţ,		69	100
S.R.	X 3/16					3/4"	15	.6667'	42	201
4.0 5	L 3 X 3					(1)	25'	18 @ 6	83	80
							17.		43	
Π	4						-0		25	60
	1/2 X 1						1		26	401
S.R.	1/2 X 3						75'		08	40
4.25	٢3						20.		58	201
	X 1/4					3/4"	ۍ ت	t0'	38	20
	L 3 X 3	۷	ပ		3	(2)	22.	2@	60	0'
Π					s		÷	leight	t	0
	nals	ontals	als	lagonal	lorizonta	Bolts	ace Wid	Count/	in Weigh	
Legs	Diago	Horizo	Intern.	Sub-D	Sub-F	Brace	Top F	Panel	Sectic	



#### Design Criteria - ANSI/TIA-222-H

Wind Speed (No Ice)	107 mph
Wind Speed (Ice)	30 mph
Design Ice Thickness	1.50 in
Risk Category	I
Exposure Category	С
Topographic Factor Procedure	Method 1 (Simplified)
Topographic Category	1
Ground Elevation	361 ft

#### **Base Reactions**

Total For	undation	Individual A	Footing
Shear (kips)	61.54	Shear (kips)	37.71
Axial (kips)	189.36	Compression (kips)	443
Moment (ft-kips)	8768	Uplift (kips)	382
Torgion (ft.king)	28.07		

### Material List

Display	Value
А	L 3 X 3 X 1/4
В	L 2 X 2 X 1/8
С	L 3 X 3 X 3/16
D	L 2 1/2 X 2 1/2 X 3/16

#### Notes

1) All legs are A572 Grade 50.

2) All braces are A572 Grade 50.

3) All brace bolts are A325-X.

4) The tower model is S3R Series SD.

5) Transmission lines are to be attached to standard 12 hole waveguide ladders with stackable hangers.

6) Azimuths are relative (not based on true north).

7) Foundation loads shown are maximums.

8) All unequal angles are oriented with the short leg vertical.

9) Weights shown are estimates. Final weights may vary.

10) This tower design and, if applicable, the foundation design(s) shown on the following page(s) also meet or exceed the requirements of the 2012 International Building Code.

11) Tower Rating: 97.02%

	Sabre Communications Corporation	Job:	19-5171-TJH	
Sabre Industries	IP.O. Box 658	Customer:	VERIZON WIRELES	iS
Towers and Poles	Sloux City, IA 51102-0658	Site Name:	Barlow, KY 232179	
Information contained herein is the sole pro	perty of Sabre Communications Corporation, constitutes a	Description:	280' S3R	
or part for any purpose whatsoever without i	the prior written consent of Sabre Communications	Date:	12/13/2018	^{By:} NM

**Designed Appurtenance Loading** 

Elev	Description	Tx-Line	Elev	Description	Tx-Line
275	3V-Boom - 12ft Face - 3ft Standoff		245	(6) JAHH-65C-R3B	(12) 1 5/8"
275	(1) RCMDC-6627-PF-48		230	3V-Boom - 10ft Face - 3ft Standoff	
275	(3) 4449 B13 + B5		230	(6) JAHH-65C-R3B	(12) 1 5/8"
275	(3) 8843 B2 + B66A		215	3V-Boom - 10ft Face - 3ft Standoff	
275	(3) CBC78T-DS-43		215	(6) JAHH-65C-R3B	(12) 1 5/8"
275	(6) JAHH-65C-R3B	(2) 1 1/4"	200	3V-Boom - 10ft Face - 3ft Standoff	
260	Leg Dish Mount		200	(6) JAHH-65C-R3B	(12) 1 5/8"
260	(1) 8' Solid Dish W/ Radome	(1) 1 5/8"	185	Leg Dish Mount	
245	3V-Boom - 10ft Face - 3ft Standoff		185	(1) 8' Solid Dish W/ Radome	(1) 1 5/8"

Sabre Communications Corporation 7101 Southbridge Drive P.O. Box 658 Slow City, IA 51102-0658 Prome (712) 258-0680 Prome (712) 278-0640 Prome (712) 278-0600 Prome (712)	Job: 19-5171-TJH Customer: VERIZON WIRELESS Site Name: Barlow, KY 232179
Information contained herein is the sole property of Sabre Communications Corporation, constitutes a	Description: 280' S3R
trade sector is defined by low code cft, 550 and shar his tie reproduced, capied is used in where or part for any purpose whatsoever without the prior written consent of Sabre Communications Corporation	Date: 12/13/2018 By: NM

No.: 19-5171-TJH

Date: 12/13/2018 By: NM

## Customer: VERIZON WIRELESS

Site: Barlow, KY 232179

280 ft. Model S3R Series SD Self Supporting Tower



Sabre Industries

**Towers and Poles** 



CAUTION: Center of tower is not in center of slab.

### Notes:

- Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.
- 2) Rebar to conform to ASTM specification A615 Grade 60.
- 3) All rebar to have a minimum of 3" concrete cover.
- 4) All exposed concrete corners to be chamfered 3/4".
- The foundation design is based on the geotechnical report by Alt & Witzig Engineering, Inc., Project No. 18IN0510 dated: August 30th, 2018.
- 6) See the geotechnical report for compaction requirements, if specified.
- 4.5' of soil cover is required over the entire area of the foundation slab.
- The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.
- Tie overlaps shall be staggered with a nominal 180° separation.

Ŕ	Rebar Schedule per Mat and per Pier						
	(18) #7 vertical rebar w/ hooks at bottom w/						
Pier	#4 rebar ties, two (2) within top 5" of pier then						
	4" C/C						
Mat	(59) #9 horizontal rebar evenly spaced each						
	way top and bottom. (236 total)						
	Anchor Bolts per Leg						
(6) 1.25" c	(6) 1.25" dia. x 63" F1554-105 on a 10" B.C. w/ 7.5" max.						
	projection above concrete.						

Information contained herein is the sole property of Sabre Towers & Poles, constitutes a trade secret as defined by Iowa Code Ch. 550 and shall not be reproduced, copied or used in whole or part for any purpose whatsoever without the prior written consent of Sabre Towers & Poles.

No.: 19-5171-TJH

Date: 12/13/2018 By: NM



Customer: VERIZON WIRELESS

Site: Barlow, KY 232179

280 ft. Model S3R Series SD Self Supporting Tower



ELEVATION VIEW (15.8 cu. yds.) (3 REQUIRED; NOT TO SCALE)

### Notes:

- 1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.
- 2) Rebar to conform to ASTM specification A615 Grade 60.
- 3) All rebar to have a minimum of 3" concrete cover.
- 4) All exposed concrete corners to be chamfered 3/4".
- 5) The foundation design is based on the geotechnical report by Alt & Witzig Engineering, Inc., Project No. 18IN0510 dated: August 30th, 2018.
- 6) See the geotechnical report for drilled pier installation requirements, if specified.
- 7) The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.
- Tie overlaps shall be staggered with a nominal 180° separation.

	Rebar Schedule per Pier				
Dior	(12) #9 vertical rebar w/ #4 ties, two (2) within				
Pier	top 5" of pier then 12" C/C				
	Anchor Bolts per Leg				
(6) 1.25" dia. x 63" F1554-105 on a 10" B.C. w/ 7.5" max.					
projection above concrete.					

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Maximum



Maximum





Licensed to: Sabre Towers and Poles

Maximum





13 dec 2018 13:23:34

Lattico	d Towa				(c) 2015 Cuvmast Inc. $416-726-7452$				
Process	ed und	er licens	e at:	(C	.)2013	Guymast 1	anc. 410-73	0-7433	
Sabre T	owers	and Poles				on:	13 dec 2	018 at: 1	.3:23:34
MAST GE	OMETRY	(ft)							
	======:								
PANEL TYPE	NO.OF LEGS	ELEV BOT	'.АТ Е ТОМ	ELEV.AT TOP	F.W BOTT	AT OM	F.WAT TOP	TYPICAL PANEL HEIGHT	
****	3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	275 260 255 240 235 220 180 160 140 120 100 80 60 40 20 10	.00 .00 .00 .00 .00 .00 .00 .00 .00 .00	$\begin{array}{c} 280.00\\ 275.00\\ 260.00\\ 255.00\\ 240.00\\ 235.00\\ 220.00\\ 215.00\\ 220.00\\ 180.00\\ 160.00\\ 140.00\\ 120.00\\ 100.00\\ 80.00\\ 60.00\\ 40.00\\ 20.00\\ 10.00\\ \end{array}$	5. 5. 5. 5. 10. 12. 13. 17. 19. 22. 23. 24.	00 00 00 00 00 00 00 44 55 52 50 52 50 55 52 50 55 52 50 537 537 525	5.00 5.00 5.00 5.00 5.00 5.44 6.75 8.50 10.25 12.00 13.75 15.50 17.25 19.00 20.75 22.50 23.37	5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00 0.67 6.67 6.67 10.00 10.00	
MEMBER	PROPER	TIES							
		DOTTOM	TOP	Y CECTN		THE		TUEDMAL	
T	YPE	ELEV	ELEV ft	AREA in.sq	OF GY	RAT in	MODULUS ksi	EXPANSN /deg	
	LE LE LE LE LE LE LE LE DI DI DI DI HO HO HO HO BR	$\begin{array}{c} 260.00\\ 240.00\\ 220.00\\ 200.00\\ 160.00\\ 140.00\\ 100.00\\ 60.00\\ 0.00\\ 140.00\\ 140.00\\ 140.00\\ 140.00\\ 100.00\\ 20.00\\ 0.00\\ 275.00\\ 235.00\\ 235.00\\ 235.00\\ 235.00\\ 215.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.0$	280.00 260.00 240.00 220.00 160.00 140.00 140.00 140.00 140.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 20.00 280.00 280.00 280.00 280.00 20.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 280.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.000	$\begin{array}{c} 2.405\\ 3.142\\ 4.909\\ 5.940\\ 7.069\\ 8.296\\ 11.045\\ 12.566\\ 14.186\\ 0.484\\ 0.715\\ 0.902\\ 1.090\\ 1.688\\ 1.438\\ 0.484\\ 0.484\\ 0.484\\ 0.484\\ 1.438\\ 1.090\\ \end{array}$		438 438 438 438 438 438 438 438 438 438	29000. 0 29000. 0	.0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117 .0000117	
FACTORE	D MEMBI	ER RESIST	ANCES						
BOTTOM ELEV ft	TOP ELEV ft	L COMP kip	EGS TENS kip	DIAGON COMP kip	IALS TENS kip	HOR COMP kip	IZONTALS TENS kip	INT BR COMP kip	ACING TENS kip
275.0 260.0 255.0 240.0 235.0 220.0	280.0 275.0 260.0 255.0 240.0 235.0	28.89 28.89 49.29 49.29 112.60 112.60	108.24 108.24 120.41 120.41 220.89 220.89	7.62 7.62 7.62 7.62 7.62 7.62 7.62	7.62 7.62 7.62 7.62 7.62 7.62 7.62	7.37 0.00 7.37 0.00 7.37 0.00	7.37 0.00 7.37 0.00 7.37 0.00	$\begin{array}{c} 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \end{array}$	0.00 0.00 0.00 0.00 0.00 0.00

					1	19-5171-1	гэн		
215.0	220.0	153.15	267.28	7.62	7.62	7.37	7.37	0.00	0.00
200.0	215.0	153.15	267.28	7.62	7.62	0.00	0.00	0.00	0.00
180.0	200.0	199.21	318.09	7.62	7.62	0.00	0.00	0.00	0.00
160.0	180.0	199.21	318.09	5.68	5.68	0.00	0.00	0.00	0.00
140.0	160.0	250.56	373.31	6.19	6.19	0.00	0.00	0.00	0.00
120.0	140.0	291.83	457.90	8.39	8.39	0.00	0.00	0.00	0.00
100.0	120.0	291.83	457.90	6.77	6.77	0.00	0.00	0.00	0.00
80.0	100.0	354.16	457.90	10.03	10.03	0.00	0.00	0.00	0.00
60.0	80.0	354.16	457.90	8.35	8.35	0.00	0.00	0.00	0.00
40.0	60.0	421.75	457.90	15.39	15.39	0.00	0.00	0.00	0.00
20.0	40.0	421.75	457.90	13.14	13.14	0.00	0.00	0.00	0.00
10.0	20.0	505.61	545.12	14.02	14.02	0.00	0.00	0.00	0.00
0.0	10.0	505.61	545.12	12.71	12.71	13.05	13.05	9.39	9.39

* Only 3 condition(s) shown in full
* RRUs/TMAs were assumed to be behind antennas _____ _____ * Some wind loads may have been derived from full-scale wind tunnel testing _____

LOADING CONDITION A -----_____ PL - 0

107 mph wind with no ice. Wind Azimuth: 0.

MAST LOADING ________

LOAD	ELEV	ELEV APPLY., LOAD., AT		FORCES.		MOMENTS	
TYPE	ft	RADIUS	AZI AZI	HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
с с с с с	275.0 245.0 230.0 215.0 200.0	$\begin{array}{c} 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \end{array}$	$\begin{array}{cccc} 0.0 & 0.0 \\ 0.0 & 0.0 \\ 0.0 & 0.0 \\ 0.0 & 0.0 \\ 0.0 & 0.0 \\ 0.0 & 0.0 \end{array}$	1.98 1.74 1.71 1.69 1.66	2.79 2.10 2.10 2.10 2.10 2.10	$\begin{array}{c} 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \end{array}$	$\begin{array}{c} 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\end{array}$
	280.0 275.0 260.0 245.0 245.0 245.0 240.0 230.0 230.0 220.0 215.0 220.0 220.0 220.0 220.0 220.0 220.0 220.0 220.0 220.0 220.0 200.0 185.0 160.0 140.0 120.0 160.0 120.0 100.0 100.0 40.0 20.0 100.0 20.0 100.0 20.0 100.0 20.0 2	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	0.06 0.07 0.07 0.08 0.07 0.11 0.12 0.11 0.12 0.11 0.13 0.14 0.14 0.14 0.14 0.15 0.15 0.15 0.15 0.18 0.18 0.18 0.19 0.19 0.19 0.19 0.19 0.20 0.20 0.21 0.20 0.21 0.20 0.215 0.15 0.15 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.20 0.21 0.20 0.21 0.20 0.21 0.20 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.18 0.19 0.19 0.20 0.20 0.20 0.20 0.20 0.15 0.15 0.18 0.18 0.19 0.19 0.19 0.20 0.20 0.20 0.15 0.15 0.18 0.18 0.18 0.18 0.19 0.19 0.19 0.20 0.20 0.20 0.15 0.15 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.15 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.1	$\begin{array}{c} 0.05\\ 0.06\\ 0.06\\ 0.07\\ 0.06\\ 0.08\\ 0.011\\ 0.10\\ 0.12\\ 0.14\\ 0.15\\ 0.19\\ 0.19\\ 0.19\\ 0.22\\ 0.23\\ 0.27\\ 0.27\\ 0.27\\ 0.31\\ 0.31\\ 0.31\\ 0.31\\ 0.39\\ 0.39\\ 0.39\\ 0.34\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41\\ 0.41$	$\begin{array}{c} 0.00\\ 0.01\\ 0.02\\ 0.02\\ 0.05\\ 0.05\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.01\\ 0.12\\ 0.12\\ 0.14\\ 0.16\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.16\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.16\\ 0.15\\ 0.15\\ 0.16\\ 0.15\\ 0.15\\ 0.16\\ 0.15\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.16\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\$	0.00 0.03 0.03 0.03 0.08 0.08 0.08 0.10 0.10 0.10 0.10 0.10 0.06 0.06 0.06 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.06 0.066 0.066 0.066 0.066 0.067 0.07 0.07 0.07 0.07 0.07 0.07 0.07 0.060 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066 0.066
# ANTENNA LOADING

ANTENNA TYPE	ELEV ft	AZI	ATTACH RAD ft	MENT AZI	AXIAL kip	ANTEN SHEAR kip	NA FORCES GRAVITY kip	TORSION ft-kip
STD+R STD+R	260.0 185.0	0.0	4.4 6.2	$\begin{array}{c} 0.0\\ 0.0\end{array}$	1.40 1.31	$0.00 \\ 0.00$	$0.40 \\ 0.40$	0.00 0.00
LOADING CONDITION	k ==== o ice. Wi	nd Az	imuth:	0.				PL - 0

# MAST LOADING

•			•	_	-	 _	• •	-
_					_	 _	_	_
-	-	_		-	_	 _	_	_

LOAD TYPE	ELEV ft	APPLYLO RADIUS ft	ADAT AZI	LOAD AZI	FORCES HORIZ kip	DOWN kip	VERTICAL ft-kip	NTS TORSNAL ft-kip
c c c c c c	275.0 245.0 230.0 215.0 200.0	$\begin{array}{c} 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\end{array}$	$0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 $	$\begin{array}{c} 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\end{array}$	1.98 1.74 1.71 1.69 1.66	2.09 1.58 1.58 1.58 1.58	$\begin{array}{c} 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \end{array}$	$\begin{array}{c} 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\end{array}$
D D D D D D D D D D D D D D D D D D D	280.0 275.0 275.0 260.0 245.0 245.0 245.0 240.0 230.0 230.0 220.0 220.0 220.0 220.0 220.0 220.0 20.0 20.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 160.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 100.0 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0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	$\begin{array}{c} 180.0\\ 42.0\\ 42.0\\ 42.0\\ 42.0\\ 42.0\\ 42.0\\ 42.0\\ 42.0\\ 57.2\\ 83.4\\ 20.0\\ 57.2\\ 87.2\\ 87.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2\\ 77.2$	$\begin{array}{c} 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0$	0.06 0.07 0.08 0.07 0.11 0.11 0.12 0.11 0.13 0.14 0.14 0.14 0.14 0.15 0.15 0.17 0.18 0.18 0.19 0.19 0.19 0.19 0.19 0.20 0.21 0.20 0.21 0.20 0.21 0.20 0.15 0.15 0.15 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.20 0.21 0.20 0.21 0.215 0.215 0.15 0.15 0.17 0.18 0.19 0.19 0.19 0.19 0.19 0.19 0.19 0.20 0.21 0.20 0.21 0.215 0.15 0.15 0.15 0.15 0.19 0.20 0.21 0.20 0.21 0.20 0.21 0.20 0.21 0.20 0.21 0.20 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.19 0.20 0.21 0.20 0.21 0.20 0.21 0.20 0.21 0.20 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.15 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18 0.18	0.04 0.04 0.04 0.05 0.05 0.06 0.08 0.09 0.11 0.11 0.11 0.14 0.14 0.14 0.15 0.20 0.20 0.20 0.23 0.23 0.23 0.24 0.29 0.29 0.20 0.20 0.23 0.23 0.23 0.24 0.29 0.29 0.23 0.23 0.23 0.24 0.26 0.31 0.31	0.00 0.01 0.01 0.01 0.03 0.03 0.03 0.03 0.05 0.05 0.05 0.05 0.04 0.04 0.04 0.04 0.04 0.05 0.05 0.05 0.05 0.05 0.04 0.04 0.06 0.07 0.06 0.07 0.08 0.09 0.09 0.10 0.11 0.12 0.12 0.12	$\begin{array}{c} 0.00\\ 0.03\\ 0.03\\ 0.03\\ 0.03\\ 0.08\\ 0.08\\ 0.08\\ 0.08\\ 0.10\\ 0.10\\ 0.10\\ 0.10\\ 0.10\\ 0.10\\ 0.10\\ 0.10\\ 0.06\\ 0.06\\ 0.06\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.07\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 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0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\ 0.06\\$

ANTENNA			ATTAC	HMENT		ANTEN	NA FORCES	
TYPE	ELEV	AZI	RAD	AZI	AXIAL	SHEAR	GRAVITY	TORSION
	ft		ft		kip	kip	kip	ft-kip
STD+R STD+R	260.0 185.0	0.0 0.0	4.4 6.2	0.0	1.40 1.31	0.00	$\begin{array}{c} 0.30\\ 0.30\end{array}$	0.00 0.00

19-5171-тэн

LOADING CONDITION AU ______

30 mph wind with 1.5 ice. Wind Azimuth: 0.

PL - 0

MAST LOADING

LOAD TYPE	ELEV	APPLYLO RADIUS	ADAT	LOAD	FO	RCES	MOM	IENTS TORSNAL
	ft	ft		,	kip	kip	ft-kip	ft-kip
c	275.0	0.00	0.0	0.0	0.29	5.78	0.00	0.00
č	230.0	0.00	ŏ.ŏ	0.0	0.25	4.64	0.00	0.00
c	215.0	0.00	0.0		0.24	4.63	0.00	0.00
	200.0	0.00	190.0	0.0	0.24	0.10	0.00	0.00
D	275.0	0.00	180.0	0.0	0.01	0.19	0.00	0.00
D	275.0	0.00	42.0		0.01	0.19	0.07	0.01
D	260.0	0.00	42.0	0.0	0.01	0.24	0.08	0.01
D	255.0	0.00	42.0		0.01	0.24	0.08	0.01
D	245.0	0.00	42.0	0.0	0.01	0.21	0.08	0.01
D	245.0	0.00	42.0		0.01	0.26	0.20	0.01
D	240.0	0.00	42.0	0.0	0.01	0. <u>3</u> 2	0.20	0.01
D	235.0	0.00	42.0		0.01	0.32	0.20	0.01
D	230.0	0.00	42.0	0.0	0.01	0.29	0.20	0.01
D	230.0	0.00	62.1	. 0.0	0.02	0.35	0.21	$0.01 \\ 0.01$
D	220.0	0.00	63.2	0.0	0.02	0.40	0.21	0.01
D	215.0	0.00	63.2	0.0	0.02	0.40	0.21	0.01
D	205.0	0.00	93.2	0.0	0.02	0.43	0.22	0.01
D	205.0	$0.00 \\ 0.00$	95.1	. 0.0	0.02	0.43	0.21	$0.01 \\ 0.01$
D	200.0	ŏ.ŏŏ	85.6	ŏ.ŏ	0.02	0.52	0.17	0.00
D	185.0	$0.00 \\ 0.00$	87.6	0.0	0.02	0.53	$0.15 \\ 0.19$	0.00
D	160.0	0.00	80.5	0.0	0.02	0.56	0.17	0.00
D	$160.0 \\ 140.0$	0.00	76.3	0.0	0.02	0.59	0.22	0.00
D	140.0	0.00	74.3	0.0	0.02	0.63	0.26	0.00
D D	120.0	0.00	75.2	0.0	0.02	0.64	0.23	0.00
D	100.0	0.00	73.4	0.0	0.02	0.66	0.27	0.00
D	80.0	0.00	71.4	0.0	0.02	0.71	0.32	0.00
D	80.0	0.00	70.4	0.0	0.02	0.72	0.36	0.00
D	60.0	0.00	69.6	0.0	0.02	0.82	0.39	0.00
D	40.0	0.00	70.0		0.02	0.84	0.37	0.00
D	20.0	0.00	69.3	0.0	0.02	0.84	0.40	0.00
D	$20.0 \\ 10.0$	0.00	68.4	0.0	0.02	0.74	0.50	0.00
D	10.0	0.00	68.4	0.0	0.02	0.99	0.62	0.00
D	0.0	0.00	00.4	0.0	0.02	0.99	0.02	0.00
ANTEN	INA LOAD	ING ===						
	ANTENN	A		ATT	ACHMENT		ANTENNA FO	RCES
TYPE		Ē	LEV	AZI RAI	D AZI	AXIAL	SHEAR GRAV	ITY TORSION
		1				kip A 10	кір кір	пс-кір
STD+R STD+R		26	0.0 5.0	0.0 4.4	4 0.0 2 0.0	0.12 0.11	0.00 1	.57 0.00
MAXIMU	M ANTEN	NA AND REF	LECTOR	ROTATI	DNS:			

.....BEAM DEFLECTIONS (deg).....

				19-5171-TJ	н	
ft	deg	*	PITCH	YAW	ROLL	TOTAL
260.0 185.0	$0.0 \\ 0.0$	STD+R STD+R	1.315 0.827	J 0.469 J 0.218	AL -1.551 AL -0.978	s 1.396 b s 0.855 j

# MAXIMUM TENSION IN MAST MEMBERS (kip)

ELEV ft	LEGS	DIAG	HORIZ	BRACE
280.0			0.24 C	0.00 A
275.0	0.16 AE	0.38 5	0.03 g	0.00 A
270.0	0.63 k	1.16 n	0.02 A	0.00 A
265.0	3.78 k	1.35 D	0.03 1	0.00 A
260.0	6.96 k	1.51 AF	0 49 AF	
200.0	11.26 k	2.58 D	0.08 м	0.00 A
255.0	17.18 k	2.87 D	0.00 m	0.00 A
250.0	23.37 k	2.96 AF	0.02 m	0.00 A
245.0	30.25 k	4.07 D	0.07 A	0.00 A
240.0	39.77 k	4.27 n	0.51 A	0.00 A
235.0	49.79 k	4.63 D	0.18 A	0.00 A
230.0	60.71 k	5.68 D	0.07 AC	0.00 A
225.0	74 39 k	6 01 D	0.20 A	0.00 A
220.0	94.01 k		0.55 AC	0.00 A
215.0	04.01 K	3.43 K	0.16 A	0.00 A
210.0	91.47 K	3.91 X	0.03 A	0.00 A
205.0	98.93 K	3.77 AH	0.16 A	0.00 A
200.0	106.43 k	3.75 X	0.01 AU	0.00 A
195.0	113.33 k	4.40 k	0.12 A	0.00 A
190.0	121.72 k	4.35 s	0.07 e	0.00 A
185.0	129.31 k	4.37 AH	0.09 A	0.00 A
180 0	137.53 k	5.29 AH	0.09 4	
175 0	144.93 k	5.44 F	0.09 0	
170.0	153.56 k	5.15 AH	0.06 A	0.00 A
1/0.0	160.78 k	5.31 F	0.06 e	0.00 A
165.0	168.77 k	5.14 AH	0.07 A	0.00 A
160.0	175.84 k	5.31 F	0.05 J	0.00 A
155.0	183.39 k	5.21 AH	0.05 A	0.00 A
150.0	190.30 k	5.37 F	0.05 M	0.00 A
145.0	197.57 k	5.33 AH	0.05 A	0.00 A
140.0	205 47 k	5 90 F	0.05 M	0.00 A
133.3	203.43 K	5.00 F	0.05 A	0.00 A
126.7	214.// K	5.03 AE	0.04 M	0.00 A
120.0	223.61 K	5.99 F	0.05 A	0.00 A
113.3	232.70 k	6.12 AE	0.04 A	0.00 A
106.7	241.44 k	6.23 F	0.04 A	0.00 A
	250.34 k	6.43 AE		

			19-51	71-тэн
100.0	258 06 k	 6 54 i	0.03 A	0.00 A
93.3	238.90 K	0.34 J	0.04 A	0.00 A
86.7	267.70 k	6.80 AE	0.03 A	0.00 A
80.0	276.27 k	6.94 U	0.03.4	0.00 A
<b>50.0</b>	284.97 k	7.20 AE	0.03 A	0.00 A
73.3	293.52 k	7.36 U	0.03 A	0.00 A
66.7		7 60 AF	0.03 A	0.00 A
60.0			0.03 A	0.00 A
53.3	310.05 K	7.78 0	0.03 A	0.00 A
46.7	319.16 k	8.03 AE	0.02 A	0.00 A
40.0	327.59 k	8.22 U	0.02.4	0.00 A
40.0	336.10 k	8.48 U	0.02 A	0.00 A
33.3	344.54 k	8.70 U	0.04 AE	0.00 A
26.7	353 00 k	8 06 c	0.16 AC	0.00 A
20.0		0.50 5	0.22 A	0.00 A
10.0	369.91 k	11.17 AT	0.62 k	0.00 AF
0.0	368.97 k	11.50 AT	0 00 4	0 00 4
0.0			0.00 A	0.00 A

# MAXIMUM COMPRESSION IN MAST MEMBERS (kip)

ELEV ft	LEGS	DIAG	HORIZ	BRACE
280.0	-0.25.0	-0.37 A	-0.24 U	0.00 A
275.0	-0.25 C	-0.37 A	-0.02 y	0.00 A
270.0	-6 01 6	-1.17 V	-0.02 AC	0.00 A
265.0	-0.01 5	-1.54 V	-0.01 AO	0.00 A
260.0	-9.43 5	-1.33 0	-0.62 F	0.00 A
255.0	-14.38 5	-2.73 G	-0.05 AC	0.00 A
250.0	-20.00 5	-2.77 AF	-0.02 CA	0.00 A
245.0	-20.00 5	-3.04 D	-0.05 AC	0.00 A
240.0	-35.31 5	-4.03 V	-0.43 AC	0.00 A
235.0	-45.07 5	-4.37 D	-0.16 AC	0.00 A
230.0	-55.75 5	-4.57 n	-0.07 A	0.00 A
225.0	-68.29 5	-5.73 D	-0.17 AC	0.00 A
220.0	-82.62 5	-6.01 D	-0.64 A	0.00 A
215.0	-92.65 5	-3.64 5	-0.14 AC	0.00 A
210.0	-102.21 S	-3.87 AH	-0.02 AC	0.00 A
205.0	-110.19 5	-3.8/ 5	-0.13 AC	0.00 A
200.0	-118.42 5	-3.74 X	0.00 AC	0.00 A
195.0	-127.18 5	-4.58 S	-0.10 AC	0.00 A
190.0	-136.47 S	-4.35 AH	-0.04 w	0.00 A
185.0	-144.73 S	-4.48 S	-0.09 k	0.00 A

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180.0	-154.26 S	-5.59 F	-0.05	t	0.00 A
175.0	-162.45 s	-5.21 AH	-0.07	AC	0.00 A
170.0	-171.51 S	-5.37 F	-0 04	40	0 00 4
165 0	-179.61 s	-5.14 AH	-0.06	AC	
160.0	-188.17 s	-5.31 F	0.00	AC	0.00 A
100.0	-196.14 s	-5.19 AE	-0.04	AU	0.00 A
155.0	-204.45 s	-5.35 F	-0.04	AC	0.00 A
150.0	-212.31 S	 -5.35 AE	-0.04	AO	0.00 A
145.0	-220.40 s	 -5.44 F	-0.04	AC	0.00 A
140.0	-229.45 \$	 -5.89 U	-0.04	AO	0.00 A
133.3	-240 11 s	-5 91 F	-0.05	AC	0.00 A
126.7	-240.11 3	-5.51 F	-0.04	AC	0.00 A
120.0	-230.42 S	-6.10 U	-0.04	AC	0.00 A
113.3	-260.90 S	-6.20 0	-0.03	AC	0.00 A
106.7	-271.14 S	-6.46 U	-0.04	AC	0.00 A
100.0	-281.50 S	-6.54 U	-0.03	AC	0.00 A
93.3	-291.73 s	-6.80 U	-0.03	AC	0.00 A
86.7	-302.12 S	-6.93 U	-0.03	AC	
80.0	-312.42 s	-7.20 U	0.03		0.00 A
72.2	-322.83 S	-7.35 U	-0.03	AC	0.00 A
/3.3	-333.17 s	-7.59 U	-0.02	AC	0.00 A
66.7	-343.59 s	-7.75 U	-0.03	AC	0.00 A
60.0	-354.04 s	 -8.01 U	-0.02	AC	0.00 A
53.3	-364.65 5	 -8.20 U	-0.02	AC	0.00 A
46.7			-0.02	AC	0.00 A
40.0		• • • • • • • • • • • • • • • • • • •	-0.02	AC	0.00 A
33.3	-363.92 3	-8.04 0	-0.05	с	0.00 A
26.7	-390.60 S	-8.82 U	-0.19	A	0.00 A
20.0	-407.30 S	-8.96 U	-0.18	AC	0.00 A
10.0	-427.23 S	-11.91 s	-0.75	s	0.00 D
0.0	-428.48 S	-12.27 s	0.00	Δ	0.00 🗛
			2.00		

# FORCE/RESISTANCE RATIO IN LEGS

ΜΔςτ	LE	G COMPRE	SSION -		LEG TENSION		
ELEV ft	MAX COMP	COMP RESIST	RESIST RATIO	MAX TENS	TENS RESIST	RESIST RATIO	
280.00							
275 00	0.25	28.89	0.01	0.16	108.24	0.00	
273.00	2.64	28.89	0.09	0.63	108.24	0.01	
2/0.00	6.01	28.89	0.21	3.78	108.24	0.03	
265.00	9.45	28.89	0.33	6.96	108.24	0.06	
260.00	14.58	49.29	0.30	11.26	120.41	0.09	
255.00	20.66	49.29	0.42	17.18	120.41	0.14	

250 00					19	-21/1-1
250.00	26.88	49.29	0.55	23.37	120.41	0.19
245.00	35.31	49.29	0.72	30.25	120.41	0.25
240.00	45.07	112.60	0.40	39.77	220.89	0.18
235.00	55.75	112.60	0.50	49.79	220.89	0.23
230.00	68.29	112.60	0.61	60.71	220.89	0.27
225.00	82.62	112.60	0.73	74.39	220.89	0.34
220.00	92.65	153.15	0.60	84.01	267.28	0.31
215.00	102.21	153.15	0.67	91.47	267.28	0.34
210.00	110,19	153.15	0.72	98.93	267.28	0.37
205.00	118 42	153.15	0 77	106.43	267.28	0.40
200.00	127 18	100 21	0.64	113 33	318 09	0.40
195.00	127.10	100 21		121 72	318 00	0.30
190.00	144 72	100 21	0.03	121.72	218 00	
185.00	164.75	199.21	0.73	129.31	210.09	0.41
180.00	104.20	199.21	0.77	137.33	310.09	0.45
175.00	162.45	199.21	0.82	144.93	318.09	0.40
170.00	1/1.51	199.21	0.86	153.56	318.09	0.48
165.00	179.61	199.21	0.90	160.78	318.09	0.51
160.00	188.17	199.21	0.94	168.77	318.09	0.53
155.00	196.14	250.56	0.78	175.84	373.31	0.47
150.00	204.45	250.56	0.82	183.39	373.31	0.49
145.00	212.31	250.56	0.85	190.30	373.31	0.51
140.00	220.40	250.56	0.88	197.57	373.31	0.53
133 33	229.45	291.83	0.79	205.43	457.90	0.45
126 67	240.11	291.83	0.82	214.77	457.90	0.47
120.00	250.42	291.83	0.86	223.61	457.90	0.49
112 22	260.90	291.83	0.89	232.70	457.90	0.51
106 67	271.14	291.83	0.93	241.44	457.90	0.53
100.07	281.50	291.83	0.96	250.34	457.90	0.55
100.00	291.73	354.16	0.82	258.96	457.90	0.57
93.33	302.12	354.16	0.85	267.70	457.90	0.58
80.67	312.42	354.16	0.88	276.27	457.90	0.60
80.00	322.83	354.16	0.91	284.97	457.90	0.62
73.33	333.17	354.16	0.94	293.52	457.90	0.64
66.67	343.59	354.16	0.97	302.16	457.90	0.66
60.00	354.04	421.75	0.84	310.65	457.90	0.68
53.33	364.65	421.75	0.86	319.16	457.90	0.70
46.67	375.25	421.75	0.89	327.59	457.90	0.72
40.00	385.92	421.75	0.92	336.10	457.90	0.73
33.33	396.60	421.75	0.94	344.54	457.90	0.75
26.67	407.30	421.75	0.97	353.00	457.90	0.77

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20.00						
10.00	427.23	505.61	0.84	369.91	545.12	0.68
10.00	428.48	505.61	0.85	368.97	545.12	0.68
0.00						

FORCE/RESISTANCE RATIO IN DIAGONALS

маст	- DIA	G COMPRE	SSION -		DIAG TEN	SION
ELEV	MAX	COMP	RESIST	MAX	TENS	RESIST
ft	COMP	RESIST	RATIO	TENS	RESIST	RATIO
280.00	0.37	7.62	0.05	0.38	7.62	0.05
275.00	1.17	7.62	0.15	1.16	7.62	0.15
270.00	1.34	7.62	0.18	1.35	7.62	0.18
265.00	1.55	7.62	0.20	1.51	7.62	0.20
260.00	2.75	7.62	0.36	2.58	7.62	0.34
255.00	2 77	7 62	0 36		7 62	0 38
250.00					7 67	0.30
245.00			0.40		7.02	
240.00	4.05	7.02		4.07	7.02	0.33
235.00	4.37	/.62	0.57	4.2/	/.62	0.56
230.00	4.57	7.62	0.60	4.63	7.62	0.61
225.00	5.73	7.62	0.75	5.68	7.62	0.75
220.00	6.01	7.62	0.79	6.01	7.62	0.79
215 00	3.64	7.62	0.48	3.43	7.62	0.45
210.00	3.87	7.62	0.51	3.91	7.62	0.51
210.00	3.87	7.62	0.51	3.77	7.62	0.49
205.00	3.74	7.62	0.49	3.75	7.62	0.49
200.00	4.58	7.62	0.60	4.40	7.62	0.58
195.00	4.35	7.62	0.57	4.35	7.62	0.57
190.00	4.48	7.62	0.59	4.37	7.62	0.57
185.00	5.59	7.62	0.73	5.29	7.62	0.69
180.00	5.21	5.68	0.92	5.44	5.68	0.96
175.00	5.37	5.68	0.95	5.15	5.68	0.91
170.00			0 91	5 31		0.01
165.00		5.00		5.51 		
160.00		 C 10	0.95	J.14	5.00	0.90
155.00	5.19	6.19	0.84	5.31	6.19	0.86
150.00	5.35	6.19	0.86	5.21	6.19	0.84
145.00	5.35	6.19	0.86	5.37	6.19	0.87
140.00	5.44	6.19	0.88	5.33	6.19	0.86
133.33	5.89	8.39	0.70	5.80	8.39	0.69
126 67	5.91	8.39	0.70	5.83	8.39	0.69
120.00	6.16	8.39	0.73	5.99	8.39	0.71
112 22	6.20	6.77	0.92	6.12	6.77	0.90
112.22	6.46	6.77	0.95	6.23	6.77	0.92

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106.67 -	6.54	6.77	0.97	6.43	6.77	0.95		
100.00 -	6.80	10.03	0.68	6.54	10.03	0.65		
93.33 -	6.93	10.03	0.69	6.80	10.03	0.68		
86.67 -	7.20	10.03	0.72	6.94	10.03	0.69		
80.00 -	7.35	8.35	0.88	7.20	8.35	0.86		
73.33 -	7.59	8.35	0.91	7.36	8.35	0.88		
66.67 -	7.75	8.35	0.93	7.60	8.35	0.91		
60.00 -	8.01	15.39	0.52	7.78	15.39	0.51		
53.33 -	8.20	15.39	0.53	8.03	15.39	0.52		
46.67 -	8.46	15.39	0.55	8.22	15.39	0.53		
40.00 -	8.64	13.14	0.66	8.48	13.14	0.65		
33.33 -	8.82	13.14	0.67	8.70	13.14	0.66		
26.67 -	8.96	13.14	0.68	8.96	13.14	0.68		
20.00 -	11.91	14.02	0.85	11.17	14.02	0.80		
10.00 -	12.27	12.71	0.97	11.50	12.71	0.91		
0.00 -					• 、			
MAXIMUM	INDIVID	JAL FOUND	ATION LC	ADS: (k [.]	ip) ===			
		LOAD	-COMPONE	NTS		т	TAL	
NOR	TH	EAST		OWN	UPLIF	SF		
37.7	/1 S	31.84 e	443	50 S	-381.60	) к 37	./1 S	
MAXIMUM	TOTAL LO	DADS ON FO	DUNDATIC	DN : (kij	p & kip-1	ft)		
								OBSTON
NORTH	EAST	TOTAL @ 0.0	DOWN	N	ORTH	EAST	TOTAL @ 0.0	ORSION
61.5 S	52.9 b	61.5 S	189.4 ВК	87	67.6 S	7480.3 b	8767.6 S	29.0 AT
Latticed Processe	d Tower A	Analysis license	Unguyec	I)	(c)	)2015 Guyma	st Inc. 416	-736-7453
Sabre To	owers and	d Poles				on: 13 d	lec 2018 at	: 13:24:12
======	********							
******	*******	*******	* servi	ice Load	Conditio	**************************************	***********	********
*****	******	******	*****	*******	******	*******	******	****
				=======				
* Only 1 * RRUs/TM	conditio MAs were	on(s) show assumed	wn in fu to be be	hind an	tennas			
* Some w	ind loads	s may have	e been o	derived	from ful	l-scale wir	nd tunnel te	sting
LOADING	CONDITIO	======================================						

MAST LOADING

LOAD TYPE	ELEV ft	APPLYLO RADIUS ft	ADAT AZI	LOAD AZI	FOR HORIZ kip	CES DOWN kip	MOME VERTICAL ft-kip	NTS TORSNAL ft-kip
c c c c c c	275.0 245.0 230.0 215.0 200.0	$\begin{array}{c} 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\end{array}$	$0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0$	$0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0 \\ 0.0$	0.62 0.55 0.54 0.53 0.52	2.33 1.75 1.75 1.75 1.75	$\begin{array}{c} 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \\ 0.00 \end{array}$	$\begin{array}{c} 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00 \end{array}$
000000000000000000000000000000000000000	$\begin{array}{c} 280.0\\ 260.0\\ 245.0\\ 245.0\\ 240.0\\ 240.0\\ 230.0\\ 220.0\\ 220.0\\ 220.0\\ 220.0\\ 220.0\\ 215.0\\ 200.0\\ 185.0\\ 160.0\\ 185.0\\ 160.0\\ 140.0\\ 120.0\\ 120.0\\ 140.0\\ 120.0\\ 100.0\\ 80.0\\ 60.0\\ 40.0\\ 20.0\\ 10.0\\ 10.0\\ 10.0\\ 0.0\\ \end{array}$	$\begin{array}{c} 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\ 0.00\\$	180.0 42.0 42.0 42.0 42.0 56.0 57.2 57.2 87.2 90.5 57.2 87.2 90.5 93.1 76.2 77.5 76.2 77.5 73.4 74.1 72.9 71.5 70.8 71.1	$\begin{array}{c} 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0$	0.02 0.03 0.03 0.03 0.04 0.04 0.04 0.05 0.05 0.05 0.05 0.05 0.05 0.05 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06	0.05 0.06 0.07 0.09 0.10 0.12 0.13 0.13 0.16 0.16 0.16 0.16 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.22 0.23 0.26 0.26 0.26 0.26 0.26 0.26 0.33 0.33 0.34	0.00 0.02 0.01 0.04 0.04 0.04 0.05 0.05 0.05 0.05 0.05	0.00 0.01 0.01 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.03 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02
ANTEN	NA LOAD	ING ===			CUMENT			
ТҮРЕ	. AN I ENN	AE f	LEV AZ t	I RAD ft	AZI	AXIAL S kip	SHEAR GRAVI kip kip	TY TORSION ft-kip
STD+R STD+R		26 18	0.0 0. 5.0 0.	0 4.4 0 6.2	0.0 0.0	0.44 0.41	0.00 0	.34 0.00 .34 0.00
MAXIMU	M MAST	DISPLACEME	NTS:					
	ELEV ft	DE NORTH	FLECTION	s (ft)	DOWN	TILTS NORTH	(DEG) EAST	TWIST DEG
2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	80.0 75.0 70.0 65.0 60.0 55.0 50.0 45.0 45.0 40.0	1.192 s 1.148 s 1.104 s 1.061 s 1.017 s 0.974 s 0.932 s 0.889 s 0.848 s	-1.009 -0.972 -0.935 -0.898 -0.862 -0.825 -0.789 -0.753 -0.718	) ) ) ) ) ) ) ) ) ) )	0.012 S 0.012 S 0.012 S 0.011 S 0.011 S 0.011 S 0.011 S 0.011 S 0.010 S	0.498 S 0.498 S 0.497 S 0.495 S 0.492 S 0.487 S 0.480 S 0.480 S 0.472 S 0.460 S	-0.422 J -0.423 J -0.422 J -0.420 J -0.416 J -0.412 J -0.406 J -0.399 J -0.389 J	-0.147 d -0.147 d -0.147 d -0.147 d -0.148 d -0.141 d -0.134 d -0.128 d -0.122 d

				10-5171-T	10	
235.0 230.0 225.0 220.0 215.0 210.0 205.0 200.0	0.807 S 0.768 S 0.728 S 0.691 S 0.656 S 0.621 S 0.588 S 0.557 S	-0.684 J -0.650 J -0.617 J -0.586 J -0.556 J -0.527 J -0.499 J -0.472 J	0.010 S 0.010 S 0.009 S 0.009 S 0.009 S 0.009 S 0.009 S 0.009 S	19-5171-T: 0.451 S 0.425 S 0.425 S 0.407 S 0.392 S 0.378 S 0.362 S 0.347 S	DH -0.381 J -0.371 J -0.359 J -0.344 J -0.331 J -0.319 J -0.306 J -0.293 J -0.293 J	-0.116 d -0.110 d -0.098 d -0.092 d -0.088 d -0.083 d -0.075 d
190.0	0.497 S	-0.421 J	0.008 S	0.322 S	-0.272 J	-0.072 d
180.0	0.441 S	-0.374 J	0.007 S	0.297 S	-0.251 J	-0.063 d
170.0	0.389 S	-0.330 J	0.007 S	0.271 S	-0.229 J	-0.052 d
160.0	0.341 S	-0.290 J	0.007 S	0.245 S	-0.207 J	-0.047 d
155.0	0.319 S	-0.272 J -0 254 J	0.006 S	0.234 S	-0.198 J -0 189 J	-0.039 d
145.0	0.279 s	-0.237 j	0.006 s	0.212 s	-0.179 j	-0.034 d
140.0	0.260 S	-0.221 J	0.006 S	0.201 S	-0.170 J	-0.031 d
126.7	0.213 s	-0.182 j	0.005 s	0.178 S	-0.151 j	-0.026 d
120.0	0.192 S	-0.164 J	0.005 S	0.167 S	-0.142 J	-0.024 d
106.7	0.172 S	-0.131 J	0.005 S	0.145 S	-0.123 J	-0.022 u
100.0	0.136 S	-0.116 J	0.004 s	0.134 s	-0.114 ]	-0.018 d
93.3 86.7	0.120 S 0.104 S	-0.102 J	0.004 S	0.125 S	-0.106 J -0.098 1	-0.016 d
80.0	0.090 s	-0.077 J	0.004 s	0.105 s	-0.090 J	-0.013 d
/3.3	0.077 S	-0.066 J -0.055 J	0.003 S	0.096 S	-0.082 J	-0.011 d
60.0	0.054 s	-0.046 j	0.003 s	0.077 S	-0.065 j	-0.008 d
53.3	0.044 S	-0.038 J	0.003 S	0.068 S	-0.058 J	-0.007 d
40.0	0.028 S	-0.024 J	0.002 S	0.051 S	-0.044 J	0.005 h
33.3	0.021 S	-0.018 J	0.002 U	0.043 s	-0.037 ]	0.004 h
20.7	0.015 5	-0.013 J	0.001 C	0.034 5	-0.029 J	0.003 h
10.0	0.002 0	-0.002 L	0.001 c	0.013 s	-0.011 5	0.001 h
0.0	0.000 A	0.000 A	0.000 A	0.000 A	0.000 A	0.000 A

# MAXIMUM ANTENNA AND REFLECTOR ROTATIONS:

ELEV	AZI	TYPE	BEAM	DEFLECTION	S (deg)	TOTAL
ft	deg	*	PITCH	YAW	ROLL	
260.0	0.0	STD+R	0.416 J	0.148 d	-0.492 s	0.442 J
185.0	0.0	STD+R	0.262 J	0.069 d	-0.309 s	0.270 J

# MAXIMUM TENSION IN MAST MEMBERS (kip)

ELEV ft	LEGS	DIAG	HORIZ	BRACE
280.0		0 12 -	0.08 O	0.00 A
275.0	0.03 g	0.12 e	0.01 g	0.00 A
270.0	0.00 A	0.36 D	0.01 A	0.00 A
265.0	0.51 A	0.43 D	0.01 b	0.00 A
260.0	1.48 A	0.47 V	0.15 X	0.00 A
255.0	2.64 M	0.84 D	0.03 M	0.00 A
250.0	4.38 A	0.92 D	0.00 C	0.00 A
245.0	6.32 A	0.92 V	0.03 A	0.00 A
240 0	7.99 A	1.29 D	0 19 A	0.00 A
235 0	10.97 A	1.32 D	0.15 A	
230.0	13.98 A	1.48 D	0.07 A	0.00 A
230.0	16.96 A	1.79 D	0.02 5	0.00 A

225.0			0.07	19-5171 A	-ТЈН 0.00 а
220.0	21.17 A	1.91 D	0.14	s	0.00 A
215.0	24.16 A	1.04 A	0.06	А	0.00 A
210.0	25.88 A	1.25 X	0.01	А	0.00 A
205.0	28.13 A	1.17 X	0.06	А	0.00 A
200.0	30.33 A	1.18 ×	0.00	А	0.00 A
195.0	31.95 A	1.37 A	0.04	A	0.00 A
190.0	34.38 A	1.38 S	0.03	e	0.00 A
185.0	36.62 A	1.35 X	0.03	۵	0.00 4
180 0	38.80 A	1.62 X	0.03	6	
175 0	40.94 A	1.74 F	0.03	۲ ۸	
170.0	43.53 A	1.59 X	0.03	~	0.00 A
165 0	45.58 A	1.69 F	0.02	e •	0.00 A
103.0	47.94 A	1.59 X	0.02	A -	0.00 A
160.0	49.94 A	1.68 F	0.02	J	0.00 A
155.0	52.13 A	1.62 X	0.02	A	0.00 A
150.0	54.07 A	1.70 F	0.02	М	0.00 A
145.0	56.16 A	1.66 x	0.02	А	0.00 A
140.0	58.34 A	1.83 F	0.02	М	0.00 A
133.3	60.96 A	1.82 s	0.02	А	0.00 A
126.7	63.39 A	1.89 F	0.02	м	0.00 A
120.0	65.92 A	1.92 5	0.02	А	0.00 A
113.3	68.32 A	1.97 F	0.01	Μ	0.00 A
106.7	70.78 Δ	2 01 5	0.02	А	0.00 A
100.0	73 12 Δ	2 07 i	0.01	Μ	0.00 A
93.3	75 49 A	2.07 J  2 14 s	0.01	Α	0.00 A
86.7	77.78 A	2.14 5  2 18 s	0.01	А	0.00 A
80.0	80 13 4	2.10 3	0.01	Α	0.00 A
73.3	80.13 A	2.20 5	0.01	Α	0.00 A
66.7	82.42 A	2.32 5	0.01	А	0.00 A
60.0	84.74 A	2.39 5	0.01	А	0.00 A
53.3	86.96 A	2.45 5	0.01	А	0.00 A
46.7	89.14 A	2.53 5	0.01	A	0.00 A
40.0	91.29 A	2.59 S	0.01	A	0.00 A
33.3	93.47 A	2.67 S	0.01	9	0.00 A
26.7	95.61 A	2.74 S	0.04	s	0.00 A
20.0	97.76 A	2.83 S	0.08	A	0.00 A
10.0	102.56 A	3.49 j	0.17	А	0.00 Y
0.0	101.51 A	3.59 j	0.00	A	0.00 A

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# MAXIMUM COMPRESSION IN MAST MEMBERS (kip)

ELEV ft	LEGS	DIAG	HORIZ	BRACE
280.0			-0.08 g	0.00 A
275.0	-0.10 0	-0.12 M	0.00 o	0.00 A
270.0	-1.36 s	-0.38 D	0.00 s	0.00 A
265.0	-2.51 e	-0.42 V	0.00 A	0.00 A
260.0	-3.62 S	-0.50 D	-0.20 h	0.00 A
255.0	-5.48 S	-0.90 G	-0.01 s	0.00 A
250.0	-7.45 S	-0.86 V	-0.01 F	0.00 A
245.0	-9.43 s	-0.97 D	-0.01 s	0.00 A
240.0	-12.54 S	-1.28 V	-0.11 G	0.00 A
235.0	-15.68 S	-1.41 D	-0.04 s	0.00 A
230.0	-19.21 s	-1.43 D	-0.02 A	0.00 A
225.0	-23.61 S	-1.83 D	-0.04 s	0.00 A
220.0	-28.27 S	-1.91 D	-0.23 A	0.00 A
215.0	-31.50 s	-1.20 S	-0.03 s	0.00 A
210.0	-35.09 S	-1.21 X	0.00 s	0.00 A
205.0	-37.70 s	-1.24 S	-0.03 s	0.00 A
200.0	-40.45 S	-1.18 X	0.00 A	0.00 A
195.0	-43.69 S	-1.46 S	-0.03 s	0.00 A
190.0	-46.82 S	-1.36 X	-0.01 M	0.00 A
185.0	-49.55 S	-1.43 S	-0.03 A	0.00 A
180.0	-52.91 S	-1.80 F	-0.01 b	0.00 A
175.0	-55.66 S	-1.62 X	-0.02 A	0.00 A
170.0	-58.63 S	-1.72 F	-0.01 s	0.00 A
165.0	-61.38 S	-1.60 X	-0.02 h	0.00 A
160.0	-64.21 S	-1.69 F	-0.01 s	0.00 A
155.0	-66.93 s	-1.63 S	-0.01 h	0.00 A
150.0	-69.72 S	-1.70 F	-0.01 s	0.00 A
145.0	-72.40 s	-1.68 S	-0.01 s	0.00 A
140.0	-75.14 S	-1.73 F	-0.01 s	0.00 A
133.3	-78.24 S	-1.86 S	-0.01 s	0.00 A
126.7	-81.90 S	-1.88 F	-0.01 s	0.00 A
120.0	-85.46 S	-1.94 S	-0.01 s	0.00 A
113.3	-89.06 s	-1.97 S	-0.01 s	0.00 A
106.7	-92.61 S	-2.04 S	-0.01 s	0.00 A
100.0	-96.18 S	-2.07 s	-0.01 s	0.00 A
93.3	-99.74 s	-2.15 S	-0.01 s	0.00 A
86.7	-103.36 S	-2.19 S	-0.01 s	0.00 A

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80.0	-106.97 S	-2.27 S	-0.01	s	0.00 A
73.3	-110.60 s	-2.33 s	-0.01	s	0.00 A
66 7	-114.23 s	-2.40 s	-0.01	s	0 00 4
60.0	-117.87 s	-2.45 s	_0_01	с с	
50.0	-121.58 S	-2.54 s	-0.01	5	0.00 A
55.5	-125.36 s	-2.60 s	-0.01	5	0.00 A
46.7	-129.15 s	-2.67 S	0.00	S	0.00 A
40.0	-132.97 S	-2.74 S	0.00	S	0.00 A
33.3	-136 80 S		-0.02	с	0.00 A
26.7	140 62 6	2.75 5	-0.07	А	0.00 A
20.0	-140.03 5	-2.03 0	-0.04	s	0.00 A
10.0	-147.40 S	-3.80 S	-0.25	s	0.00 G
0.0	-148.44 S	-3.91 S	0.00	A	0.00 A

# MAXIMUM INDIVIDUAL FOUNDATION LOADS: (kip)

	TOTAL			
NORTH	EAST	DOWN	UPLIFT	SHEAR
12.59 s	10.65 e	153.57 s	-105.06 A	12.59 s

# MAXIMUM TOTAL LOADS ON FOUNDATION : (kip & kip-ft)

H NORTH	IORIZONTA EAST @	L TOTAL 0.0	DOWN	NORTH	OVERTURNING EAST	Б ТС ТОТАL @ 0.0	DRSION
19.4	-16.6	19.4	65.0	2769.9	-2364.1	2769.9	9.1
S	J	S	X	S	J	S	h

#### **MAT FOUNDATION DESIGN BY SABRE TOWERS & POLES**

Tower Description 280' S3R Series SD Customer VERIZON WIRELESS Project Number 19-5171-TJH Date 12/13/2018 Engineer NM

**Overall Loads:** 

Factored Moment (ft-kips) Factored Axial (kips) Factored Shear (kips) Individual Leg Loads: Factored Uplift (kips) Factored Download (kips) Factored Shear (kips)

100.00	
61.54	
382.00	
444.00	
38.00	

0.12

0.15

58.73

8767.62

Width of Tower (ft) **Ultimate Bearing Pressure** Bearing Φs

Bearing Design Strength (ksf) Water Table Below Grade (ft) Width of Mat (ft) Thickness of Mat (ft) Depth to Bottom of Slab (ft) Bolt Circle Diameter (in) **Effective Anchor Bolt Embedment** Diameter of Pier (ft) Ht. of Pier Above Ground (ft) Ht. of Pier Below Ground (ft) Quantity of Bars in Mat Bar Diameter in Mat (in) Area of Bars in Mat (in²) Spacing of Bars in Mat (in) Quantity of Bars Pier Bar Diameter in Pier (in) Tie Bar Diameter in Pier (in) Spacing of Ties (in) Area of Bars in Pier (in²) Spacing of Bars in Pier (in) f'c (ksi) fy (ksi) Unit Wt. of Soil (kcf) Unit Wt. of Concrete (kcf) Volume of Concrete (yd³)

189.36		
61.54		
	Tower eccentric from mat (ft)	= 2
382.00		
444.00		
38.00		
24.25	Allowable Bearing Pressure (ksf)	3.00
9.00	Safety Factor	3.00
0.75		
6.75	Max. Factored Net Bearing Pressure (ksf)	4.49
999		
31	Minimum Mat Width (ft)	30.08
1.5		
6		
10		
52.625		
3.5	Minimum Pier Diameter (ft)	2.17
0.5	Equivalent Square b (ft)	3.10
4.5		
59		
1.128		
58.96		
6.29	Recommended Spacing (in)	6 to 12
18		-
0.875		
0.5		
4		
10.82	Minimum Pier A _s (in ² )	6.93
5.93	Recommended Spacing (in)	5 to 12
4.5		•
60		

# MAT FOUNDATION DESIGN BY SABRE TOWERS & POLES (CONTINUED)

Two-Way Shear:			
Average d (in)	13.872		
φv _c (ksi)	0.201	v _u (ksi)	0.158
$\phi v_{c} = \phi (2 + 4/\beta_{c}) f'_{c}^{1/2}$	0.302		
$\phi v_{a} = \phi (\alpha_{a} d/b_{a} + 2) f'_{a}^{1/2}$	0.237		
$\varphi \cdot c = \varphi(\alpha_{S} \alpha_{S} $	0.201		
$\psi \mathbf{v}_{c} = \psi \mathbf{H}_{c}$	0.201		
Snear perimeter, $b_o$ (in)	204.37		
β _c	1		
Stability:			
Overturning Design Strength (ft-k) One-Way Shear:	11045.8	Factored Overturning Moment (ft-k)	9167.6
φV _c (kips)	519.3	V,, (kips)	509.1
Pier Design:		,	The second state in the second state
Design Tensile Strength (kips) Shear:	584.5	Tu (kips)	382.0
φ	0.75		
V _c (kips)	84.9		
V _s (kips)	197.9	V _{s max} (kips)	757.3
$\phi V_{e}$ (kips)	212.1	V. (kips)	38.0
Maximum Spacing (in)	11.15	(Only if Shear Ties are Bequired)	
Actual Hook Development (in)	12 74	Beg'd Hook Development L. (in) - Tension	10.96
	12.7	Reald Hook Development L. (in) - Compression	11.81
Anchor Bolt Bull-Out:			11.01
N $/ \sigma N$	0.81		0.17
Pier Rebar Development Length (in)	/1 18	Bequired Length of Development (in)	23.48
Flexure in Slab:	41.10		23.40
φM _e (ft-kips)	3350.7	M. (ft-kips)	3335.0
a (in)	2.49		
Steel Ratio	0.01143		
βı	0.825		
Maximum Steel Batio (o.)	0.0197		
Minimum Steel Batio	0.0018		
Rebar Development in Pad (in)	93.61	Required Development in Pad (in)	16.77
		· · · · · · · · · · · · · · · · · · ·	
Condition	1 is OK, 0 Fails		
Minimum Mat Width			
Maximum Soil Bearing Pressure			
Pier Area of Steel			
Two-Way Shear	1		
Overturning	1		
Anchor Bolt Pull-Out	1		
Flexure	1		
Steel Ratio	1		
Interaction Diagram Visual Check	1		
One-Way Shear	1		
Hook Development	1		
Minimum Mat Depth	1		
Anchor Bolt Punching Shear	1		

### **DRILLED STRAIGHT PIER DESIGN BY SABRE TOWERS & POLES**

Tower Description 280' S3R Series SD Customer Name VERIZON WIRELESS Job Number 19-5171-TJH Date 12/13/2018 Engineer NM

Factored Uplift (kips)	382		
Factored Download (kips)	444		
Factored Shear (kips)	38		
Ultimate Bearing Pressure	12		
Bearing $\phi_s$	0.75		
Bearing Design Strength (ksf)	9		
Water Table Below Grade (ft)	999		
Bolt Circle Diameter (in)	10		
Effective Anchor			
Bolt Embedment	52.625		
Pier Diameter (ft)	4	Minimum Pier Diameter (ft)	2.17
Ht. Above Ground (ft)	0.5		
Pier Length Below Ground (ft)	33.5		
Quantity of Bars	12		
Bar Diameter (in)	1.128		
Area of Bars (in ² )	11.99		
Spacing of Bars (in)	10.32	Minimum Area of Steel (in ² )	9.05
Tie Bar Diameter (in)	0.5		
Spacing of Ties (in)	12		
f' _c (ksi)	4.5		
f _v (ksi)	60		
y c v			
Unit Wt. of Concrete (kcf)	0.15		
Download Friction $\phi_s$	0.75		
Uplift Friction $\phi_s$	0.75		
Volume of Concrete (yd ³ )	15.82		
Skin Friction Factor for Uplift	esergia de <b>1</b> de la	Length to Ignore Download (ft)	
Ignore Bottom Length in Download?		0	
Depth at Bottom of Layer (ft)	Ult. Skin Friction (ksf)	(Ult. Skin Friction)*(Uplift Factor)	γ (kcf)
6	0.00	0.00	0.12
23.5	1.20	1.20	0.12
28.5	1.20	1.20	0.13
35	2.00	2.00	0.13
0	0.00	0.00	0
0	0.00	0.00	0
0	0.00	0.00	0
0	0.00	0.00	0
0	0.00	0.00	0
0	0.00	0.00	0

# DRILLED STRAIGHT PIER DESIGN BY SABRE TOWERS & POLES (CONTINUED)

Download:			
Factored Net Weight of Concrete (kips)	14.8	]	
Bearing Design Strength (kips)	113.1	1	
Skin Friction Design Strength (kips)	348.7	)	0000000
Download Design Strength (kips)	461.8	Factored Net Download (kips)	458.8
	105.0		
Nominal Skin Friction (kips)	465.0		
wc, weight of Concrete (kips)	64.1		
W _R , Soil Resistance (kips)	2070.4		
$\phi_{s}W_{r}+0.9W_{c}$ (kips)	1610.5		
Uplift Design Strength (kips)	406.4	Factored Uplift (kips)	382.0
Topology			
Design Tanaila Otranath (kina)	047.0		
Design Tensile Strength (kips)	047.0		382.0
Shear:			
φ	0.75		
V _c (kips)	142.9		
V _s (kips)	75.4	V _{s,max} (kips)	989.2
φV _n (kips)	163.7	V _u (kips)	38.0
Anchor Bolt Pull-Out:			140 100 100 100 100 100 100 100 100 100
N _{ua} /φN _n	0.81	V _{ua} / φV _n	0.17
Rebar Development Length (in)	39.17	Required Length of Development (i	n) 30.27
Condition	1 is OK. 0 Fails	l	
Download	1	1	
Uplift	1		
Area of Steel	1		
Shear	1		
Anchor Bolt Puli-Out	1		
Interaction Diagram Visual Check	1		



SITE #5: FCC# 1061534 SBA PROPERTIES, LLC N37°01'59.6", W88°55'53.8" SITE #6: FCC# 1222068 AMERICAN FAMILY ASSOCIATION N36°59'32.1", W88°59'19.2" SITE #7: FCC# 1229412 TOWERS III, LLC N37°04'30.1", W88°52'42.7"

SITE #9: FCC# 1252613 KENTUCKY RSA NO. 1 PARTNERSHIP N37°10'55.4", W88°56'43.7" SITE #10:FCC# 1265272 TV6 HOLDINGS, LLC N37°05'12.6", W88°52'56.7" SITE #11: FCC# 1265530 KENTUCKY RSA NO. 1 PARTNERSHIP N36°59'01.1", W89°04'29.2" SITE #12: FCC# 1313667 KENTUCKY RSA NO. 1 PARTNERSHIP N37°01'45.6", W89°00'07.6" SITE #13: FCC# 1318625 KENTUCKY STATE POLICE N36°58'24.9", W89°04'58.4"



# **BALLARD COUNTY, KENTUCKY** VERIZON WIRELESS TOWER SITE EV BARLOW TOWER LOCATION EXHIBIT

BROADCAST AND TRANSMIT STRUCTURE LOCATIONS DEPICTED ARE ALL KNOWN STRUCTURE SITES THAT HAVE BEEN REGISTERED WITH THE FEDERAL COMMUNICATIONS COMMISSION WITHIN 1/2 MILE OF THE LIMITS OF BALLARD COUNTY ON OR BEFORE JANUARY 31, 2022



Kentucky

Aeronautical Study No. 2018-ASO-17808-OE



Mail Processing Center Federal Aviation Administration Southwest Regional Office Obstruction Evaluation Group 10101 Hillwood Parkway Fort Worth, TX 76177

Issued Date: 10/01/2018

Network Regulatory Kentucky RSA No. 1 Partnership 5055 North Point Pkwy Alpharetta, GA 30005

# **** DETERMINATION OF NO HAZARD TO AIR NAVIGATION ****

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Antenna Tower EV Barlow - B (2580839)
Barlow, KY
37-06-42.14N NAD 83
89-02-44.58W
364 feet site elevation (SE)
285 feet above ground level (AGL)
649 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

As a condition to this Determination, the structure is to be marked/lighted in accordance with FAA Advisory circular 70/7460-1 L Change 2, Obstruction Marking and Lighting, a med-dual system - Chapters 4,8(M-Dual),&12.

Any failure or malfunction that lasts more than thirty (30) minutes and affects a top light or flashing obstruction light, regardless of its position, should be reported immediately to (877) 487-6867 so a Notice to Airmen (NOTAM) can be issued. As soon as the normal operation is restored, notify the same number.

It is required that FAA Form 7460-2, Notice of Actual Construction or Alteration, be e-filed any time the project is abandoned or:

At least 10 days prior to start of construction (7460-2, Part 1) X Within 5 days after the construction reaches its greatest height (7460-2, Part 2)

This determination expires on 04/01/2020 unless:

- (a) the construction is started (not necessarily completed) and FAA Form 7460-2, Notice of Actual Construction or Alteration, is received by this office.
- (b) extended, revised, or terminated by the issuing office.

(c) the construction is subject to the licensing authority of the Federal Communications Commission (FCC) and an application for a construction permit has been filed, as required by the FCC, within 6 months of the date of this determination. In such case, the determination expires on the date prescribed by the FCC for completion of construction, or the date the FCC denies the application.

NOTE: REQUEST FOR EXTENSION OF THE EFFECTIVE PERIOD OF THIS DETERMINATION MUST BE E-FILED AT LEAST 15 DAYS PRIOR TO THE EXPIRATION DATE. AFTER RE-EVALUATION OF CURRENT OPERATIONS IN THE AREA OF THE STRUCTURE TO DETERMINE THAT NO SIGNIFICANT AERONAUTICAL CHANGES HAVE OCCURRED, YOUR DETERMINATION MAY BE ELIGIBLE FOR ONE EXTENSION OF THE EFFECTIVE PERIOD.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power, except those frequencies specified in the Colo Void Clause Coalition; Antenna System Co-Location; Voluntary Best Practices, effective 21 Nov 2007, will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA.This determination includes all previously filed frequencies and power for this structure.

If construction or alteration is dismantled or destroyed, you must submit notice to the FAA within 5 days after the construction or alteration is dismantled or destroyed.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

A copy of this determination will be forwarded to the Federal Communications Commission (FCC) because the structure is subject to their licensing authority.

If we can be of further assistance, please contact our office at (718) 553-2611, or angelique.eersteling@faa.gov. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2018-ASO-17808-OE.

**Signature Control No: 381530823-386435140** Angelique Eersteling Technician

Attachment(s) Frequency Data Map(s)

cc: FCC

(DNE)

LOW	HIGH	FREQUENCY		ERP
FREQUENCY	FREQUENCY	UNIT	ERP	UNIT
6	7	GHz	55	dBW
6	7	GHz	42	dBW
10	11.7	GHz	55	dBW
10	11.7	GHz	42	dBW
17.7	19.7	GHz	55	dBW
17.7	19.7	GHz	42	dBW
21.2	23.6	GHz	55	dBW
21.2	23.6	GHz	42	dBW
614	698	MHz	1000	W
614	698	MHz	2000	W
698	806	MHz	1000	W
806	901	MHz	500	W
806	824	MHz	500	W
824	849	MHz	500	W
851	866	MHz	500	W
869	894	MHz	500	W
896	901	MHz	500	W
901	902	MHz	7	W
929	932	MHz	3500	W
930	931	MHz	3500	W
931	932	MHz	3500	W
932	932.5	MHz	17	dBW
935	940	MHz	1000	W
940	941	MHz	3500	W
1670	1675	MHz	500	W
1710	1755	MHz	500	W
1850	1910	MHz	1640	W
1850	1990	MHz	1640	W
1930	1990	MHz	1640	W
1990	2025	MHz	500	W
2110	2200	MHz	500	W
2305	2360	MHz	2000	W
2305	2310	MHz	2000	W
2345	2360	MHz	2000	W
2496	2690	MHz	500	W

# Frequency Data for ASN 2018-ASO-17808-OE

# TOPO Map for ASN 2018-ASO-17808-OE





KENTUCKY AIRPORT ZONING COMMISSION

MATTHEW BEVIN Governor 421 Buttermilk Pike Covington, KY 41017 www.transportation.ky.gov 859-341-2700

#### CONSTRUCTION/ALTERATION STATUS REPORT

December 21, 2018

AERONAUTICIAL STUDY NUMBER: AS-004-PAH-2018-092

Verizon Wireless Tennessee Verizon Wireless Tennessee 5055 North Point Pkwy, NP2NE Alpharetta, GA 30022

This concerns the permit which was issued to you by the Kentucky Airport Zoning Commission on December 21, 2018. This permit is valid for a period of 18 Month(s) from its date of issuance. If construction is not completed within the said 18-Month period, this permit shall lapse and be void, and no work shall be performed without the issuance of a new permit. When appropriate, please indicate the status of the project in the place below and return this letter to John Houlihan, Administrator, Kentucky Airport Zoning Commission, 421 Buttermilk Pike, Covington, KY, 41017. 859-341-2700.

STRUCTURE:	Antenna Tower
LOCATION:	Barlow, KY
COORDINATES:	37° 6' 42.14" N / 89° 2' 44.58" W
HEIGHT:	285' AGL /649'AMSL

CONSTRUCTION/ALTERATION STATUS

- 1. The project ( ) is abandoned. ( ) is not abandoned.
- Construction status is as follows: Structure reached its greatest height of ______ ft. AGL _____ ft. AMSL on ______ (date).

Date construction was completed.

Type of obstruction	marking/painting.	
	0.1.0.0	

Type of obstruction lighting.	

As built coordinates.	

Miscellaneous Information.

DATE _____

SIGNATURE/TITLE



An Equal Opportunity Employer M/F/D

# **Geotechnical Report and Resistivity**

# Verizon Wireless EV Barlow

2244 Steve Denton Road Barlow, Kentucky

August 30, 2018

# **Prepared For:**



Verizon Wireless 250 East 96th Street Suite 175 Indianapolis, Indiana

# **Prepared By:**



# SUBSURFACE INVESTIGATION & GEOTECHNICAL RECOMMENDATIONS

EV BARLOW – CELL TOWER 2244 STEVE DENTON ROAD BARLOW, KENTUCKY A&W PROJECT NO: 18IN0510

> PREPARED FOR: GPD GROUP INDIANAPOLIS, INDIANA

PREPARED BY: ALT & WITZIG ENGINEERING, INC. GEOTECHNICAL DIVISION

AUGUST 30, 2018



Alt & Witzig Engineering, Inc.

4105 West 99th Street • Indianapolis • Indiana • 46032 Ph (317) 875-7000 • Fax (317) 876-3705

August 30, 2018

GPD Group 8275 Allison Pointe Trail, Suite 220 Indianapolis, Indiana 46250 ATTN: Ms. Traci Preble

# **Report of Subsurface Investigation & Geotechnical Recommendations**

RE: EV Barlow – Cell Tower 2244 Steve Denton Road Barlow, Kentucky Alt & Witzig File: **18IN0510** 

Dear Ms. Preble:

In compliance with your request, we have completed a subsurface investigation and geotechnical evaluation for the above referenced project. It is our pleasure to transmit herewith one (1) electronic copy of our report.

The purpose of this subsurface investigation was to determine the various soils profile components and the engineering characteristics of the materials encountered in order to provide information to be used for preparing a foundation for the proposed cellular tower and equipment building.

# **Project Description**

It is anticipated that a new 285-foot tall self-support cell tower will be constructed at this site. A prefabricated equipment building will also be constructed at this site

The site is located west of State Road 1105 and approximately one-hundred (100) feet south of Sallie Crice Road near Barlow, Kentucky (Exhibit 1). The site may be located using the Barlow Quadrangle, Kentucky-Illinois 7¹/₂ minute topographic map.

Based upon the project plans provided by GPD to Alt & Witzig Engineering, the ground surface elevation at the tower center is taken to be 348.0' AMSL. All depths referred to in this report and on the Boring Logs are referenced from the existing ground surface.

GPD Group EV Barlow – Cell Tower Alt & Witzig File: 18IN0510 August 30, 2018 Page 2

### Exhibit 1: 2017 Aerial Photograph with Overlay



### Field Methods

The field investigation included a reconnaissance of the project site, performing one (1) soil boring (B-1) for the proposed tower and one (1) soil boring for the equipment building (B-2), performing standard penetration tests, and obtaining soil samples retained in the standard split-spoon sampler. The apparent groundwater level at the boring location was also determined.

The soil boring was performed with an all terrain vehicle-mounted drilling rig equipped with a rotary head. Conventional hollow-stem augers were used to advance the holes. The advancement of the borings was temporarily stopped at regular intervals in order to perform standard penetration tests in accordance with ASTM Procedure D-1586. The standard penetration test involves driving a split spoon soil sampler into the ground by dropping a 140-pound hammer, thirty (30) inches. The number of hammer drops required to advance the split-spoon sampler one (1) foot into the soil is defined as the standard penetration tests were obtained, classified, and labeled for further laboratory investigation.



### Laboratory Investigation

A laboratory investigation was conducted to ascertain additional pertinent engineering characteristics of the subsurface materials at the site of the proposed tower. The laboratory testing program included:

- Visual classification of soils.
- Moisture content determination in accordance with ASTM D-2216.
- Samples of the cohesive soil were frequently tested in unconfined compression by use of a calibrated spring testing machine.
- A pocket penetrometer was used as an aid in determining the strength of the soil.

The values of the unconfined compressive strength as determined on soil samples from the split-spoon sampling must be considered approximate recognizing the manner in which they were obtained since the split-spoon sampling techniques provide a representative but somewhat disturbed soil sample.

### Site Specific Subsurface Conditions

At the ground surface, the borings encountered approximately six (6) inches of topsoil. Beneath the topsoil, the borings encountered very soft to stiff silty clays with varying amounts of sand and gravel extending to depths of twenty-three and one-half  $(23\frac{1}{2})$  feet (Elev. 319.5 feet) in boring B-1 and ten (10) feet (Elev. 338.0 feet) in boring B-2. In boring B-1, these soils transitioned into a hard consistency that extended to a depth of twenty-eight and one-half  $(28\frac{1}{2})$  feet. At this depth, dry, very dense, clayey sand was encountered to the termination depth of the boring at thirty-seven (37) feet, where auger refusal was encountered. Detailed soil descriptions at the boring location have been included on the *Boring Logs* in the Appendix of this report.

### Bedrock

The site is located along the Mississippi Embayment of the Mississippi Alluvial Plain within the Jackson Purchase Region of Kentucky. This part of Kentucky is relatively flat-lying, with numerous lakes, ponds, sloughs, and swamps. Geologic maps published by the US Geological Service indicate the Mississippi Embayment is the northward continuation of the fluvial sediments of the Mississippi River Delta. The current sedimentary area was formed in the Cretaceous and early Cenozoic periods by the filling with sediment of an existing basin. The soils in this region consists primarily of loess. The underlying bedrock in this region consists primarily of limestone formed in the Ordovician period.



### Groundwater

Water level observations made during and upon completion of drilling operations yielded dry boreholes. These measurements are noted on the *Boring Logs* presented herewith. The exact location at which water is encountered should be anticipated to fluctuate somewhat depending upon normal seasonal variations in precipitation and surface runoff.

It should be noted that the groundwater level measurement recorded on the individual *Boring Logs* in the Appendix of this report is accurate for the specific date on which the measurements was performed. It must be understood that the groundwater level will fluctuate throughout the year. The *Boring Logs* do not indicate these fluctuations.

### Seismic Parameters

An evaluation of the seismic site class has been performed for this site. The State of Kentucky has integrated the 2015 International Building Code into the Indiana Building Code (IBC). The seismic site class is determined by averaging soil conditions within the top 100 feet with respect to the shear wave velocity in accordance with ASCE 7. Our evaluation is based on data obtained for borings performed to depths of 33 feet at this site and information provided by the Indiana Geological Survey for a depth of 100 feet. A detailed report generated by the USGS Earthquake Hazard program (http://earthquake.usgs.gov/designmaps/us/application.php) has been attached to this letter. Following are the summarized requested seismic parameters.

Seismic Parameters					
Site Soil Classification	Site Class D				
MCE Spectral Response Accelerations	$S_s = 2.506$ $S_1 = 0.951$				
Site Coefficients	$F_a = 1.0$ $F_v = 1.5$				

### **Geotechnical Recommendations**

Information provided by GPD Group indicates that the proposed 285-foot self-support cell tower will be constructed in the general vicinity of soil boring B-1; and an equipment building will be constructed in the general vicinity of boring B-2. Our experience with this type of structure indicates that the structural loads of the tower will be supported by an extended mat foundation or a caisson system and the buildings will be supported by conventional spread footings and continuous wall footings. It is recommended that a representative of Alt & Witzig Engineering, Inc. be on-site to monitor the excavation and inspect the base of the foundations.



## Tower Foundation Recommendations

## Extended Footing or Extended Mat Foundation

If spread footings are desired, they should be founded at a minimum depth of four (4) feet below existing grade. The soil parameters presented in *Table 1* may be utilized for the design of a shallow foundation.

### **Table 1: Shallow Foundation Soil Parameters**

Soil Description	Depth Below Existing Grade (feet)	Allowable Bearing Pressure (psf) FS=3	Unit Weight (pcf)	С (psf)/ Ф (°)	Adhesion (psf)
Silty Clay	4-9	3,000	120	2,000	1250

It is anticipated that lateral wind loads and overturning moments will act on the spread footing. To help resist the overturning moment, it may be necessary to place a larger footing than necessary for bearing capacity. Also, any soil placed above the footing may be considered to help resist overturning moments if compacted to a minimum of 98 percent of the maximum dry density as determined from ASTM D-698 (Standard Proctor).

Depending upon the time of the year that the excavations are made, seepage from surface runoff may occur. Since these foundation materials tend to soften/loosen when exposed to free water, every effort should be made to keep the excavations dry should water be encountered. It is also recommended that concrete for footings be poured as soon as possible after the excavations are complete. A mud mat may be placed to provide the contractors a firm working surface and protect the exposed subgrade soils from softening.

# Caissons/Drilled Piers

A caisson type foundation is advantageous to use when it is necessary to resist large overturning moments such as those caused by wind loads against the proposed structure. As an alternative to a shallow foundation system, a caisson type foundation system may be considered to support this tower structure. A straight shaft caisson/drilled pier may be considered. If a caisson or drilled shaft is used to support the structure, it should be designed using the soil parameters provided in *Table 2*.



Soil Type	Depth Below Grade (Feet)	Allowable Skin Friction for Gravity Loads SF=2	Design End Bearing Pressure SF=3	Effective Unit Weight (pcf)	С (psf) / Ф (°)
Silty Clay	6-23.5	600 psf	NA	120 pcf	2000 psf
Hard Silty Clay	23.5 - 28.5	600 psf	4,000 psf	130 pcf	2000 psf
Clayey Sand	28.5+	1000 psf	4,000 psf	130 pcf	28°

*Skin friction may be utilized in shaft compression and tension. The top one-shaft diameter should be neglected.

### **Equipment Building Foundation Recommendations**

A net allowable bearing pressure of 2,000 psf is recommended for dimensioning continuous wall footings at this site. The above-suggested bearing pressure is provided assuming the footings will be founded on medium stiff natural soils or properly compacted fill materials at a minimum depth of three (3) feet below grade.

### **Equipment Building Slab Recommendations**

This structure will be a slab-on-grade supported by natural soils and/or compacted fill materials. In those areas where the existing grade is lower than the design floor elevation, a well-compacted structural fill will be necessary to raise the site to the desired grade. The fill material shall consist of INDOT No. 53 Stone.

After the building areas have been raised to the proper elevation, a granular fill should be placed immediately beneath the floor slab. It is recommended that all material placed in the floor slab areas be compacted to a density of 100 percent of maximum dry density in accordance with ASTM D-698. Recommendations for proper filling procedures are presented later in the Appendix of this report.

#### **Statement of Limitations**

Our subsurface investigation was conducted in accordance with guidelines set forth in the scope of services and applicable industry standards.

An inherent limitation of any geotechnical engineering study is that conclusions must be drawn on the basis of data collected at a limited number of discrete locations. The geotechnical parameters provided in this report were developed from the information obtained from the test borings that depict subsurface conditions only at these specific locations and on the particular date indicated on the boring logs. Soil conditions at other locations may differ from conditions encountered at these GPD Group EV Barlow – Cell Tower Alt & Witzig File: 18IN0510 August 30, 2018 Page 7



boring locations and groundwater levels shall be expected to vary with time. The nature and extent of variations between the borings may not become evident until the course of construction.

Often, because of design and construction details that occur on a project, questions rise concerning the soil conditions. If we can give further service in these matters, please contact us at your convenience.

Very truly yours,

Alt & Witzig Engineering, Inc.

David M. Shumate Staff Geologist

and C. Hamon

David C. Harness, P.E.



## APPENDIX

Recommended Specifications for Compacted Fills and Backfills Site Location Map Boring Location Plan Boring Logs General Notes USGS Design Maps Summary Custom Soil Resource Report for Ballard and McCracken Counties, Kentucky

#### RECOMMENDED SPECIFICATIONS FOR COMPACTED FILLS AND BACKFILLS

All fill shall be formed from material free of vegetable matter, rubbish, large rock, and other deleterious material. Prior to placement of fill, a sample of the proposed fill material should be submitted to the soils engineer for his approval. The fill material should be placed in layers not to exceed eight (8) inches in loose thickness and should be sprinkled with water as required to secure specified compactions. Each layer should be uniformly compacted by means of suitable equipment of the type required by the materials composing the fill. Under no circumstances should a bulldozer or similar tracked vehicles be used as compacting equipment. Material containing an excess of water so the specified compaction limits cannot be attained should be spread and dried to a moisture content which will permit proper compaction. All fill should be compacted to the specified percent of the maximum density obtained in accordance with ASTM density Test D-698 (100 percent of maximum dry density below and above the base of footing elevation). Should the results of the in-place density tests indicate that the specified compaction limits are not obtained; the areas represented by such tests should be reworked and retested as required until the specified limits are reached.

# SITE LOCATION MAP




## **BORING LOG**



## Alt & Witzig Engineering, Inc.

CLIENT GPD Group	BORING #	B-1	
PROJECT NAME EV Barlow Cell Tower	ALT & WITZIG FILE #	18IN0510	
PROJECT LOCATION Barlow, Kentucky		Lonaitude	-89.045767



## **BORING LOG**



## Alt & Witzig Engineering, Inc.

CLIENT GPD Group	BORING #	B-2	
PROJECT NAME _ EV Barlow Cell Tower	ALT & WITZIG FILE #	18IN0510	
PROJECT LOCATION Barlow. Kentucky		Longitude	-89.045823



### MATERIAL GRAPHICS LEGEND



CL-ML: USCS Low Plasticity Silty Clay



SC: USCS Clayey Sand

TOPSOIL

SAMPLER SYMBOLS

### SOIL PROPERTY SYMBOLS

N: Standard "N" penetration value. Blows per foot of a 140-lb hammer falling 30" on a 2" O.D. split-spoon.Qu:Unconfined Compressive Strength, tsfPP:Pocket Penetrometer, tsfLL: Liquid Limit, %PL: Plastic Limit, %PI: Plasticity Index, %

#### DRILLING AND SAMPLING SYMBOLS

#### **GROUNDWATER SYMBOLS**

O Apparent water level noted while drilling.

♀ Apparent water level noted upon completion.

▼ Apparent water level noted upon delayed time.

#### RELATIVE DENSITY & CONSISTANCY CLASSIFICATION (NON-COHESIVE SOILS)

<u>TERM</u> Very Loose Loose Medium Dense Dense Very Dense <u>BLOWS PER FOOT</u> 0 - 5 6 - 10 11 - 30 31 - 50 >51

SS: Split Spoon

#### RELATIVE DENSITY & CONSISTANCY CLASSIFICATION (COHESIVE SOILS)

<u>TERM</u> Very Soft Soft Medium Stiff Stiff Very Stiff Hard

<u>BLOWS PER FOOT</u> 0 - 3 4 - 5 6 - 10 11 - 15 16 - 30 >31



Alt & Witzig Engineering, Inc. 4105 West 99th St. Carmel, IN 46032 Telephone: 317-875-7000 Fax:

## **GENERAL NOTES**

Project: EV Barlow Cell Tower Location: Barlow, Kentucky

Number: 18IN0510

NOTES - PROJECT SPECIFIC 18IN0510 GINT.GPJ US EVAL.GDT 8/30/18

## S Design Maps Summary Report

#### **User-Specified Input**

8/15/2018

Report Title 18IN0510

Building Code Reference Document 2012/2015 International Building Code

Site Coordinates 37.11175°N, 89.04577°W

Site Soil Classification Site Class D - "Stiff Soil"

Wed August 15, 2018 15:23:27 UTC

(which utilizes USGS hazard data available in 2008)

Risk Category I/II/III



#### **USGS**-Provided Output

$S_s =$	2.506 g	S _{MS} =	2.506 g	S _{ps} =	1.671 g
S1 =	0.951 g	S _{M1} =	1.426 g	S _{D1} =	0.951 g

For information on how the SS and S1 values above have been calculated from probabilistic (risk-targeted) and deterministic ground motions in the direction of maximum horizontal response, please return to the application and select the "2009 NEHRP" building code reference document.



Although this information is a product of the U.S. Geological Survey, we provide no warranty, expressed or implied, as to the accuracy of the data contained therein. This tool is not a substitute for technical subject-matter knowledge.

8/15/2018	Design Map	os Detailed Report	l
<b>USGS</b> Design Maps	Detailed Report		
2012/2015 International Buildi	ing Code (37.11175°N,	89.04577°V	V)
Site Class D – "Stiff Soil", Risk Categ	ory I/II/III		
Section 1613.3.1 — Mapped ad	cceleration parameters		
Note: Ground motion values provide spectral response acceleration. They mean ground motions computed by $1.3$ (to obtain S ₁ ). Maps in the 2012, Site Class B. Adjustments for other S 1613.3.3.	d below are for the direction have been converted from o the USGS by applying factor (2015 International Building Site Classes are made, as ne	of maximum l corresponding s of 1.1 (to ob Code are prov eded, in Sectio	horizontal geometric tain S _s ) and vided for on
From <u>Figure 1613.3.1(1)</u> ^[1]			S _s = 2.506 g
From <u>Figure 1613.3.1(2)</u> ^[2]			S ₁ = 0.951 g
The authority having jurisdiction (no the default has classified the site as accordance with Section 1613. 2010 ASC	t the USGS), site-specific ge Site Class D, based on the s CE-7 Standard – Table 20.3-1	otechnical dat ite soil properi	a, and/or ties in
Site SI		<b>N N</b>	7
Site Class	<b>۷</b> s	N OF N _{ch}	<u>Su</u>
A. Hard Rock	>5,000 IL/S	N/A	N/A
C. Very dance call and caft rack	2,500 to 5,000 ft/s	N/A	N/A
C. Very dense soit and soit rock	1,200 to 2,500 tt/s	20U	>2,000 psi
D. Suir Soli		15 (0 50	1,000 to 2,000 psi
	Any profile with more than characteristics: • Plasticity index PI • Moisture content w • Undrained shear st	a 10 ft of soil h > 20, $v \ge 40\%$ , and crength $\overline{s}_v < 50$	aving the
F. Soils requiring site response	See	Section 20.3.	

analysis in accordance with Section

21.1

For SI:  $1ft/s = 0.3048 \text{ m/s} 11b/ft^2 = 0.0479 \text{ kN/m}^2$ 

Section 1613.3.3 — Site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters

Site Class	Mapped Spectral Response Acceleration at Short Period				
	S _s ≤ 0.25	$S_{s} = 0.50$	S _s = 0.75	$S_{s} = 1.00$	S _s ≥ 1.25
А	0.8	0.8	0.8	0.8	0.8
В	1.0	1.0	1.0	1.0	1.0
С	1.2	1.2	1.1	1.0	1.0
D	1.6	1.4	1.2	1.1	1.0
E	2.5	1.7	1.2	0.9	0.9
F		See Se	ction 11.4.7 of	ASCE 7	

TABLE 1613.3.3(1) VALUES OF SITE COEFFICIENT F,

Note: Use straight-line interpolation for intermediate values of S₅

For Site Class = D and  $S_s = 2.506 \text{ g}$ ,  $F_s = 1.000$ 

TABLE 1613.3.3(2) VALUES OF SITE COEFFICIENT  $F_{\rm v}$ 

Site Class	Mapped Spectral Response Acceleration at 1-s Period				
	$S_1 \leq 0.10$	$S_1 = 0.20$	$S_1 = 0.30$	$S_1 = 0.40$	$S_1 \ge 0.50$
A	0.8	0.8	0.8	0.8	0.8
В	1.0	1.0	1.0	1.0	1.0
С	1.7	1.6	1.5	1.4	1.3
D	2.4	2.0	1.8	1.6	1.5
Е	3.5	3.2	2.8	2,4	2,4
F		See Se	ction 11.4.7 of	ASCE 7	

Note: Use straight-line interpolation for intermediate values of  $S_1$ 

For Site Class = D and S  $_{\rm t}$  = 0.951 g, F  $_{\rm v}$  = 1.500

Design Maps Detailed Report

Equation (16-37):	$S_{MS} = F_a S_S = 1.000 \times 2.506 = 2.506 g$
Equation (16-38):	$S_{M1} = F_v S_1 = 1.500 \times 0.951 = 1.426 g$
Section 1613.3.4 — Design spect	tral response acceleration parameters
Equation (16-39):	$S_{DS} = \frac{2}{3} S_{MS} = \frac{2}{3} \times 2.506 = 1.671 g$
Equation (16-40):	$S_{D1} = \frac{3}{3} S_{M1} = \frac{3}{3} \times 1.426 = 0.951 g$

#### Section 1613.3.5 — Determination of seismic design category

TABLE 1613.3.5(1)	
SEISMIC DESIGN CATEGORY BASED ON SHORT-PERIOD (0.2 second) RESPONSE ACCELERATION	

VALUE OF S _{DS}		RISK CATEGORY	
	I or II	III	IV
S _{ps} < 0.167g	A	A	A
$0.167g \le S_{DS} < 0.33g$	В	В	С
$0.33g \le S_{DS} < 0.50g$	С	С	D
0.50g ≤ S _{ps}	D	D	D

For Risk Category = I and  $S_{DS}$  = 1.671 g, Seismic Design Category = D

TABLE 1613.3.5(2)

SEISMIC DESIGN	CATEGORY BAS	ED ON 1-SECONE	) PERIOD RES	SPONSE ACCELERATION
00101110 0001011	CALEGOIAL DAG	CD ON I DECOME		of office house end have

VALUE OF S _{D1}	RISK CATEGORY				
	I or II	III	IV		
S _{D1} < 0.067g	А	A	А		
$0.067g \le S_{D1} < 0.133g$	В	В	С		
$0.133g \le S_{D1} < 0.20g$	С	с	D		
0.20g ≤ S ₀₁	D	D	D		

For Risk Category = I and  $S_{p1}$  = 0.951 g, Seismic Design Category = D

Note: When  $S_1$  is greater than or equal to 0.75g, the Seismic Design Category is **E** for buildings in Risk Categories I, II, and III, and **F** for those in Risk Category IV, irrespective of the above.

Seismic Design Category  $\equiv$  "the more severe design category in accordance with Table 1613.3.5(1) or 1613.3.5(2)" = E

Note: See Section 1613.3.5.1 for alternative approaches to calculating Seismic Design Category.

#### References

- 1. *Figure 1613.3.1(1)*: https://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/IBC-2012-Fig1613p3p1(1).pdf
- 2. *Figure 1613.3.1(2)*: https://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/IBC-2012-Fig1613p3p1(2).pdf



United States Department of Agriculture

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Ballard and McCracken Counties, Kentucky

18IN0510



# Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

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# Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP LEGEND				MAP INFORMATION		
Area of In	Area of Interest (AOI)		Spoil Area	The soil surveys that comprise your AOI were mapped at		
	Area of Interest (AOI)	â	Stony Spot	1:12,000.		
Soils	Soil Map Unit Polygons	0	Very Stony Spot	Warning: Soil Map may not be valid at this scale.		
	Soil Man Unit Lines	Ŷ	Wet Spot			
<u> </u>	Soil Map Unit Points		Other	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soi		
L.			Special Line Features	line placement. The maps do not show the small areas of		
Blowout		Water Features		contrasting soils that could have been shown at a more detaile scale.		
	Borrow Pit	$\sim$	Streams and Canals			
	Clay Spot	Transport	ation	Please rely on the bar scale on each map sheet for map		
茂	Closed Depression	+++	Rails	measurements.		
0	Crouel Dit	~	Interstate Highways	Source of Map: Natural Resources Conservation Service		
n An		~	US Routes	Web Soil Survey URL:		
	Gravelly Spot	~	Major Roads			
Q	Landfill		Local Roads	Maps from the Web Soil Survey are based on the Web Mercat		
A.	Lava Flow	Backgrou	nd	projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the		
ale	Marsh or swamp	No.	Aerial Photography	Albers equal-area conic projection, should be used if more		
Ŧ	Mine or Quarry			accurate calculations of distance or area are required.		
0	Miscellaneous Water			This product is generated from the USDA-NRCS certified data		
0	Perennial Water			of the version date(s) listed below.		
V	Rock Outcrop			Soil Survey Area: Ballard and McCracken Counties, Kentuck		
+	Saline Spot			Survey Area Data: Version 11, Oct 3, 2017		
	Sandy Spot			Soil map units are labeled (as space allows) for map scales		
-	Severely Eroded Spot			1:50,000 or larger.		
Ô	Sinkhole			Date(s) aerial images were photographed: Sep 13, 2011—O		
6	Slide or Slip			2011		
Ś	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor		

## **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
GrB2	Grenada silt loam, 2 to 6 percent slopes, eroded	0.2	21.8%
GrC3	Grenada silt loam, 6 to 12 percent slopes, severely eroded	0.9	78.2%
Totals for Area of Interest	8	1.1	100.0%

## **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the

development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## **Ballard and McCracken Counties, Kentucky**

### GrB2—Grenada silt loam, 2 to 6 percent slopes, eroded

#### **Map Unit Setting**

National map unit symbol: 2wn5t Elevation: 310 to 640 feet Mean annual precipitation: 52 to 62 inches Mean annual air temperature: 48 to 69 degrees F Frost-free period: 175 to 244 days Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Grenada, eroded, and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### Description of Grenada, Eroded

#### Setting

Landform: Ridges Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Nose slope Down-slope shape: Convex Across-slope shape: Linear Parent material: Fine-silty noncalcareous loess

#### **Typical profile**

Ap - 0 to 5 inches: silt loam Bw - 5 to 21 inches: silt loam E - 21 to 28 inches: silt loam Btx/E - 28 to 38 inches: silt loam Btx - 38 to 80 inches: silt loam

#### Properties and qualities

Slope: 2 to 6 percent
Depth to restrictive feature: 17 to 36 inches to fragipan
Natural drainage class: Moderately well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 18 to 32 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Moderate (about 6.0 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: C Ecological site: Northern Loess Fragipan Upland - PROVISIONAL (F134XY012AL) Hydric soil rating: No

#### **Minor Components**

#### Calloway

Percent of map unit: 6 percent Landform: Flats Landform position (three-dimensional): Tread Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

#### Collins

Percent of map unit: 4 percent Landform: Flood-plain steps Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### GrC3—Grenada silt loam, 6 to 12 percent slopes, severely eroded

#### **Map Unit Setting**

National map unit symbol: 1qls1 Elevation: 320 to 500 feet Mean annual precipitation: 40 to 56 inches Mean annual air temperature: 46 to 69 degrees F Frost-free period: 177 to 222 days Farmland classification: Not prime farmland

#### Map Unit Composition

*Grenada, severely eroded, and similar soils:* 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Grenada, Severely Eroded**

#### Setting

Landform: Ridges Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Linear Parent material: Thick fine-silty noncalcareous loess

#### **Typical profile**

H1 - 0 to 4 inches: silt loam H2 - 4 to 18 inches: silt loam H3 - 18 to 22 inches: silt loam H4 - 22 to 32 inches: silt loam H5 - 32 to 80 inches: silt loam

#### **Properties and qualities**

Slope: 6 to 12 percent
Depth to restrictive feature: 18 to 23 inches to fragipan
Natural drainage class: Moderately well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 18 to 24 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 4.9 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4e Hydrologic Soil Group: D Hydric soil rating: No

#### Minor Components

#### Purchase, severely eroded

Percent of map unit: 7 percent Landform: Ridges Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

#### Calloway

Percent of map unit: 4 percent Landform: Ridges Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

#### Falaya

Percent of map unit: 2 percent Landform: Drainageways Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### Collins

Percent of map unit: 2 percent Landform: Drainageways Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No



## Alt & Witzig Engineering, Inc.

4105 West 99th Street • Indianapolis • Indiana • 46032 Ph (317) 875-7000 • Fax (317) 876-3705

August 30, 2018

GPD Group 8275 Allison Pointe Trail, Suite 220 Indianapolis, Indiana 46250 ATTN: Ms. Traci Preble

### **Resistivity Results**

RE: EV Barlow – Cell Tower 2244 Steve Denton Road Barlow, Kentucky Alt & Witzig File: **18IN0510** 

Dear Ms. Preble:

To aid in the design of the grounding equipment for the referenced project, soil resistivity tests were performed at the site. The resistivity testing was performed using an AEMC Model 6472 Soil Resistance Meter per ASTM G-57 (The Wenner Vertical Profiling Method). A qualified technician familiar with this equipment and testing procedure performed the appropriate test to obtain the resistivity values at multiple depths. Alt & Witzig Engineering, Inc. was able to gather the necessary resistivity information in all four (4) directions.

The Wenner Vertical Profiling Method was used by centering the potential electrodes on a traverse line between the current electrodes and maintaining an equal "a" spacing between the electrodes. The depths of interests or "a" spacing of  $2\frac{1}{2}$  feet, 5 feet,  $12\frac{1}{2}$  feet, 20 feet and 50 feet.

The resistivity test was performed on August 28, 2018. The weather during data collection was between 80 and 92 degrees and sunny. The measurements were taken in general vicinity of the proposed tower location and approximately one-hundred (100) feet south of Sallie Crice Road. The layouts of the arrays are shown below in *Exhibit 1*.

GPD Group EV Barlow – Cell Tower Alt & Witzig File: 18IN0510 August 30, 2018 Page 2





Exhibit 1: Aerial Photograph of Site Showing the Layout of the Resistivity Array.

We appreciate the opportunity to be of service to you on this project. If we can give further service in these matters, please contact us at your convenience.

Very truly yours, *Alt & Witzig Engineering, Inc.* 

David M. Shumate Geologist

Jurid C. Hamen

David C. Harness, P.E. Sr. Geotechnical Engineer

Attachments: Boring Location Plan Resistivity Testing Results



Alt & Witzig Engineering, Inc 4105 West 99th Street Carmel, IN 46032 (317) 875-7000 www.altwitzig.com					RESISTIVTIY TESTING WENNER 4-ELECTRODE METHOD ASTM G57					
A&W Project ID: 18		18IN051	0	Sit	Site Location:		EV Barlow			
A&W Field Technician:		L		Engin	eer:	D. Harn	ess			
Weather C	Weather Conditions		Sunny		Meter Used		AEMC 6	472		
Air Tem	Air Temperature: 80 - 9		92	Ground Condi		Condition:	Corn Field			
Start Date		8/28/2018		9	itart Time		10:30am			
End Date		8/28/2018			End Time		2:30pm			
a R Important note (1) large, r soil, bou (2) conduc they are	= electrode = resistanc s: nonconduc ulders, con ctive struct e at right a	e separation, ft e, Ω tive bodies sha acrete foundati tures such as pi ngles to the sp	all not be in ons, pes and ca an.	honcludec	, <b>S 2</b> • <b>C</b> I in the surv	cm = vey. Nonco vithin 1/2 a	191.5 <i>d</i> inductive bodie	s include: frozen de span unless		
Location	Spaci	ing between	range	Dia	al Resi	stance, R	Multiplier	Resistivity, p		
Location	elect	rodes, a (ft)	switch	Read	ing	ohms	Multiplier Ω·cm	Ω·cm		
		2.5	1	14.1	13	14.13	478.75	6,765		
		5	1	6.82	L8	6.818	957.5	6,528		
NORTH		12.5	1	2.0	6	2.06	2393.75	4,931		
		20	1	1.45	59	1.459	3830	5,588		
		50	1	0.77	78	0.778	9575	7,449		
		2.5	1	9.24	12	9.242	478.75	4,425		
		5	1	6.04	18	6.048	957.5	5,791		
SOUTH		12.5	1	2.19	96	2.196	2393.75	5,257		
		20	1	1.46	56	1.466	3830	5,615		
		50	1	0.82	28	0.828	9575	7,928		
		2.5	1	25.9	94	25.94	478.75	12,419		
EAST		5	1	7.3	6	7.36	957.5	7,047		
		12.5	1	4.2	3	4.23	2393.75	10,126		
		20	1	0.86	56	0.866	3830	3,317		
		50		0.67	75	0.675	9575	6,463		
-		2.5	1	55.4	14	55.44	478.75	26,542		
		5	1	14.8	32	14.82	957.5	14,190		
WEST		12.5	1	2.32	11	2.311	2393.75	5,532		
		20	1	1.35	55	1.355	3830	5,190		
		50	1	1.06	54	1.064	9575	10,188		

#### **DIRECTIONS TO WFC SITE:**

FROM BALLARD COUNTY SEAT; TAKE US-60 E TO N 6TH ST IN BARLOW (6. 8 MI), HEAD NORTH ON 4TH ST TOWARD OHIO ST, PASS BY NAPA AUTO PARTS AUTO TIRE AND PARTS OF WICKCKLIFFE (ON THE RIGHT IN 0.2 MI)(0.5 MI), 4TH ST TURNS SLIGHTLY RIGHT AND BECOMES LEE ST (0.1 MI), CONTINUE ONTO US-60 E/N 6TH ST (CONTINUE TO FOLLOW US-60 E) (6.1 MI), TURN RIGHT ONTO BROADWAY ST (0.2 MI), TAKE KY-1105 N/OSCAR RD TO STEVE DENTON RD (5.3 MI), TURN LEFT AT THE 2ND CROSS STREET ONTO N 6TH ST (0.2 M), CONTINUE ONTO KY-1105 N/OSCAR RD (4.3 MI), TURN LEFT ONTO SALLIE CRICE RD (226 FT), TURN LEFT ONTO STEVE DENTON RD, ARRIVE AT DESTINATION 2244 STEVE DENTON RD.



PREPARED BY: GPD GROUP, INC. (317) 299-2996

**EV Barlow** 

Prepared By and Upon Recording, Return to:

Matthew R. Clark, Esq. CLARK, QUINN, MOSES, SCOTT & GRAHN, LLP 320 North Meridian Street, Suite 1100 Indianapolis, IN 46204

COMMONWEALTH OF KENTUCKY)

COUNTY OF BALLARD

Deed Reference: Deed Book 112, Page 227

#### MEMORANDUM OF LAND LEASE AGREEMENT

This Memorandum of Land Lease Agreement is made this MAM day of <u>NOVEMEN</u>, 2018, between The Myatt Family Trust, dated September 9, 2011, by and between Charles Myatt and Deena Myatt, Trustees, with a mailing address of 2224 Steve Denton Road, Barlow, Kentucky 42024, hereinafter collectively referred to as "LESSOR", and Kentucky RSA 1 Partnership d/b/a Verizon Wireless with its principal offices at One Verizon Way, Mail Stop 4AW100, Basking Ridge, New Jersey 07920, hereinafter referred to as "LESSEE". LESSOR and LESSEE are at times collectively referred to hereinafter as the "Parties" or individually as the "Party".

- LESSOR and LESSEE entered into a Land Lease Agreement (the "Agreement") on <u>M M</u>, 2018 for an initial term of five (5) years, commencing on the Commencement Date. The Land Lease Agreement shall automatically be extended for four (4) additional five (5) year terms unless the LESSEE terminates it at the end of the then current term by giving the LESSOR written notice of the intent to terminate at least three (3) months prior to the end of the then current term.
- 2. LESSOR hereby leases to LESSEE a portion of that certain parcel of property (the entirety of LESSOR's property is referred to hereinafter as the "Property"), located at 2557 Steve Denton Road, Barlow, Kentucky 42024 and being described as a 100' x 100' parcel containing 10,000 square feet, as shown on the Tax Map of Ballard County as a portion of Tax Parcel No. 24-30 and 24 -30 CH, and being part of that real property further described in Deed Book 112, at Page 227, recorded in the Office of the Register of Deeds for Ballard County, together with the non-exclusive right for ingress and egress, seven (7) days a week twenty-four (24) hours a day, on foot or motor vehicle, Including trucks, and for the installation and maintenance of utility wires, poles, cables, conduits, and pipes over, under, or along a thirty (30) foot wide right-of-way extending from the nearest public right-of-way are hereinafter collectively referred to as the "Premises". The Premises are described in Exhibit A attached hereto and made a part hereof, and as shown on the plat of survey attached hereto and Incorporated herein as Exhibit B. In the event any public

utility is unable to use the aforementioned right-of-way, LESSOR has agreed to grant an additional right-of-way either to the LESSEE or to the public utility at no cost to the LESSEE.

- The Commencement Date of the Agreement, of which this is a Memorandum, is the first day of the month after LESSEE begins installation of LESSEE'S communication equipment.
- LESSEE has the right of first refusal to purchase the Premises during the initial term and all renewal terms of the Agreement.
- The terms, covenants and provisions of the Agreement, the terms of which are hereby Incorporated by reference into this Memorandum, shall extend to and be binding upon the respective executors, administrators, heirs, successors and assigns of LESSOR and LESSEE.

IN WITNESS WHEREOF, hereunto and to a duplicate hereof, LESSOR and LESSEE have caused this Memorandum to be duly executed on the date first written hereinabove.

#### LESSOR:

THE MYATT FAMILY TRUST, DATED SEPTEMBER 9, 2011, BY AND THROUGH CHARLES MYATT AND DEENA MYATT, TRUSTEES

BY: Charles Mvatt, Trustee

Deena Myatt,

LESSEE:

BY:

KENTUCKY RSA 1 PARTNERSHIP D/B/A VERIZON WIRELESS

Bv:

Name: Ed Maher Title: Director Network Field Engineering

#### COMMONWEALTH OF KENTUCKY)

COUNTY OF BALLARD

#### ACKNOWLEDGEMENT

I, Kray Derton, a Notary Public for said County and State, do hereby certify that Charles Myatt and Deena Myatt, personally came before me this day and acknowledged that they are the Trustees of The Myatt Family Trust, dated September 9, 2011, and being authorized to do so, executed the foregoing Memorandum of Land Lease Agreement as their own act and deed on behalf of The Myatt Family Trust, dated September 9, 2011.

)

WITNESS my hand and official Notarial Seal, this Unday of October , 2018.

D. DENTO EG

My Commission Expires: ______ つーいー みつえみ

STATE OF MICHIGAN

#### COUNTY OF OAKLAND

#### ACKNOWLEDGMENT

I, MANUAL UNITY, a Notary Public for said County and State, do hereby certify that Ed Maher personally came before me this day and acknowledged that he is the Director Network Field Engineering for Kentucky RSA 1 Partnership d/b/a Verizon Wireless, and that he, as Director Network Field Engineering, being authorized to do so, executed the foregoing Memorandum of Land Lease Agreement on behalf of Kentucky RSA 1 Partnership d/b/a Verizon Wireless.

WITNESS my hand and official Notarial Seal, this Juliday of BARBARA MADIGAN EVANS NOTARY PUBLIC, STATE OF MICHIGAN County Of Oakland My Commission Expires 05-10-2020 Notary Public ACTING IN THE COUNTY OF My Commission Expires:

[&]quot;I affirm, under the penalties for perjury, that I have taken reasonable care to redact each Social Security number in this document, unless required by law." Matthew R. Clark

## EXHIBIT A

### [WRITTEN METES AND BOUNDS OF THE PREMISES AND INGRESS/EGRESS AND UTILITY EASEMENT]

#### LEASE AREA DESCRIPTION

A PART OF A 69 ACRE PARCEL OF LAND OWNED BY THE MYATT FAMILY TRUST AS RECORDED IN DEED BOOK 112, PAGE 227, AND LYING SOUTHWEST OF THE INTERSECTION OF SALIE CIRCLE ROAD AND STEVE DENTON ROAD, BALLARD COUNTY, KENTUCKY. verizo

1961 NORTHPOINT BLVD.

BENCHMARK

Consulting Engineers Land Surveyors 318 North Main Street Huntingourg, IN 47542 (812) 1833-3049

benchmark@mw twcbc.com

20161506655

EV BARLOW

2557 STEVE DENTON RD

BARLOW, KY 42024

10000 SQ. FT.

RLES MYATT & DEENA MYATT, TRU

2244 STEVE DENTON ROAD

BARLOW, KENTUCKY 42024

BALLARD COUNTY

CHKO BY:

RMW

SURVEY PLAN

2 OF 2

REVISION/ISSUE

DATE:

12.22.17

DATE:

24-30

PROJECT NUMBER:

SITE NAME:

SITE ADDRESS:

LEASE AREA:

PROPERTY OWNER: MYATT FAMILY TRUST

TAX PARCIL ID:

SOURCE OF TITLE: DEED BK 112, PG 227 UTTUDE: 37° 06' 42.145" N LONGTUDE: 89° 02' 44.583" W

COUNTY:

DWG BY:

ND.

TITLE

SHEET:

GVW

T

PLS NO. 80040185

SERVICES, INC.

SUITE 130

HIXSON, TN 37343

Wireless

COMMENCING AT AN IRON PIN FOUND AT THE INTERSECTION OF SAID SALLE CIRCLE ROAD AND STEVE DENTON ROAD, SAID POINT ALSO BEING THE NORTHEAST CORNER OF A PARCEL OF LAND OWNED BY DAVID L JONES AS RECORDED IN DEED BOOK 93, PAGE 150, THENCE ALONG THE CENTERLINE OF SALLE CIRCLE ROAD NORTH 68 DEGREES 26 MINUTES 09 SECONDS WEST 172.37 FEET; THENCE SOUTH 71 DEGREES 49 MINUTES 20 SECONDS WEST 78.70 FET; THENCE NORTH 76 DEGREES 16 MINUTES 40 SECONDS WEST 128.76 FEET; THENCE SOUTH 32 DEGREES 16 MINUTES 50 SECONDS WEST 132.35 FEET; THENCE NORTH 76 DEGREES 16 MINUTES 50 SECONDS WEST 30.00 FEET; THENCE NORTH 57 DEGREES 43 MINUTES 50 SECONDS WEST 30.00 FEET; THENCE NORTH 57 DEGREES 43 MINUTES 50 SECONDS WEST 30.00 FEET; THENCE NORTH 57 DEGREES 43 MINUTES 50 SECONDS WEST 30.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTES 50 SECONDS WEST 30.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTES 50 SECONDS SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTES 40 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 20 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTES 40 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTES 50 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 20 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 20 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 20 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 50 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 50 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 50 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 50 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTE 50 SECONDS EAST 100.00 FEET; THENCE SOUTH 57 DEGREES 43 MINUTES 50 SUBJER 50

#### 30' ACCESS & UTILITY EASEMENT DESCRIPTION

A PART OF A 69 ACRE PARCEL OF LAND OWNED BY THE MYATT FAMILY TRUST AS RECORDED IN DEED BOOK 112, PAGE 227, AND LYING SOUTHWEST OF THE INTERSECTION OF SALIE CIRCLE ROAD AND STEVE DENTON ROAD, BALLARD COUNTY, KENTUCKY.

-COMMENCING AT AN IRON PIN FOUND AT THE INTERSECTION OF SAID SALIE CIRCLE ROAD AND STEVE DENTON ROAD, SAID POINT ALSO BEING THE NORTHEAST CORNER OF A PARCEL OF LAND OWNED BY DAVID L. JONES AS RECORDED IN DEED BOOK 93. PAGE 150. THENCE ALONG THE CENTERLINE OF SALIE CIRCLE ROAD NORTH 68 = DEGREES 26 MINUTES 09 SECONDS WEST 172.37 FEET TO THE TRUE PLACE OF BEGINNING OF THIS ACCESS AND EASEMENT DESCRIPTION; THENCE SOUTH 71 DEGREES 49 MINUTES 20 SECONDS WEST 78.70 FEET; THENCE NORTH 76 DEGREES 06 MINUTES 17 SECONDS WEST 128.76 FEET; THENCE SOUTH 32 DEGREES 16 MINUTES 40 SECONDS WEST 132.35 FEET; THENCE NORTH 57 DEGREES 43 MINUTES 20 SECONDS WEST 30.00 FEET TO THE SOUTHERNMOST LEASE CORNER: THENCE  $\sim$ NORTH 32 DEGREES 16 MINUTES 40 SECONDS EAST 143.46 FEET; THENCE SOUTH 76 DEGREES 06 MINUTES 17 SECONDS EAST 141.78 FEET; THENCE NORTH 00 DEGREES OO MINUTES OO SECONDS EAST 23.37 FEET; THENCE SOUTH 68 DEGREES 26 MINUTES 09 SECONDS EAST 81.66 FEET TO THE TRUE PLACE OF BEGINNING AND CONTAINING 10,105.1 SQUARE FEET, (0.23 ACRES), MORE OR LESS.



LAND SURVEYOR'S CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAT AND SURVEY WERE MADE UNDER MY SUPERVISION AND THAT THE ANGULAR AND LINEAR MEASUREMENTS AS WITNESSED BY MONUMENTS SHOWN HEREON ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

~ Well

NOTE: THIS DRAWING DOES NOT REPRESENT A BOUNDARY SURVEY.

**EV Barlow** 

### EXHIBIT B

## [BOUNDARY SURVEY OF THE PREMISES AND INGRESS/EGRESS AND UTILITY EASEMENT]

.



Filed for record this	3r	2	day of
Dec 2018.	at 11:36	e'cloc	k <u>A</u> m
recorded in Deed	Bookils	pa	K287
LYNN W. LANE, I	Ballard Co	inty (	Clerk
by Halle	STY	en	ca
Fee 8 2	9.00 1		

29



265748 Filed on:12/03/2018 12:02:56 PM Book: DEED Number: 118 Pages: 287 - 293 Lynn Lane ,Ballard County Clerk DC: KATIE Deed Tax:#0.00

## NOTICE LIST

- Commonwealth of Kentucky Dept. of Fish & Wildlife Oscar Road Highway 1105 La Center, KY 42056
- Myatt Family Trust Charles Myatt & Dee Ann Myatt, Trustees
   2224 Steve Denton Road Barlow, KY 42024
- David L. Jones
   2925 Steve Denton Road Barlow, KY 42024
- Flint Renfo 4540 Oscar Road Barlow, KY 42024
- Rhonda Rice & Coy Simmons 11930 Wallace Rd. Kevil, KY 42053



February 2, 2022

Matthew R. Clark Robert B. Scott Charles R. Grahn Frank D. Otte* John "Bart" Herriman William W. Gooden** Michael P. Maxwell Russell L. Brown**† Jennifer F. Perry Keith L. Feall N. Davey Neal Travis W. Cohron Maggie L. Sadler Kristin A. McIlwain Olivia A. Hess

Land Use Consultant Elizabeth Bentz Williams, AICP

Notice of Proposed Construction of Wireless Communications Facility Site Name: Barlow

Raymond J. Grahn (2015) Alex M. Clark (1991) Peter A. Pappas (1986) Thomas M. Quinn (1973) Joseph M. Howard (1964)

> *Also admitted in Montana [†]Also admitted in Kentucky **Registered Civil Mediator

Cellco Partnership, d/b/a Verizon Wireless has filed an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located at Steve Denton Road, Barlow, KY, 42024 (North Latitude: (37° 06' 42.15", West Longitude 89° 02' 44.58"). The proposed facility will include a 285-foot tall antenna tower, plus a 5-foot lightning arrestor, for a total height of 290 feet with related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

This notice is being sent to you because the County Property Valuation Administrator's records indicate that you may own property that is within a 500' radius of the proposed tower site or contiguous to the property on which the tower is to be constructed. You have a right to submit testimony to the Kentucky Public Service Commission ("PSC"), either in writing or to request intervention in the PSC's proceedings on the application. You may contact the PSC for additional information concerning this matter at: Kentucky Public Service Commission, Executive Director, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2022-00016 in any correspondence sent in connection with this matter.

We have attached a map showing the site location for the proposed tower. Applicant's radio frequency engineers assisted in selecting the proposed site for the facility, and they have determined it is the proper location and elevation needed to provide quality service to wireless customers in the area. Please feel free to contact us at 317-637-1321 if you have any comments or questions about this proposal.

Sincerely, Russell L. Brown

Attorney for Applicant RLB/jdj enclosure

## VICINITY MAP




Matthew R. Clark Robert B. Scott Charles R. Grahn Frank D. Otte* John "Bart" Herriman William W. Gooden** Michael P. Maxwell Russell L. Brown**† Jennifer F. Perry Keith L. Beall N. Davey Neal Travis W. Cohron Maggie L. Sadler Kristin A. McIlwain Olivia A. Hess

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> > *Also admitted in Montana [†]Also admitted in Kentucky **Registered Civil Mediator

February 2, 2022

VIA CERTIFIED MAIL 7020 1810 0002 1853 0596

Hon. Todd Cooper 437 Ohio Street Wickliffe, KY 402087

> RE: Notice of Proposal to Construct Wireless Communications Facility Kentucky Public Service Commission Docket No. 2022- 00016 Site Name: Barlow

Dear Judge Cooper:

Cellco Partnership, d/b/a Verizon Wireless has filed an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located at Steve Denton Road, Barlow, KY, 42024 (North Latitude: (37° 06' 42.15", West Longitude 89° 02' 44.58"). The proposed facility will include a 285-foot tall antenna tower, plus a 5-foot lightning arrestor, for a total height of 290 feet with related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

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Sincerely Russell L. Brown

Attorney for Applicants

RLB/jdj enclosure VICINITY MAP





February 2, 2022

Matthew R. Clark Robert B. Scott Charles R. Grahn Frank D. Otte* John "Bart" Herriman William W. Gooden** Michael P. Maxwell Russell L. Brown**† Jennifer F. Perry Keith L. Feall N. Davey Neal Travis W. Cohron Maggie L. Sadler Kristin A. McIlwain Olivia A. Hess

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Sincerely, Russell L. Brown

Attorney for Applicant RLB/jdj enclosure

## VICINITY MAP



# SITE NAME: Barlow NOTICE SIGNS

The signs are at least (2) feet by four (4) feet in size, of durable material, with the text printed in black letters at least one (1) inch in height against a white background, except for the word "**tower**," which is at least four (4) inches in height.

Cellco Partnership, d/b/a Verizon Wireless propose to construct a telecommunications **tower** on this site. If you have questions, please contact Clark, Quinn, Moses, Scott & Grahn, LLP, 320 N. Meridian Street, Indianapolis, IN 46204; 317-637-1321, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2022-00016 in your correspondence.

Cellco Partnership, d/b/a Verizon Wireless propose to construct a telecommunications **tower** on this site. If you have questions, please contact Clark, Quinn, Moses, Scott & Grahn, LLP, 320 N. Meridian Street, Indianapolis, IN 46204; 317-637-1321, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2022-00016 in your correspondence.

#### VIA EMAIL: <u>larrah@ky-news.com</u> <u>advanceyeoman@gmail.com</u>

Kentucky Publishing Inc. 1540 McCracken Blvd. Paducah, KY 42001

February 1, 2022

RE: Legal Notice Advertisement Site Name: Barlow

Dear Ms. Workman:

Please publish the following legal notice advertisement in the next available edition of the *Advance Yeoman:* 

#### NOTICE

Cellco Partnership, d/b/a Verizon Wireless has filed an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located at Steve Denton Road, Barlow, KY, 42024 (North Latitude: (37° 06' 42.15", West Longitude 89° 02' 44.58"). You may contact the PSC for additional information concerning this matter at: Kentucky Public Service Commission, Executive Director, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2022-00016 in any correspondence sent in connection with this matter.

After this advertisement has been published, please forward a tearsheet copy, affidavit of publication, and invoice to Clark, Quinn, Moses, Scott & Grahn, LLC, 320 N. Meridian Street, Indianapolis, IN 46204 or by email to ebw@clarkquinnlaw.com. Please call me or Elizabeth Bentz Williams, in our offices at (317) 637-1321 if you have any questions. Thank you for your assistance.

Sincerely

Hight Bat William

Elizabeth Bentz Williams Clark, Quinn, Moses, Scott & Grahn, LLC

## Radio Frequency Design Search Area





Wednesday, December 5, 2018

RE: Proposed Verizon Wireless Communications Facility
Site Name: EV Barlow.
Type of Tower: 280' self-support Tower.
Location: 2557 Steve Denton Rd Barlow, KY 42024.

To Whom It May Concern:

As a radio frequency engineer for Verizon Wireless, I am providing this letter to state the need for a Verizon Wireless site called **EV Barlow**.

The EV Barlow site is proposed with the below objectives:

- 1. Offload 4G traffic from busy site to the northwest.
- 2. Offload 4G traffic from busy site to the northeast.
- 3. Improve 4G throughput to existing heavy data users.
- 4. Improve 4G network reliability by increasing the amount of time our customers operate on 4G instead of 3G.

Currently the area is experiencing high demand for wireless high-speed data. Growth forecasts have triggered the need for an additional site in the area. The tower is needed to provide all Verizon customers in the area with the best experience on their 4G wireless devices.

Raw Land – Design plans for a new tower would provide tower height of **280'**. The new structure height was decided upon to best cover the offload area and interact with the existing Verizon sites. If we are limited to building a structure less than the proposed height, another tower would be needed in the vicinity in the near future. In addition, building a structure that is too short can cause existing taller sites to shoot over the proposed site and building a site that is too tall can cause the proposed site to shoot over existing sites. Both situations create a poor experience from a user perspective. The new structure will be placed near the center of the area with high traffic demand and offload the surrounding sites greatly. The new tower design meets stated objectives.

Verizon Wireless cares about the communities as well as the environment and prefers to collocate on existing structures when available. It can be noticed from any map that Verizon Wireless is currently collocated on many existing structures in the area. We prefer collocation due to reduced construction costs, faster deployment, and environment protection. However, Verizon Wireless was unable to find a suitable structure within the center of demand area to collocate the proposed **EV Barlow** site.



,

Verizon Wireless design engineers establish search area criteria in order to effectively meet coverage objectives as well as offload existing Verizon cell sites. When met, the criterion also reduces the need for a new site to cover the area in the immediate future. Each cellular site covers a limited area, depending on site configuration and the surrounding terrain. Cell sites are built in an interconnected network; which means each cell site must be located so that their respective coverage areas are contiguous. This provides uninterrupted communications throughout the coverage area.

Since collocation is generally the most cost-effective means for prompt deployment of new facilities, Verizon Wireless makes every effort to investigate the feasibility for using existing towers or other tall structures for collocation when designing a new site or system expansion. However, collocation on an existing tower or tall structure is not always feasible due to location of existing cell sites. Cell sites are placed in a way so they provide smooth hand off to each other and are placed at some distance from each other to eliminate too much overlap. Too much overlap may result in a waste of resources and raise a system capacity overload concern.

This cell site has been designed, and shall be constructed and operated in a manner that satisfies regulations and requirements of all applicable governmental agencies that have been charged with regulating tower specifications, operation, construction, and placement, including the FAA and FCC.

Sincerely,

Michael Fahim.

**RF Engineer, Verizon Wireless** 

Milmel



STATE OF INDIANA

AMILTON COUNTY OF

Subscribed and sworn to before me this  $5^{1}$ 

**Notary Public** 

Signature

____day of DECEMBER, 2018.

THOMAS D. HERNDON Notary Public, State of Indiana SEAL My Commission Expires 9/2/2023

Thom HERNDON Printed

-150 County of Residence

My Commission expires: 9 - 2 - 2023



Wednesday, December 5, 2018

**RE: Ballard County Zoning Plots** 

Site Name: EV Barlow.

To Whom It May Concern:

This map is not a guarantee of coverage and may contain areas with no service. This map reflects a depiction of predicted and approximate wireless coverage of the network and is intended to provide a relative comparison of coverage. The depictions of coverage do not guarantee service availability as there are many factors that can influence coverage and service availability. These factors vary from location to location and change over time. The coverage areas may include locations with limited or no coverage. Even within a coverage area shown, there are many factors, including but not limited to, usage volumes, service, outage, and customer's equipment, and terrain, proximity to buildings, foliage, and weather that may impact service.

The proposed site is needed to offload capacity from existing sites. This map reflects the predicted coverage area that will be offloaded from existing sites and transferred to the proposed site.

Michael Fahim.

RF Engineer, Verizon Wireless



### EV Barlow Pre







**EV Barlow Post** 



Legend:	
Existing Verizon Sites	0
Proposed Verizon Site	•
Future Verizon Site	0
County Border	

Page **3** of **4** 



