



# Final Groundwater and Non-Groundwater Corrective Action Remedy Selection Report

Green Landfill Sebree Station Webster County, Kentucky

Prepared for:



Big Rivers Electric Corporation Sebree Generating Station 9000 Highway 2096 Robards, KY 42452

Prepared by:

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AECOM PN 60626688

November 18, 2020

# Certification Statement 40 CFR § 257.97(a) – Selection of a Remedy for the Corrective Action Program for Green Station CCR Landfill

#### Big Rivers Electric Corporation Sebree Generating Station, Green CCR Landfill

AECOM ("Consultant") has been retained by Big Rivers Electric Corporation to certify whether the selected groundwater remedy presented herein for the Green Station coal combustion residuals (CCR) landfill meets the requirements of Chapter 40 of the Code of Federal Regulations (CFR) §257.97.

#### **LIMITATIONS**

The signature of Consultant's authorized representative on this document represents that to the best of Consultant's knowledge, information, and belief in the exercise of its professional judgment, it is Consultant's professional opinion that the aforementioned information is accurate as of the date of such signature. Any opinion or decisions by Consultant are made on the basis of Consultant's experience, qualifications, and professional judgment and are not to be construed as warranties or guaranties. In addition, opinions relating to environmental, geologic, and geotechnical conditions or other estimates are based on available data, and actual conditions may vary from those encountered at the times and locations where data are obtained, despite the use of due care.

#### **CERTIFICATION**

I, Brian Cole, being a Registered Professional Engineer in the State of Kentucky, certify to the best of my knowledge, information, and belief, that the remedy selected by Big Rivers Electric Corporation for the Green Station CCR Landfill meets the requirements of 40 CFR § 257.97, and that this certification is true and correct and has been prepared in accordance with generally accepted good engineering practices.

M. Brian Cole	
Printed Name	
11/18/2020	
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#### **Revision History**

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#### **Distribution List**

# Hard Copies	PDF Required	Association / Company Name
	1	Big Rivers Electric Corporation

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#### 1. Introduction

In accordance with provisions of the United States Environmental Protection Agency's (USEPA) coal combustion residual (CCR) rule, Title 40 of the Code of Federal Regulations (CFR) Part 257.97, Big Rivers Electric Corperation (BREC) is required to select a remedy to address groundwater impacts identified at the Green Station CCR Landfill (the Unit) at the Sebree Generating Station located in Webster County, Robards, Kentucky (Figure 1). Previous monitoring results indicate the presence of lithium at a Statistically Significant Level (SSL) above the Ground Water Protection Standard (GWPS) in four monitoring wells (MW-3A, MW-4, MW-5, and MW-6) at the Unit. In June 2019, BREC performed an Assessment of Corrective Measures (ACM) for the Unit to identify remedial alternatives to address groundwater impacts. A public meeting was held on July 16, 2020 in Henderson, Kentucky to dicuss the results of the ACM. No public input was received at this meeting. Additional technical assessment has been utilized by BREC to select the final remedy for the Unit in accordance with 40 CFR Part 257.97, which is presented in this report.

On December 16, 2019, an Agreed Order was filed with the Kentucky Office of Administrative Hearings between BREC and the Commonwealth of Kentucky, Energy and Environment Cabinet, Division of Waste Management (KDWM) to address Notices of Violation (NOVs) received in regard to unpermitted discharges and seepage emanating form the Unit (see Section 1.2). Within the AGREED ORDER are requirements for remedy selection reporting, including a timeline for review by the KDWM. These requirements are discussed in Paragraphs 18 and 23 of the Agreed Order and listed in Exhibit 4 to the Agreed Order. This report has been prepared to address these requirements in the Agreed Order and Exhibit 4 to the Agreed Order, in addition to the Federal CCR Rule requirements.

In parallel with addressing groundwater impacts, BREC performed an ACM for non-groundwater release surface seeps at the Unit in June 2019. In September and October 2019, BREC initiated interim corrective measures (ICMs) to address non-groundwater releases at the Unit. The ICMs are currently being evaluated through performance monitoring and are expected to benefit corrective action as a whole for the Unit. As a result, no separate remedy selection report is currently being developed for non-groundwater releases. BREC intendes for this report to address the remedy selection requirements for both groundwater and non-groundwater impacts under 40 CFR Part 257.

#### 1.1 Regulatory Background

Kentucky Revised Statue (KRS) Chapter 224.50-760 governs the disposal of special waste, including utility wastes. The Commonwealth of Kentucky, Energy and Environment Cabinet (The Cabinet) promulgated regulations under Title 401 of the Kentucky Administrative Regulations (KAR) Chapters 45 and 46 to regulate the disposal of special wastes The Unit is a Kentucky permitted landfill (Permit No. SW11700007) subject to permitting requirements for special wastes established under 401 KAR Chapter 45.

In 2015 the USEPA promulgated 40 CFR Parts 257.50 through 257.107 which established national standards to govern the location, design, construction, and operation of landfills and surface impoundments utilized to manage CCR. In 2017, the Cabinet promulgated 401 KAR 46:110 which incorporates the federal CCR standards by reference into Kentucky regulations. As noted in the Agreed Order, the Unit is an existing CCR landfill under the Federal CCR rule and therefore subject to the operating criteria and corrective action standards of 401 KAR 46:110.

Corrective actions at the Unit are being performed to address both the federal requirements in 40 CFR Part 257 and state requirements in 401 KAR Chapter 46 as described below.

#### 1.1.1 Federal CCR Background

In response to SSL exceedances in groundwater at the Unit, BREC evaluated the nature and extent of groundwater impacts as required by Title 40 CFR Part 257.95(g) for characterization monitoring.

Following chracterization monitoring, BREC performed an ACM, to identify potential corrective measures to address lithium impacts in groundwater pursuant to Title 40 CFR Part 257.96. A notice of ACM initiation dated January 14, 2019 was posted to BREC's publicly-accessible CCR reporting website. A report summarizing the results of the groundwater ACM (AECOM, June 2019) was posted to BREC's publicly-accessible CCR reporting website on June 14, 2019.

On March 15, 2018, the USEPA proposed a modification to the federal CCR regualtions to address four provisions within 40 CFR Section 257 that were remanded back to the USEPA on June 14, 2016 by the United States Court of Appeals for the District of Columbia Circuit. The proposed modifications to 40 CFR 257 (also known as the Remand Rule) also included provisions for owners and operators of CCR units in states that have approved CCR permit programs. Title 40 CFR Part 257.99 established procedures for owners and operators of CCR units to perform corrective action for eligible non-groundwater releases at a CCR unit. In alignment with corrective actions being performed to address the NOVs received from the KDWM for unpermitted discharges and seepage emanating from the Unit, BREC perfomed an ACM for non-groundwater releases in addition to the ACM for groundwater impacts. A report summarizing the results of the groundwater ACM (AECOM, June 2019) was posted to BREC's publicly-accessible CCR reporting website on June 28, 2019. In 2019, pursuant to 40 CFR Parts 257.90(d) and 257.84(b)(5), BREC initiated design of ICMs (i.e., containment systems) intended to reduce and prevent non-groundwater releases from reaching the Green River. In September and October 2019, BREC intiated construction of ICMs to address non-groundwater releases at the Unit (which are referred to herein as river seeps), including:

- Construction of a collection trench along the east side of the Green Landfill (refered to as the Deep Seep Collection Trench) to address seeps adjacent to the Green River; and
- Construction of a series of collection trenchs along the north side of the Green Landfill (refered to as the Northwest Seep Collection Trench) to address seeps near the northwest corner of the landfill discharging toward an east-flowing unnamed tributary to the Green River.

Construction of the ICMs was functionally completed in Janaury 2020, within the 180 day required timeframe required under proposed 40 CFR Part 257.99, although piping, pumping, and control system installation, and installation of supplemental collector systems were not completed until later in 2020. The ICMs completed to address non-groundwater releases under 40 CFR Part 257 and the Agreed Order are expected to benefit groundwater corrective action and are discussed collectively within this report (see Section 2.4).

Title 40 CFR Section 257.97(a) requires that progress reports be prepared on a semi-annual basis describing progress made in selecting and designing a remedy. The first Remedy Selection Progress Report was finalized on December 9, 2019 and posted to BREC's publicly-accessible CCR reporting website on December 12, 2019.

BREC held a public meeting on July 16, 2020 in Henderson, Kentucky to discuss the results of the Groundwater ACM in accordance with 40 CFR Part 257.96(e). No public input influencing the remedy for the Unit was received during the meeting. BREC has selected the remedy for groundwater and non-groundwater impacts at the Unit in accordance with 40 CFR Part 257.97 as detailed within this report.

#### 1.1.2 Kentucky Division of Waste Management Background

On December 6, 2019, BREC signed Agreed Order #18-3-0138 with the KDWM to address NOVs received regarding unpermitted discharges and seepage emanating from the Unit. The Agreed Order was filed on December 16, 2019. Under the Agreed Order the following actions were required:

- Development of Standard Operating Procedures (SOPs) to characterize and mitigate leachate and seep releases to the surface (Exhibit 1);
- Development of construction and post-construction plans for implementing the Northwestern Seep Collection Trench Remedy (Exhibit 2);

- Development of construction and post-construction plans for implementing the Eastern "Deep Seep" Collection Trench Remedy (Exhibit 3); and
- Establishment of the process to complete the evaluation of groundwater corrective action remedies at the Unit pursuant to 401 KAR 46:110 (Exhibit 4).

Within Exhibit 4 of the Agreed Order, the following milestones for groundwater corrective action were identified:

- 1) Within 180 days of the entry of the Agreed Order, BREC shall conduct a public meeting as required by 40 CFR 257.96(e) and 401 KAR 46:110.
- 2) Within 90 days of the public meeting, BREC shall submit a draft groundwater remedy selection report for submittal to KDWM for a 30-day review and comment period.
- 3) As soon as possible, following receipt of KDWM comments on the draft groundwater remedy selection report, select the final groundwater corrective action remedy.
- 4) Posting of the Final *Groundwater and Non-Groundwater Corrective Action Remedy Selection Report* to BREC's CCR Rule compliance website in accordance with 40 CFR 257.97 and 257.107 (no timeline specified).

Although the milestone schedule has been adjusted due to the COVID-19 pandemic, which prevented BREC from holding the public meeting at an earlier date, BREC has moved forward with the activities required in the Agreed Order as documented in this report. A revised schedule for corrective action implementation is discussed in Section 5.0.

#### 2. Site Background

#### 2.1 Site Description

BREC owns and operates Sebree Station, which is a coal-fired power generating facility located on the Green River northeast of Sebree, Kentucky. Sebree Station is composed of Green Station and Reid/Henderson Municipal Power & Light (HMP&L) Station. The Sebree Station is bounded by Interstate-69 to the west and the Green River to the east (see **Figure 1**). Reid Unit 1 (65 Megawatts [MW]) began commercial operation in 1966 and is scheduled to be retired in 2020 pending regulatory approval from the Kentucky Public Service Commission and Rural Utilities Service. The Reid Combustion Turbine (65 MW) was commercialized in 1976. HMP&L Station 2, Units 1 (167 MW) and 2 (168 MW) began commercial operation in 1973 and 1974 respectively. Both HMP&L units were retired as of February 1, 2019. Green Station Units 1 (250 MW) and 2 (242 MW) began commercial operation in 1979 and 1981, respectively.

The location of the Green Landfill is illustrated on **Figure 1**. The Green Landfill is located directly south of Sebree Station, situated south of the Green Station CCR Surface Impoundment. The Green Landfill is a Kentucky permitted landfill (Permit No. SW11700007) that receives special wastes generated by burning coal (CCRs) from Green and Reid/HMP&L Stations. The landfill began receiving CCR wastes in 1980. The current Green Landfill footprint is approximately 170 acres.

As stated in the published CCR monitoring well network certification, available on the BREC website (http://www.bigrivers.com/), the original ground surface within the landfill footprint was irregular and the dominant features were small stream valleys draining towards the Green River, which is located just east of the landfill; and towards Groves Creek, which is located just south of the landfill. There was also historic oil and gas production at and in the immediate vicinity of the Green Landfill. A review of the records from the Kentucky Geological Survey (KGS) showed that at or immediately adjacent to the Site, there were a number of dry exploratory oil/gas exploration holes, oil production wells, one gas production well, and one secondary recovery injection well. There were also former brine ponds at the Site. Most of these wells were abandoned in accordance with applicable regulations by BREC in 1997 and 1998. The last existing oil well was decommissioned in 2019.

#### 2.2 Groundwater Investigation Summary

Monitoring wells were installed at the Unit beginning in November 1996 prior to the implementation of the CCR Rule. However, the existing wells meet the requirements of Title 40 CFR Section 257.90 of the CCR Rule for installation of a groundwater monitoring system. These regulations require that monitoring wells adequately represent the quality of background groundwater and groundwater representing the downgradient waste boundary. The existing wells are located along the perimeter of the landfill footprint. One upgradient monitoring well (MW-1) and five downgradient monitoring wells (MW-2, MW-3A, MW-4, MW-5 and MW-6) were installed at the Unit to determine the general direction of groundwater movement and to monitor groundwater impacts. One additional characterization monitoring well (MW-104) was installed downgradient of the Unit in 2018. All monitoring wells were installed in the uppermost saturated portion of the sandstone bedrock aquifer. A map illustrating the location of all program monitoring wells is presented as **Figure 2**.

Nine rounds of Baseline groundwater sampling for Appendix III constituents were conducted between March 2016 and October 2017. Statistical evaluation for Detection monitoring indicated that statistically significant increases (SSIs) over background had occurred, and therefore, Assessment monitoring was triggered. Detection monitoring activities and data are presented in the annual reports that have been prepared to date, (AECOM 2018, 2019, and 2020).

As part of Assessment monitoring, upgradient and downgradient wells for the Unit were sampled for Appendix IV constituents in June, July, and September 2018. GWPSs were established for the Appendix IV constituents occurring at SSIs (lithium only), and statistical evaluation of the lithium concentrations indicated exceedances of GWPSs at SSLs, as detailed in **Table 1** below.

**Table 1. Green Landfill Constituents of Concern** 

Monitoring Well (Date)	Parameter Lithium GWPS 0.04 (mg/L)
MW-3A (Jun 2018)	0.699
MW-3A (Jul 2018)	0.790
MW-3A (Sep 2018)	0.766
MW-4 (Jun 2018)	1.81
MW-4 (Jul 2018)	1.91
MW-4(Sep 2018)	1.81
MW-5(Jun 2018)	0.459
MW-5 (Jul 2018)	0.481
MW-5 (Sep 2018)	0.425
MW-6 (Jun 2018)	0.0650
MW-6 (Jul 2018)	0.0590
MW-6 (Sep 2018)	0.0558

GWPSs are the greater of the site-specific background concentrations, the USEPA primary drinking water standard maximum contaminant limits (MCL), or GWPS provided in 40 CFR 257.95(3)(h)(2)

An additional characterization well, MW-104, was subsequently installed to estimate the downgradient extent of impacted groundwater. Sample collection from MW-104 for Appendix III and IV parameters took place in March and April 2019. The analytical results for lithium were below the GWPS. The additional characterization data are summarized in **Table 2** below.

Table 2. Green Landfill -2019 Characterization Sample Results

	Parameter
Monitoring Well (Date)	Lithium GWPS 0.04 <sup>a</sup>
	(mg/L)
MW-104 (March 2019)	0.0281
MW-104 (April 2019)	0.0288

a The Upper Prediction Limit for lithium was calculated as 0.008 mg/L.

The results from both characterization sampling events helped to confirm the downgradient (southwestern) extent of constituent of concern (COC) impacts above GWPS at the Unit.

Semi-annual Assessment monitoring continued at the Unit in 2019 and 2020 in accordance with 40 CFR 257.95.

#### 2.3 Conceptual Site Model

Development and refinement of a Conceptual Site Model (CSM) is necessary to support remedy selection for the Unit. A CSM is based on a set of working hypotheses regarding how contaminants of concern (COCs) entered the environment at a site, how they were and continue to be transported to various media, what the potential routes of exposure are, and who may be exposed, including both human and ecological receptors. As such, the CSM is a "living" model. As new data become available or site conditions change, a CSM should be evaluated and updated as necessary.

The CSM for the Unit was first provided in the June 2019 ACM for the Unit (AECOM 2019). The CSM presents the physical setting of the Unit (adjacent to the Green River), the unconsolidated and bedrock geologic strata underling the Unit, the occurrence and movement of groundwater, the distribution of COCs in groundwater, and the potential receptors (or lack thereof) for impacted groundwater. These elements are described in detail below and have been updated with new information for this report as appropriate.

#### 2.3.1 Physical Setting

The Unit is located within the Interior Low Plateaus physiographic province. The province is part of the Interior Plains division of the United States. Characteristic features of the province include unglaciated rolling limestone plains with alluvial valleys and entrenched rivers and streams. Several large rivers are in the region, including the Green, Ohio, Kentucky, Tennessee, and the Cumberland Rivers. The geology underlying the Unit consists of unconsolidated materials, including loess and alluvial deposits, underlain by Upper to Middle Pennsylvanian-age clastic and carbonate bedrock consisting primarily of sandstone and shale. The unconsolidated materials also include fill, silty and clayey residuum, and minor amounts of sandy, clayey channel fill alluvium.

The Unit is located on an upland adjacent to the west bank of the Green River at an elevation of approximately 436 feet, above mean sea level [ft., amsl] (at the north end of the landfill) and 397 ft., amsl (at the south end of the landfill), with a maximum elevation of 608 ft., amsl at the landfill crest. Precipitation falling within the Green Landfill is directed to ponds on the north and south sides of the Unit and then to the river under Kentucky Pollution Discharge and Elimination System (KPDES) permit No. KY0001929. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands (Associated Engineers 2016, Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan).

#### 2.3.2 Geology

The Unit lies in the Western Kentucky Coalfields section, characterized by rolling uplands underlain by coal-bearing bedrock of the Pennsylvanian Period. Near the Unit, maximum topographic relief is on the order of 80 feet. The geologic quadrangle (Geologic map of the Robards quadrangle, Henderson and Webster Counties, Kentucky, 1973) for the area published by the Kentucky Geologic Survey (KGS) shows the surficial material in portions of the western half of the Unit to be unconsolidated loess representing the Pleistocene geologic epoch. The loess consists of sandy and clayey silt. Underlying the loess deposits and exposed at the surface on the eastern half of the Unit are broadly distributed Pleistocene and Holocene alluvium deposits consisting of intermixed and interlensing clay, silt, sand, and gravel. In close proximity to the Unit, the alluvium is generally a low permeability unit that forms terraces along the Green River at elevations of roughly 380 and 395 ft., amsl. The unconsolidated surficial materials range from approximately 10 feet (MW-5) to 52 feet (MW-104) in thickness surrounding the Unit. Figure 3 provides an excerpt from the geologic quadrangle for the immediate area surrounding the Unit.

The unconsolidated materials are underlain by bedrock of the Upper Pennsylvanian Shelburn Formation [formerly identified as the Lisman Formation (Fairer, 1973)] and the Middle Pennsylvanian Carbondale Formation. At the base of the Shelburn Formation is the Providence Limestone Member, consisting of two distinct limestone beds separated by a sandy shale. The member is exposed in a streambed near the northwest corner of the Unit but is absent beneath much of the Unit footprint due to erosional channeling.

The underlying Carbondale Formation consists of cyclic sequences of sandstones, shales, siltstones and coals. The Carbondale sediments were deposited in a fluvial-deltaic system. As a result of this depositional environment, the lithologic units of the Carbondale tend to be lenticular bodies rather than continuous sheet-like strata. Gradational and abrupt horizontal changes in lithology are often encountered.

Cross-sections have been developed to support the CSM and are presented as **Figures 4**, **5**, **6** and **7**. Cross-section locations are shown on **Figure 2**. These sections illustrate the sequence of geologic materials present under the Unit as interpreted using the currently available data.

#### 2.3.3 Groundwater Hydrogeology

For purposes of compliance with the CCR Rule groundwater monitoring requirements, the interbedded sandstone and shale of the Carbondale Formation is considered the uppermost aquifer underlying the Unit. The uppermost aquifer is hydraulically confined and first encountered at an elevation of approximately 401 ft., amsl at the northwest end of the landfill, and 367 ft., amsl at the southeast end of the landfill (AECOM, 2019).

Groundwater elevation data collected in April 2020 are summarized on **Table 3** below. These data were utilized to construct a piezometric surface map illustrating groundwater flow conditions for the uppermost aquifer (see **Figure 8**). Overall groundwater flow beneath the footprint of the Unit is to the east towards the Green River and south-southeast towards Groves Creek.

Monitoring Well	Top of Casing Elevation (ft) <sup>1</sup>	Depth to Groundwater (ft)	Groundwater Elevation (ft, amsl)
MW-1	423.23	19.52	403.71
MW-2	392.37	16.24	376.13
MW-3A	386.48	12.08	374.40
MW-4	391.33	17.90	373.43
MW-5	390.18	17.62	372.56
MW-6	388.17	15.62	372.55
MW-12 <sup>2</sup>	395.54	22.15	373.39

Table 3. Green Landfill -April 2020 Groundwater Elevation Data

Slug tests were performed on April 25, 2019 at monitoring wells MW-3A, MW-4, MW-6, and MW-104 to assess the hydraulic characteristics of the uppermost aquifer. The estimated hydraulic conductivity of the monitoring wells tested ranged from  $2 \times 10^{-5}$  to  $3 \times 10^{-3}$  centimeters per second (cm/sec).

Although previous site-specific investigations have noted the presence of perched zones of saturation in the overlying unconsolidated materials, these discontinuous zones do not qualify as an uppermost aquifer under the CCR Rule because they do not produce usable quantities of groundwater (40CFR Part 257.53).

#### 2.3.4 Non-Groundwater Hydrogeology

Two types of non-groundwater releases have been identified through inspection and investigation of the Unit: river seeps and perimeter seeps. The river seeps are those found along the Green River and its tributary streams. River seeps have been observed on the bank of the river/tributary and on the slope between the river/tributary and the landfill perimeter road. The river seeps on the northwest side of the

<sup>1</sup> Reference elevation of monitoring wells surveyed by Fuller, Mossbarger, Scott and May, Civil Engineers, Inc., Lexington, Kentucky, December 1996, December 1999. Survey coordinates were based on the Kentucky State Plane, Kentucky Southern Zone, NAD27 datum.

MW-12 is utilized for collection of piezometric data only and is not part of the CCR monitoring well network for the Green Landfill.

landfill drain to a KPDES permitted outfall, whereas the river seeps on the Green River side do not. Perimeter seeps are more surficial in nature and have been observed in various surface ditches located around the perimeter of the Green Landfill, all of which drain to sedimentation basins that discharge to a KPDES permitted outfall.

#### 2.3.4.1 River Seeps

An investigation of the seeps along the Green River was conducted in July 2018 and was reported in a Technical Memorandum from AECOM to BREC dated September 6, 2018. The results of laboratory analysis of seep samples collected during this investigation are summarized in **Appendix A**. During this investigation, the banks of the Green River were surveyed by boat for evidence of seepage. The survey was conducted when the river stage had retreated to a low pool after a prolonged elevated stage so that the maximum number of seeps might be surveyed, and seepage rates might be high enough to allow sampling. Samples of seeps having visible flow were collected and tested for CCR indicator parameters (40 CFR Part 257 Appendix III), CCR constituents of concern (40 CFR Part 257 Appendix IV), and general chemistry parameters. The data from these analyses were used to evaluate whether individual seeps were likely associated with the Landfill.

Riverbank seeps were identified at sixteen discrete locations in the vicinity of Sebree Station. Seeps were recorded at locations on both the east and west banks of the river over two miles upstream of the landfill footprint and over 1.5 miles downstream of the landfill footprint. Some seeps appeared to potentially be associated with a surface water drainage feature, such as RS-11 where there appears to be a beaver pond beyond the riverbank, but most emanated from otherwise nondescript sections of riverbank. Some of the seeps resulted in a green discoloration of the riverbank, but most had orange staining.

Of the seven seeps tested, only three, RS-05, RS-07, and RS-08 as illustrated on Figure 1 in **Appendix A**, were found to have similar chemistry to leachate generated by the Green Landfill. These seeps did not differ greatly from the majority of the other riverbank seeps in that they were broadly seeping from the bank sediments and had a general orange discoloration, except that RS-07 had a some relatively discrete seepage points emanating from a few feet higher on the bank and RS-08 appeared to be emanating from on top of bedrock outcropping on the river bank. Seeps RS-05 and RS-07 are located near the center of the Landfill between monitoring wells MW-2 and MW-3A. This is the same area in which seeps have been observed higher on the slope between the river and the perimeter road, suggesting that they have a similar origin. Seep RS-08 is located adjacent to the South Sediment Basin and appears to be tied to that surface water feature. The approximate vertical position of the river seep locations relative to the Green Landfill are shown on **Figure 4**. It should be noted that the seep designated RS-06, located between RS-05 and RS-07, is likely to be of similar character and origin but was not generating enough flow to be sampled at the time of the survey.

The analytical results from the July 2018 river seep samples were compared to Kentucky Water Quality criteria for warm water aquatic habitat identified in 401 KAR 10:031 Section 6. Where there are no Kentucky Water Quality criteria for a specific constituent, the USEPA Region 4 surface water screening values were utilized for comparison. It should be noted that the Region 4 screening values are not compliance criteria, but rather values used to determine whether further evaluation is warranted. Samples from RS-05, -07 and -08 were found to exceed the 600 milligrams per liter (mg/L) limit for chloride. RS-05 also exceeded the current criteria for cadmium (0.00029 mg/L) and lead (0.0036 mg/L), but Kentucky has introduced a new cadmium criterion that may bring RS-05 back into compliance. Follow-up sampling conducted in December 2018 by the Kentucky Division of Water (KDoW) and BREC confirmed the exceedance of the chloride criteria. Accordingly, this parameter (chloride) is regarded as the primary COC for non-groundwater releases at the Unit requiring corrective action. Addressing the river seeps was included as a stipulation in the Agreed Order signed between BREC and the KDWM.

The analytical results for the river seep samples are summarized in **Appendix A**. Presented in parallel with the river seep results are deep in-stream river samples that were collected immediately adjacent to the river seeps to characterize the river water quality that is most likely to be impacted by seepage. The

deep samples were collected within 1 foot of the riverbed within 3 to 5 feet of the water line. None of the river sample results exceed the water quality or screening criteria suggesting that the identified river seeps are not impacting the Green River.

Additional data regarding the river seeps is provided in the Assessment of Corrective Measures Non-Groundwater Release Under the CCR Rule, Green Station CCR Landfill (AECOM June 28, 2019).

In April 2019, inspection of the Landfill site by the KDWM and KDoW identified an area of seepage outside the perimeter road on the northwest side of the Landfill (see Figure 2). This seepage (herein identified as the NW Seep) is adjacent to a tributary ditch that flows eastward to an unnamed outfall which has a KPDES discharge permit. The outfall was sampled by KDoW and BREC on April 2, 2019. A sample from this seep area (identified as sample 023) was collected by BREC personnel on April 11, 2019. The results indicated that the seep sample exceeded Kentucky Warm Water Aquatic Habitat criteria for Chronic Exposure for chloride and cadmium. As a result, this area was identified for corrective action. Addressing this seep area was included as a stipulation in the Agreed Order signed between BREC and the KDWM.



Photo 1: Bedrock outcrop located west of the NW Seep as observed on April 2, 2019.

The NW Seep appears to emanate from a horizon in or above a natural limestone ledge adjacent to the ditch. This conclusion is based on the observation of natural springs of groundwater upstream from the seep that clearly flows from fractures in the ledge. A series of three soil borings drilled between the landfill and the NW Seep area in May 2019 further suggest the seepage is controlled by this feature. **Figure 7** provides a cross-section illustrating the sequence of geologic materials present within the NW seep area as interpreted using the currently available data.

#### 2.3.4.2 Perimeter Seeps

During the July 2018 investigation of Green River seeps, the area inside the Landfill perimeter road was also inspected for seeps. Four areas of perimeter seepage were identified (see **Figure 2**): along the west side of the landfill (LS-01), the southwest corner (LS-04), the south end adjacent to the South Sediment Basin (LS03), and the east side north of MW-2 vicinity (LS02). LS-01, LS-02 LS-03, and LS-04 are directed to the South Sediment Basin, which is pumped to the Northeast Sediment Basin and then further to the Green surface impoundment and eventually discharged to the Green River under KPDES permitted outfall #001.

Samples of a select set of these perimeter seeps were collected in July 2018 and tested for the Appendix III, Appendix IV, and general chemistry parameters. As previously noted, these seeps do not directly discharge to surface waters, but they may have the potential to influence groundwater and other non-groundwater releases. As such, they will be addressed by future corrective action to manage those potentials (see Section 4).

Additional data regarding the perimeter seeps is provided in the Assessment of Corrective Measures Non-Groundwater Release Under the CCR Rule, Green Station CCR Landfill (AECOM June 28,2019).

#### 2.3.5 Constituents of Concern

Groundwater analytical data obtained from groundwater sampling events performed at the Unit through 2019 are summarized in **Appendix B**. A summary of the statistical evaluation conducted on the Appendix III and Assessment Appendix IV parameters for the Green Landfill is provided in **Appendix C**. Combined, these data indicate that the only COC detected at SSLs above its GWPS in groundwater at the Unit is lithium. Lithium has been detected at SSLs in the wells MW-4, MW-5, and MW-6 surrounding the South Sediment Basin and in MW-3A located north (downstream on the Green River) of MW-4.

Chloride is regarded as the primary COC for non-groundwater releases at the Unit requiring corrective action. Although there have been Appendix IV (Part 257) constituents detected in the surface seeps identified within the perimeter footprint of the landfill, these seeps are contained within a KPDES permitted discharge area that are monitored routinely to ensure compliance with applicable surface water quality standards.

#### 2.3.6 Impacted Media

Both groundwater and surface water have been identified as impacted media of concern requiring corrective measures at the Unit.

#### 2.3.7 Distribution of COCs

Groundwater sampling was performed at the Unit most recently in April 2020. Laboratory analytical data from the April 2020 sampling event is provided in **Appendix D**. The additional lithium data collected during this event are summarized below in **Table 4**.

	Parameter
Monitoring Well (Date)	Lithium GWPS 0.04 (mg/L)
MW-1	0.03
MW-2	0.007
MW-3A	0.68
MW-4	0.82
MW-5	0.38
MW-6	0.05

Table 4. Green Landfill - April 2020 Lithium Analytical Results

Figure 9 illustrates the distribution of COCs and other groundwater quality constituents in groundwater at the Unit. This distribution of COCs in groundwater suggests that impacts to groundwater likely originate from two primary source area. Impacts observed at MW-4, MW-5 and MW-6 likely originated as infiltration from the South Sediment Basin where storm water and landfill seepage accumulate on the south side of the landfill before being pumped to the Green Surface Impoundment. Data from characterization well MW-104 indicate that MW-3A may be effectively separated from the South Sediment Basin by a buried valley in the bedrock aquifer where groundwater does not appear to be impacted. This suggests that the impact observed at MW-3A may have instead originated from a different source, potentially from localized landfill seepage, which is now captured by the Deep Seep Collection Trench (see Section 2.4). It is possible that the MW-3A impact originates from the western end of the South Sediment Basin, but there is currently no feasible means of directly tracing that potential under the footprint of the landfill. It is, however, possible to evaluate this potential by monitoring MW-3A over time after the South Sediment Basin is rehabilitated as is currently planned (see Section 4). Ongoing monitoring of MW-3A also has the potential to demonstrate whether the landfill seepage intercepted by the Deep Seep Collection Trench is the source of impact.

#### 2.3.8 Potential Receptors/Exposure Pathways

Contact with water (e.g., shallow groundwater or surface water) impacted by COCs at levels above GWPS or Water Quality Criteria is regarded as the potential pathway for exposure of potential receptors. Based on data published by KGS, there are no known groundwater wells used for drinking water within a 1-mile radius of the Unit, thus limiting the potential receptors to the surface water, i.e., the Green River and its tributary, Groves Creek. The potential pathways to these receptors include seepage of water from the Unit through manmade and natural hydraulic conduits.

Other potential exposure pathways (e.g., soil or vapor) are not considered a risk as the CCR material is isolated in the Unit. This isolation prevents direct access by individuals that might result in direct contact or ingestion. In addition, the inherent non-volatile nature of the Unit-specific COCs eliminates the potential for a complete vapor pathway (i.e., vapor intrusion to indoor air).

#### 2.4 Interim Corrective Measures

In September and October 2019, BREC initiated design and construction of two containment systems intended as an interim corrective measure to reduce and prevent non-groundwater releases at the Unit from reaching the Green River. The containment systems are identified as the Deep Seep Collection Trench (also known as the Eastern Collection Trench) and the Northwest Seep Collection Trench.

No formal interim corrective measures have been performed at the Green Landfill to address groundwater impacts. However, the interim corrective measures for known non-groundwater releases completed at the Unit are expected to benefit corrective action for groundwater impacts. The compatibility of those corrective measures with potential groundwater remedies is currently being evaluated as part of the Unit's assessment monitoring and will continued to be evaluated in the future as part of systematic performance reviews (see Section 5.2).

#### 2.4.1 Deep Seep Collection Trench

BREC began construction of the Deep Seep Collection Trench on October 7, 2019. The installation of four partially overlapping trenches and corresponding individual sumps was completed on November 11, 2019. This completion allowed removal of collected seepage using temporary pumping and piping until the permanent system components were completed.

The Deep Seep Collection Trench is located on the eastern side of the landfill, adjacent to the Green River. This collection system consists of 1,065 lineal feet of perforated (HDPE) pipe and four (4) stainless steel sumps. The HDPE perforated pipe is surrounded by a washed river gravel, with profiles set at a 0.5% slope toward the associated pumping (sump) station. Each section of HDPE pipe overlaps at the sump interconnection to prevent seepage bypass and to ensure all deep seeps are properly captured. Each sump was set at an elevation of 352 ft., amsl. The approximate vertical position of the



Photo 2: Installation of the Deep Seep Collection Trench in October 2019.

Deep Seep Collection Trench relative to the Green Landfill is shown on **Figure 4**. The location of the trench in plan view is provided on **Figure 10**.

The electrical and mechanical portion of the project that allows the system to become fully automated was finalized on May 29, 2020.

### 2.4.2 Northwest Seep Collection

BREC began construction of the Northwest Seep Collection Trench on September 3, 2019. The construction of the collection trench was completed on January 22, 2020. The system is located in the northwest corner of the landfill and consists of 357 lineal feet of HDPE perforated pipe within the primary collection trench installed at an elevation of 391.4 ft, amsl. The HDPE perforated pipe is surrounded by a washed river gravel, with profiles set at a 0.5% slope toward the associated pumping (sump) station. Since the installation of the primary trench, BREC



Photo 3: Installation of the Northwest Seep Collection Trench in September 2019.

has installed two relay stations to ensure all possible seeps are captured and pumped to a permitted KPDES outfall. The Northwest Seep Collection Trench is configured to pump the incoming flow to a target manhole, which is located on the northeast corner of the landfill. The target manhole subsequently discharges to KPDES permitted outfall #009. The approximate vertical position of the Northwest Seep Collection Trench relative to the Green Landfill is shown on **Figure 7**. The location of the trench in plan view is provided on **Figure 10**.

#### 2.5 Assessment of Corrective Measures Summary

#### 2.5.1 Assessment of Corrective Measures for Groundwater Impacts

In June 2019, BREC performed an ACM for the Unit to identify remedial alternatives to address groundwater impacts. Title 40 CFR Section 257.96(c) requires that the ACM include an analysis of the effectiveness of potential corrective measures in meeting the objectives for remedies identified under Section 257.97(b), by addressing at least the following:

- The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- 2) The time required to begin and complete the remedy; and
- 3) The institutional requirements, such as state or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the remedy(s).

As part of the groundwater ACM, several potential corrective measures technologies were evaluated to identify which ones could be carried forward as components of corrective measures alternatives. The results of the corrective measures technology evaluation are presented below in **Table 5**.

Table 5 – Potential Corrective Measures Options for Groundwater Impacts

Potentially Applicable Technology	Status	Description/Overview	
No Action	Not retained as standalone technology, but carried forward for baseline comparisons	This technology has been included in the preliminary evaluation/screening but is not retained because it wi not meet the established Corrective Action Objectives (CAOs).	
Institutional Controls (ICs)	Retained as supplement to corrective measures alternatives	The use of ICs (i.e., Environmental Covenant, groundwater use restrictions, etc.) is retained as a useful technology. However, it is noted the ICs are not anticipated to be used as a stand-alone technology. Environmental Covenants, groundwater use restrictions, etc., are expected to be combined with other applicable technologies as part of corrective measures alternatives.	
Groundwater Monitoring (Assessment and Detection mode)	Retained as supplement to corrective measures alternatives	The use of groundwater monitoring (Assessment and/or Detection modes as appropriate) when combined with other applicable technologies as part of any proposed corrective measures alternative is retained to address the CAO and to track the effectiveness of the overall remedy. However, it is not retained as a standalone technology.	
Hydraulic Containment	Retained	The use of hydraulic containment is retained because it is an effective means of preventing offsite migration of soluble contaminants. Hydraulic containment requires management and potential ex-situ treatment of extracted groundwater, so it is not a stand-alone technology. The CSM will guide the design of any groundwater extraction system to optimize the total discharge of groundwater needed to provide hydraulic containment.	
Physical Containment	Retained	The use of physical containment is retained because it can be an effective means of managing groundwater flow. Physical containment often requires pairing with hydraulic containment and/or in-situ treatment (funnel and gate style) to manage the flux of groundwater flow into the system. The CSM will guide the design of any physical barrier system, but technology limitations increase implementation difficulty with scale.	
Ex-situ Treatment (Physical, Chemical or Biological)	Retained	Ex-situ treatment technologies are retained as a way of removing contaminants from extracted groundwater from a hydraulic containment system. Ex-situ treatment may be paired with wastewater treatment, non-groundwater release treatment systems, or with permitted discharge to manage groundwater contamination. The CSM and data gaps investigations will guide the design of any ex-situ treatment	
Closure in Place (CiP) (of the regulated unit)	Retained	The use of CiP as a source control technology and is amenable with respect to CAO attainment.	
Closure by Removal (CbR) (of the regulated unit)	Retained	The use of CbR as a source control technology is amenable with respect to CAO attainment.	

Potentially Applicable Technology	Status	Description/Overview
Other Source Control Technologies	Retained	Control of source area non-groundwater related releases. For the purposes of this groundwater ACM, management of non-groundwater releases are not included in the alternatives evaluation. Engineering measures, including leachate collection, lining of trenches and/or ponds, and other isolation methods are regarded as part of closure technologies selected by other means.

Note:

Technologies that were retained may be used as components of a corrective action alternative, but when evaluated in conjunction with other available technologies any single technology may not be utilized.

Preliminary assembly of corrective measures alternatives was performed based on site-specific and regional geology and groundwater conditions. For the Unit, six corrective measures alternatives were developed from this list of applicable corrective measures technologies during the ACM screening process:

- Alternative #1 No Action and Groundwater Monitoring
- Alternative #2a Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring
- Alternative #2b Closure by Removal (CbR), ICs, and Groundwater Monitoring
- Alternative #3 CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #4 CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #5 CiP, Other Source Control, ICs, and Groundwater Monitoring

The assembly of corrective measures alternatives presented in the ACM was considered preliminary and subject to revision following additional evaluation during the remedy selection process and/or following comment from the regulatory community and public. Further evaluation of the alternatives is discussed in the following sections.

#### 2.5.2 Assessment of Corrective Measures for Non-Groundwater Impacts

Pursuant to Title 40 of the Code of Federal Regulations (CFR) parts 257.90(d) and 257.84(b)(5), BREC initiated design of containment systems intended to reduce and prevent non-groundwater releases from reaching the Green River as an interim corrective measure. Plans for these measures were submitted to the KDWM for review and comment in 2019. KDWM conditionally approved the interim corrective measures for implementation at the Unit and they were constructed in 2019 and 2020 (see Section 2.4).

In June 2019, BREC performed an ACM to evaluate whether additional remedial measures, that would be supplemental to the ICMs already planned, were warranted to address non-groundwater releases. Several potential corrective measures technologies were evaluated in order to identify which ones could be carried forward as components of corrective measure alternatives for non-groundwater releases, if required. The results of the corrective measures technology evaluation are presented below in **Table 6**.

Table 6 – Potential Corrective Measures Options for Non-Groundwater Impacts

Potentially Applicable Technology	Status	Description/Overview
No Action	Not retained as stand-alone technology, but carried forward for baseline comparisons	This technology has been included in the preliminary evaluation/screening but is not retained because it will not meet the established CAOs.

Potentially Applicable Technology	Status	Description/Overview		
Hydraulic Containment	Retained	Hydraulic containment in the form of pumping of vertical or horizontal wells would potentially be used to provide spot control of seepage if the interim corrective measures are unable to fully capture the seepage.		
Physical Containment	Retained	Physical containment in the form of a cutoff wall would potentially be used to re-direct or otherwise intercept seepage that was not adequately captured by the interim corrective measures.		
Ex-situ Physical/Chemical/Biological Treatment	Retained	Ex-situ treatment is retained as a potential supplement to the interim corrective measures in the event that discharge via the station's KPDES permit is not possible.		
In-situ Physical/Chemical Treatment	Retained	In-situ treatment is retained in the form of spot treatment or fixation of seepage areas in the event that the interim corrective measures do not adequately address all seepage areas.		
Permeable Reactive Barriers (PRB)	Retained	The use of PRBs is retained in the form of a reactive cell in the event that interim measures result in seepage concentrations that require pre-treatment insitu prior to discharge.		
Closure in Place (CiP) (of the regulated unit)	Retained	The use of CiP as a source control technology and is amenable with respect to CAO attainment.		
Closure by Removal (CbR) (of the regulated unit)	Retained	The use of CbR as a source control technology is amenable with respect to CAO attainment.		
Other Source Control Technologies	Retained	Control of source area non-groundwater releases is being implemented as interim corrective measures but is retained in the event that interim measures need to be evaluated for expansion.		

The ICMs implemented at the Unit in 2019 were designed to address river seepage and divert it to KPDES outfalls, eliminating any potential exposure to public health or the environment. During ACM development, it was anticipated that the ICMs would meet the CAOs by effectively eliminating any future river seepage through source control, and as a result, no supplemental remedies were considered warranted. Data collected at the Unit since installation of the ICMs suggests that the CAOs are being met and in compliance with the conditions of the Agreed Order.

Performance monitoring is ongoing and will continue to be performed in the future to demonstrate source control and evaluate the ability of the ICMs to meet the CAO. The ICMs implemented at the Unit in 2019 and 2020 are considered the final remedy for non-groundwater releases and are expected to benefit corrective action as a whole for the Unit. As a result, no separate remedy selection report is currently being developed for non-groundwater releases. If warranted based on performance monitoring results, additional evaluation of the non-groundwater corrective measures will be performed consistent with 40 CFR 257.98(b).

#### 3. Corrective Measure Evaluation

To address the remedy selection requirement under 40 CFR Part 257.97, a corrective measure evaluation was performed to address groundwater impacts at the Unit. Currently, no separate corrective measure evaluation is planned for non-groundwater releases, as the ICMs implemented at the Unit in 2019 and 2020 are considered the final remedy for non-groundwater releases. The discussion included below details the evaluation performed to address groundwater impacts at the Unit.

#### 3.1 Corrective Action Objectives

Corrective Action Objectives (CAOs) for the Unit were identified during the groundwater ACM completed for the Unit in June 2019. CAOs are overall descriptions of what remedial action is expected to accomplish at a given site. CAOs also provide a basis for evaluating the performance of a corrective measure. Title 40 CFR Section 257.97 (b) outlines the CAOs for corrective measures under the CCR Rule as follows:

- (1) Be protective of human health and the environment;
- (2) Attain the GWPS as specified pursuant to Section 257.95(h);
- (3) Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into the environment;
- (4) Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors such as avoiding inappropriate disturbance of sensitive ecosystems;
- (5) Comply with standards for management of wastes as specified in Section 257.98(d). [note: this statute refences all applicable requirements under the Resource Conservation and Recovery Act (RCRA)].

The corrective measure alternative selected for the Unit must ultimately demonstrate attainment of the CAOs. Compliance with the CAOs will be a primary factor in determining the effectiveness of the corrective measure alternative selected for the Unit during future systematic performance reviews.

Each of the CAOs have been adopted as Threshold Criteria (see Section 3.3.1 below) for evaluating potential corrective measures in alignment with 40 CFR Part 257.97 (b).

#### 3.2 Corrective Measures Alternatives Assembly

The groundwater ACM performed for the Unit in June 2019 identified a total of six (6) corrective measures alternatives to be carried forward into the remedy selection process. In December 2019, BREC provided a *Semi-annual Remedy Selection Progress Report* (AECOM, December 2019) as required under 40 CFR 257.97(a). As part of this submittal, two (2) corrective measures alternatives were eliminated from further consideration, including:

- Alternative #1 (No Action and Groundwater Monitoring) This alternative does not control or remove COCs from the environment and therefore does not achieve the RAOs.
- Alternative #2b (CbR, ICs, and Groundwater Monitoring) Implementing a CbR approach is considered cost prohibitive. In addition, any CbR approach would require relocating waste to an existing disposal unit or construction of a new waste disposal unit, which does not align with the one of the fundamental goals of RCRA (conserving energy and natural resources).

Four (4) potential corrective measures alternatives have been identified by BREC as viable options to address lithium impacts in groundwater and non-groundwater releases at the Unit, including:

Alternative #2a (Alt 2a): CiP, ICs, and Groundwater Monitoring

- Alternative #3 (Alt 3): CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #4 (Alt 4): CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #5 (Alt 5): CiP, Other Source Control, ICs, and Groundwater Monitoring

Each of the remining 4 corrective measures alternatives was evaluated against the threshold, balancing, and modifying criteria as discussed below.

#### 3.3 Corrective Measures Criteria Evaluation

40 CFR Part 257.97(a) outlines the criteria for evaluating corrective measures under the Federal CCR Rule. Although not specifically stated as such, these criteria mirror the criteria outlined for the National Oil and Hazardous Substance Contingency Plan, more commonly referred to as the National Contingency Plan (NCP), established under 40 CFR 300. 40 CFR 300.430 identifies 9 criteria for evaluating remedial alternatives which are further divided into 3 categories:

- 1) Threshold Criteria;
- 2) Balancing Criteria, and
- 3) Modifying Criteria.

These criteria were utilized by BREC to evaluate the potential corrective measures alternatives for the Unit. Each of the remaining 4 corrective measures alternatives was evaluated against each other and scored on a scale from 1 to 4 (1 being lowest and 4 being highest). Where multiple corrective measures alternatives were considered equal with respect to a given criteria, the available points were combined and divided equally. The results of analysis performed to evaluate each of the corrective measures alternative is discussed below and summarized in **Appendix E**.

#### 3.3.1 Threshold Criteria Evaluation

Title 40 CFR Part 257.97 (b) outlines the threshold criteria (also viewed as CAOs) for evaluating corrective measures under the CCR Rule, and these criteria were presented in Section 3.1 above. The results of the threshold criteria evaluation are summarized below in **Table 7**.

40 CFR 257.97 Reference	Alternative 2a	Alternative 3	Alternative 4	Alternative 5
(b)(1)	1	3	3	3
(b)(2)	1	3.5	2	3.5
(b)(3)	1	3	2	4
(b)(4)	1	3	2	4
(b)(5)	2.5	2.5	2.5	2.5

**Table 7. Threshold Criteria Evaluation Summary** 

Further detail regarding how threshold criteria were evaluated in provided on Table E-2 in Appendix E.

#### 3.3.2 Balancing Criteria Evaluation

Title 40 CFR Section 257.97 (c) outlines the balancing criteria for evaluating corrective measures under the CCR Rule as follows:

1) The long and short-term effectiveness of the potential remedy(s), along with the degree of certainty that the remedy will prove successful based on a consideration of the following:

- i. Magnitude of reduction of existing risks;
- ii. Magnitude of residual risks in terms of likelihood of further releases due to CCR remaining following implementation of a remedy;
- iii. The type and degree of long-term management required, including monitoring, operation, and maintenance;
- iv. Short-term risks that might be posed to the community or the environment during implementation of such a remedy, including potential threats to human health and the environment associated with excavation, transportation, and re-disposal of contaminant;
- v. Time until full protection is achieved;
- vi. Potential for exposure of humans and environmental receptors to remaining wastes, considering the potential threat to human health and the environment associated with excavation, transportation, re-disposal, or containment;
- vii. Long-term reliability of the engineering and institutional controls; and
- viii Potential need for replacement of the remedy
- 2) The effectiveness of the remedy in controlling the source to reduce further releases based on consideration of the following factors:
  - i. The extent to which containment practices will reduce further releases; and
  - ii. The extent to which treatment technologies may be used.
- 3) The ease or difficulty of implementing a potential remedy(s) based on consideration of the following types of factors:
  - i. Degree of difficulty associated with constructing the technology;
  - ii. Expected operational reliability of the technologies;
  - iii. Need to coordinate with and obtain necessary approvals and permits from other agencies;
  - iv. Availability of necessary equipment and specialists; and
  - v. Available capacity and location of needed treatment, storage, and disposal services.

The results of the threshold criteria evaluation are summarized below in Table 8.

**Table 8. Balancing Criteria Evaluation Summary** 

40 CFR 257.97 Reference	Alternative 2a	Alternative 3	Alternative 4	Alternative 5
(c)(1)(i)	1	4	3	2
(c)(1)(ii)	1	3.5	3.5	2
(c)(1)(iii)	1	2.5	2.5	4
(c)(1)(iv)	1	3	2	4
(c)(1)(v)	1	3	2	4
(c)(1)(vi)	1	3	2	4
(c)(1)(vii)	1	3	2	4
(c)(1)(viii)	4	2	1	3
(c)(2)(i)	1	3	2	4
(c)(2)(ii)	1	4	3	2
(c)(3)(i)	4	2	1	3
(c)(3)(ii)	4	2	1	3
(c)(3)(iii)	2.5	2.5	2.5	2.5
(c)(3)(iv)	4	2	1	3

(c)(3)(v)	1	2	3	4
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Further detail regarding how threshold criteria were evaluated in provided on Table E-3 in Appendix E.

#### 3.3.3 Modifying Criteria Evaluation

Title 40 CFR Section 257.97 (c) defines modifying criteria as "the degree to which community concerns are addressed by a potential remedy(s)". Given that an Agreed Order was signed between BREC and the KDWM for the Unit, the modifying criteria were expanded as part of this evaluation to include separate criteria for state and community acceptance (40 CFR 300.430 divides modifying criteria into two categories).

The results of the modifying criteria evaluation are summarized below in Table 9.

**Table 9. Modifying Criteria Evaluation Summary** 

40 CFR 257.97 Reference	Alternative 2a	Alternative 3	Alternative 4	Alternative 5
NA - state acceptance	1	3.5	3.5	2
(c)(4)	1	3.5	3.5	2

Further detail regarding how threshold criteria were evaluated in provided on Table E-4 in Appendix E.

#### 3.3.4 Corrective Measures Alternative Evaluation Summary

The cumulative scoring of the criteria evaluation is summarized below in **Table 10**.

**Table 10. Cumulative Criteria Evaluation Scoring Summary** 

40 CFR 257.97	Alternative	Alternative	Alternative	Alternative
Reference	2a	3	4	5
Total Score	37	63.5	50	69.5

Further detail regarding the cumulative scoring criteria is provided on Table E-1 in **Appendix E**. Alternative 5 scored highest of all the alternatives during the evaluation.

#### 4. Remedy Selection

In alignment with the scoring completed as part of the corrective measure evaluation (see **Appendix E**), BREC has selected Alternative #5 (CiP, Other Source Control, ICs, and Groundwater Monitoring) as the remedy to address groundwater and non-groundwater impacts at the Unit. A description of each corrective measure technology incorporated into the selected remedy is provided below.

#### 4.1 Closure in Place

In adherence with the BREC's permit conditions, the Site will continue to operate as a solid waste disposal facility through its life cycle and will be closed in accordance with the requirements of the permit. The current life cycle estimates for the Green Landfill predict that the Unit will reach capacity in approximately 2041. Source control through landfill closure will include installation of final cover that will prevent infiltration and contribute to groundwater quality restoration.

#### 4.2 Source Control

To comply with the Agreed Order signed by BREC and KDWM for the Unit, additional source control measures will be implemented in 2020 and 2021 to reduce/eliminate the downward migration of COC into groundwater. As currently planned, theses measure will include the following:

- Landfill perimeter collection trenches; and
- Removal of CCR material from the South Sediment Basin.

Interim corrective measures for the perimeter seeps are being planned in a phased approach. The first step is to divert the seepage to the Northeast Sediment Basin which is routed to the KPDES outfall of the Green Surface Impoundment. Removing the seeps from stormwater channels will prevent mixing with impounded stormwater. The use of the South Sediment Basin requires that CCR materials be removed so that the seepage does not have the potential to impact groundwater. Corrective measures for the South Sediment Basin will involve the removal of any residual CCR material and creation of two lined sump areas, one on the east end to collect the South and East perimeter seeps and one on the west end to collect Southwest corner perimeter seeps. Additionally, perimeter seeps on the north side of the landfill will be similarly controlled but will be directly routed to the collector sump on the north side of the landfill.

Design of the additional source control remedies is currently being performed by BREC. A draft design package will be provided to KDWM as part of a separate submittal to comply with the conditions of Agreed Order #18-3-0138. The implementation schedule for source control measures is discussed in Section 5.

#### 4.3 Institutional Controls

The use of ICs (i.e., Environmental Covenant, groundwater use restrictions, etc.) is retained as a useful technology. However, it is noted the ICs are not anticipated to be used as a stand-alone technology. Environmental Covenants, groundwater use restrictions, etc., are expected to be combined with other applicable technologies as part of the remedy for the Unit.

#### 4.4 Groundwater Monitoring

Assessment monitoring is expected to continue at the Unit until the CAOs have been met.

#### 5. Remedy Implementation Schedule

#### 5.1 Schedule Evaluation Factors

The schedule for remedy implementation is provided in **Appendix F**. 40 CFR Part 257.97(d) outlines the factors that must be considered in specifying a schedule to remedial implementation at a CCR unit as follows.

- 1) Extent and nature of contamination, as determined by the characterization required under § 257.95(g);
- 2) Reasonable probabilities of remedial technologies in achieving compliance with the groundwater protection standards established under § 257.95(h) and other objectives of the remedy;
- 3) Availability of treatment or disposal capacity for CCR managed during implementation of the remedy;
- 4) Potential risks to human health and the environment from exposure to contamination prior to completion of the remedy;
- 5) Resource value of the aquifer including:
  - i. Current and future uses;
  - ii. Proximity and withdraw rate of users;
  - iii. Groundwater quantity and quality;
  - iv. The potential damage to wildlife, crops, vegetation, and physical structures caused by exposure to CCR constituents;
  - v. The hydrogeologic characteristic of the facility and surrounding land;
  - vi. The availability of alternative water supplies; and
- 6) Other relevant factors.

Each of these factors was consider by BREC as part of the remedy selection process as described below.

#### **5.1.1** Nature and Extent of Contamination

The data obtained during characterization monitoring performed at the Unit under 40 CFR Part 257.95(g) indicates that the extent of groundwater and non-groundwater impacts is confined to Sebree Station. Source control measures implemented to date will ensure that non-groundwater releases are captured and will not migrate beyond the functional perimeter of the Unit and the property controlled by BREC.

Assessment monitoring will continue at the Unit to confirm that the nature and extent of contamination is defined and progressing in accordance with the CAOs.

#### 5.1.2 Compliance Probability

Implementation of the selected remedy is expected to have a high probability of meeting the CAOs. There is firm evidence of a relatively direct connection between infiltration of co-mingled leachate and stormwater at the South Sediment Basin and the observed impact to monitoring wells MW-4, -5, and -6. Consequently, removal of that infiltration by the planned corrective measures (excavating CCR from the South Sediment Basin and containing leachate in a series of sumps and piped conveyance) is expected to have a direct influence on groundwater quality. The time required to achieve GWPSs at the affected wells has not been modeled but is expected to be on the order of one to five years if the remedy is implemented as planned.

Impacts observed at MW-3A may be tied to the nearby non-groundwater release captured by the Deep Seep Collection Trench, in which case, the time to achieve CAOs may be relatively quick now that the

seepage is being hydraulically controlled. However, there are unknowns regarding the nature of how lithium is transported to that well location. Those uncertainties cannot be evaluated given the physical constraints of the site (proximity of the landfill to the river), so the time frame required to meet CAOs cannot be predicted until additional Assessment monitoring data are available.

#### 5.1.3 CCR Treatment and Disposal Capacity

Wastes generated by the groundwater corrective measures activities will include residual CCR content removed from the South Sediment Basin and seepage collected from the perimeter seepage controls. Wastes generated by the non-groundwater corrective measures activities will be seepage collected from the Deep Seep Collection Trench and the Northwest Seep Collection Trench.

The solids (dredged material from the South Sediment Basin) will be interred in the Landfill as allowed under the existing solid waste permit. The Landfill has sufficient capacity for this one-time waste stream volume. The liquid wastes will be managed under the KPDES permit for the station.

#### 5.1.4 Exposure Risk

As detailed in Section 2.3.8, there is no data to suggest that human health and the environment are currently being exposed to COC emanating from the Unit. This condition is not expected to change prior to implementation of the remedy but will continue to be evaluated through Assessment monitoring and systematic performance reviews.

#### 5.1.5 Aguifer Resource Value

Based on data published by KGS, there are no known groundwater wells used for drinking water within a 1-mile radius of the Unit. This is not expected to change in the future but will be re-examined during future performance reviews. Therefore, the significance of aquifer resource value is not considered pertinent to this evaluation or the resulting schedule.

#### **5.1.6** Other Relevant Factors

Within Exhibit 4 of the Agreed Order, a milestone schedule was provided for groundwater corrective action. Although the milestone schedule has been adjusted due to the work conditions imposed by the COVID-19 pandemic, which includes holding the public meeting at an earlier date, BREC has moved forward with the activities required in the Agreed Order.

#### 5.2 Performance Review

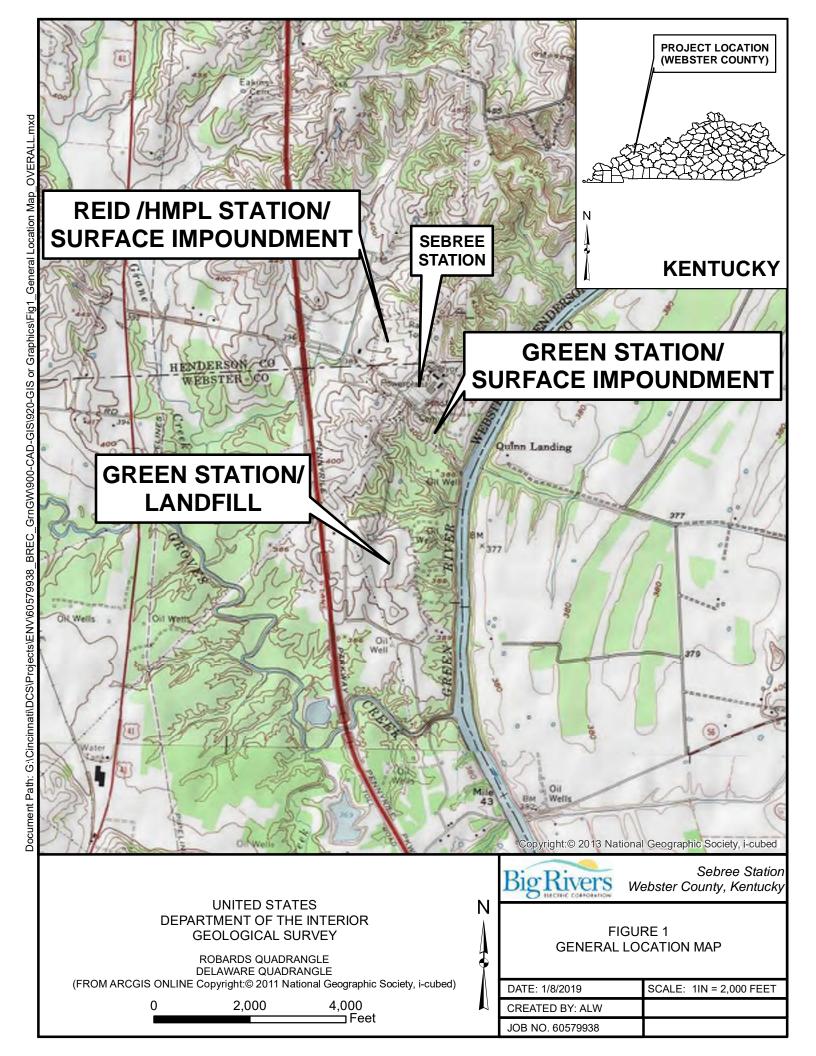
Source control measures are viewed as the remedial component likely to have the most significant shortand long-term benefit on reducing groundwater and non-groundwater impacts at the Unit. As such, evaluating the performance of source control measures constructed at the Unit should be evaluated through systematic review.

Although not specifically mandated under the CCR Rule, five-year reviews are generally required by the regulatory agency under corrective action programs (i.e. CERCLA) when hazardous substances remain at a site above levels that permit unlimited use and unrestricted exposure. Five-year reviews provide an opportunity to evaluate the implementation and performance of a remedy to determine whether it remains protective of human health and the environment. Generally, reviews take place five years following the start of corrective action and are repeated every succeeding five years so long as future uses remain restricted. BREC will perform a five-year review to evaluate compliance with the CAOs and evaluate the effectiveness of the remedy selected for the Unit five years after construction completion (approximately 2026).

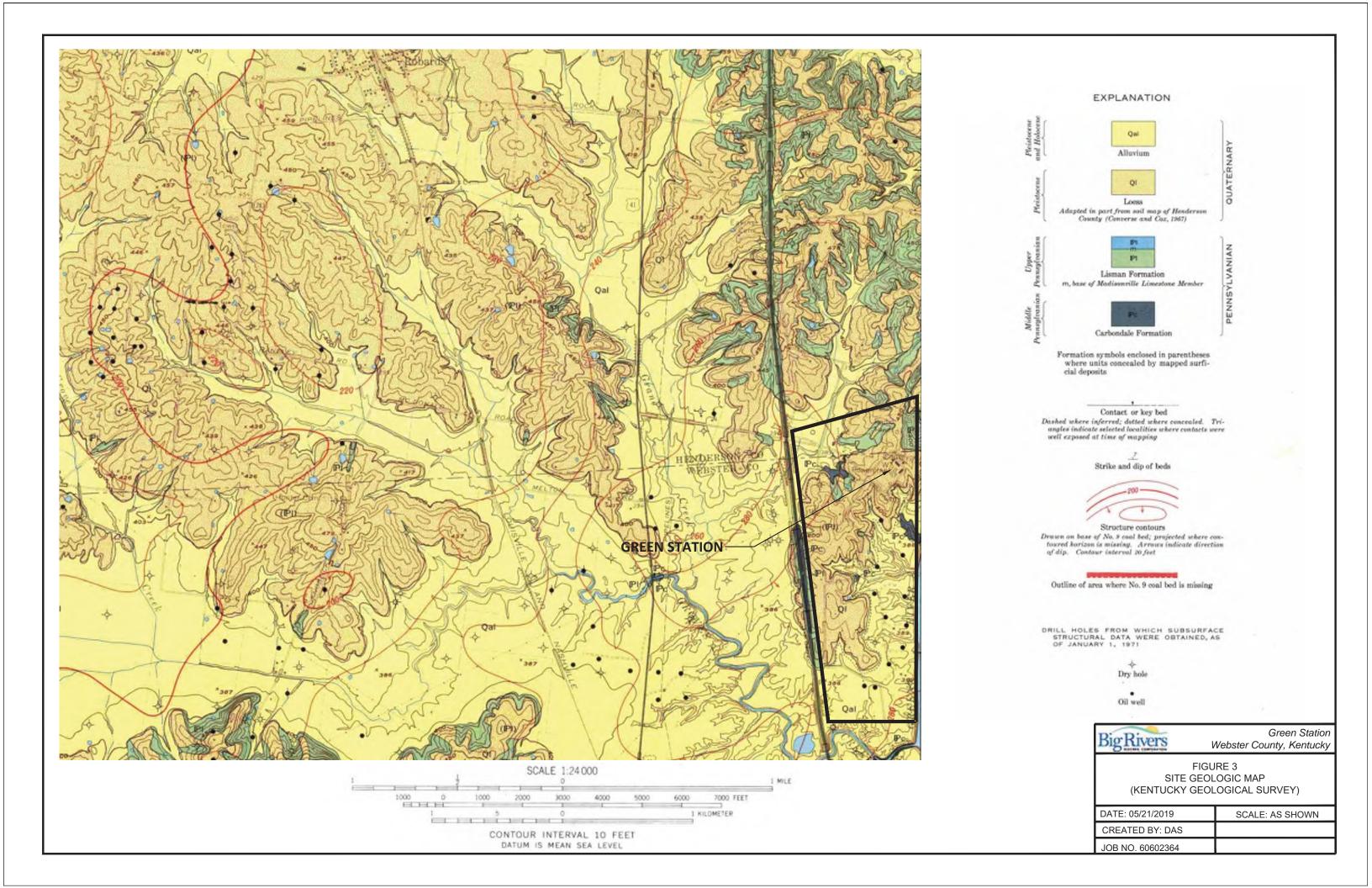
#### 6. References

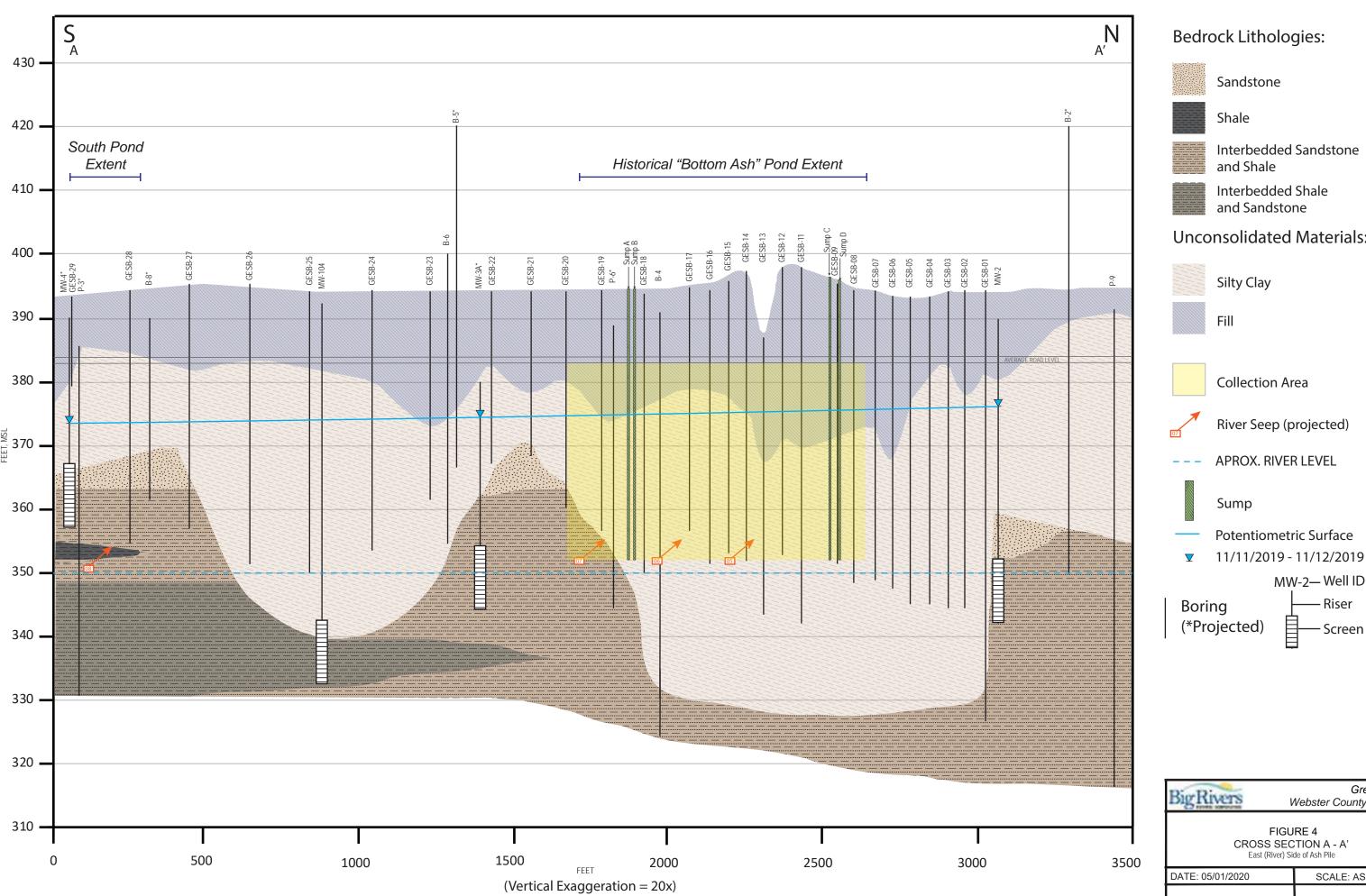
- AECOM, 2018. Annual Groundwater Monitoring and Corrective Action Report, 2016-2017; Green Station CCR Landfill, Webster County, Kentucky.
- AECOM, 2019. Annual Groundwater Monitoring and Corrective Action Report, 2018; Green Station CCR Landfill, Webster County, Kentucky.
- AECOM, 2019. Assessment of Corrective Measures Under the CCR Rule; Green Station CCR Landfill, Green Station, Webster County, Kentucky.
- AECOM, 2019. Assessment of Corrective Measures, Non-Groundwater Releases Under the CCR Rule; Green Station CCR Landfill, Green Station, Webster County, Kentucky.
- AECOM, 2020. 2019 Annual Groundwater Monitoring and Corrective Action Report, Sebree Generating Station, Henderson and Webster Counties Kentucky.
- Associated Engineers 2016. Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan.
- Fairer, G.M., Geologic Map of the Robards Quadrangle, Henderson and Webster Counties, Kentucky, U.S. Geological Survey, 1973.
- USEPA, 40 CFR Part 257. [EPA-HQ-RCRA-2015-0331; FRL-9928-44-OSWER]. RIN-2050-AE81. Technical Amendments to the Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals From Electric Utilities—Correction of the Effective Date. Federal Register / Vol. 80, No. 127 / Thursday, July 2, 2015 / Rules and Regulations.
- USEPA, 40 CFR Part 257. [EPA-HQ-OLEM-2017-0286; FRL-9973-31-OLEM]. RIN-2050-AG88. Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals From Electric Utilities; Amendments to the National Minimum Criteria (Phase One); Proposed Rule. Federal Register / Vol. 83, No. 51 / Thursday, March 15, 2018 / Proposed Rules.

## **Figures**





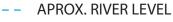




Interbedded Sandstone



### **Unconsolidated Materials:**



Potentiometric Surface

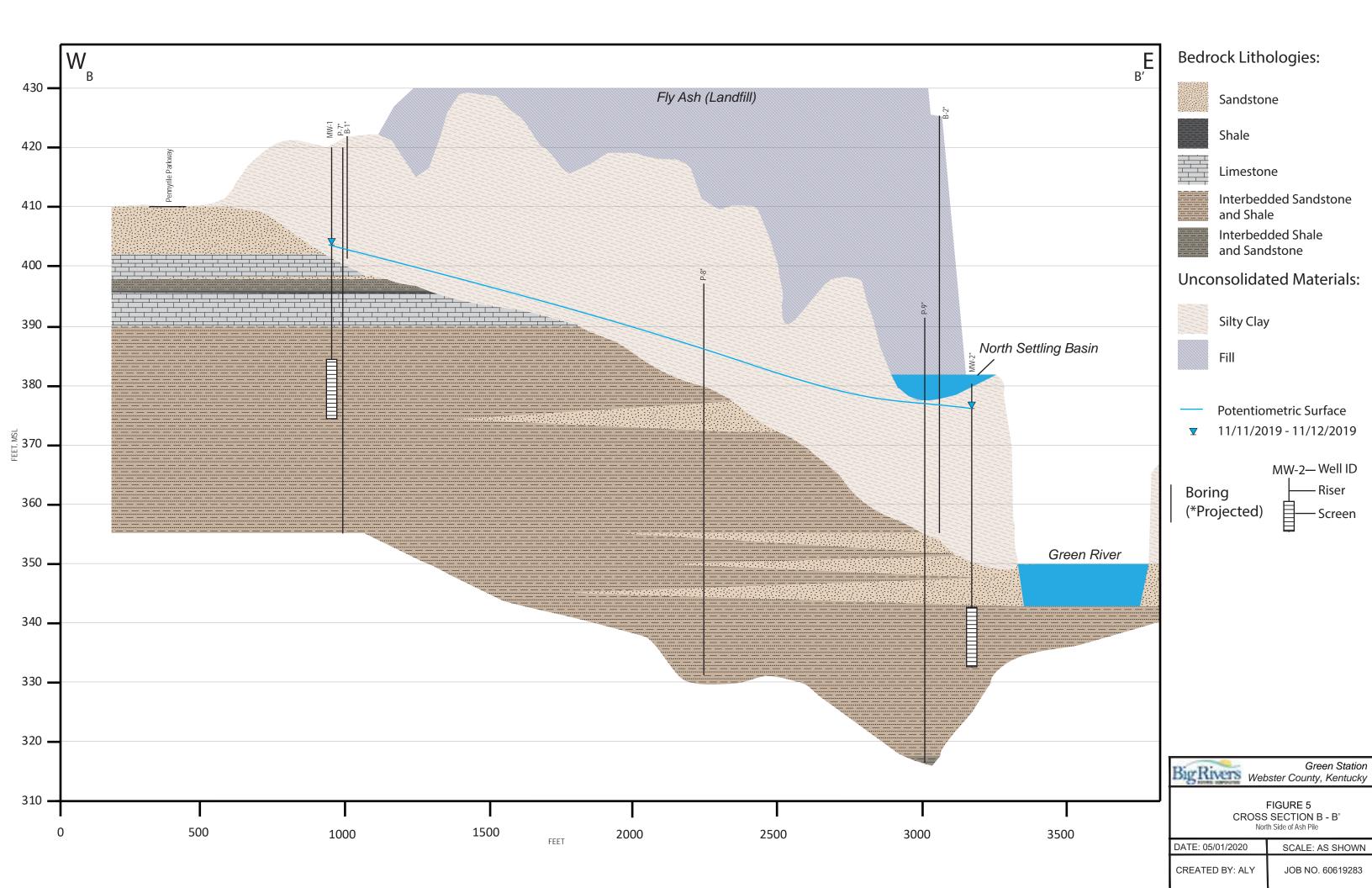
11/11/2019 - 11/12/2019

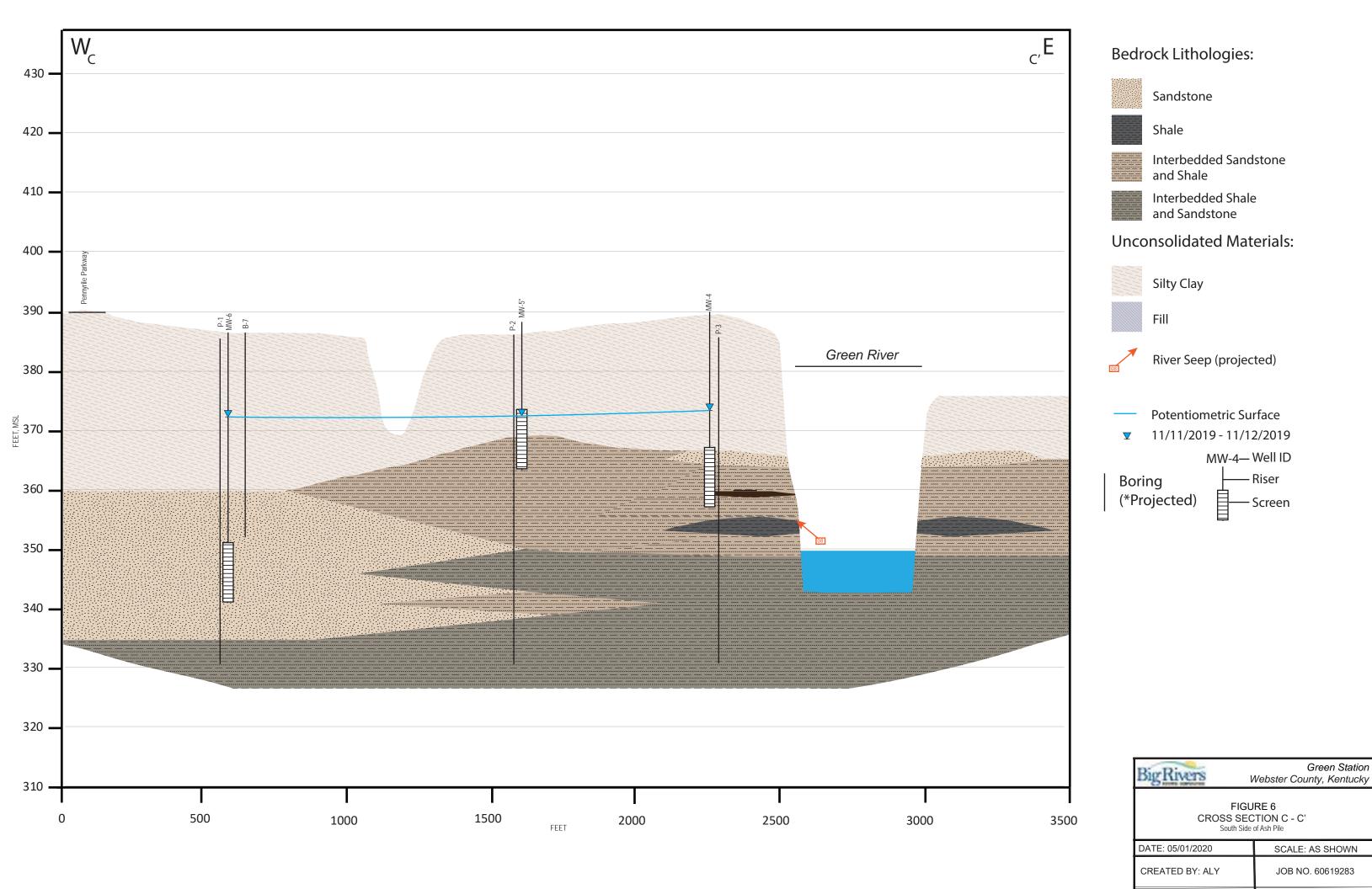
- Riser - Screen

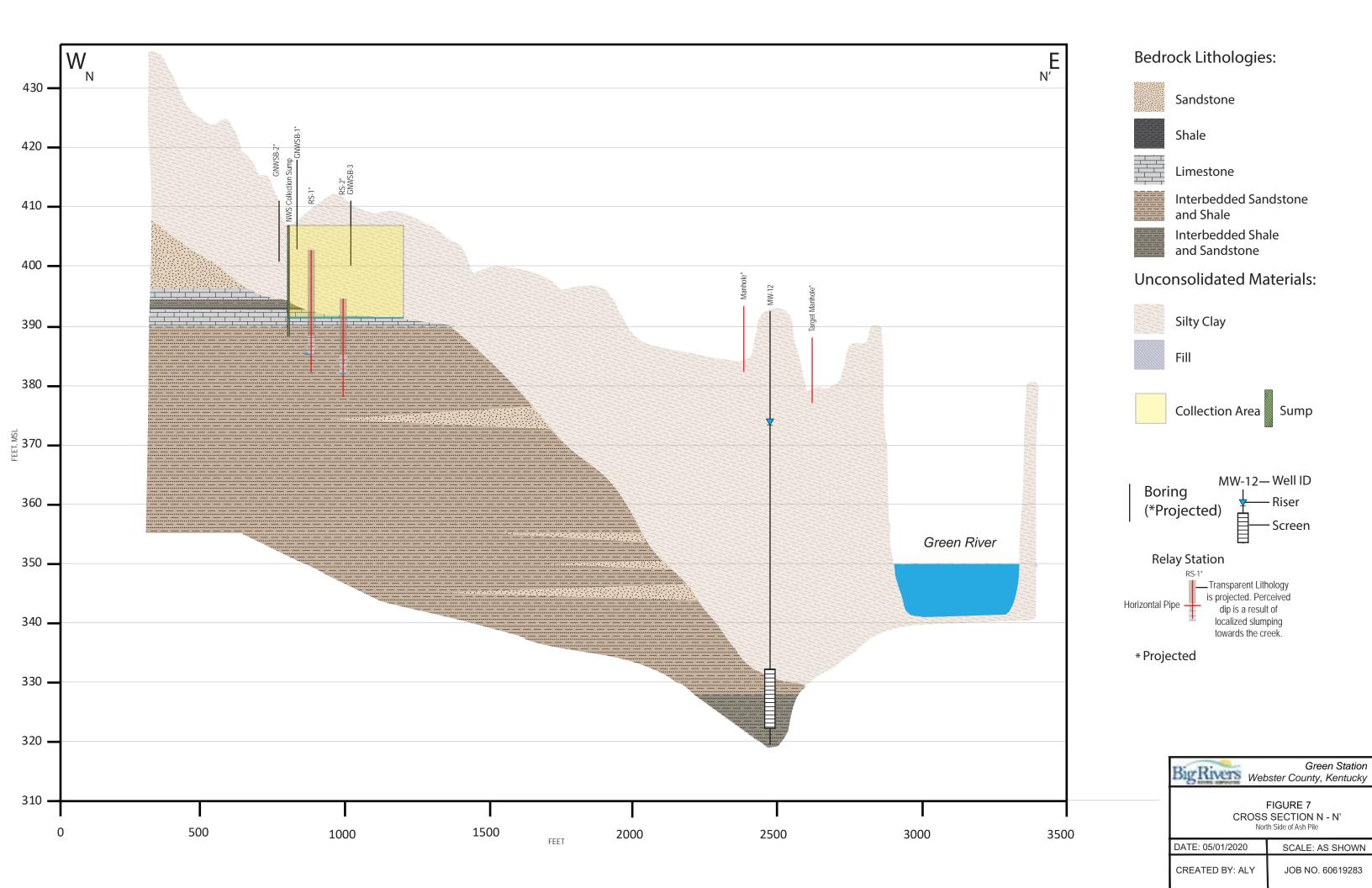
> Green Station Webster County, Kentucky

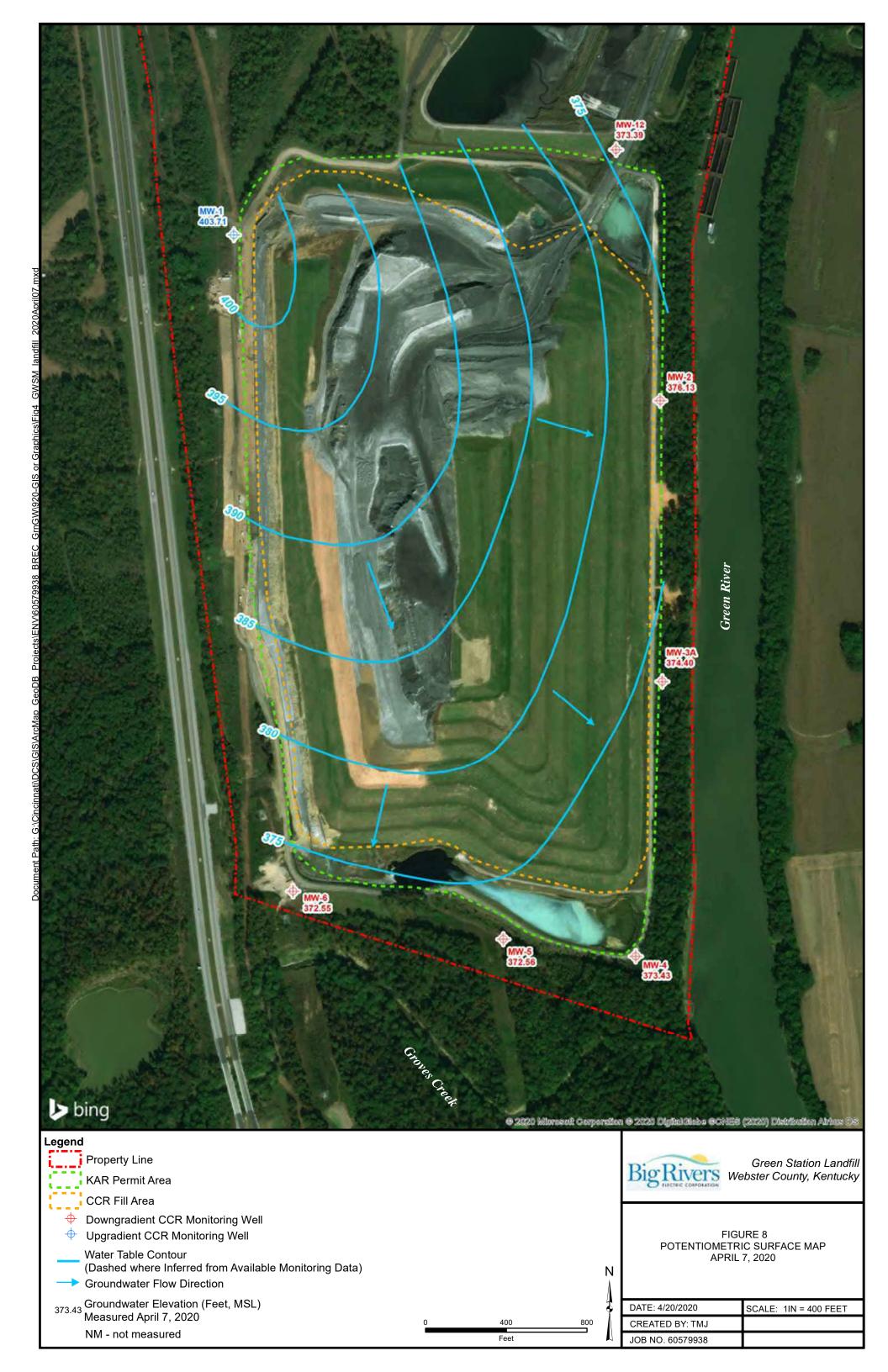
FIGURE 4 CROSS SECTION A - A' East (River) Side of Ash Pile

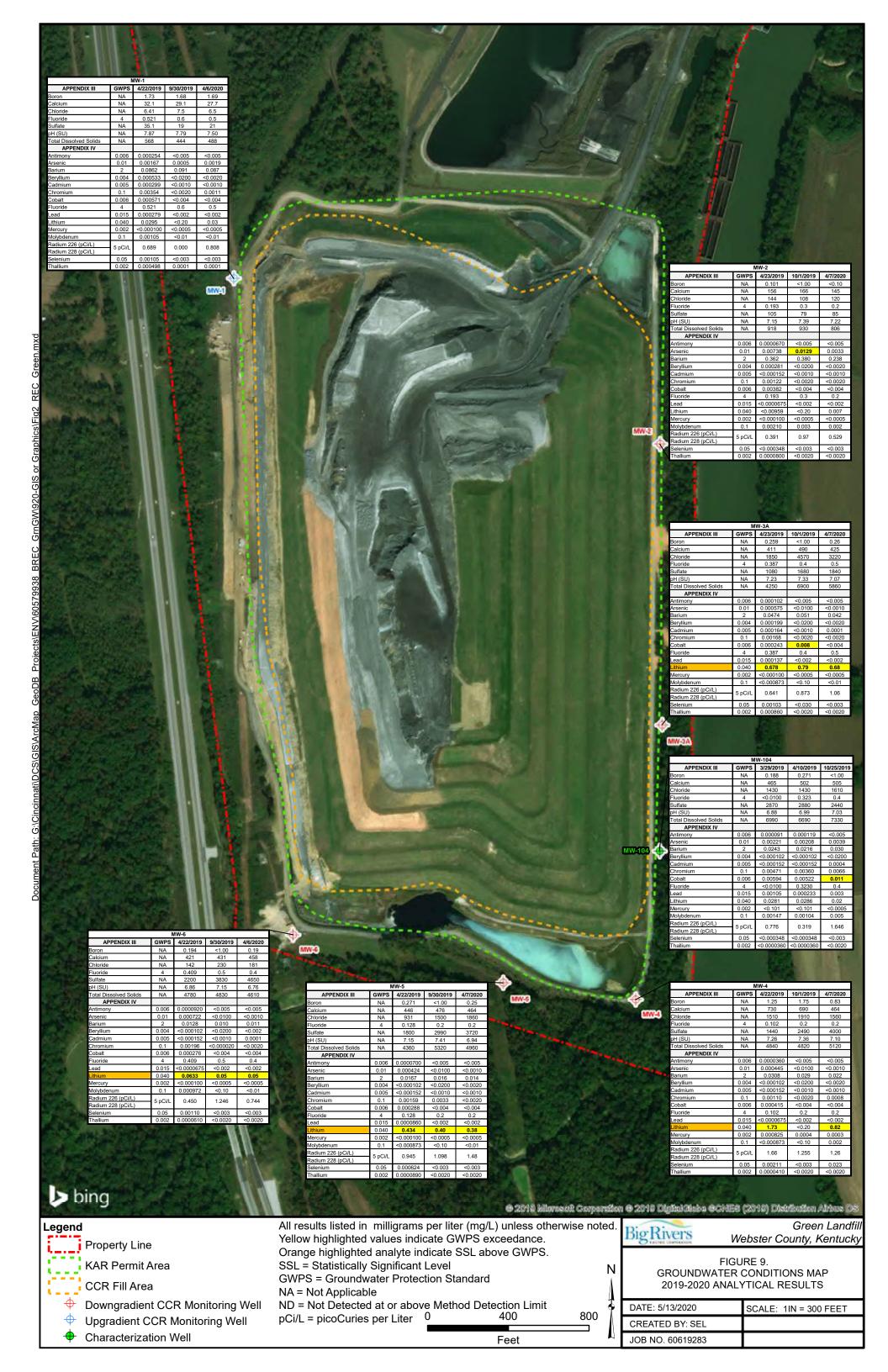
SCALE: AS SHOWN CREATED BY: ALY JOB NO. 60619283













# **Appendix A July 2018 River and Seep Sampling and Analysis Data**

#### TABLE 1

### CCR ANALYTICAL SUMMARY RIVER SEEP AND RIVER SAMPLE EVALUATION

JULY 2018

BIG RIVERS ELECTRIC CORPORATION GREEN STATION LANDFILL WEBSTER COUNTY, KENTUCKY

			Water Qu	ality Criteria	(mg/L)	River Seep-14-	River Seep-12-	RiverSeep-16-	River 01A	River 01B	RiverSeep-08-	RiverSeep-07-	River 02A	River 02B	RiverSeep-05-	River 03A	River 03B	River 04A	River 04B	River-Seep-04-
	PRIMARY MCL	Human H	lealth	Warm W	later Aquatic Habitat	71318	71318	71318	71218	71218	71318	71218	71218	71218	71218	71218	71218	71218	71218	71218
Field Parameters	and CCR LIMITS	Domestic Water Supply Source	Fish	Acute	Chronic	Lat 37.661126 Long -87.4894	Lat 37.61732 Long -87.4936	Lat 37.62167 Long -87.4967	Lat 37.64610 Long -87.5059	Lat 37.64610 Long -87.5059	Lat 37.62860 Long -87.5003	Lat 37.63299 Long -87.5003	Lat 37.63303 Long -87.5002	Lat 37.63303 Long -87.5002	Lat 37.63433 Long -87.5003	Lat 37.63433 Long -87.5002	Lat 37.63433 Long -87.5002	Lat 37.63789 Long -87.5004	Lat 37.63789 Long -87.5004	Lat 37.64122 Long -87.4997
pH (Field Measurement) SU	NA					7.54	7.37	7.46	7.94	7.94	7.09	7.27	7.91	7.91	6.92	7.94	7.94	7.86	7.86	5.13
pH (Lab Measurement) SU	NA					8.14	8.00	8.40	7.64	7.62	8.16	8.01	7.45	7.50	7.95	7.50	7.51	7.52	7.53	5.26
Conductivity (µmhos/cm)	NA					1207	226.2	654	268	268	7674	7715	267.7	267.7	6174	262.2	262.2	265.1	265.1	2545
Temperature (°F)	NA					88.34	84.0	91.58	82.9	82.9	70.52	79.7	84.2	84.2	94.28	84.2	84.2	82.6	82.6	71.6
Oxidation-Reduction Potential	(m NA					-92	-98	-48	131	131	29	-123	98	98	-137	133	133	133	133	125
APPENDIX III CONSTITUENTS	s				•															
Boron	NA					0.0694	J 0.0379 J	0.0321	J 0.0281	J 0.0252 J	0.510 J	1.46	0.0323	J 0.0322 J	0.853 J	0.0251 J	0.0235	J 0.0229	J 0.0234 J	2.19
Calcium	NA					171	21.1	93.8	31.8	33.2	801	1120	32.8	35.8	916	34.8	32.6	32.9	34.5	460
Chloride	NA	250	-	1200	600	22.7	32.7	23.2	4.58	B 4.52 B	2040	1990	6.75 E	6.69 B	1670	5.33 B	5.59	B 4.83	B 4.75 B	189
Fluoride	4 mg/L	4	-	-	-	0.144	J 0.0803 J	0.177	J 0.111	J 0.105 J	0.0915 J	0.102 J	0.0958	J 0.0979 J	0.0795 J	0.100 J	0.0954	J 0.0948	J 0.0945 J	0.239 J F
Sulfate	NA	250	-	-	-	159	B 16.1 B	26.5 E	3 28.5	28.3	1440 B	1480 B	30.6	30.1	1170 B	28.8	28.9	28.6	28.6	1310 B
Total Dissolved Solids	NA	250	-	-	-	790	157	504	169	161	5310	6080	173	170	5140	175	170	174	156	2130
APPENDIX IV CONSTITUENTS	S		•																	
Antimony	0.006 mg/L	0.0056	0.64	-	-	0.000312	J 0.000499 J	0.000270	J 0.000591	JB 0.000476 JB	0.00141 J	ND	0.00276 E	3 0.00106 JB	0.000366 J	0.000571 JB	0.000514	JB 0.000504	JB 0.000360 JB	0.000200 J
Arsenic	0.01 mg/L	0.01	-	0.340	0.150	0.0173	0.00467 J	0.0247	0.00124	J 0.00137 J	0.000404 J	0.00182 J	0.00131	J 0.00135 J	0.0192	0.00126 J	0.00131	J 0.00118	J 0.00109 J	0.00188 J
Barium	2 mg/L	1	-	-	-	0.242	0.0757 J	0.190	J 0.0330	J 0.0374 J	0.0443 J	0.0605 J	0.0350	J 0.0396 J	0.718	0.0366 J	0.0362	J 0.0382	J 0.0402 J	0.0384 J
Beryllium	0.004 mg/L	0.004	-	-	-	0.000497	J 0.000145 J	0.000211	J ND	ND	ND	ND	ND	ND	0.000545 J	ND	ND	ND	ND	0.00372
Cadmium	0.005 mg/L	0.005	-	0.00235	0.00029	0.000312	J 0.000183 J	0.000196	J ND	ND	ND	ND	ND	ND	0.000563 J	ND	ND	ND	ND	0.00307
Chromium	0.1 mg/L	0.1	-	-	-	0.00969	0.00200 J	0.00383	0.000676	J 0.00143 J	0.000560 J	0.000340 J	0.00111	J 0.00155 J	0.0124	0.00112 J	0.00119	J 0.00134	J 0.00105 J	0.00386
Cobalt	0.006 mg/L					0.0125	0.00581	0.00613	0.000401	J 0.000623 J	0.000691 J	0.0218	0.000730	J 0.000937 J	0.0327	0.000934 J	0.000800	J 0.000841	J 0.000738 J	0.0447
Fluoride	4 mg/L	4	-	-	-	0.144	J 0.0803 J	0.177	J 0.111	J 0.105 J	0.0915 J	0.102 J	0.0958	J 0.0979 J	0.0795 J	0.100 J	0.0954	J 0.0948	J 0.0945 J	0.239 J F
Lead	0.015 mg/L	0.015	-	0.092	0.0036	0.0109	0.00221 J	0.00521	0.000994	JB 0.00600 B	0.000769 J	0.000523 J	0.00125 J	B 0.00199 JE	0.0104	0.00115 JB	0.00166	JB 0.00141	JB 0.00147 JB	0.00507
Lithium	0.040 mg/L					0.0126	J ND	ND	ND	ND	1.80	0.772	ND	ND	0.340	ND	ND	ND	ND	0.0209 J
Mercury	0.002 mg/L	0.002	0.000051	0.0014	0.00077	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND
Molybdenum	0.1 mg/L					0.00550	J 0.000948 J	0.00878	J 0.00217	J 0.00130 J	0.00296 J	0.00219 J	0.00222	J 0.00145 J	0.00442 J	0.00105 J	0.00103	J 0.00101	J 0.000981 J	ND
Radium 226 Radium 228	5 pCi/L	5 pCi/L				NS	1.17	NS	0.417	0.249 U	1.31	1.4	0.554	0.735	7.64	0.404 U	0.391	U 0.544	0.423 U	1.48
Selenium	0.05 mg/L	0.17	4.2	-	0.005	0.000582	J ND	0.000906	J ND	ND F2	ND	ND	0.000423	J 0.000636 J	0.00121 J	ND	ND	0.000402	J ND	0.00216 J
Thallium	0.002 mg/L	0.00024	0.00047	-	_	0.000126	J ND	ND	0.0000500	J ND	ND	ND	ND	ND	0.000164 J	ND	ND	ND	ND	ND
IONIC CONSTITUENTS				•																
Total Alkalinity	NA					443	38.2	393	85.6	85.6	174	87.7	85.7	85.8	229	86.1	86.4	80.9	85.8	ND
Hardness (as mg/L of CaCO3)	** NA					578	74	318	106	110	3198	3010	108	117	2608	115	108	109	114	1411
Magnesium	NA					36.6	5.20	20.3	6.41	6.62	291	51.8	6.32	6.76	77.8	6.87	6.41	6.45	6.73	63.6
Potassium	NA					4.96	2.37	4.85	2.68	2.91	125	262	3.01	3.65	285	3.06	2.87	2.85	2.95	9.51
Sodium	NA					18.5	5.52	26.7	3.79	3.95	274	277	3.98	4 63	285	4.64	4.01	3.87	4.02	42.1

\*All results listed in milligrams per liter (mg/L) unless otherwise noted by the Maximum Contaminant Level (MCL)

Na = Not available
pCi/L = picoCuries per Liter
SU = Standards units
purbos/cm = microSiems per centimeter
"F = Degrees Fahrenheit
m/ = millivolts
ND = Not detected above the Method Detection Limit
J = Result is less than the Reporting Limit but greater than or equal to the Method Detection Limit and the concentration is an approximate value.
B = Compound was found in the blank and sample.
F1 = MS and/or MSD Recovery is outside acceptance limits.
NM = Not measured

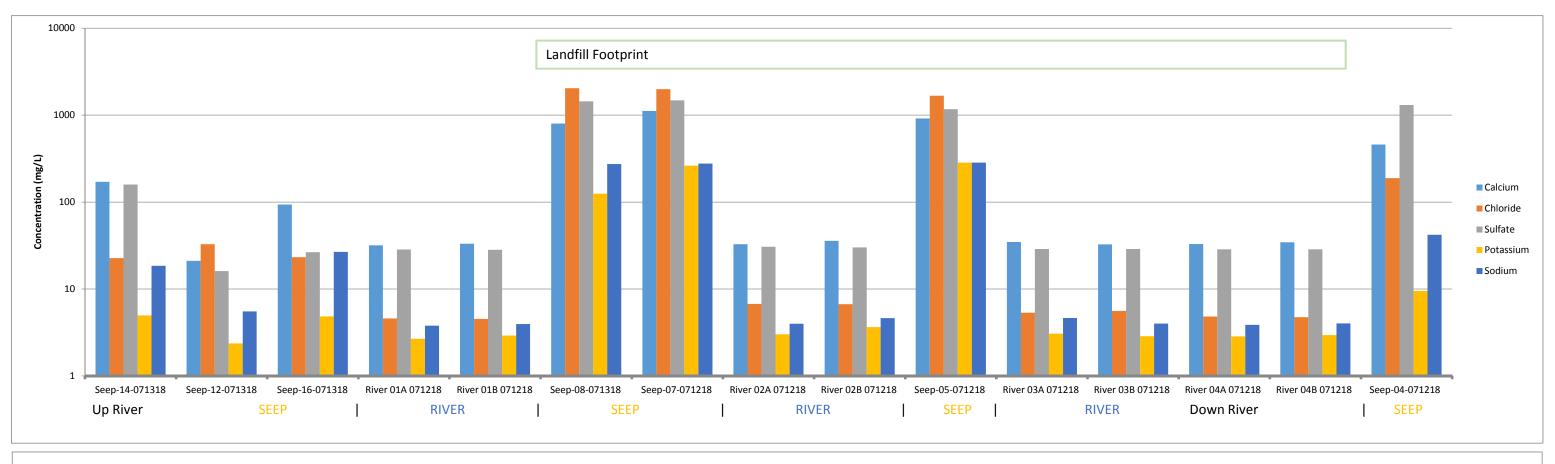
NM = Not measured
U = Result is less than the sample detection limit

Note: River "A" samples collected from surface River "B" samples collected <1 foot above river bed

\*\* The water hardness is using American degree equivalent to mg/L. Water hardness(mg/L)=Ca(mg/L)×2.497 + Mg(mg/L)×4.118

		Hardness	Hardness**		
		(mg/L CaCO <sub>3</sub> )	(mg/L CaCO <sub>3</sub> )		
Constituent	KY Acute Warm Water Habitat Equation	50	110		
		Criterion	Criterion		
		(ug/L)	(ug/L)		
Cadmium	Criterion = e(1.0166 (In Hard*)-3.924)	1.05	2.35		
Lead	Criterion = e(1.273 (In Hard*)-1.460)	34	92		
		Hardness	Hardness**		
	KY Chronic Warm Water Habitat	(mg/L CaCO <sub>3</sub> )	(mg/L CaCO <sub>3</sub> )		
Constituent	Equation	50	110		
	Equation	Criterion	Criterion		
		(ug/L)	(ug/L)		
Cadmium	Criterion = e(0.7409 (In Hard*)-4.719)	0.16	0.29		
Lead	Criterion = e(1.273 (In Hard*)-4.705)	1.3	3.6		
	*Hard = Hardness as mg/L CaCO <sub>3</sub>	**Average hardn	ess concentration	from collected Rive	r Samples (7/12/18)





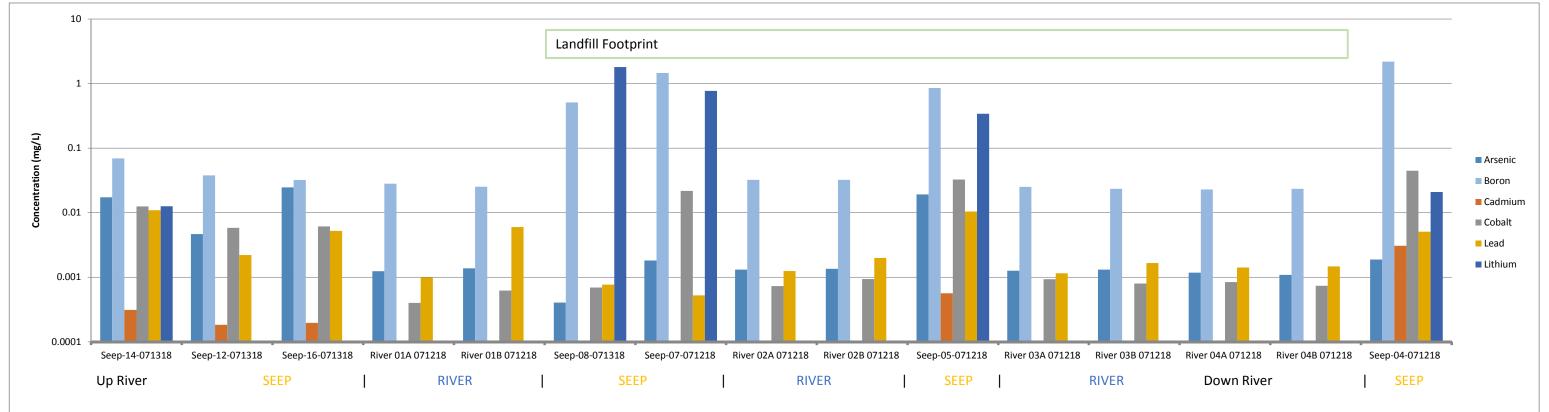


FIGURE 2 CCR ANALYTICAL SUMMARY - GREEN STATION LANDFILL RIVER SEEP AND RIVER SAMPLE EVALUATION, JULY 2018

# **Appendix B Green Landfill Analytical Summary Tables**

### **GREEN LANDFILL ANALYTICAL SUMMARY TABLES**

									DA	TE						
APPENDIX III CONSTITUENTS	<b>Detection Limit</b>	GWPS	3/26/2016	5/23/2016	8/18/2016	10/26/2016	2/1/2017	5/2/2017	8/7/2017	9/5/2017	10/5/2017	6/4/2018	7/10/2018	9/28/2018	4/22/2019	9/30/2019
							Baseline Events	S				Assessment	Re-Sampling		Assessment	
Boron	0.08		1.67	1.49	2.25	1.70	1.71 J	1.68	1.85 B	1.79	1.92		1.41	1.94 B	1.73 B	1.68 D2 M4
Calcium	0.5		29.1	31.8 B	33.0	30.9	20.8	28.1	27.1	29.9 B	26.4		26.5	28.5 B	32.1	29.1 D2
Chloride	3		9.03 JB	0.501 JB	6.60 B	6.02 B	5.56 B F1	5.30 B	5.12 B F1	5.71 B	4.07 F1 B		6.34 B	6.17 B	6.41 B F1	7.5
Fluoride	1		ND J	ND JB	ND J	ND JB	ND J F1	ND JB	ND J F1	ND J	ND J F1		ND J	ND JB	0.521 J	0.6
Sulfate	5		25.2	22.8 JB	22.9	20.7 B	28.4	24.0 B	25.3 B	23.4	24.9 JB		23.5	22.5 B	35.1 B F1	19
pH (SU)	0.10		7.39	7.24	7.57	7.19	7.63	7.54	7.45	7.48	7.63		7.08	8.43	7.87	7.79 H3
Total Dissolved Solids	10		598	588	585	585	605	630	614	627	636		585	616	568 B	444 H1
APPENDIX IV CONSTITUENTS																
Antimony	0.002	0.006 mg/L	ND	ND J	ND B	ND	ND	ND JB	0.00297 B	ND JB		ND JB	ND J	NA	0.000254 JB	ND M1 V1 U
Arsenic	0.005	0.01 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND J F1	ND JB		ND JB	ND J	ND JB	0.00167 JB	0.0005 V1 J
Barium	0.2	2 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND J F1	ND J		ND JB	ND J	ND J	0.0862 J	0.091 D2
Beryllium	0.002	0.004 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	0.000533 J	ND D2 U
Cadmium	0.001	0.005 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	0.000299 J	ND VI U
Chromimum	0.003	0.1 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND J		ND JB	ND	NA	0.00354 B	ND U
Cobalt	0.005	0.006 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND J	ND J		ND JB	ND J	NA	0.000571 J	ND U
Fluoride	1	4 mg/L	ND J	ND J	ND J	ND JB	ND J F1	ND JB	ND J F1	ND J		ND J	ND J	ND JB	0.521 J	0.6
Lead	0.005	0.015 mg/L	ND J	ND J	ND J	ND	ND	ND	ND	ND J		ND	ND J	NA	0.000279 J	ND V1 U
Lithium	0.05	0.040 mg/L	0.0293 J	0.0317 J	0.0326 J	0.0286 J	0.0342 J	0.0396 J	0.0314 J	0.0315 J		0.0319 J	0.0298 J	0.0279 J	0.0295 J	ND D2 M3 U
Mercury	0.0002	0.002 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	ND	ND	ND V1 U
Molybdenum	0.01	0.1 mg/L	ND	ND J	ND J	ND J	ND J	ND J	ND J	ND		ND J	ND J	NA	0.00105 J	ND U
Radium 226	1	5 pCi/L	1.05	1.02	0.676	1.02	0.694	0.666	0.491	0.601		1.92	0.882	0.905	0.689	0.782
Radium 228	ı	3 poi/L	1.03	1.02	0.070	1.02	0.034	0.000	0.431	0.001		1.32	0.002	0.303	0.003	0.733
Selenium	0.01	0.05 mg/L	ND	ND	ND	ND	ND	ND	ND J	ND		ND	ND	NA	0.00105 J	ND U
Thallium	0.001	0.002 mg/L	ND	ND J	ND	ND J	ND	ND	ND J	ND		ND	ND	NA	0.000498 J	0.0001 V1 J

<sup>\*</sup>All results listed in milligrams per liter (mg/L) unless otherwise noted by the Maximum Contaminant Level (MCL)

GWPS = Groundwater Protection Standard

NA = Not Analyzed

ND = Not Detected at or above Method Detection Limit

- J = Result is less than the Reporting Limit but greater than or equal to the Method Detection Limit and the concentration is an approximate value.
- B = Compound was found in the blank and sample.
- F1 = MS and/or MSD Recovery is outside acceptance limits.
- D2 = Sample required dilution due to matrix interference
- H1 = Sample analysis performed pasts holding time
- H3 = Sample received and analyzed past holding time
- M3 = The accuracy of the spike recovery value is reduced since the analyte concentration in the sample is disproportionate to spike level. The method control sample recovery was acceptable
- M4 = The analysis of the spike sample required a dilution such that the spike concentration was diluted below the reporting limit. The method control sample recovery was acceptable
- U = Target analyte was analyzed for, but was below detection limit
- V1 = CCV recovery was above method acceptance limits. This target analyte not detected in the sample

										DATE						
APPENDIX III CONSTITUENTS	<b>Detection Limit</b>	GWPS	3/26/2016	5/23/2016	8/18/2016	11/14/2016	2/1/2017	5/2/2017	8/8/2017	9/7/2017	10/6/2017	6/5/2018	7/11/2018	9/28/2018	4/23/2019	10/1/2019
						E	Baseline Events					Assessment	Re-Sampling		Assessment	
Boron	0.08		ND J	ND J	ND J	ND J	ND JB	ND J	0.113 JB	ND JB	ND J		ND J	0.0630 JB	0.101 JB	ND D2 U
Calcium	0.5		119	116 B	140	140 B	126	152	154	121	150		155	165 B	156	166 D1
Chloride	3		126 B	125 B	129 B	133	142 B	129 B	145 B	136 B	129 B		154 B	159 B	144	108 D
Fluoride	1		ND J	ND	ND J	ND JB F1	ND J	ND JB	ND JB	ND JB F1	ND J		ND J	ND JB	0.193 J	0.3
Sulfate	5		80.0	84.5 J	85.5 J	90.1	89.8	83.2	92.0 JB	90.8	88.6 JB		107	108 B	105	79.0 D
pH (SU)	0.10		6.81	6.59	6.7	6.78	7.12	7.04	6.77	6.69	6.86	6.64	6.40	7.02	7.15	7.39 H3
Total Dissolved Solids	10		764	780	830	880	862	918	913	818	970		884	937	918 B	930 H1
APPENDIX IV CONSTITUENTS																
Antimony	0.002	0.006 mg/L	ND	ND J	ND JB	ND JB	ND	ND JB	ND B	ND JB		ND JB	ND J	NA	0.0000670 JB	ND V1 U
Arsenic	0.005	0.01 mg/L	0.00703 J	0.00633	0.0110	0.0159	0.0462	0.00755	0.0381	0.00527		0.0327 B	0.0119	0.0211 B	0.00738 B	0.0129 D2
Barium	0.2	2 mg/L	ND J	ND J	0.280	0.319	0.347	0.332	0.308	ND J		0.369	0.323	0.367	0.362	0.380 D2
Beryllium	0.002	0.004 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	0.000281 J	ND D2 U
Cadmium	0.001	0.005 mg/L	ND J	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND V1 U
Chromimum	0.003	0.1 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND		ND JB	ND	NA	0.00122 JB	ND D2 U
Cobalt	0.005	0.006 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND JB	ND J		ND JB	ND J	NA	0.00382 J	ND D2 U
Fluoride	1	4 mg/L	ND J	ND	ND J	ND JB F1	ND J	ND JB	ND JB	ND JB F1		ND J	ND J	ND JB	0.193 J	0.3
Lead	0.005	0.015 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND J	NA	ND	ND V1 U
Lithium	0.05	0.040 mg/L	ND J	ND	ND	ND	ND J	ND J	ND JB	ND		ND	ND	ND	ND	ND D2 VI U
Mercury	0.0002	0.002 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	ND	ND	ND V1 U
Molybdenum	0.01	0.1 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND JB	ND JB		ND J	ND J	NA	0.00210 J	0.003 J
Radium 226	1	5 pCi/L	0.533	ND	0.46	ND	0.856	0.73	0.968	0.537		1.18	0.733	0.803	0.391	0.136
Radium 228	· .	3 pc//L	0.333	IND	0.40	IND	0.656	0.73	0.900	0.557		1.10	0.733	0.003	0.391	0.834
Selenium	0.01	0.05 mg/L	ND	ND	ND	ND JB	ND	ND	ND JB	ND		ND	ND	NA	ND	ND U
Thallium	0.001	0.002 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND		ND	ND	NA	0.0000800 J	ND V1 U

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pCi/L = picoCuries per Liter

J = Result is less than the Reporting Limit but greater than or equal to the Method Detection Limit and the concentration is an approximate value.

B = Compound was found in the blank and sample.

F1 = MS and/or MSD Recovery is outside acceptance limits.

D1 = Sample required dilution due to high concentration of target analyte

D2 = Sample required dilution due to matrix interference

H1 = Sample analysis performed pasts holding time

H3 = Sample received and analyzed past holding time

U = Target analyte was analyzed for, but was below detection limit

V1 = CCV recovery was above method acceptance limits. This target analyte not detected in the sample

										DATE						
APPENDIX III CONSTITUENTS	Detection Limit	GWPS	3/26/2016	5/23/2016	8/18/2016	11/14/2016	2/1/2017	5/2/2017	8/8/2017	9/6/2017	10/6/2017	6/5/2018	7/11/2018	9/28/2018	4/23/2019	10/1/2019
						E	Baseline Events					Assessment	Re-Sampling		Assessmen	t
Boron	0.08		0.145	0.135 J	0.279 J	0.213 J	0.235 JB	0.232 J	0.304 JB	0.376 J	0.313		0.177 J	0.257 JB	0.259 JB	ND D2 U
Calcium	0.5		431	322 B	362	365 B	327	420	421	438 B	408		469	447 B	411	490 D1
Chloride	3		2630 HB	3070	2150 B	2150 B	2220 B	2120 B	1790 B	2270 B	1870 B		2180 B	2040 B	1850	4570 D
Fluoride	1		ND J	ND J	ND J	ND JB	ND J	ND JB	ND	3.16	ND J		ND J	ND JB	0.387 J	0.4
Sulfate	5		1330	1330	1190	1660	1080	1030 B	942	1130	1030 B		1010	1130 B	1080	1680 D
pH (SU)	0.10		6.92	6.86	6.95	6.75	7.17	7.11	6.81	6.9	6.95	6.84	6.55	7.98	7.23	7.33 H3
Total Dissolved Solids	10		4440	5010	4170	4450	4270	5170	5010	5020	5300		4540	4940	4250 B	6900 H1
APPENDIX IV CONSTITUENTS																
Antimony	0.002	0.006 mg/L	ND	ND J	ND JB	ND JB	ND	ND JB	ND JB	ND JB		ND JB	ND	NA	0.000102 JB	ND V1 U
Arsenic	0.005	0.01 mg/L	ND	ND J	ND J	ND J	ND J	ND J	ND J	ND JB		ND JB	ND J	ND JB	0.000575 JB	ND D2 U
Barium	0.2	2 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND J	ND J		ND J	ND J	ND J	0.0474 J	0.051 D2 U
Beryllium	0.002	0.004 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	0.000199 J	ND D2 U
Cadmium	0.001	0.005 mg/L	ND J	ND J	ND	ND	ND J	ND J	ND	ND		ND J	ND J	NA	0.000164 J	ND V1 U
Chromimum	0.003	0.1 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND J		ND JB	ND	NA	0.00168 JB	ND D2 U
Cobalt	0.005	0.006 mg/L	ND	ND J	ND J	ND J	ND J	ND J	ND J	ND J		ND JB	ND J	NA	0.000243 J	0.008
Fluoride	1	4 mg/L	ND J	ND J	ND J	ND JB	ND J	ND JB	ND	3.16		ND J	ND J	ND JB	0.387 J	0.4
Lead	0.005	0.015 mg/L	ND J	ND	ND	ND	ND	ND	ND J	ND J		ND	ND J	NA	0.000137 J	ND V1 U
Lithium	0.05	0.040 mg/L	0.669	0.516	0.648	0.677	0.689	0.746	0.767	0.762		0.699	0.790	0.766	0.678	0.79 D1
Mercury	0.0002	0.002 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	ND	ND	ND V1 U
Molybdenum	0.01	0.1 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND D2 U
Radium 226	1	5 pCi/L	1.38	0.386	0.472	1.15	1.15	0.923	1.53	1.03		1.18	1.43	1.21	0.641	0.139
Radium 228	ı	3 po//L	1.50	0.300	0.712	1.10	1.15	0.020	1.55	1.00		1.10	1.45	1.21	0.041	0.734
Selenium	0.01	0.05 mg/L	ND	ND	ND J	ND JB	ND	ND	ND	ND		ND J	ND	NA	0.00103 J	ND D2 U
Thallium	0.001	0.002 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND J		ND	ND	NA	0.000860 J	ND V1 U

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										DATE						
APPENDIX III CONSTITUENTS	<b>Detection Limit</b>	GWPS	3/29/2016	5/23/2016	8/18/2016	10/26/2016	2/1/2017	5/2/2017	8/8/2017	9/7/2017	10/6/2017	6/5/2018	7/11/2018	9/28/2018	4/22/2019	10/1/2019
							Baseline Events	3				Assessmen	Re-Sampling		Assessment	
Boron	0.08		0.602	0.498 J	1.58	1.7	1.54 B	2.09	2.51 B	2.87 B	1.36		0.751 J	1.33 B	1.25 B	1.75 D2
Calcium	0.5		660	386 B	464	558	591	774	743	739	828		822	722 B	730	690 D1
Chloride	3		1450 B	939 B	952 B	1000 B	1420 B	1320 B	1360 B	1880 B	1730 B		1430 B	1310 B	1510	1910 D
Fluoride	1		ND J	ND	ND J	ND JB	ND J	1.06 B	ND	ND JB	ND J		ND J	ND JB	0.102 J	0.2
Sulfate	5		1830	1640	1420	1420 B	1620	1430 B	1600 B	2020	1590 B		1460	1400 B	1440	2490 D
pH (SU)	0.10		6.36	6.83	7.08	6.61	7.28	7.1	6.84	6.64	6.93	6.86	6.58	8.06	7.26	7.36 H3
Total Dissolved Solids	10		3700	4250	3440	3250	4420	4550	4890	4700 H	6220		4880	5170	4840 B	4820 H1
APPENDIX IV CONSTITUENTS																
Antimony	0.002	0.006 mg/L	ND	ND J	ND JB	ND	ND	ND JB	ND JB	ND JB		ND JE	ND	NA	0.0000360 JB	ND V1 U
Arsenic	0.005	0.01 mg/L	ND	ND J	ND J	ND	ND J	ND J	ND J	ND JB		ND JE	ND J	ND JB	0.000445 JB	ND D2 U
Barium	0.2	2 mg/L	ND J	ND J	ND J	ND JB	ND J	ND J	ND J	ND JB		ND J	ND J	ND J	0.0308 JB	0.029 D2 J
Beryllium	0.002	0.004 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND D2 U
Cadmium	0.001	0.005 mg/L	ND J	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND V1 U
Chromimum	0.003	0.1 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND		ND JE	ND	NA	0.00110 JB	ND D2 U
Cobalt	0.005	0.006 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND		ND JE	ND J	NA	0.000415 J	ND U
Fluoride	1	4 mg/L	ND	ND	ND J	ND JB	ND J	ND B	ND	ND JB		ND J	ND J	ND JB	0.102 J	0.2
Lead	0.005	0.015 mg/L	ND J	ND J	ND	ND	ND	ND	ND	ND		ND	ND J	NA	ND	ND V1 U
Lithium	0.05	0.040 mg/L	1.39	0.838	1.13	1.25	1.35	1.59	1.77	1.66		1.81	1.91	1.81	1.73	ND D2 V1 U
Mercury	0.0002	0.002 mg/L	0.00027	0.000224	ND J	0.000248	0.000302	0.000717	0.000825	0.000485		0.000824	0.000832	0.000680	0.000825	0.0004 V1 J
Molybdenum	0.01	0.1 mg/L	ND J	ND J	ND	ND	ND J	ND	ND	ND		ND	ND	NA	ND	ND D2 U
Radium 226	] 1	5 pCi/L	1.26	0.592	ND	0.536	1.22	1.43	1.94	1.19		1.62	2.00	1.51	1.66	0.451
Radium 228	'	3 poi/L		0.592	ND	0.550	1.22	1.43	1.54	1.19		1.02	2.00	1.51	1.00	0.804
Selenium	0.01	0.05 mg/L	ND J	ND J	ND J	ND	ND J	ND	ND	ND J		ND J	ND	NA	0.00211 J	ND U
Thallium	0.001	0.002 mg/L	ND	ND	ND	ND J	ND	ND	ND	ND		ND	ND	NA	0.0000410 J	ND V1 U

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APPENDIX III CONSTITUENTS	<b>Detection Limit</b>	GWPS	3/29/2016	5/23/2016	8/18/2016	10/26/2016	2/1/2017	5/2/2017	8/7/2017	9/7/2017	10/6/2017	6/5/2018	7/11/2018	9/28/2018	4/22/2019	9/30/2019
							Baseline Events				-	Assessment	Re-Sampling		Assessme	nt
Boron	0.08		0.217	0.0896 J	0.216 J	0.214 J	0.222 JB	0.241 J	0.257 JB	0.276 B	0.262		0.207 J	0.263 JB	0.271 J	B ND D2 U
Calcium	0.5		452	189 B	374	399	335	464	423	407 B	383		469	441 B	446	476 D1
Chloride	3		1630 B	521	688 B	755 B	734 B	722 B	945 B	779 B	608 B		941 B	1140 B	931	1500 D
Fluoride	1		ND J	ND	ND J	ND	ND J	ND JB	ND	3.69	ND J		ND J	ND JB	0.128	J 0.2
Sulfate	5		1760 HE	876	1780	1740 B	1880	1760 B	2060 B	1920	1600 B		1800	1890 B	1800	2990 D
pH (SU)	0.10		6.76	6.74	6.99	6.61	7.14	7.44	6.87	7.13	7.06	6.88	6.40	7.99	7.15	7.41 H3
Total Dissolved Solids	10		4210	1660	3470	3610	3680	4250	4130	4120	4390		4100	4540	4360	B 5320 H1
APPENDIX IV CONSTITUENTS																
Antimony	0.002	0.006 mg/L	ND	ND J	ND JB	ND	ND	ND JB	ND JB	ND JB		ND JB	ND	NA	0.0000700 J	B ND V1 U
Arsenic	0.005	0.01 mg/L	ND	ND J	ND JB	ND J	ND J	ND J	ND J	ND JB		ND JB	ND J	ND JB	0.000424 J	B ND D2 U
Barium	0.2	2 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND J	ND J		ND J	ND J	ND J	0.0167	J 0.016 D2 J
Beryllium	0.002	0.004 mg/L	ND	ND	ND J	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND D2 U
Cadmium	0.001	0.005 mg/L	ND J	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND V1 U
Chromimum	0.003	0.1 mg/L	ND	ND J	ND	ND	ND J	ND J	ND	ND J		0.00363 B	ND	NA	0.00159 J	B 0.0033
Cobalt	0.005	0.006 mg/L	ND	ND J	ND J	ND J	ND	ND J	ND	ND J		ND JB	ND J	NA	0.000288	J ND U
Fluoride	1	4 mg/L	ND J	ND	ND J	ND	ND J	ND	ND	3.69		ND J	ND J	ND JB	0.128	J 0.2
Lead	0.005	0.015 mg/L	ND J	ND J	ND	ND	ND	ND	ND	ND		ND J	ND J	NA	0.0000860	J ND V1 U
Lithium	0.05	0.040 mg/L	0.521	0.136	0.305	0.325	0.368	0.415	0.405	0.353		0.459	0.481	0.425	0.434	0.40 D1
Mercury	0.0002	0.002 mg/L	ND	ND	ND	ND	ND	ND	0.00351	ND		ND	ND	ND	ND	ND V1 U
Molybdenum	0.01	0.1 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND D2 U
Radium 226	1	5 pCi/L	1.16	0.736	0.959	0.957	0.765	0.888	1.54	0.773		0.862	1.42	1.37	0.945	0.368
Radium 228	1	5 poi/L	1.10	0.750	0.555	0.557	0.703	0.000	1.04	0.110		0.002	1.72	1.07	0.545	0.730
Selenium	0.01	0.05 mg/L	ND	ND	ND	ND	ND J	ND J	ND	ND		ND J	ND	NA	0.000624 J	ND U
Thallium	0.001	0.002 mg/L	ND	ND	ND J	ND J	ND	ND J	ND	ND J		ND J	ND	NA	0.0000890 J	ND V1 U

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										DATE						
APPENDIX III CONSTITUENTS	Detection Limit	GWPS	3/29/2016	5/23/2016	8/18/2016	10/26/2016	2/1/2017	5/2/2017	8/7/2017	9/5/2017	10/5/2017	6/4/2018	7/10/2018	9/28/2018	4/22/2019	9/30/2019
						В	Baseline Events	S				Assessment	Re-Sampling		Assessme	nt
Boron	0.08		0.156	0.137 J	0.193 J	0.168 J	0.173 B	0.179 J	0.167 JB	0.199 J	0.178		0.155 J	0.196 J	0.194 JB	ND D2 U
Calcium	0.5		467	374 B	373	400	320	415	365	382 B	376		386	356 B	421	431 D1
Chloride	3		167 B	149 B	136 JB	150 B	125 B	129 B	128 B	123 B	138 B		147 B	142 B	142	230 D
Fluoride	1		ND J	ND J	ND J	ND JB	ND J	ND JB	ND	ND J	ND J		ND J	ND J	3 0.409 J	0.5
Sulfate	5		2250 HB	3340	2550	2610 B	2700	2600 B	2820 B	2490	2700 B		2120	2420	2200	3830 D
pH (SU)	0.10		6.66	6.65	6.96	6.6	6.92	6.97	6.76	6.95	6.86		6.50	7.94	6.86	7.15 H3
Total Dissolved Solids	10		4060	4280	4350	4470	4720	4700	4830	4890	4910		4500	4820	4780 B	4830 H1
APPENDIX IV CONSTITUENTS																
Antimony	0.002	0.006 mg/L	ND	ND J	ND JB	ND	ND	ND JB	ND JB	ND JB		ND JB	ND	NA	0.0000920 JB	ND V1 U
Arsenic	0.005	0.01 mg/L	ND	ND J	ND J	ND J	ND J	ND J	ND J	ND JB		ND JB	ND J	ND J	3 0.000722 JB	ND V1 U
Barium	0.2	2 mg/L	ND J	ND J	ND J	ND J	ND J	ND J	ND J	ND J		ND J	ND J	ND J	0.0128 J	0.010 D2 J
Beryllium	0.002	0.004 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND D2 U
Cadmium	0.001	0.005 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	NA	ND	ND V1 U
Chromimum	0.003	0.1 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND J		ND JB	ND	NA	0.00196 JB	ND U
Cobalt	0.005	0.006 mg/L	ND	ND J	ND J	ND J	ND J	ND J	ND J	ND J		ND JB	ND J	NA	0.000276 J	ND U
Fluoride	1	4 mg/L	ND J	ND J	ND J	ND JB	ND J	ND JB	ND	ND J		ND J	ND J	ND J	3 0.409 J	0.5
Lead	0.005	0.015 mg/L	ND J	ND J	ND	ND	ND	ND	ND	ND		ND	ND J	NA	ND	ND V1 U
Lithium	0.05	0.040 mg/L	0.0475 J	0.0527	0.0555	0.0524	0.0607	0.0724	0.0589	0.0554		0.0650	0.0592	0.0558	0.0633	0.05 D2 V1 J
Mercury	0.0002	0.002 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND	ND	ND	ND	ND V1 U
Molybdenum	0.01	0.1 mg/L	ND J	ND J	ND J	ND B	ND J	ND J	ND J	ND J		ND J	ND J	NA	0.000972 J	ND D2 U
Radium 226	1	5 pCi/L	0.741	0.386	ND	0.751	ND	ND	0.462	ND		0.392	0.532	NDL	0.450	0.548
Radium 228	1	0 p0//L	0.741	0.500	ND	0.751	ND	ND	0.402	ND		0.002	0.002	IND	0.400	0.698
Selenium	0.01	0.05 mg/L	ND	ND	ND	ND	ND	ND	ND	ND		ND J	ND	NA	0.00110 J	ND U
Thallium	0.001	0.002 mg/L	ND	ND J	ND	ND	ND	ND	ND	ND		ND	ND	NA	0.0000610 J	ND V1 U

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					DATE			
APPENDIX III CONSTITUENTS	Detection Limit	GWPS	3/29/201	9	4/10/20 <sup>-</sup>	19	10/25/20	19
					Characteriz	ation		
Boron	0.08		0.1880	JB	0.2710	JB	ND	D2, U
Calcium	0.5		465	В	502		505	D1
Chloride	3		1430		1430	В	1610	D
Fluoride	1		ND		0.3230	JB	0.4	
Sulfate	5		2870		2880	В	2440	D
pH (Field Measurement)	0.10		6.88		6.99		6.86	
Total Dissolved Solids	10		6990		6690		7330	
APPENDIX IV CONSTITUENTS								
Antimony	0.002	0.006 mg/L	0.0001	JB	0.0001	JB	ND	U
Arsenic	0.005	0.01 mg/L	0.0022	J	0.0021	J	0.0039	
Barium	0.2	2 mg/L	0.0243	J	0.0216	JB	0.030	
Beryllium	0.002	0.004 mg/L	ND		ND		ND	U
Cadmium	0.001	0.005 mg/L	ND		ND		0.0004	J
Chromium	0.003	0.1 mg/L	0.0047	В	0.0036		0.0066	
Cobalt	0.005	0.006 mg/L	0.0059	В	0.0052		0.011	
Fluoride	1	4 mg/L	ND		0.3230	JB	0.4	
Lead	0.005	0.015 mg/L	0.0011	J	0.0002	J	0.003	
Lithium	0.05	0.040 mg/L	0.0281	J	0.0286	J	0.02	
Mercury	0.0002	0.002 mg/L	ND		ND	^	ND	U
Molybdenum	0.01	0.1 mg/L	0.0015	J	0.0010	J	0.005	J
Radium 226	1	5 pCi/L	0.7760		0.3190	U	0.126	
Radium 228		3 po//L	0.7760		0.5180		1.52	
Selenium	0.01	0.05 mg/L	ND		ND		ND	U
Thallium	0.001	0.002 mg/L	ND		ND		ND	U

<sup>\*</sup>All results listed in milligrams per liter (mg/L) unless otherwise noted by the Maximum Contaminant Level (MCL)

GWPS = Groundwater Protection Standard

ND = Not Detected at or above Method Detection Limit

J = Result is less than the Reporting Limit but greater than or equal to the Method Detection Limit and the concentration is an approximate value.

B = Compound was found in the blank and sample.

D1 = Sample required dilution due to high concentration of target analyte

D2 = Sample required dilution due to matrix interference

U = Target analyte was analyzed for, but was below detection limit

# **Appendix C Green Landfill Statistical Procedures and Results**

#### 1.0 GREEN LANDFILL STATISTICAL PROCEDURES AND RESULTS

The Appendix III and IV groundwater quality data for the Green Landfill were evaluated using an interwell approach that statistically compared constituent concentrations at downgradient compliance monitoring wells to those present at a background monitoring well. For the Green Landfill, monitoring well MW-1 is designated as the background well because it is located upgradient, whereas monitoring wells MW-2, MW-3A, MW-4, MW-5, and MW-6 are designated as compliance wells because they are located downgradient.

The statistical analyses were performed in accordance with the U.S. Environmental Protection Agency's Final CCR Rule 40 CFR Parts 257.93(f), 257.93(g), and 257.93(h) and the Groundwater Monitoring System and Statistical Methods Certification. Prediction limits (i.e., parametric or nonparametric) with 1 of 2 retesting were developed for each constituent based on the frequency of non-detect values and whether the background data for that constituent exhibited a normal, lognormal, or nonparametric distribution. For the statistical analysis, non-detect values were represented as one-half the detection limit. No outliers were identified in the background data. Analytical data from the background monitoring wells collected between March 2016 and October 2019 were used to develop an upper prediction limit (UPL) for the Appendix III and IV background data at 95 percent confidence. Data from the downgradient monitoring wells for the same time period were compared to the UPL to identify statistically significant increases (SSIs) over background. Mann-Kendall trend analysis was used to identify statistically significant increasing trends for constituents with SSIs. ProUCL Version 5.1 was used to store the data and run the statistical analyses. The results of the analyses, including the UPLs, are provided in **Tables C1** and **C2**.

The statistical analysis results indicate that Appendix III constituents calcium, chloride, sulfate, and total dissolved solids (TDS) at monitoring wells MW-2, MW-3A, MW-4, MW-5, and MW-6 have SSIs over background (**Table C3**) that were confirmed by subsequent sampling events. Boron, fluoride, and pH did not have any verified SSIs over background. pH at MW-6 had a verified SSI below the background lower prediction limit (LPL). Based on these results, assessment monitoring was conducted at the landfill. Statistical analysis of the April and October 2019 Appendix IV assessment monitoring results indicate that arsenic and barium at monitoring well MW-2, lithium at monitoring wells MW-3A, MW-4, MW-5, and MW-6, and mercury at monitoring well MW-4 have verified SSIs over background (**Table C4**).

The Appendix IV constituents with SSIs were further evaluated to determine whether they are present at statistically significant levels (SSLs) over the groundwater protection standards (GWPS) by calculating the lower confidence limit at 95% confidence (95LCL) for each well and constituent identified as a SSI using the baseline, detection, and assessment monitoring results collected to date. For a constituent to be present at a SSL over the GWPS, its 95LCL must be greater than the GWPS. **Table C5** provides a summary of the 95LCLs and GWPS for arsenic, barium, lithium, and mercury at monitoring wells MW-2, MW-3A, MW-4, MW-5, and MW-6. The results indicate that lithium at monitoring wells MW-3A, MW-4, MW-5, and MW-6 (yellow highlight) is present as a SSL above the GWPS. The LCLs for the remaining wells and constituents (arsenic, barium, and mercury) are less than the GWPS and thus are not considered SSLs.

Table C1. Well MW-1 Appendix III Constituents Background Upper Prediction Limits

Parameter (Units)	Number of Samples	Percent Non-detects	Normal or Lognormal Distribution?	Statistical Test	Background Limit
Boron (mg/L)	13	0	Yes/Yes	Parametric	2.122
Calcium (mg/L)	13	0	Yes/Yes	Parametric	35
Chloride (mg/L)	13	0	Yes/No	Parametric	9.3
Fluoride (mg/L)	13	0	No/No	Nonparametric	0.89
pH (std units)	13	0	Yes/Yes	Parametric	6.99/7.93
Sulfate (mg/L)	13	0	Yes/Yes	Parametric	33
TDS (mg/L)	13	0	No/No	Nonparametric	636

Note: pH has both a lower prediction limit (LPL) and upper prediction limit (UPL); all other constituents are represented as UPLs

Table C2. Well MW-1 Appendix IV Constituents Background Upper Prediction Limits

Parameter (Units)	Number of Samples	Percent Non- detects	Normal or Lognormal Distribution?	Statistical Test	Background Limit
Antimony (mg/L)	13	31	No/No	Nonparametric	0.003
Arsenic (mg/L)	12	0	No/No	Nonparametric	0.0026
Barium (mg/L)	12	0	Yes/Yes	Parametric	0.098
Beryllium (mg/L)	12	92	No/No	Nonparametric	0.002
Cadmium (mg/L)	12	92	No/No	Nonparametric	0.001
Chromium (mg/L)	12	62	Yes/Yes	Parametric	0.0024
Cobalt (mg/L)	12	8	Yes/Yes	Parametric	0.0014
Fluoride (mg/L)	13	0	No/No	Nonparametric	0.89
Lead (mg/L)	13	46	Yes/No	Parametric	0.0003
Lithium (mg/L)	12	8	Yes/Yes	Parametric	0.037
Mercury (mg/L)	13	100	No/No	Nonparametric	0.0002
Molybdenum (mg/L)	13	31	No/No	Nonparametric	0.01
Ra-226+228 (pCi/L)	12	0	No/Yes	Parametric	1.74
Selenium (mg/L)	12	85	No/No	Nonparametric	0.01
Thallium (mg/L)	13	61	No/No	Nonparametric	0.0006

Note: The UPL for constituents with 100 percent nondetects (Be, Cd, and Hg) is established as the maximum laboratory analytical reporting limit.

Table C3. Big Rivers Green Landfill Appendix III SSI Summary

Well	Location	В	Ca	Cl	F	_	H 'UPL)	SO4	TDS
MW-1	Upgradient	Р	Р	Р	NP	Р	Р	Р	NP
MW-2	Downgradient								
MW-3A	Downgradient								
MW-4	Downgradient								
MW-5	Downgradient								
MW-6	Downgradient								

#### Notes:

SSIs determined using interwell prediction limits; MW-8 is upgradient background well

P = parametric prediction limit; NP = nonparametric prediction limit

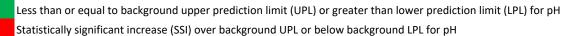


Table C4. Big Rivers Green Landfill Appendix IV SSI Summary

Well	Location	Sb	As	Ва	Ве	Cd	Cr	Co	F	Pb	Li	Hg	Мо	Ra-226+228	Se	ті
MW-1	Upgradient	NP	Np	Р	NP	NP	Р	Р	NP	Pb	Р	NP	NP	Р	NP	NP
MW-2	Downgradient															
MW-3A	Downgradient															
MW-4	Downgradient															
MW-5	Downgradient															
MW-6	Downgradient															

Notes:

SSIs determined using interwell prediction limits; MW-8 is upgradient background well

P = parametric prediction limit; NP = nonparametric prediction limit

Less than or equal to background upper prediction limit (UPL) or greater than lower prediction limit (LPL) for pH

Statistically significant increase (SSI) over background UPL or below background LPL for pH

Table C5. Summary of LCLs and GWPS for Arsenic, Barium, Lithium, and Mercury

Well	Parameter	95%LCL (mg/L)	GWPS (mg/L)
MW-2	Arsenic	0.008	0.01
MW-2	Barium	0.25	2.0
MW-3A	Lithium	0.65	0.04
MW-3A	Mercury	0.0001	0.002
MW-4	Lithium	1.04	0.04
MW-5	Lithium	0.32	0.04
MW-6	Lithium	0.055	0.04

95%LCL = lower confidence limit at 95% confidence. Yellow highlighted results exhibit a statistically significant level (SSL) above the GWPS.

# Appendix D Green Landfill – April 2020 Groundwater Analytical Data



0041376



## Certificate of Analysis 0041376

Chad Phillips
Big Rivers Electric Corporation Reid/Green Station
PO Box 24
Henderson KY, 42419

Customer ID: Report Printed:

44-102032 04/30/2020 14:59

Project Name: Green Landfill Semiannual Groundwater

Workorder:

Dear Chad Phillips

Enclosed are the analytical results for samples received at one of our laboratories on 04/07/2020 15:49.

Pace Analytical Services LLC Kentucky is a commercial laboratory accredited by various state and national authorities, including Indiana, Kentucky, Tennessee, and Virginia's National Environmental Laboratory Accreditation Program (NELAP). With the NELAP accreditation, applicable test results are certified to meet the requirements of the National Environmental Laboratory Accreditation Program.

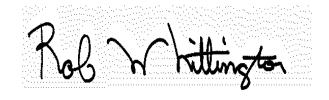
If you have any questions concerning this report please contact the individual listed below.

Please note that this certificate of analysis may not be reproduced without the written consent of Pace Analytical Services, LLC Kentucky.



#460210 Madisonville, KY #460293 Pikeville, KY

> This page is included as part of the Analytical Report and must be retained as a permanent record thereof.



Rob Whittington, Project Manager





#### **SAMPLE SUMMARY**

	SAINT LE SUMMANT												
Lab ID	Client Sample ID/Alias		Matrix	Date Collected	Date Received	Sampled By							
0041376-01	MW1/		Groundwater	04/06/2020 13:05	04/07/2020 15:49	Phillip Hill							
0041376-02	MW2/		Groundwater	04/07/2020 11:40	04/07/2020 15:49	Phillip Hill							
0041376-03	MW3A/		Groundwater	04/07/2020 13:55	04/07/2020 15:49	Phillip Hill							
0041376-04	MW4/		Groundwater	04/07/2020 09:55	04/07/2020 15:49	Phillip Hill							
0041376-05	MW5/		Groundwater	04/07/2020 10:10	04/07/2020 15:49	Phillip Hill							
0041376-06	MW6/		Groundwater	04/06/2020 14:20	04/07/2020 15:49	Phillip Hill							
0041376-07	DUPLICATE/		Groundwater	04/07/2020 10:20	04/07/2020 15:49	Phillip Hill							
0041376-08	FIELD BLANK/		Water	04/07/2020 11:50	04/07/2020 15:49	Phillip Hill							
<u>LabNumber</u>	<u>Measurement</u>	<u>Value</u>											
0041376-01	Field Conductance	867											
	Field pH	7.22											
	Field Temp (C)	18.23											
0041376-02	Field Conductance	1590											
	Field pH	6.92											
	Field Temp (C)	16.86											
0041376-03	Field Conductance	8090											
	Field pH	6.92											
	Field Temp (C)	16.86											
0041376-04	Field Conductance	6770											
	Field pH	6.70											
	Field Temp (C)	16.47											
0041376-05	Field Conductance	6250											
	Field pH	6.77											
	Field Temp (C)	14.85											
0041376-06	Field Conductance	5010											
	Field pH	6.36											
	Field Temp (C)	20.50											
0041376-07	Field Conductance	6770											
	Field pH	6.70											
	Field Temp (C)	16.47											



#### **ANALYTICAL RESULTS**

Lab Sample ID: 0041376-01

Description: MW1

Sample Collection Date Time: 04/06/2020 13:05 Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Arsenic	0.0019		mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Barium	0.087		mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Boron	1.69	D1, M3	mg/L	1.00	1.00	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:42	DMH
Cadmium	ND	U	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Calcium	27.7	D1, M3	mg/L	4.00	1.30	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:42	DMH
Chromium	0.0011	J	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Iron	1.57		mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:39	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Lithium	0.03		mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Mercury	ND	U	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Molybdenum	ND	U	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Selenium	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH
Sodium	206	D1, M3	mg/L	26.0	10.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:46	DMH
Thallium	0.0001	J	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:13	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	ND	U	mg/L	8	8	HACH 8000	04/10/2020 13:13	04/10/2020 13:13	ALT
Specific Conductance (Lab)	962		umhos/cm	1	1	2510 B-2011	04/09/2020 15:52	04/09/2020 15:52	JLW
pH (Lab)	7.50	H3	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:14	04/09/2020 16:14	GAT
Total Dissolved Solids	488		mg/L	50	50	2540 C-2011	04/13/2020 10:14	04/14/2020 12:26	MAG
Total Organic Carbon	1.0		mg/L	0.5		5310 C-2011	04/14/2020 10:27	04/14/2020 10:27	HMF

#### Subcontracted Analyses

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.340	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	0.468	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	0.808	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	6.5	mg/L	2.0	1.3	SW846 9056	04/16/2020 00:56	04/16/2020 00:56	CSC
Fluoride	0.5	mg/L	0.2	0.1	SW846 9056	04/16/2020 00:56	04/16/2020 00:56	CSC
Sulfate	21	mg/L	1	0.5	SW846 9056	04/16/2020 00:56	04/16/2020 00:56	CSC



#### **ANALYTICAL RESULTS**

Lab Sample ID: 0041376-02

Description: MW2

Sample Collection Date Time: 04/07/2020 11:40 Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Arsenic	0.0033		mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Barium	0.238		mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Boron	ND	U	mg/L	0.10	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:49	DMH
Cadmium	ND	U	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Calcium	145	D1	mg/L	40.0	13.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:55	DMH
Chromium	ND	U	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Iron	0.459		mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:49	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Lithium	0.007	J	mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Mercury	ND	U	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Molybdenum	0.002	J	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Selenium	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH
Sodium	66.5	D1	mg/L	26.0	10.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 16:55	DMH
Thallium	ND	U	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:16	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	12		mg/L	8	8	HACH 8000	04/10/2020 13:13	04/10/2020 13:13	ALT
Specific Conductance (Lab)	1530		umhos/cm	1	1	2510 B-2011	04/09/2020 15:53	04/09/2020 15:53	JLW
pH (Lab)	7.22	H3	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:15	04/09/2020 16:15	CML
Total Dissolved Solids	806		mg/L	50	50	2540 C-2011	04/13/2020 10:18	04/14/2020 12:26	MAG
Total Organic Carbon	1.0		mg/L	0.5		5310 C-2011	04/14/2020 10:48	04/14/2020 10:48	HMF

#### **Subcontracted Analyses**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.513	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	0.016	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	0.529	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	120	D	mg/L	100	64.0	SW846 9056	04/16/2020 01:29	04/16/2020 01:29	CSC
Fluoride	0.2		mg/L	0.2	0.1	SW846 9056	04/16/2020 01:12	04/16/2020 01:12	CSC
Sulfate	85	D	mg/L	50	25	SW846 9056	04/16/2020 01:29	04/16/2020 01:29	CSC



Sample Collection Date Time: 04/07/2020 13:55

#### **ANALYTICAL RESULTS**

Lab Sample ID: 0041376-03

Description: MW3A Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte Re	esult	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Arsenic	ND	U	mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Barium 0	.042		mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Boron	0.26		mg/L	0.10	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:11	DMH
Cadmium 0.0	0001	J	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Calcium	425	D1	mg/L	40.0	13.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:17	DMH
Chromium	ND	U	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Iron	ND	U	mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:11	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Lithium	0.68		mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Mercury	ND	U	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Molybdenum	ND	U	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Selenium	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH
Sodium	352	D1	mg/L	26.0	10.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:17	DMH
Thallium	ND	U	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:20	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	160		mg/L	8	8	HACH 8000	04/10/2020 13:14	04/10/2020 13:14	ALT
Specific Conductance	7660		umhos/cm	1	1	2510 B-2011	04/09/2020 15:54	04/09/2020 15:54	JLW
(Lab)									
pH (Lab)	7.07	H3	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:16	04/09/2020 16:16	CML
Total Dissolved Solids	5860		mg/L	50	50	2540 C-2011	04/13/2020 10:22	04/14/2020 12:26	MAG
Total Organic Carbon	ND	U	mg/L	0.5		5310 C-2011	04/14/2020 12:15	04/14/2020 12:15	HMF

#### **Subcontracted Analyses**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.603	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	0.460	_Sub	pCi/L			EPA 904.0 Radium	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	1.06	_Sub	pCi/L			Sum Calc EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	3220	D	mg/L	200	128	SW846 9056	04/16/2020 02:02	04/16/2020 02:02	CSC
Fluoride	0.5		mg/L	0.2	0.1	SW846 9056	04/16/2020 01:45	04/16/2020 01:45	CSC
Sulfate	1840	D	mg/L	100	50	SW846 9056	04/16/2020 02:02	04/16/2020 02:02	CSC



#### **ANALYTICAL RESULTS**

Lab Sample ID: 0041376-04

Description: MW4

Sample Collection Date Time: 04/07/2020 09:55 Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Arsenic	ND	U	mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Barium	0.022		mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Boron	0.83		mg/L	0.10	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:20	DMH
Cadmium	ND	U	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Calcium	464	D1	mg/L	40.0	13.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:27	DMH
Chromium	0.0008	J	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Iron	ND	U	mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:20	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Lithium	0.82		mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Mercury	0.0003	J	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Molybdenum	0.002	J	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Selenium	0.023		mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH
Sodium	433	D1	mg/L	26.0	10.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:27	DMH
Thallium	ND	U	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:24	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	44		mg/L	8	8	HACH 8000	04/10/2020 13:14	04/10/2020 13:14	ALT
Specific Conductance (Lab)	6460		umhos/cm	1	1	2510 B-2011	04/09/2020 15:55	04/09/2020 15:55	JLW
pH (Lab)	7.10	H3	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:17	04/09/2020 16:17	CML
Total Dissolved Solids	5120		mg/L	50	50	2540 C-2011	04/13/2020 10:26	04/14/2020 12:26	MAG
Total Organic Carbon	0.6		mg/L	0.5		5310 C-2011	04/14/2020 12:37	04/14/2020 12:37	HMF

#### **Subcontracted Analyses**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.476	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	0.787	_Sub	pCi/L			EPA 904.0 Radium	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	1.26	_Sub	pCi/L			Sum Calc EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	1560	D	mg/L	200	128	SW846 9056	04/16/2020 02:34	04/16/2020 02:34	CSC
Fluoride	0.2		mg/L	0.2	0.1	SW846 9056	04/16/2020 02:18	04/16/2020 02:18	CSC
Sulfate	4000	D	mg/L	100	50	SW846 9056	04/16/2020 02:34	04/16/2020 02:34	CSC



#### **ANALYTICAL RESULTS**

Lab Sample ID: 0041376-05

Description: MW5

Sample Collection Date Time: 04/07/2020 10:10 Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte Re	esult	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Arsenic	ND	U	mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Barium 0	0.014		mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Boron	0.25		mg/L	0.10	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:30	DMH
Cadmium	ND	U	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Calcium	464	D1	mg/L	40.0	13.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:36	DMH
Chromium	ND	U	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Iron	ND	U	mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:30	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Lithium	0.38		mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Mercury	ND	U	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Molybdenum	ND	U	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Selenium	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH
Sodium	217	D1	mg/L	26.0	10.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:36	DMH
Thallium	ND	U	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:28	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	463		mg/L	8	8	HACH 8000	04/10/2020 13:14	04/10/2020 13:14	ALT
Specific Conductance	5950		umhos/cm	1	1	2510 B-2011	04/09/2020 15:56	04/09/2020 15:56	JLW
(Lab) pH (Lab)	6.94	НЗ	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:18	04/09/2020 16:18	CML
Total Dissolved Solids	4960		mg/L	50	50	2540 C-2011	04/13/2020 10:30	04/14/2020 12:26	MAG
Total Organic Carbon	0.6		mg/L	0.5		5310 C-2011	04/16/2020 21:48	04/16/2020 21:48	HMF

#### **Subcontracted Analyses**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.302	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	1.18	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	1.48	_Sub	pCi/L			EPA 904.0 Radium	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	1860	D	mg/L	200	128	SW846 9056	04/16/2020 03:07	04/16/2020 03:07	CSC
Fluoride	0.2		mg/L	0.2	0.1	SW846 9056	04/16/2020 02:51	04/16/2020 02:51	CSC
Sulfate	3720	D	mg/L	100	50	SW846 9056	04/16/2020 03:07	04/16/2020 03:07	CSC



#### **ANALYTICAL RESULTS**

Lab Sample ID: 0041376-06

Description: MW6

Sample Collection Date Time: 04/06/2020 14:20 Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Arsenic	ND	U	mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Barium	0.011		mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Boron	0.19		mg/L	0.10	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:49	DMH
Cadmium	0.0001	J	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Calcium	458	D1	mg/L	40.0	13.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:55	DMH
Chromium	ND	U	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Iron	0.078	J	mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:49	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Lithium	0.05		mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Mercury	ND	U	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Molybdenum	ND	U	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Selenium	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH
Sodium	435	D1	mg/L	26.0	10.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:55	DMH
Thallium	ND	U	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:32	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	22		mg/L	8	8	HACH 8000	04/10/2020 13:14	04/10/2020 13:14	ALT
Specific Conductance	4960		umhos/cm	1	1	2510 B-2011	04/09/2020 15:57	04/09/2020 15:57	JLW
(Lab)									
pH (Lab)	6.76	H3	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:19	04/09/2020 16:19	CML
Total Dissolved Solids	4610		mg/L	50	50	2540 C-2011	04/13/2020 10:34	04/14/2020 12:26	MAG
Total Organic Carbon	2.0		mg/L	0.5		5310 C-2011	04/16/2020 22:11	04/16/2020 22:11	HMF

#### **Subcontracted Analyses**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.061	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	0.683	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	0.744	_Sub	pCi/L			EPA 904.0 Radium	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	181	D	mg/L	100	64.0	SW846 9056	04/16/2020 04:13	04/16/2020 04:13	CSC
Fluoride	0.4		mg/L	0.2	0.1	SW846 9056	04/16/2020 03:57	04/16/2020 03:57	CSC
Sulfate	4650	D	mg/L	100	50	SW846 9056	04/16/2020 12:57	04/16/2020 12:57	CSC



#### **ANALYTICAL RESULTS**

Lab Sample ID: **0041376-07**Description: **DUPLICATE**Sample Collection Date Time: 04/07/2020 10:20
Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Arsenic	ND	U	mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Barium	0.022		mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Boron	0.86		mg/L	0.10	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:58	DMH
Cadmium	ND	U	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Calcium	503	D1	mg/L	40.0	13.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 18:05	DMH
Chromium	0.0009	J	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Iron	ND	U	mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 17:58	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Lithium	0.84		mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Mercury	0.0003	J	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Molybdenum	0.003	J	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Selenium	0.025		mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH
Sodium	468	D1	mg/L	26.0	10.0	SW846 6010 B	04/09/2020 07:40	04/12/2020 18:05	DMH
Thallium	ND	U	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:36	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	62		mg/L	8	8	HACH 8000	04/10/2020 13:14	04/10/2020 13:14	ALT
Specific Conductance	6410		umhos/cm	1	1	2510 B-2011	04/09/2020 15:58	04/09/2020 15:58	JLW
(Lab)									
pH (Lab)	7.12	НЗ	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:20	04/09/2020 16:20	CML
Total Dissolved Solids	4700		mg/L	50	50	2540 C-2011	04/13/2020 10:38	04/14/2020 12:26	MAG
Total Organic Carbon	0.8		mg/L	0.5		5310 C-2011	04/16/2020 22:34	04/16/2020 22:34	HMF

#### **Subcontracted Analyses**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.371	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	1.10	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	1.47	_Sub	pCi/L			EPA 904.0 Radium	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	1480	D	mg/L	100	64.0	SW846 9056	04/21/2020 14:14	04/21/2020 14:14	CSC
Fluoride	0.2		mg/L	0.2	0.1	SW846 9056	04/16/2020 04:46	04/16/2020 04:46	CSC
Sulfate	4050	D	mg/L	100	50	SW846 9056	04/23/2020 12:44	04/23/2020 12:44	CSC



#### **ANALYTICAL RESULTS**

Lab Sample ID: **0041376-08**Description: **FIELD BLANK**Sample Collection Date Time: 04/07/2020 11:50
Sample Received Date Time: 04/07/2020 15:49

#### Metals by SW846 6000 Series Methods

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Antimony	ND	U	mg/L	0.005	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Arsenic	ND	U	mg/L	0.0010	0.0004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Barium	ND	U	mg/L	0.004	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Beryllium	ND	U	mg/L	0.0020	0.0010	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Boron	ND	U	mg/L	0.10	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 18:08	DMH
Cadmium	ND	U	mg/L	0.0010	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Calcium	ND	U	mg/L	0.40	0.13	SW846 6010 B	04/09/2020 07:40	04/12/2020 18:08	DMH
Chromium	ND	U	mg/L	0.0020	0.0006	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Cobalt	ND	U	mg/L	0.004	0.004	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Copper	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Iron	ND	U	mg/L	0.100	0.050	SW846 6010 B	04/09/2020 07:40	04/12/2020 18:08	DMH
Lead	ND	U	mg/L	0.002	0.0005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Lithium	ND	U	mg/L	0.02	0.005	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Mercury	ND	U	mg/L	0.0005	0.0002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Molybdenum	ND	U	mg/L	0.01	0.002	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Selenium	ND	U	mg/L	0.003	0.001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH
Sodium	ND	U	mg/L	0.26	0.10	SW846 6010 B	04/09/2020 07:40	04/12/2020 18:08	DMH
Thallium	ND	U	mg/L	0.0020	0.0001	SW846-6020 A	04/09/2020 07:40	04/12/2020 16:55	DMH

#### **Conventional Chemistry Analyses Madisonville**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chemical Oxygen Demand	ND	U	mg/L	8	8	HACH 8000	04/10/2020 13:15	04/10/2020 13:15	ALT
Specific Conductance	8		umhos/cm	1	1	2510 B-2011	04/09/2020 15:59	04/09/2020 15:59	JLW
(Lab)									
pH (Lab)	7.62	H3	Std. Units	0.10	0.10	4500-H+ B-2000	04/09/2020 16:21	04/09/2020 16:21	CML
Total Dissolved Solids	ND	U	mg/L	50	50	2540 C-2011	04/13/2020 10:42	04/14/2020 12:26	MAG
Total Organic Carbon	ND	U	mg/L	0.5		5310 C-2011	04/16/2020 22:57	04/16/2020 22:57	HMF

#### **Subcontracted Analyses**

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Radium-226	0.224	_Sub	pCi/L			EPA 903.1	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium-228	0.262	_Sub	pCi/L			EPA 904.0 Radium Sum Calc	04/30/2020 14:07	04/30/2020 14:09	RCW
Radium	0.486	_Sub	pCi/L			EPA 904.0 Radium	04/30/2020 14:07	04/30/2020 14:09	RCW

Analyte	Result	Flag	Units	MRL	MDL	Method	Prepared	Analyzed	Analyst
Chloride	ND	M1, U	mg/L	2.0	1.3	SW846 9056	04/16/2020 05:03	04/16/2020 05:03	CSC
Fluoride	ND	M1, U	mg/L	0.2	0.1	SW846 9056	04/16/2020 05:03	04/16/2020 05:03	CSC
Sulfate	ND	M1, U	mg/L	1	0.5	SW846 9056	04/16/2020 05:03	04/16/2020 05:03	CSC



#### Notes for work order 0041376

- Samples collected by MMLI personnel are done so in accordance with procedures set forth in MMLI field services SOPs.
- Results contained in this report are only representative of the samples received.
- MMLI does not provide interpretation of these results unless otherwise stated.
- All Waste Water analyses comply with methodology requirements of 40 CFR Part 136.
- All Drinking Water analyses comply with methodology requirements of 40 CFR Part 141.
- Unless otherwise noted, all quantitative results for soils are reported on a dry weight basis.
- The Chain of Custody document is included as part of this report.

See subcontractors report.

- All Library Search analytes should be regarded as tentative identification based on the presumptive evidence of the mass spectra. Concentrations reported are estimated values.

#### Qualifiers

\_Sub

D	Results reported from dilution.
D1	Sample required dilution due to high concentration of target analyte.
D2	Sample required dilution due to matrix interference.
НЗ	Sample received and analyzed past holding time.
J	Estimated value.
M1	Matrix spike recovery was high; the method control sample recovery was acceptable.
M3	The accuracy of the spike recovery value is reduced since the analyte concentration in the sample is disproportionate to spike level. The method control sample recovery was acceptable.
U	Target analyte was analyzed for, but was below detection limit (the value associated with the qualifier is the laboratory method detection limit in our LIMS system).

#### Standard Qualifiers/Acronymns

	,
MDL	Method Detection Limit
MRL	Minimum Reporting Limit
ND	Not Detected
LCS	Laboratory Control Sample
MS	Matrix Spike
MSD	Matrix Spike Duplicate

DUP Sample Duplicate
% Rec Percent Recovery

RPD Relative Percent Difference

> Greater than < Less than





## Metals by SW846 6000 Series Methods - Quality Control

		Domontin :		Cmilea	Causas		0/ DEC		DDD	
Amelida	Daguit	Reporting	l luita	Spike	Source	0/ DEC	%REC	DDD	RPD	Natas
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B015276 - EPA 200.2										
Blank (B015276-BLK1)										
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0	5									
Molybdenum	ND	0.01	mg/L							U
Antimony	ND	0.005	mg/L							U
Mercury	ND	0.0005	mg/L							U
Arsenic	ND	0.0010	mg/L							U
Barium	ND	0.004	mg/L							U
Beryllium	ND	0.0020	mg/L							U
Cadmium	ND	0.0010	mg/L							U
Chromium	ND	0.0020	mg/L							U
Cobalt	ND	0.004	mg/L							U
Copper	ND	0.003	mg/L							U
Lead	ND	0.002	mg/L							U
Lithium	ND	0.02	mg/L							U
Selenium	ND	0.003	mg/L							U
Thallium	ND	0.0020	mg/L							U
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:3 Boron	ND	0.10	mg/L							U
Calcium	ND	0.40	mg/L							U
Iron	ND	0.100	mg/L							U
Sodium	ND	0.26	mg/L							U
LCS (B015276-BS1)										
EGG (D0:02:0-D0:)										
·	9									
·	9 0.07	0.01	mg/L	0.0625		105	85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum		0.01 0.005	mg/L mg/L	0.0625 0.0625		105 109	85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0	0.07		_							
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony	0.07 0.068	0.005	mg/L	0.0625		109	85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury	0.07 0.068 0.0025	0.005 0.0005	mg/L mg/L	0.0625 0.00250		109 98.3	85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic	0.07 0.068 0.0025 0.0645	0.005 0.0005 0.0010	mg/L mg/L mg/L	0.0625 0.00250 0.0625		109 98.3 103	85-115 85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic Barium	0.07 0.068 0.0025 0.0645 0.062	0.005 0.0005 0.0010 0.004	mg/L mg/L mg/L mg/L	0.0625 0.00250 0.0625 0.0625		109 98.3 103 99.5	85-115 85-115 85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic Barium Beryllium	0.07 0.068 0.0025 0.0645 0.062 0.0613	0.005 0.0005 0.0010 0.004 0.0020	mg/L mg/L mg/L mg/L mg/L	0.0625 0.00250 0.0625 0.0625 0.0625		109 98.3 103 99.5 98.1	85-115 85-115 85-115 85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic Barium Beryllium Cadmium	0.07 0.068 0.0025 0.0645 0.062 0.0613 0.0621	0.005 0.0005 0.0010 0.004 0.0020 0.0010	mg/L mg/L mg/L mg/L mg/L mg/L	0.0625 0.00250 0.0625 0.0625 0.0625 0.0625		109 98.3 103 99.5 98.1 99.4	85-115 85-115 85-115 85-115 85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic Barium Beryllium Cadmium Chromium	0.07 0.068 0.0025 0.0645 0.062 0.0613 0.0621 0.0641	0.005 0.0005 0.0010 0.004 0.0020 0.0010 0.0020	mg/L mg/L mg/L mg/L mg/L mg/L	0.0625 0.00250 0.0625 0.0625 0.0625 0.0625		109 98.3 103 99.5 98.1 99.4 103	85-115 85-115 85-115 85-115 85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic Barium Beryllium Cadmium Chromium Cobalt Copper	0.07 0.068 0.0025 0.0645 0.062 0.0613 0.0621 0.0641	0.005 0.0005 0.0010 0.004 0.0020 0.0010 0.0020 0.004	mg/L mg/L mg/L mg/L mg/L mg/L mg/L	0.0625 0.00250 0.0625 0.0625 0.0625 0.0625 0.0625 0.0625		109 98.3 103 99.5 98.1 99.4 103 102	85-115 85-115 85-115 85-115 85-115 85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic Barium Beryllium Cadmium Chromium Cobalt Copper Lead	0.07 0.068 0.0025 0.0645 0.062 0.0613 0.0621 0.0641 0.064 0.060	0.005 0.0005 0.0010 0.004 0.0020 0.0010 0.0020 0.004 0.003	mg/L mg/L mg/L mg/L mg/L mg/L mg/L mg/L	0.0625 0.00250 0.0625 0.0625 0.0625 0.0625 0.0625 0.0625 0.0625		109 98.3 103 99.5 98.1 99.4 103 102 95.6	85-115 85-115 85-115 85-115 85-115 85-115 85-115 85-115			
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020 16:0 Molybdenum Antimony Mercury Arsenic Barium Beryllium Cadmium Chromium Cobalt	0.07 0.068 0.0025 0.0645 0.062 0.0613 0.0621 0.0641 0.064 0.060	0.005 0.0005 0.0010 0.004 0.0020 0.0010 0.0020 0.004 0.003 0.002	mg/L mg/L mg/L mg/L mg/L mg/L mg/L mg/L	0.0625 0.00250 0.0625 0.0625 0.0625 0.0625 0.0625 0.0625 0.0625		109 98.3 103 99.5 98.1 99.4 103 102 95.6 98.7	85-115 85-115 85-115 85-115 85-115 85-115 85-115 85-115 85-115			





### Metals by SW846 6000 Series Methods - Quality Control

	Metals by S		OCITES IV	iotilous - (	guanty 0					
		Reporting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B015276 - EPA 200.2										
LCS (B015276-BS2)										
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020	0 16:36									
Boron	0.12	0.10	mg/L	0.125		94.1	85-115			
Calcium	5.92	0.40	mg/L	6.25		94.8	85-115			
Iron	6.27	0.100	mg/L	6.25		100	85-115			
Sodium	6.12	0.26	mg/L	6.25		97.9	85-115			
Matrix Spike (B015276-MS1)	Source: 0041376-0	1								
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020	0 16:59									
Molybdenum	0.06	0.01	mg/L	0.0625	ND	102	80-120			
Antimony	0.066	0.005	mg/L	0.0625	ND	106	80-120			
Mercury	0.0023	0.0005	mg/L	0.00250	ND	93.6	80-120			
Arsenic	0.0634	0.0010	mg/L	0.0625	0.0019	98.3	80-120			
Barium	0.150	0.004	mg/L	0.0625	0.087	101	80-120			
Beryllium	0.0547	0.0020	mg/L	0.0625	ND	87.4	80-120			
Cadmium	0.0562	0.0010	mg/L	0.0625	ND	89.9	80-120			
Chromium	0.0656	0.0020	mg/L	0.0625	0.0011	103	80-120			
Cobalt	0.063	0.004	mg/L	0.0625	ND	101	80-120			
Copper	0.056	0.003	mg/L	0.0625	ND	89.6	80-120			
Lead	0.056	0.002	mg/L	0.0625	ND	90.2	80-120			
Lithium	0.09	0.02	mg/L	0.0625	0.03	95.1	80-120			
Selenium	0.055	0.003	mg/L	0.0625	ND	88.1	80-120			
Thallium	0.0579	0.0020	mg/L	0.0625	0.0001	92.5	80-120			
Matrix Spike (B015276-MS2)	Source: 0041376-0	1								
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020	0 16:58									
Boron	1.85	1.00	mg/L	0.125	1.69	132	80-120			D2, M3
Calcium	34.4	4.00	mg/L	6.25	27.7	106	80-120			D2
Iron	7.68	1.00	mg/L	6.25	1.57	97.8	80-120			D2
Sodium	205	2.60	mg/L	6.25	206	NR	80-120			D2, M3
Matrix Spike Dup (B015276-MSD1)	Source: 0041376-0	1								
Prepared: 4/9/2020 7:40, Analyzed: 4/12/2020	0 17:03									
Antimony	0.071	0.005	mg/L	0.0625	ND	114	80-120	7.69	20	
Mercury	0.0025	0.0005	mg/L	0.00250	ND	99.2	80-120	5.81	20	
Molybdenum	0.07	0.01	mg/L	0.0625	ND	107	80-120	4.09	20	
Arsenic	0.0677	0.0010	mg/L	0.0625	0.0019	105	80-120	6.64	20	
Barium	0.157	0.004	mg/L	0.0625	0.087	111	80-120	4.16	20	
Beryllium	0.0585	0.0020	mg/L	0.0625	ND	93.6	80-120	6.82	20	
Cadmium	0.0610	0.0010	mg/L	0.0625	ND	97.6	80-120	8.15	20	
Chromium	0.0684	0.0020	mg/L	0.0625	0.0011	108	80-120	4.12	20	
Cobalt	0.066	0.004	mg/L	0.0625	ND	106	80-120	4.34	20	
Copper	0.059	0.003	mg/L	0.0625	ND	94.0	80-120	4.78	20	
Lead	0.061	0.002	mg/L	0.0625	ND	97.1	80-120	7.36	20	
Lithium	0.09	0.02	mg/L	0.0625	0.03	98.1	80-120	2.10	20	
			-							
Selenium	0.061	0.003	mg/L	0.0625	ND	97.1	80-120	9.79	20	





## Metals by SW846 6000 Series Methods - Quality Control

					•					
		Reporting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B015276 - EPA 200.2										
Matrix Spike Dup (B015276-MSD2)	Source: 0041376-0	1								
Prepared: 4/9/2020 7:40, Analyzed: 4/12/202	0 18:11									
Boron	1.80	1.00	mg/L	0.125	1.69	88.6	80-120	2.95	20	D2
Calcium	35.3	4.00	mg/L	6.25	27.7	121	80-120	2.64	20	D2, M3
Iron	8.28	1.00	mg/L	6.25	1.57	107	80-120	7.50	20	D2
Sodium	208	2.60	mg/L	6.25	206	23.5	80-120	1.56	20	D2, M3
Post Spike (B015276-PS1)	Source: 0041376-0	1	-							
Prepared: 4/9/2020 7:40, Analyzed: 4/12/202										
Antimony	65.3		ug/L	62.5	0.087	104	75-125			
Mercury	2.49		ug/L	2.50	0.0595	97.1	75-125			
Molybdenum	62.9		ug/L	62.5	1.02	99.0	75-125			
Arsenic	63.0		ug/L	62.5	1.92	97.7	75-125			
Barium	153		ug/L	62.5	87.2	105	75-125			
Beryllium	55.2		ug/L	62.5	-0.0177	88.4	75-125			
Cadmium	57.4		ug/L	62.5	0.0329	91.8	75-125			
Chromium	63.2		ug/L	62.5	1.10	99.4	75-125			
Cobalt	61.3		ug/L	62.5	0.695	96.9	75-125			
Copper	54.1		ug/L	62.5	-2.87	86.6	75-125			
Lead	56.6		ug/L	62.5	0.013	90.6	75-115			
Lithium	85.9		ug/L	62.5	28.0	92.7	75-125			
Selenium	56.3		ug/L	62.5	0.072	89.9	75-125			
Thallium	57.4		ug/L	62.5	0.118	91.7	75-125			
Post Spike (B015276-PS2)	Source: 0041376-0	1								
Prepared: 4/9/2020 7:40, Analyzed: 4/12/202	0 18:14									
Boron	1820		ug/L	125	1690	107	75-125			D2
Calcium	33800		ug/L	6250	27700	96.6	75-125			D2
Iron	7590		ug/L	6250	1570	96.4	75-125			D2
Sodium	202000		ug/L	6250	206000	NR	75-125			D2, M3





		-	-			-				
		Reporting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B015432 - Default Prep Wet Chem										
Blank (B015432-BLK1)										
Prepared: 4/14/2020 1:48, Analyzed: 4/14/2020	1:48									
Total Organic Carbon	ND	0.5	mg/L							U
LCS (B015432-BS1)										
Prepared: 4/14/2020 2:09, Analyzed: 4/14/2020	2:09									
Total Organic Carbon	4.8	0.5	mg/L	5.00		95.5	80-120			
Duplicate (B015432-DUP1)	ource: 0040539-01									
Prepared: 4/14/2020 7:34, Analyzed: 4/14/2020	7:34									
Total Organic Carbon	2.0	0.5	mg/L		2.0			1.22	25	
Duplicate (B015432-DUP2)	ource: 0041286-01									
Prepared: 4/14/2020 12:59, Analyzed: 4/14/2020	12:59									
Total Organic Carbon	1.1	0.5	mg/L		1.1			5.36	25	
Matrix Spike (B015432-MS1)	ource: 0040539-02									
Prepared: 4/14/2020 7:55, Analyzed: 4/14/2020	7:55									
Total Organic Carbon	3.6	0.5	mg/L	2.50	1.1	102	80-120			
Matrix Spike (B015432-MS2)	ource: 0041286-02									
Prepared: 4/14/2020 13:20, Analyzed: 4/14/2020	13:20									
Total Organic Carbon	5.9	0.5	mg/L	5.00	0.9	100	80-120			
Batch B015433 - Default Prep Wet Chem										
Blank (B015433-BLK2)										
Prepared: 4/16/2020 20:16, Analyzed: 4/16/2020	20:16									
Total Organic Carbon	ND	0.5	mg/L							U
LCS (B015433-BS2)										
Prepared: 4/16/2020 20:39, Analyzed: 4/16/2020	20:39									
Total Organic Carbon	4.9	0.5	mg/L	5.00		98.4	80-120			
Duplicate (B015433-DUP1) Se	ource: 0041409-01									
Prepared: 4/14/2020 23:44, Analyzed: 4/14/2020	23:44									
Total Organic Carbon	1.0	0.5	mg/L		1.0			2.11	25	





			-							
		Reporting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B015433 - Default Prep Wet Chem										
Duplicate (B015433-DUP3)	Source: 0042383-0	1								
Prepared: 4/15/2020 12:23, Analyzed: 4/15/	2020 12:23									
Total Organic Carbon	2.2	0.5	mg/L		2.2			1.81	25	
Matrix Spike (B015433-MS1)	Source: 0041409-02	2								
Prepared: 4/15/2020 0:07, Analyzed: 4/15/2	020 0:07									
Total Organic Carbon	3.4	0.5	mg/L	2.50	0.9	99.6	80-120			
Matrix Spike (B015433-MS3)	Source: 0042383-02	2RE1								
Prepared: 4/15/2020 12:46, Analyzed: 4/15/	2020 12:46									
Total Organic Carbon	6.4	0.5	mg/L	5.00	1.4	101	80-120			
Batch B015469 - Default Prep Wet Chem										
LCS (B015469-BS1)										
Prepared: 4/9/2020 16:08, Analyzed: 4/9/20	20 16:08									
pH (Lab)	7.98		Std. Units	8.00		99.8	98.8-101.2			
LCS (B015469-BS2)										
Prepared: 4/9/2020 16:26, Analyzed: 4/9/20	20 16:26									
pH (Lab)	8.04		Std. Units	8.00		100	98.8-101.2			
Duplicate (B015469-DUP1)	Source: 0041388-0	2								
Prepared: 4/9/2020 16:24, Analyzed: 4/9/20	20 16:24									
pH (Lab)	7.29	0.10	Std. Units		7.27			0.275	10	
Duplicate (B015469-DUP2)	Source: 0060028-0	1								
Prepared: 4/9/2020 16:34, Analyzed: 4/9/20	20 16:34									
pH (Lab)	7.77	0.10	Std. Units		7.76			0.129	10	
Batch B015470 - Default Prep Wet Chem										
Blank (B015470-BLK1)										
Prepared: 4/9/2020 15:46, Analyzed: 4/9/20.	20 15:46									
Specific Conductance (Lab)	ND	1	umhos/cm							U



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	R	eporting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B015470 - Default Prep Wet Chem										
LCS (B015470-BS1)										
Prepared: 4/9/2020 15:47, Analyzed: 4/9/2	020 15:47									
Specific Conductance (Lab)	1410		umhos/cm	1410		99.9	80-120			
Duplicate (B015470-DUP1)	Source: 0042630-01									
Prepared: 4/9/2020 16:02, Analyzed: 4/9/2	020 16:02									
Specific Conductance (Lab)	202	1	umhos/cm		202			0.148	1.24	
Batch B015517 - Default Prep Wet Chem										
Blank (B015517-BLK1)										
Prepared: 4/10/2020 13:09, Analyzed: 4/10	0/2020 13:09									
Chemical Oxygen Demand	ND	8	mg/L							U
LCS (B015517-BS1)										
Prepared: 4/10/2020 13:09, Analyzed: 4/10	0/2020 13:09									
Chemical Oxygen Demand	116	8	mg/L	125		93.0	90-110			
Duplicate (B015517-DUP1)	Source: 0041376-01									
Prepared: 4/10/2020 13:18, Analyzed: 4/10	0/2020 13:18									
Chemical Oxygen Demand	ND	8	mg/L		ND				25	U
Matrix Spike (B015517-MS1)	Source: 0041376-01									
Prepared: 4/10/2020 13:18, Analyzed: 4/10	0/2020 13:18									
Chemical Oxygen Demand	262	8	mg/L	250	ND	105	90-110			
Matrix Spike Dup (B015517-MSD1)	Source: 0041376-01									
Prepared: 4/10/2020 13:18, Analyzed: 4/10	0/2020 13:18									
Chemical Oxygen Demand	256	8	mg/L	250	ND	102	90-110	2.46	10	
Batch B016032 - Default Prep Wet Chem										
Blank (B016032-BLK1)										
Prepared: 4/13/2020 9:34, Analyzed: 4/14	/2020 12:26									
Total Dissolved Solids	ND	25	mg/L							U



Pace Analytical Services, LLC P.O. Box 907 Madisonville, KY 42431 270.821.7375

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	Re	porting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B016032 - Default Prep Wet Chem										
LCS (B016032-BS1)										
Prepared: 4/13/2020 9:38, Analyzed: 4/14	/2020 12:26									
Total Dissolved Solids	1480	25	mg/L	1500		98.7	80-120			
Duplicate (B016032-DUP1)	Source: 0040819-01									
Prepared: 4/13/2020 10:50, Analyzed: 4/14	4/2020 12:26									
Total Dissolved Solids	206	50	mg/L		226			9.26	10	
Duplicate (B016032-DUP2)	Source: 0041376-08									
Prepared: 4/13/2020 10:54, Analyzed: 4/14/2020 12:26										
Total Dissolved Solids	ND	50	mg/L		ND				10	U





## Ion Chromatography Madisonville - Quality Control

		Reporting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B016360 - Default Prep IC										
Blank (B016360-BLK1)										
Prepared: 4/16/2020 0:39, Analyzed: 4/16/2020 0	:39									
Chloride	ND	2.0	mg/L							U
Fluoride	ND	0.2	mg/L							U
Sulfate	ND	1	mg/L							U
LCS (B016360-BS1)										
Prepared: 4/16/2020 0:23, Analyzed: 4/16/2020 0	:23									
Fluoride	9.5		mg/L	10.0		95.0	90-110			
Chloride	9.5		mg/L	10.0		94.9	90-110			
Sulfate	10		mg/L	10.0		98.1	90-110			
Matrix Spike (B016360-MS1) Sou	ırce: 0041376-	08								
Prepared: 4/16/2020 5:20, Analyzed: 4/16/2020 5	:20									
Fluoride	13.2		mg/L	10.0	0.0	132	75-125			M1
Chloride	13.1		mg/L	10.0	0.1	130	75-125			M1
Sulfate	14		mg/L	10.0	0.1	139	75-125			M1
Matrix Spike Dup (B016360-MSD1) Sou	ırce: 0041376-	08								
Prepared: 4/16/2020 5:37, Analyzed: 4/16/2020 5	:37									
Chloride	12.3		mg/L	10.0	0.1	122	75-125	6.11	15	
Fluoride	12.5		mg/L	10.0	0.0	125	75-125	5.37	15	
Culfata	13		mg/L	10.0	0.1	125	75-125	10.7	15	
Sulfate										
Batch B016418 - Default Prep IC										
Batch B016418 - Default Prep IC										
Batch B016418 - Default Prep IC Blank (B016418-BLK1)	2:41									
Batch B016418 - Default Prep IC Blank (B016418-BLK1)	2:41 ND	1	mg/L							U
Batch B016418 - Default Prep IC  Blank (B016418-BLK1)  Prepared: 4/16/2020 12:41, Analyzed: 4/16/2020 1:  Sulfate		1	mg/L							U
Batch B016418 - Default Prep IC  Blank (B016418-BLK1)  Prepared: 4/16/2020 12:41, Analyzed: 4/16/2020 1:	ND	1	mg/L							U





## Ion Chromatography Madisonville - Quality Control

g.up.yg.up.y control										
	F	Reporting		Spike	Source		%REC		RPD	
Analyte	Result	Limit	Units	Level	Result	%REC	Limits	RPD	Limit	Notes
Batch B016418 - Default Prep IC										
Matrix Spike (B016418-MS1)	Source: 0043228-02									
Prepared: 4/16/2020 14:36, Analyzed: 4	4/16/2020 14:36									
Sulfate	30		mg/L	10.0	17	121	75-125			
Matrix Spike Dup (B016418-MSD1)	Source: 0043228-02									
Prepared: 4/16/2020 14:52, Analyzed: 4	4/16/2020 14:52									
Sulfate	30		mg/L	10.0	17	130	75-125	2.87	15	M1
Certified Analyses included in this Rep	port									
Analyte	Certifications									
2510 B-2011 in Water  Specific Conductance (Lab)	KY Drinking Water Mdv (	00030)								
2540 C-2011 in Water										
Total Dissolved Solids	KY Drinking Water Mdv (	00030)								
4500-H+ B-2000 in Water										
pH (Lab)	KY Drinking Water Mdv (00030) TN Drinking Water (02819)									
5310 C-2011 in Water										
Total Organic Carbon	KY Drinking Water Mdv (	KY Drinking Water Mdv (00030)								

HACH 8000 in Water

Chemical Oxygen Demand KY Wastewater Mdv (00030)

SW846 6010 B in Water

	Sample Acceptance Checklist for Work Order 0041376
Shipped By: Client	Temperature: 1.90° Celcius
Condition	
Check if Custody Seals are Present/Intact	
Check if Custody Signatures are Present	☑
Check if Collector Signature Present	☑
Check if bottles are intact	oxdot
Check if bottles are correct	☑
Check if bottles have sufficient volume	☑
Check if samples received on ice	☑
Check if VOA headspace is acceptable	
Check if samples received in holding time.	
Check if samples are preserved properly	

# **Chain of Custody**



	Scrieduleu	101. 04/01/2020		51 # 36 a 6   11 & 1 2 m m p at m (1 p #13
Client: Big Rivers Electric Corporation Reid/Green Station  Project: Green Landfill Semiannual Groundwater	Report To: Big Rivers Elect Station Chad Phillips PO Box 24 Henderson, KY	ric Corporation Reid/Green	Invoice To: Big Rivers E Chad Phillip PO Box 24 Henderson,	Electric Corporation Reid/Green Stations
Please Print Legibly	Phone: (270) 84 PWS ID#: State:	<u>4-6000</u> LY	PO#:	
Collected by (Signature):			<del>-</del>	ance Monitoring? Yes No
required mito				es Chlorinated? Yes No
*For composite samples please indicate begin time, end Influent: Start Date Start time			Tomp (oC)	
Effluent: Start Date Start time				
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr): Bottle a	and Preservative	n E E Sample Description	Composite	Sample Analysis Requested
<del></del>	c 500mL pH<2 1 w/HNO3	MW1	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum
0041376-01 B 4/6/20 1305 Plastic	tion Check: pH : c 500mL pH<2 1 w/HNO3	MW1	g/c	Tot 6020 Sodium Tot 6010B  Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Cobalt Tot 6020 Chromium Tot 6020 Boron Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B
11/1/20 1200	tion Check: pH : Plastic 1L 1	·	g/c	pH (Lab) Conductivity (Lab) TDS Sulfate 9056 Chloride 9056 Fluoride 9056
Preservation Check Performed by:CLH	-			
Field data collected by: Phillip Hill	Date (mm/dd/yy) _	4/6/20 Time (24 hr)	1205	
pH 7.22 Cond (umho) 0.867	Res Cl (mg/L) _	Tot CI (mg/L)	Fre	e CI (mg/L)
		DO (mg/L)		i i
Flow (MGD) or (CFS) o				
Relinquished by: (Signature)  The Sund	Received by: (Signal)	(1.9°	Date (mgn/	dd/yy) Time (24 hr) 20 14143 20 1549
PACE- Check here if trip charge applied to as	sociated COC	Printed: 3	3/25/2020 2:51:	08PM

# **Chain of Custody**



Client: Big Rivers Electric ( Reid/Green Station	Corporation		ic Corporation Reid/Green	Invoice To: Big Rivers E	lectric Corporation Reid/Green Station		
Project: Green Landfill Sen	niannual Groundwater	Station Chad Phillips PO Box 24 Henderson, KY 4	2419	Chad Phillips PO Box 24 Henderson, KY 42419			
·		Phone: (270) 844 PWS ID#:	i-6000	PO#:			
Please Print Legibly		State: K	<u>Y</u>	Quote#			
Collected by (Signature):	7 required in	formation"		Compli	ance Monitoring? Yes No		
*For composite samples plea	se indicate begin time, er	id time and temp(oC) a	t end time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date	Start time	_ End Date	End Time1	emp (oC)			
Effluent: Start Date	Start time	_ End Date	End Time	Temp (oC)	<del></del>		
0071010	Collection	and Preservative	Sample Description	Composite			
Sample ID# 0041376-01 D 4/6/20		tic 500mL pH<2 1 w/H2SO4 vation Check: pH :	MW1	g/c	Sample Analysis Requested COD TOC		
0041376-01 E 4/6/20	F	: 1L pH<2 w/HNO3 1 lad 226 (Sub) vation Check: pH :	MW1	g/c	Radium 226 (sub)		
0041376-01 F 4/4/20		: 1L pH<2 w/HNO3 1 lad 228 (Sub) vation Check: pH :	MW1	g/c	Radium 228 (sub)		
0041376-01 G <u>4/6/20</u>	Freser	: 1L pH<2 w/HNO3 1 lad 228 (Sub) vation Check: pH :	MW1	g/c	Radium 228 (sub)		
0041376-01 H <u>4/4/30</u>		3 250mL pH<2 1 w/H2SO4 vation Check: pH : _~	MW1	g/c	TOC		
Preservation Check Perforn	ned by: <u>CLH</u>				•		
Field data collected by: _ P	hillip Hill	Date (mm/dd/yy)	Time (24 hr)	····	-		
<sub>рн</sub> <u>7.аа</u> с	cond <del>(umho)</del> 0.867	Res Cl (mg/L)	Tot Cl (mg/L)	Fre	e CI (mg/L)		
Temp (oC) 18:23 o	(oF)	Static Water Level	DO (mg/L)	To	urb. (NTU)		
Flow (MGD) or	(CFS)	or (g/min)					
Relinquished by: (Signature)	L .	Received by: (Signato	re)	Date (mm/			
Drai Sus	<u></u>	M		4-7-	20 1549		

# **Chain of Custody**



	Scrieduled	101. 04/01/2020		\$1 S 29 2 E   F  D    M   M   B   M   K    M   M		
Client: Big Rivers Electric Corporation Reid/Green Station  Project: Green Landfill Semiannual Groundwater	Report To: Big Rivers Elect Station Chad Phillips PO Box 24 Henderson, KY	ric Corporation Reid/Green 42419	Invoice To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips PO Box 24 Henderson, KY 42419			
	Phone: (270) 84	<u>4-6000</u>	PO#:			
Please Print Legibly	State: K	<u> </u>	Quote#	·······		
Collected by (Signature):	rmation		Compli	ance Monitoring? Yes V No		
*For composite samples please indicate begin time, end	time and temp(oC) a	at end time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date Start time	End Date	End Time	Temp (oC)			
Effluent: Start Date Start time	End Date	End Time	Temp (oC)			
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr): Bottle a	nd Preservative	Sample Description	Composite	Sample Analysis Requested		
0041376-02 A 4/7/2 1/40 Plastic	c 500mL pH<2 1 w/HNO3	MW2	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
0041376-02 B 4/7/20 1/40 Plastic	500mL pH<2 1 w/HNO3	MW2	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
0041376-02 C 4/2/20 //40 F	Plastic 1L 1	MW2	g/c	pH (Lab) Conductivity (Lab) TDS Sulfate 9056 Chloride 9056 Fluoride		
_ <del>, ,</del>	: 500mL pH<2 1 v/H2SO4 tion Check: pH :		g/c	9056 COD TOC		
Preservation Check Performed by:CLH						
Field data collected by: Phillip Hill		4/7/ar_ Time (24 hr) _				
pH <u>6.92</u> Cond (umito) <u>1.59</u> Temp (oC) <u>1686</u> or (oF) SI				11		
Flow (MGD) , or (CFS) or		DO (mg/L)		3rb. (N1O)		
Relinguished M. (Signature)	Received by: (Signature)	ire)	Date (mm/ 4/7/ 4-7-	ao 1443		

# **Chain of Custody**



	001104410411					
Client: Big Rivers Electric Corporation Reid/Green Station  Project: Green Landfill Semiannual Groundwa	Station Chad Phillips	Corporation Reld/Green	Invoice To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips			
Project. Green Landini Semiannoar Groundwe	PO Box 24 Henderson, KY 424	<b>4</b> 19	PO Bex 24 Henderson,	KY 42419		
	Phone: (270) 844-6		PO#:			
Please Print Legibly	PWS ID#: State:	Y	Quote#	<u> </u>		
Collected by (Signature): 7/1,	17/			ance Monitoring? Yes V No		
*For composite samples please indicate begin tim	edAniormation*	and time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date Start time			emp (oC)			
Effluent: Start Date Start time		<del>,,,,</del>		<del></del>		
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr):	Souther and Preservative	Caracia Danasiatian	0			
Sample ID#		Sample Description	Composite	Sample Analysis Requested		
Pr	Pastic 1L pH<2 w/HNO3 1 Rad 226 (Sub) reservation Check: pH :	MW2	g/c	Radium 226 (sub)		
	Plastic 1L pH<2 w/HNO3 1 Rad 228 (Sub) reservation Check: pH :	MW2	g/c	Radium 228 (sub)		
	Plastic 1L pH<2 w/HNO3 1 Rad 228 (Sub) reservation Check: pH :	MW2	g/c	Radium 228 (sub)		
0041376-02 H <u>4/7/20</u> //40 Pr	AG 250mL pH<2 1 w/H2SO4 reservation Check: pH :	MW2	g/c	TOC		
0041376-03 A 4/7/24 1355	Plastic 500mL pH<2 1 w/HNO3	MW3A	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
Preservation Check Performed by:L	4					
Field data collected by: Phillip H	Date (mm/dd/yv) 4/	7/2 Time (24 hr) _/	746			
pH 6,92 Cond (umho) 1,5	6	Tot Cf (mg/L)		e Cl (mg/L)		
Temp (oC) 16.86 or (oF)	Static Water Level			il		
Flow (MGD) or (CFS)	or (g/min)	— /				
Relinquished by (Signature)	Received by: (Signature)		Date (mm/	1443		
- Jan Jud			<u>4-7-</u>	7549		
PACE- Check here if trip charge applie	d to associated COC	Printed: 3/	25/2020 2:51:	OBPM DOCATO		

# **Chain of Custody**



	Schedule	d 101. <u>04/01/2020</u>	J			
Client: Big Rivers Electric Corporation Reid/Green Station  Project: Green Landfill Semiannual Groundwater	Report To: Big Rivers Ele Station Chad Phillips PO Box 24	ctric Corporation Reid/Green	Invoice To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips PO Box 24			
	Henderson, K	Y 42419	Henderson,	KY 42419		
•	Phone: <u>(270) (</u> PWS ID#:	344-6000 V V	PO#:	<del>.</del>		
Please Print Legibly	State:					
Collected by (Signature):	formation*		Compli	iance Monitoring? YesNo		
*For composite samples please indicate begin time, en	d time and temp(oC	) at end time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date Start time	End Date	End Time	Temp (oC)			
Effluent: Start Date Start time	End Date	End Time	Temp (oC)			
	and Preservative	Outginers Sample Description	Composite			
Sample !D# 0041376-03 B 4/2/20 1353 Plas		<del></del>	•	Sample Analysis Requested		
0041376-03 B <u>4/7/20</u> 1355 Plas	stic 500mL pH<2 w/HNO3	1 MW3A	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
, ,	vation Check: pH:	<u> </u>		101 0020 000101/1 101 001 102		
0041376-03 C 4/7/20 1355	Plastic 1L	1 MW3A	g/c	pH (Lab) Conductivity (Lab) TDS Sulfate 9056 Chloride 9056 Fluoride 9056		
<del></del>	stic 500mt. pH<2 w/H2SO4 vation Check: pH:	1 MW3A	g/c	COD TOC		
F	: 1L pH<2 w/HNO3 Rad 226 (Sub) vation Check: pH :		g/c	Radium 226 (sub)		
0041376-03 F 4/7/30 1355 Plastic	: 1L pH<2 w/HNO3 Rad 228 (Sub) vation Check; pH :		g/¢	Radium 228 (sub)		
Preservation Check Performed by:	<u> </u>					
Field data collected by: Phillip Hil	_ Date (mm/dd/yy)	4/7/20 Time (24 hr)	1350			
pH 6.86 Cond (umho) 8.09		Tot CI (mg/L)		ee CI (mg/L)		
1/ 40		DO (mg/L) _				
Flow (MGD) or (CFS)			_			
Relinguished by: (Signature) /	Received by: (Sign	ature)	Date (mm/	/dg/yy) Time (24 hr)		
M. MIX.	Dow S.	- 1	4/2	120 1442		
Tim' Sand	2	10	4-7-	<u>-</u>		
PACE- Check here if trip charge applied to	associated COC	Printed:	3/25/2020 2:51	:08PM		

Chain of Custody
Scheduled for: 04/01/2020



	<del></del>		i			
Client: Big Rivers Electric Corporation Reid/Green Station Project: Green Landfill Semiannual Groundwate	Station Chad Phillips PO Box 24 Henderson, KY		Involce To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips PO Box 24 Henderson, KY 42419			
	Phone: (270) 84 PWS ID#:	4-6000 1/	PO#:			
Please Print Legibly	State:	<u>,r</u>	Quote#			
Collected by (Signature): required	Information*		Compli	iance Monitoring? Yes V No		
*For composite samples please indicate begin time,	end time and temp(oC) a	t end time below:	Sample	es Chiorinated? Yes No		
Influent: Start Date Start time	End Date	End Time	Temp (oC)	***************************************		
Effluent: Start Date Start time	End Date	End Time	Temp (oC)			
Sample ID#	ttle and Preservative	Sample Description	Composite	Sample Analysis Requested		
	stic 1L pH<2 w/HNO3 1 Rad 228 (Sub)	MW3A	g/c	Radium 228 (sub)		
0041376-03 H <u>4/7/20 1355</u> Ргез	ervation Check: pH : AG 250mL pH<2 t w/H2SO4 ervation Check: pH :	MW3A	g/c	тос		
	lastic 500mL pH<2 1 w/HNO3 ervation Check: pH :		g / c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
Field data collected by: Phillip Hill						
pH (e. 86 Cond (umho) 8.09						
Temp (oC) <u>/6,32</u> or (oF)	_ Static Water Level	DO (mg/L)	T:	urb. (NTU)		
Flow (MGD) or (CFS)	or(g/min)	· · · · · · · · · · · · · · · · · · ·				
Relineutshed by: (Signature)	Received by: (Signatu	P 1	Date (mm/	/dd/yy) Time (24 hr)		
71. MOT 1	derai's	and a	4/7/	20 1447		
Traibuel	41	ll-	4-7-			

Chain of Custody
Scheduled for: 04/01/2020



•	Scheduled for	. 04/01/2020		#1 E 4 F E 4 1 27 E 1 1 M # 2 M 1 E 22 E 26 E	
Client: Big Rivers Electric Corporation Reid/Green Station Big Rivers Electric Corporation Station Chad Phillips Project: Green Landfill Semiannual Groundwater Project: Green Landfill Semiannual Groundwater			Invoice To: Big Rivers Electric Corporation Reid/Green Stati Chad Phillips PO Box 24 Henderson, KY 42419		
	Phone: (270) 844-600	<u>0</u>	PO#:		
Please Print Legibly	PWS ID#: State:	<del></del>	Quote#		
Collected by (Signature):	ermation"	_	Compli	ance Monitoring? Yes V No	
*For composite samples please indicate begin time, end	time and temp(oC) at end	time below:	Sample	es Chlorinated? Yes No	
Influent: Start Date Start time	End Date E	nd Time Te	етр (оС)		
Effluent: Start Date Start time	End DateE	nd TimeT	emp (oC)		
MML! USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr): Bottle a Sample ID#	and Preservative Co	Sample Description	Composite	Sample Analysis Requested	
0041376-04 B 4/2/20 9.5.5 Plasti	c 500mL pH<2 1 w/HNO3	MW4	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Motybdenum Tot 6020 Sodium Tot 6010B	
1/2/2 255	Plastic 1L 1	MW4	g/c	pH (Lab) Conductivity (Lab) TDS Sulfate 9056 Chloride 9056 Fluoride	
// Preserva	c 500mi. pH<2 1 w/H2SO4 tion Check: pH :	MVV4	g/c	9056 COD TOC	
Ra	L pH<2 w/HNO3 1 d 226 (Sub) tion Check: pH:	MVV4	g/c	Radium 226 (sub)	
Rad	L pH<2 w/HNO3 1 d 228 (Sub) ition Check: pH :	MW4	g/c	Radium 228 (sub)	
Preservation Check Performed by: CLH					
Field data collected by: Philip Hill  pH 6.70 Cond (umho) 6.77  Temp (oC) 16.47 or (oF) s  Flow (MGD) or (CFS) or	Res CI (mg/L)	Tot CI (mg/L) DO (mg/L)	Fre	ee CI (mg/L) urb. (NTU)	
Betinquished by: (Signature)	Received by: (Signature)	d	Date (mm/	ao 1443	

Chain of Custody
Scheduled for: 04/01/2020



		O O I I O O O O O O O O O O O O O O O O	1011 0-1/0 1/2020	]			
Client: Big Rivers Electric ( Reid/Green Station	Corporation	Report To: Big Rivers Electr Station	ic Corporation Reid/Green		Involce To: Big Rivers Electric Corporation Reid/Green Station		
Project: Green Landfill Semiannual Groundwater  Chad Phillips PO Box 24 Henderson, KY 42419			PO Box 24	Chad Phillips PO Box 24 Henderson, KY 42419			
Phone: (270) 844-6000			PO#:				
Please Print Legibly		PWS ID#:	<u>Y</u>	Quote#			
Collected by (Signature):	To required in	Z: formation		Compli	ance Monitoring? Yes 1 No		
*For composite samples pleas	•		t end time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date	Start time	_ End Date	End Time	Temp (oC)	***************************************		
Effluent: Start Date	Start time	_ End Date	End Time	Temp (oC)			
00-10/0	Collection	and Preservative	Sample Description	Composite			
Sample ID# 0041376-04 G 4/7/20	R	1L pH<2 w/HNO3 1 ad 228 (Sub) /ation Check: pH :	MW4	g/c	Sample Analysis Requested Radium 228 (sub)		
0041376-04 Н <u>4/7/ао</u>	955 AG	3 250mL pH<2 1 w/H2SO4 vation Check: pH :	MW4	g/c	тос		
0041376-05 A <u>4/7/20</u>	Preserv	tic 500mL pH<2 1 w/HNO3  ration Check: pH :	MW5	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Catcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
Preservation Check Perform	ned by:				1		
		Res Cl (mg/L)	// / / / / / / / / / / / / / / / / / /	Fre	re CI (mg/L)urb. (NTU)		
Relinquished by: (Signature)	l	Received by: (Signatu		Date (mm/	dd/yy) Time (24 hr) 20 1447 20 1549		

# **Chain of Custody**



Client: Big Rivers Electric Corporation Reid/Green Station	Station	ectric Corporation Reid/Green	Big Rivers E	Involce To: Big Rivers Electric Corporation Reid/Green Station			
Project: Green Landfill Semlannual Ground	Chad Phillips water PO Box 24 Henderson, K	Y 42419	Chad Phillip PO Box 24 Henderson,				
	Phone: (270)						
	PWS ID#:	<u></u>	PO#:	,—————————————————————————————————————			
Please Print Legibly	State:	<u> </u>	Quote#	·			
Collected by (Signature): requ	lired information	···········	Compl	iance Monitoring? Yes 1 No			
*For composite samples please indicate begin t	time, end time and temp(oC	) at end time below:	Sample	es Chlorinated? Yes No			
Influent: Start Date Start time	End Date	End Time	Temp (oC)				
Effluent: Start Date Start time	End Date	End Time	_Temp (oC)	······································			
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr):	Bottle and Preservative	Sample Description	n Composite				
Sample ID#		<u>o</u>		Sample Analysis Requested			
0041376-05 B 4/7/36 10/0	Plastic 500mL pH<2 w/HNO3	1 MW5	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B			
	Preservation Check: pH :	<u>~</u>		101 0020 0000011 101 00 100			
0041376-05 C 4/7/ao 1010.	Plastic 1L	1 MW5	g/c	pH (Lab) Conductivity (Lab) TDS Sulfate 9056 Chloride 9056 Fluoride			
0041376-05 D 4/7/20 10/0	Plastic 500mL pH<2 w/H2SO4 Preservation Check: pH:	1 MW5	g/c	9056 COD TOC			
0041376-05 E <u>4/7/20 /0/0</u>	Plastic 1L pH<2 w/HNO3 Rad 226 (Sub) Preservation Check: pH :		g/c	Radium 226 (sub)			
0041376-05	Plastic 1L pH<2 w/HNO3 Rad 228 (Sub) Preservation Check: pH :		g/c	Radium 228 (sub)			
Preservation Check Performed by:	<u> </u>	·					
Field data collected by: Philip Hu	Date (mm/dd/yy)	4/7/20 Time (24 hr)	1010				
pH <u>6.77</u> Cond (umho) 6.	~ ·	Tot Cl (mg/L)	•	ee CI (mg/L)			
		DO (mg/L) _		1			
Flow (MGD) or (CFS)							
Relinquished by (Signature)	Received by: (Signature)	ature)	Date (mm/	idpl/yy) Time (24 hr)			
26.01111	Trans 1	and a	4/7/	20 1443			
Treat Ind		e	4-7				
·				***************************************			
PACE- Check here if trip charge appl	ied to associated COC	Printed:	3/25/2020 2:51:	08PM			

# **Chain of Custody**

Scheduled for: 04/01/2020



Client: Big Rivers Electric Corporation Reid/Green Station  Project: Green Landfill Semiannual Groundwa	Station Chad Phillips	tric Corporation Reid/Green	Invoice To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips PO Box 24 Henderson, KY 42419		
	Phone: <u>(270)</u> 8 PWS ID#:	44-6000	PO#:		
Please Print Legibly	State:	<u> </u>	Quote#	······	
Collected by (Signature): require	ed information	<del></del>	Compli	ance Monitoring? Yes 🖳 No	
*For composite samples please indicate begin tim	e, end time and temp(oC)	at end time below:	Sample	es Chlorinated? Yes No	
Influent: Start Date Start time	End Date	End Time *	ľemp (oC)	111	
Effluent: Start Date Start time	End Date	End Time	Temp (oC)		
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr): Sample ID#	Bottle and Preservative	Sample Description	Composite	Sample Analysis Requested	
/ / Pr	lastic 1L pH<2 w/HNO3 Rad 228 (Sub) eservation Check: pH : _	1 MW5	g/c	Radium 228 (sub)	
0041376-05 H 4/7/20 10/0 Pr	AG 250mL pH<2 w/H2SO4 reservation Check: pH:_	1 MW5	g/c	тос	
0041376-06 A 4/6/20 1420		1 MW6	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chremium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B	
Preservation Check Performed by:	<u>.</u>				
	Res CI (mg/L)	Tot CI (mg/L) DO (mg/L)	Fre	<u> </u>	
Relinquished by: (Signature)	Received by: (Signa	buel .	Date (mm/	ao <u>1443</u>	

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Chain of Custody
Scheduled for: 04/01/2020



	ociicadioo	101. 04/01/2020				
Client: Big Rivers Electric Corporation Report To: Reid/Green Station Big Rivers Electric Corporation Reid/Green Station			Involce To: Big Rivers Electric Corporation Reid/Green Station			
Project: Green Landfill Semiannual Groundwater	Chad Phillips PO Box 24 Henderson, KY	42419	Chad Phillips PO Box 24 Henderson, KY 42419			
	Phone: <u>(270) 84</u> PWS ID#:	<u>14-6000</u>	PO#:			
Please Print Legibly State:			Quote#			
Collected by (Signature):	ormation		Compli	ance Monitoring? Yes L/No		
*For composite samples please indicate begin time, en	d time and temp(oC)	at end time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date Start time	_ End Date	End Time	Temp (oC)			
Effluent: Start Date Start time	_ End Date	End Time	Temp (oC)			
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr): Bottle	and Preservative	Sample Description	Composite			
Sample ID# // / // // // // Plas	tic 500mL pH<2	<u>රි</u> 1 MW6	g/c	Sample Analysis Requested  Beryllium Tot 6020 Cadmium Tot		
	w/HNO3  vation Check: pH : _		g, c	6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
0041376-06 C 4/6/20 1420	Plastic 1L	1 MW6	g/c	pH (Lab) Conductivity (Lab) TDS Sulfate 9056 Chloride 9056 Fluoride		
<u> </u>	tic 500mL pH<2 w/H2SO4 vation Check: pH : _		g/c	9056 COD TOC		
	: 1L pH<2 w/HNO3 ad 226 (Sub) vation Check: pH : _		g/c	Radium 226 (sub)		
0041376-06 F <u>4/4/20 1420</u> Plastic	:1L pH<2 w/HNO3 :ad 228 (Sub) vation Check: pH:_	,	g/c	Radium 228 (sub)		
Preservation Check Performed by: CLH						
Field data collected by: Phillip Hill	Date (mm/dd/yy) _	4/4/20 Time (24 hr)	1420			
pH 6.36 Cond (erritto) 5.01	Res CI (mg/L)	Tot CI (mg/L)	Fre	ee Ci (mg/L)		
Temp (oC) 20,50 or (oF)	Static Water Level _	DO (mg/L)		urb. (NTU)		
Flow (MGD) or (CFS)	or (g/min) _	·				
Relinquished by: (Signature) /	Received by: (Signa	turgi)	Date (mm/	/dd/yy) Time (24 hr)		
MUMME 1	"Sreal	Suel	_ 4/2/2	10 1443		
"Trei Suel			41-7-	1549		

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# **Chain of Custody**



Client: Big Rivers Electric Corporation Reid/Green Station  Project: Green Landfill Semiannual Groundwate	Station Chad Phillips PO Box 24	Big Rivers Electric Corporation Reid/Green Station Chad Phillips		Invoice To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips PO Box 24 Henderson, KY 42419		
	Phone: (270) 844- PWS ID#:	<u> 6000</u>	PO#:	····		
Please Print Legibly	State:	<u> </u>	Quote#			
Collected by (Signature):	· Information*		Compli	ance Monitoring? Yes 2 No		
*For composite samples please indicate begin time,		end time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date Start time			Temp (oC)			
Effluent: Start Date Start time						
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr): Bot Sample ID# / /	ontainers tile and Preservative O	Sample Description	Composite	Sample Analysis Requested		
0041376-06 G 4/6/30 /430 Plas	tic 1L pH<2 w/HNO3 1 Rad 228 (Sub)	MW6	g/c	Radium 228 (sub)		
0041376-06 Н <u>4/6/20</u> 1420	ervation Check: pH ; AG 250mL pH<2 1 w/H2SO4 ervation Check: pH :	MW6	g/c	тос		
·	estic 500ml pH<2 1 w/HNO3 ervation Check: pH :(	DUPLICATE	g / c	Beryllium Tot 6020 Cadmium Tot 6020 Catcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
Preservation Check Performed by:						
Field data collected by: Phillip Hill  pH	Res CI (mg/L)	Tot CI (mg/L) _	Fre			
Reinquishersy: (Signature),	Received by: (Signature	red	Date (mm/	lao 1443		

# **Chain of Custody**



			<u></u>		·
Client: Big Rivers Electric Corporation Reid/Green Station	Invoice To: ric Corporation Reid/Green Big Rivers Electric Corporation Rei			ion Reid/Green Station	
Project: Green Landfill Semiannual Groun	dwater Chad Phillips		Chad Phillip PO Box 24	os	•
	PO Box 24 Henderson, K'	Y 42419	Henderson,	KY 42419	
	Phone: (270) 8		***************************************		
·	PWS ID#:	- 1 × /	PO#:		
Please Print Legibly	State:	<u>KY</u>	Quote#		
Collected by (Signature):	guired information	<del> </del>	Compl	iance Monitoring	]? Yes <u>L</u> No
*For composite samples please indicate begin		) at end time below:	Sampl	es Chlorinated?	Yes No
Influent: Start Date Start time			Temp (oC)		
Effluent: Start Date Start time					
	<u></u>				
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr):	Bottle and Preservative	Sample Description	n Composite		
Sample ID#		<del>_</del>	•		nalysis Requested
0041376-07 В <u>4/7/а» /о</u> до	Plastic 500mL pH<2 w/HNO3	1 DUPLICATE	g/c	6020 Calcium 6020 Chromiu 6020 Arsenic 6010B Coppel Tot 6020 Lead	5020 Cadmium Tot Tot 6010B Barium Tot m Tot 6020 Cobalt Tot Tot 6020 Boron Tot r Tot 6020 Antimony I Tot 6020 Lithium Tot Tot 6020 Molybdenum
1.1	Preservation Check: pH:	<u>l~</u>		100 5020 5008	um 101 60 105
0041376-07 c 4/7/20 1020	Plastic 1L	1 DUPLICATE	g/c	Sulfate 9056 (	ductivity (Lab) TDS Chloride 9056 Fluoride
0041376-07 D 4/2/20 1020	Plastic 500mL pH<2 w/H2SO4 Preservation Check: pH :	1 DUPLICATE	g/c	9056 COD TOC	
0041376-07 E 4/7/a0 1020	Plastic 1L pH<2 w/HNO3 Rad 226 (Sub) Preservation Check: pH:	1 DUPLICATE	g/c	Radium 226 (s	sub)
0041376-07 F 4/7/20 1020	Plastic 1L pH<2 w/HNO3 Rad 228 (Sub) Preservation Check: pH:	1 DUPLICATE	g/c	Radium 228 (s	sub)
Preservation Check Performed by:	<u>CLH</u>				
Field data collected by: Phillip }	Date (mm/dd/yy)	4/7/24 Time (24 hr)	1020		- Constitution
pH 6.70 Cond (umitro)	77 Res CI (mg/L)	Tot CI (mg/L)	Fr	ee Ci (mg/L)	
1/	Static Water Level				- 41
Flow (MGD) or (CFS)			-		
Relinquished by (Signature)	Received by: (Signi	atuse	Date (mm	/dd/yy)	Time (24 hr)
M. M/1/	ر ، ومدا	1	Uni	20	1443
		18-		-	1549
- Tree Sund				-20	<u> </u>
	<u> </u>			•	
PACE- Check here if trip charge ap	plied to associated COC	Printed	3/25/2020 2:51	:08PM	9202 13 AS 18

# **Chain of Custody**



				<b>}</b>	
Cilent: Big Rivers Electric Corporation Reid/Green Station Big Rivers Electric Corporation Reid/Green Station Chad Phillips Project: Green Landfill Semiannual Groundwater PO Box 24 Henderson, KY 42419			Involce To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips PO Box 24 Henderson, KY 42419		
		ne: <u>(270) 844-6(</u> S ID#:	000	PO#:	
Please Print Legibly	State	17.5	<u>/</u>	Quote#	
Collected by (Signature):	D F juired information	•		Compli	ance Monitoring? YesNo
*For composite samples please indicate begin	•		nd time below:	Sample	es Chlorinated? Yes No
Influent: Start Date Start time	End Dat	te	End Time	Temp (oC)	<u>.</u>
Effluent: Start Date Start time					
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr):	Bottle and Prese	Containers			
Sample ID#	.=		Sample Description	Composite	Sample Analysis Requested
0041376-07 G <u>4/7/2° 1024 a</u>	Plastic 1L pH<2 v Rad 228 (Si Preservation Che	ub)	DUPLICATE	g/c	Radium 228 (sub)
0041376-07 H <u>У/7/20</u> <u>1020</u>	AG 250mL pl w/H2SO4 Preservation Che	<b>!</b>	DUPLICATE	g/c	тос
0041376-08 A 4/7/20 1/50	Plastic 500mL w/HNO3	•	FIELD BLANK	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B
	Preservation Che		-		
Preservation Check Performed by:	<u> </u>	<del></del>			
Field data collected by: Phillip H  ph	.77 Res	CI (mg/L)		Fre	ee CI (mg/L) urb. (NTU)
Relinquished by (Signature)	Received	by: (Signature)	nel	Date (mm/	ao 1442

# **Chain of Custody**

Scheduled for: 04/01/2020



	<u> </u>		j			
Client: Big Rivers Electric Corporation Reid/Green Station Project: Green Landfill Semiannual Groun	Station Chad Phillips dwater PO Box 24	ctric Corporation Reid/Greer	Chad Phillips PO Box 24			
	Henderson, K		Henderson,	KY 42439		
	Phone: <u>(270) /</u> PWS ID#:	<u>844-6000</u>	PO#:			
Please Print Legibly	} State:	<u> </u>	Quote#			
Collected by (Signature):	juired information		Compli	iance Monitoring? Yes Vo No		
*For composite samples please indicate begin	i time, end time and temp(oC	) at end time below:	Sample	es Chlorinated? Yes No		
Influent: Start Date Start time	End Date	End Time	Temp (oC)			
Effluent: Start Date Start time _	End Date	End Time	Temp (oC)			
MMLI USE ONLY *required information* Workorder # Date Collection 0041376 (mm/dd/yy): Time (24 hr): Sample ID#	Bottle and Preservative	Sample Description	n Composite	Sample Analysis Requested		
0041376-08 B 4/7/2 1/50	Plastic 500mL pH<2 w/HNO3	1 FIELD BLANK	g/c	Beryllium Tot 6020 Cadmium Tot 6020 Calcium Tot 6010B Barium Tot 6020 Chromium Tot 6020 Cobalt Tot 6020 Arsenic Tot 6020 Boron Tot 6010B Copper Tot 6020 Antimony Tot 6020 Lead Tot 6020 Lithium Tot 6020 Mercury Tot 6020 Molybdenum Tot 6020 Sodium Tot 6010B		
0041376-08 C 4/2/2 1/50	Plastic 1L	1 FIELD BLANK	g/c	pH (Lab) Conductivity (Lab) TDS Sulfate 9056 Chloride 9056 Fluoride		
0041376-08D 4/7/20 1150	Plastic 500mi. pH<2 w/H2SO4 Preservation Check: pH :	1 FIELD BLANK	g/c	9056 COD TOC		
0041376-08 E 4/7/20 1/50	Plastic 1L pH<2 w/HNO3 Rad 226 (Sub) Preservation Check: pH:		g/c	Radium 226 (sub)		
0041376-08F <u>4/7/20</u> <u>1/50</u>	Plastic 1L pH<2 w/HNO3 Rad 228 (Sub) Preservation Check: pH:	-	g/c	Radium 228 (sub)		
Preservation Check Performed by:	71H					
Field data collected by: Phillip Hi	l] Date (mm/dd/yy)	4/7/20 Time (24 hr)	1/50			
pH Cond (umho)				ee Cl (mg/L)		
	Static Water Level			1		
Flow (MGD) or (CFS)			<u> </u>			
Relinguished by: (Signature)	Received by: (Signa	ature) ()	Date (mm/	/dd/yy) Time (24 hr)		
The mk	Dren	V //	4/5/2			
The breek				-20 1549		

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# **Chain of Custody**

Scheduled for: <u>04/01/2020</u>



Client: Big Rivers Electric Corporation Reid/Green Station  Project: Green Landfill Semiannual Groundwat	Station Chad Phillins	ric Corporation Reid/Green , 42419	Involce To: Big Rivers Electric Corporation Reid/Green Station Chad Phillips PO Box 24 Henderson, KY 42419			
Please Print Legibly	Phone: (270) 84 PWS ID#: State:	<u>4-6000</u> <u>KY</u>	PO#:	ance Monitoring? YesNo		
	d information		,	es Chlorinated? Yes No		
*For composite samples please indicate begin time Influent: Start Date Start time			Temp (pC)			
Effluent: Start Date Start time						
Sample ID#		Sample Description FIELD BLANK	Composite	Sample Analysis Requested Radium 228 (sub)		
0041376-08 H 4/7/20 1/50 Pre	servation Check: pH :	FIELD BLANK	g/c	Ber 50,020 Cadmium for 60 10 10 10 10 10 10 10 10 10 10 10 10 10		
· Pre	eservation Check: pH :	<u></u>		6020 Western For boards		
Preservation Check Performed by:	14	•				
Field data collected by: Philip Hill						
pH         Cond (umho)           Temp (oC) or (oF)		Tot CI (mg/L) DO (mg/L)		<b>I</b>		
Temp (oC) or (oF) Flow (MGD) or (CFS)			'			
Relinguished by: (Signature)	Received by: (Signat	urg d	Date (mm/	7dd/yy) Time (24 hr) 20 /447 20 /549		

(724)850-5600



April 30, 2020

Rob Whittington Pace Analytical Madisonville 825 Industrial Rd Madisonville, KY 42431

RE: Project: 41376

Pace Project No.: 30358430

### Dear Rob Whittington:

Enclosed are the analytical results for sample(s) received by the laboratory on April 10, 2020. The results relate only to the samples included in this report. Results reported herein conform to the applicable TNI/NELAC Standards and the laboratory's Quality Manual, where applicable, unless otherwise noted in the body of the report.

The test results provided in this final report were generated by each of the following laboratories within the Pace Network:

• Pace Analytical Services - Greensburg

If you have any questions concerning this report, please feel free to contact me.

Sincerely,

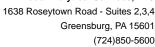
Carin a Ferris

Carin Ferris carin.ferris@pacelabs.com 724-850-5615 Project Manager

Enclosures

cc: Doug Wolfe, Pace Analytical Madisonville







### **CERTIFICATIONS**

Project: 41376
Pace Project No.: 30358430

### Pace Analytical Services Pennsylvania

1638 Roseytown Rd Suites 2,3&4, Greensburg, PA 15601

ANAB DOD-ELAP Rad Accreditation #: L2417

Alabama Certification #: 41590 Arizona Certification #: AZ0734 Arkansas Certification

California Certification #: 04222CA Colorado Certification #: PA01547 Connecticut Certification #: PH-0694

Delaware Certification EPA Region 4 DW Rad

Florida/TNI Certification #: E87683 Georgia Certification #: C040 Florida: Cert E871149 SEKS WET

Guam Certification Hawaii Certification Idaho Certification Illinois Certification Indiana Certification Iowa Certification #: 391

Kansas/TNI Certification #: E-10358 Kentucky Certification #: KY90133 KY WW Permit #: KY0098221 KY WW Permit #: KY0000221

Louisiana DHH/TNI Certification #: LA180012 Louisiana DEQ/TNI Certification #: 4086

Maine Certification #: 2017020 Maryland Certification #: 308

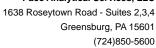
Massachusetts Certification #: M-PA1457 Michigan/PADEP Certification #: 9991 Missouri Certification #: 235
Montana Certification #: Cert0082
Nebraska Certification #: NE-OS-29-14
Nevada Certification #: PA014572018-1
New Hampshire/TNI Certification #: 297617
New Jersey/TNI Certification #: PA051
New Mexico Certification #: PA01457
New York/TNI Certification #: 10888
North Carolina Certification #: 42706
North Dakota Certification #: R-190
Ohio EPA Rad Approval: #41249

Oregon/TNI Certification #: PA200002-010 Pennsylvania/TNI Certification #: 65-00282 Puerto Rico Certification #: PA01457 Rhode Island Certification #: 65-00282

South Dakota Certification
Tennessee Certification #: 02867

Texas/TNI Certification #: T104704188-17-3 Utah/TNI Certification #: PA014572017-9 USDA Soil Permit #: P330-17-00091 Vermont Dept. of Health: ID# VT-0282 Virgin Island/PADEP Certification Virginia/VELAP Certification #: 9526 Washington Certification #: C868 West Virginia DEP Certification #: 143 West Virginia DHHR Certification #: 9964C

Wisconsin Approve List for Rad Wyoming Certification #: 8TMS-L

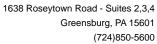




### **SAMPLE SUMMARY**

Project: 41376
Pace Project No.: 30358430

Lab ID	Sample ID	Matrix	Date Collected	Date Received
30358430001	0041376-01	Water	04/06/20 13:05	04/10/20 09:15
30358430002	0041376-02	Water	04/07/20 11:40	04/10/20 09:15
30358430003	0041376-03	Water	04/07/20 13:55	04/10/20 09:15
30358430004	0041376-04	Water	04/07/20 09:55	04/10/20 09:15
30358430005	0041376-05	Water	04/07/20 10:10	04/10/20 09:15
30358430006	0041376-06	Water	04/06/20 14:20	04/10/20 09:15
30358430007	0041376-07	Water	04/07/20 10:20	04/10/20 09:15
30358430008	0041376-08	Water	04/07/20 11:50	04/10/20 09:15





### **SAMPLE ANALYTE COUNT**

Project: 41376
Pace Project No.: 30358430

Lab ID	Sample ID	Method	Analysts	Analytes Reported	Laboratory
30358430001	0041376-01	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA
30358430002	0041376-02	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA
30358430003	0041376-03	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA
30358430004	0041376-04	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA
30358430005	0041376-05	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA
30358430006	0041376-06	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA
30358430007	0041376-07	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA
30358430008	0041376-08	EPA 903.1	MK1	1	PASI-PA
		EPA 904.0	VAL	1	PASI-PA
		Total Radium Calculation	CMC	1	PASI-PA

PASI-PA = Pace Analytical Services - Greensburg

(724)850-5600



### **ANALYTICAL RESULTS - RADIOCHEMISTRY**

 Project:
 41376

 Pace Project No.:
 30358430

Sample: 0041376-01 Lab ID: 30358430001 Collected: 04/06/20 13:05 Received: 04/10/20 09:15 Matrix: Water

PWS: Site ID: Sample Type:

Comments: • Sample collection dates and times were not present on the sample containers.

• Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement of pH

<2 for radiochemistry analysis.

Parameters	Method	Act ± Unc (MDC) Carr Trac	Units	Analyzed	CAS No.	Qual
	Pace Analytical	Services - Greensburg		,		
Radium-226	EPA 903.1	0.340 ± 0.473 (0.799) C:NA T:94%	pCi/L	04/30/20 11:27	13982-63-3	
	Pace Analytical	Services - Greensburg				
Radium-228	EPA 904.0	0.468 ± 0.409 (0.828) C:72% T:87%	pCi/L	04/28/20 11:04	15262-20-1	
	Pace Analytical	Services - Greensburg				
Total Radium	Total Radium Calculation	0.808 ± 0.882 (1.63)	pCi/L	04/30/20 14:19	7440-14-4	

Sample: 0041376-02 Lab ID: 30358430002 Collected: 04/07/20 11:40 Received: 04/10/20 09:15 Matrix: Water

PWS: Site ID: Sample Type:

Comments: • Sample collection dates and times were not present on the sample containers.

• Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement of pH

<2 for radiochemistry analysis.

Parameters	Method	Act ± Unc (MDC) Carr Trac	Units	Analyzed	CAS No.	Qual
	Pace Analytical	Services - Greensburg				
Radium-226	EPA 903.1	0.513 ± 0.402 (0.472) C:NA T:88%	pCi/L	04/30/20 11:27	13982-63-3	
	Pace Analytical	Services - Greensburg				
Radium-228	EPA 904.0	0.0161 ± 0.343 (0.794) C:70% T:88%	pCi/L	04/28/20 11:04	15262-20-1	
	Pace Analytical	Services - Greensburg				
Total Radium	Total Radium Calculation	0.529 ± 0.745 (1.27)	pCi/L	04/30/20 14:19	7440-14-4	

 Sample:
 0041376-03
 Lab ID:
 30358430003
 Collected:
 04/07/20 13:55
 Received:
 04/10/20 09:15
 Matrix: Water

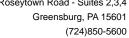
 PWS:
 Site ID:
 Sample Type:

Comments: • Sample collection dates and times were not present on the sample containers.

Parameters	Method	Act ± Unc (MDC) Carr Trac	Units	Analyzed	CAS No.	Qual
	Pace Analytica	I Services - Greensburg				
Radium-226	EPA 903.1	0.603 ± 0.577 (0.878) C:NA T:77%	pCi/L	04/30/20 11:27	13982-63-3	
	Pace Analytica	l Services - Greensburg				
Radium-228	EPA 904.0	0.460 ± 0.444 (0.914) C:68% T:85%	pCi/L	04/28/20 11:04	15262-20-1	
	Pace Analytica	l Services - Greensburg				
Total Radium	Total Radium Calculation	1.06 ± 1.02 (1.79)	pCi/L	04/30/20 14:19	7440-14-4	

### **REPORT OF LABORATORY ANALYSIS**

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41376

Project:

PWS:

Comments:

Radium-226

Radium-228

### **ANALYTICAL RESULTS - RADIOCHEMISTRY**

30358430 Pace Project No.: Sample: 0041376-04 Lab ID: 30358430004 Collected: 04/07/20 09:55 Received: 04/10/20 09:15 Matrix: Water PWS: Site ID: Sample Type: • Sample collection dates and times were not present on the sample containers. Comments: • Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement of pH <2 for radiochemistry analysis. **Parameters** Method Act ± Unc (MDC) Carr Trac Units Analyzed CAS No. Qual Pace Analytical Services - Greensburg EPA 903.1  $0.476 \pm 0.455 \quad (0.693)$ Radium-226 pCi/L 04/30/20 11:27 13982-63-3 C:NA T:95% Pace Analytical Services - Greensburg EPA 904.0  $0.787 \pm 0.428 \quad (0.770)$ Radium-228 pCi/L 04/28/20 11:04 15262-20-1 C:74% T:84% Pace Analytical Services - Greensburg Total Radium Total Radium 1.26 ± 0.883 (1.46) pCi/L 04/30/20 14:19 7440-14-4 Calculation Sample: 0041376-05 Lab ID: 30358430005 Collected: 04/07/20 10:10 Received: 04/10/20 09:15 PWS: Site ID: Sample Type: Comments: • Sample collection dates and times were not present on the sample containers. · Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement of pH <2 for radiochemistry analysis. **Parameters** Method Act ± Unc (MDC) Carr Trac Units Analyzed CAS No. Qual Pace Analytical Services - Greensburg EPA 903.1  $0.302 \pm 0.371 \quad (0.605)$ Radium-226 pCi/L 04/30/20 11:27 13982-63-3 C:NA T:95% Pace Analytical Services - Greensburg EPA 904.0 1.18 ± 0.498 (0.824) Radium-228 pCi/L 04/28/20 11:05 15262-20-1 C:71% T:90% Pace Analytical Services - Greensburg Total Radium Total Radium pCi/L  $1.48 \pm 0.869$  (1.43) 04/30/20 14:19 7440-14-4 Calculation Sample: 0041376-06 Lab ID: 30358430006 Collected: 04/06/20 14:20 Received: 04/10/20 09:15 Matrix: Water

### **REPORT OF LABORATORY ANALYSIS**

Sample Type:

Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement of pH

Act ± Unc (MDC) Carr Trac

 $0.0612 \pm 0.279 \quad (0.166)$ 

 $0.683 \pm 0.478 \quad (0.939)$ 

C:NA T:90%

C:68% T:88%

Units

pCi/L

pCi/L

Analyzed

04/30/20 11:27 13982-63-3

04/28/20 11:05 15262-20-1

CAS No.

Site ID:

Method

EPA 903.1

EPA 904.0

<2 for radiochemistry analysis.

**Parameters** 

• Sample collection dates and times were not present on the sample containers.

Pace Analytical Services - Greensburg

Pace Analytical Services - Greensburg

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Qual

(724)850-5600



Total Radium

Total Radium

Calculation

### **ANALYTICAL RESULTS - RADIOCHEMISTRY**

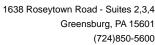
Project: 41376 30358430 Pace Project No.: Sample: 0041376-06 Lab ID: 30358430006 Collected: 04/06/20 14:20 Received: 04/10/20 09:15 Matrix: Water PWS: Site ID: Sample Type: Comments: • Sample collection dates and times were not present on the sample containers. • Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement of pH <2 for radiochemistry analysis. **Parameters** Method Act ± Unc (MDC) Carr Trac Units Analyzed CAS No. Qual Pace Analytical Services - Greensburg Total Radium Total Radium  $0.744 \pm 0.757$  (1.11) pCi/L 04/30/20 14:19 7440-14-4 Calculation Sample: 0041376-07 Lab ID: 30358430007 Collected: 04/07/20 10:20 Received: 04/10/20 09:15 Matrix: Water PWS: Site ID: Sample Type: Comments: • Sample collection dates and times were not present on the sample containers. **Parameters** Method Act ± Unc (MDC) Carr Trac Units Analyzed CAS No. Qual Pace Analytical Services - Greensburg Radium-226 EPA 903.1  $0.371 \pm 0.345 \quad (0.455)$ pCi/L 04/30/20 11:27 13982-63-3 C:NA T:83% Pace Analytical Services - Greensburg EPA 904.0 1.10 ± 0.486 (0.817) Radium-228 pCi/L 04/28/20 11:05 15262-20-1 C:74% T:84% Pace Analytical Services - Greensburg Total Radium Total Radium  $1.47 \pm 0.831$  (1.27) pCi/L 04/30/20 14:19 7440-14-4 Calculation Sample: 0041376-08 Lab ID: 30358430008 Collected: 04/07/20 11:50 Received: 04/10/20 09:15 PWS: Site ID: Sample Type: • Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement of pH Comments: <2 for radiochemistry analysis. **Parameters** Act ± Unc (MDC) Carr Trac CAS No. Method Units Analyzed Qual Pace Analytical Services - Greensburg EPA 903.1  $0.224 \pm 0.515 \quad (0.933)$ Radium-226 pCi/L 04/30/20 11:40 13982-63-3 C:NA T:94% Pace Analytical Services - Greensburg Radium-228 EPA 904.0  $0.262 \pm 0.427 \quad (0.928)$ 04/28/20 11:05 15262-20-1 pCi/L C:74% T:84% Pace Analytical Services - Greensburg

### REPORT OF LABORATORY ANALYSIS

 $0.486 \pm 0.942$  (1.86)

pCi/L

04/30/20 14:19 7440-14-4





### **QUALITY CONTROL - RADIOCHEMISTRY**

Project: 41376
Pace Project No.: 30358430

QC Batch: 392089 Analysis Method: EPA 904.0

QC Batch Method: EPA 904.0 Analysis Description: 904.0 Radium 228

Laboratory: Pace Analytical Services - Greensburg

Associated Lab Samples: 30358430001, 30358430002, 30358430003, 30358430004, 30358430005, 30358430006, 30358430007,

30358430008

METHOD BLANK: 1898525 Matrix: Water

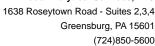
Associated Lab Samples: 30358430001, 30358430002, 30358430003, 30358430004, 30358430005, 30358430006, 30358430007,

30358430008

 Parameter
 Act ± Unc (MDC) Carr Trac
 Units
 Analyzed
 Qualifiers

 Radium-228
 0.230 ± 0.329 (0.705) C:78% T:76%
 pCi/L
 04/28/20 11:05

Results presented on this page are in the units indicated by the "Units" column except where an alternate unit is presented to the right of the result.





### **QUALITY CONTROL - RADIOCHEMISTRY**

Project: 41376
Pace Project No.: 30358430

QC Batch: 392088 Analysis Method: EPA 903.1

QC Batch Method: EPA 903.1 Analysis Description: 903.1 Radium-226

Laboratory: Pace Analytical Services - Greensburg

Associated Lab Samples: 30358430001, 30358430002, 30358430003, 30358430004, 30358430005, 30358430006, 30358430007,

30358430008

METHOD BLANK: 1898523 Matrix: Water

Associated Lab Samples: 30358430001, 30358430002, 30358430003, 30358430004, 30358430005, 30358430006, 30358430007,

30358430008

 Parameter
 Act ± Unc (MDC) Carr Trac
 Units
 Analyzed
 Qualifiers

 Radium-226
 0.176 ± 0.366 (0.660) C:NA T:95%
 pCi/L
 04/30/20 11:27

Results presented on this page are in the units indicated by the "Units" column except where an alternate unit is presented to the right of the result.

(724)850-5600



### **QUALIFIERS**

Project: 41376
Pace Project No.: 30358430

#### **DEFINITIONS**

DF - Dilution Factor, if reported, represents the factor applied to the reported data due to dilution of the sample aliquot.

ND - Not Detected at or above adjusted reporting limit.

TNTC - Too Numerous To Count

J - Estimated concentration above the adjusted method detection limit and below the adjusted reporting limit.

MDL - Adjusted Method Detection Limit.

PQL - Practical Quantitation Limit.

RL - Reporting Limit - The lowest concentration value that meets project requirements for quantitative data with known precision and bias for a specific analyte in a specific matrix.

S - Surrogate

1,2-Diphenylhydrazine decomposes to and cannot be separated from Azobenzene using Method 8270. The result for each analyte is a combined concentration.

Consistent with EPA guidelines, unrounded data are displayed and have been used to calculate % recovery and RPD values.

LCS(D) - Laboratory Control Sample (Duplicate)

MS(D) - Matrix Spike (Duplicate)

**DUP - Sample Duplicate** 

RPD - Relative Percent Difference

NC - Not Calculable.

SG - Silica Gel - Clean-Up

U - Indicates the compound was analyzed for, but not detected.

N-Nitrosodiphenylamine decomposes and cannot be separated from Diphenylamine using Method 8270. The result reported for each analyte is a combined concentration.

Act - Activity

Unc - Uncertainty: For Safe Drinking Water Act (SDWA) analyses, the reported Unc. Is the calculated Count Uncertainty (95% confidence interval) using a coverage factor of 1.96. For all other matrices (non-SDWA), the reported Unc. is the calculated Expanded Uncertainty (aka Combined Standard Uncertainty, CSU), reported at the 95% confidence interval using a coverage factor of 1.96

Gamma Spec: The Unc. reported for all gamma-spectroscopy analyses (EPA 901.1), is the calculated Expanded Uncertainty (CSU) at the 95.4% confidence interval, using a coverage factor of 2.0.

(MDC) - Minimum Detectable Concentration

Trac - Tracer Recovery (%)

Carr - Carrier Recovery (%)

Pace Analytical is TNI accredited. Contact your Pace PM for the current list of accredited analytes.

TNI - The NELAC Institute.

#### SAMPLE QUALIFIERS

Sample: 30358430007

[2] Upon receipt at the laboratory, 5 mls of nitric acid were added to the sample to meet the sample preservation requirement

of pH <2 for radiochemistry analysis.

Sample: 30358430008

Date: 04/30/2020 02:20 PM

[1] Sample collection dates and times were not present on the sample containers.

Chain of Custody

Pace Analytical "

LAB USE ONLY S 3 3 **ය**5 3 දු WO#:30358430 Comments Results Requested By: Requested Analysis 410 2020 0915 Workorder Name: Green Landfill Semiannual Owner Received Date: 4/7/2020 EPA 904.0 Radium Sum Calc Date/Time £.£09 A93 Preserved Containers Pace Analytical Services LLC Greensburg PA Water Water Water Water Water Water Water Water Reveived By 1638 Rosey Town Rd Suite 2,3,4 IR44-McCoy IR44-McCoy IR44-McCoy IR44-McCoy 1R44-McCoy IR44-McCoy IR44-McCoy IR44-McCoy Greensburg, PA 15601 Lab ID Date/Time Subcontract To: (724)850-561504/07/20 09:55 04/07/20 10:10 04/06/20 14:20 04/07/20 10:20 04/07/2011:50 04/06/20 13:05 04/07/20 13:55 04/07/20 11:40 Date/Time Collect Sample Туре r.whittington@mccoylabs.com Madisonville, KY 42409 Transfers Released By Workorder: 41376 McCoy & McCoy Labs 0041376-04 0041376-05 0041376-06 0041376-08 0041376-01 0041376-02 0041376-03 0041376-07 Item Sample ID 270-821-7375 P.O. Box 907 Report To:

Y)or N Sample Intack \*\*\*In order to maintain client confidentiality, location/name of the sampling site, sampler's name and signature may not be provided on this COO Received on Ice Y br N This chain of custody is considered complete as is since this information is available in the owner laboratory. Custody Seal Y or (N ů Cooler Temperature on Receipt

Friday, June 17, 2016 11:01:34 AM

FMT-ALL-C-002rev.00 24March2009

Page 1 of 1

Page 11 of 15 Page 47 of 51

#### SUBCONTRACT ORDER

#### Pace Analytical Services, LLC Kentucky 0041376

# 30358430

SENDING LABORATORY:

Pace Analytical Services, LLC Kentucky

PO BOX 907

Madisonville, KY 42431 Phone: (270) 821-7375 Fax: 844-270-7904

Project Manager:

Rob Whittington

RECEIVING LABORATORY:

Pace Analytical Services LLC Greensburg PA

1638 Rosey Town Rd Suite 2,3,4

Greensburg, PA 15601

Phone:(724) 850-5615

Fax:

Please return shipping cooler to return address on shipping label.

Analysis		Expires	Laboratory ID	Comments
Sample 1D: 0041376-01	Water	Sampled:04/06/2020 13:05	Specific Method	
Radium 228 (sub)	178101	10/03/2020 13:05	EPA 904.0 Radium Sum	· · · · · · · · · · · · · · · · · · ·
Radium Total (sub)		10/03/2020 13:05	EPA 904.0 Radium Sum	
Radium 226 (sub)		10/03/2020 13:05	EPA 903.1	·
Sample 1D: 0041376-02	Water	Sampled:04/07/2020 11:40	Specific Method	
Radium 226 (sub)		10/04/2020 11:40	EPA 903.1	
Radium 228 (sub)		10/04/2020 11:40	EPA 904.0 Radium Sum	(
Radium Total (sub)		10/04/2020 11:40	EPA 904.0 Radium Sum	C
Sample ID: 0041376-03	Water	Sampled:04/07/2020 13:55	Specific Method	
Radium Total (sub)		10/04/2020 13:55	EPA 904.0 Radium Sum	(
Radium 226 (sub)		10/04/2020 13:55	EPA 903.1	
Radium 228 (sub)		10/04/2020 13:55	EPA 904.0 Radium Sum	C
Sample ID: 0041376-04	Water	Sampled:04/07/2020 09:55	Specific Method	
Radium 226 (sub)		10/04/2020 09:55	EPA 903,1	
Radium 228 (sub)		10/04/2020 09:55	EPA 904.0 Radium Sum	(
Radium Total (sub)		10/04/2020 09:55	EPA 904.0 Radium Sum	(
Sample ID: 0041376-05	Water	Sampled:04/07/2020 10:10	Specific Method	
Radium 228 (sub)		10/04/2020 10:10	EPA 904.0 Radium Sum	(
Radium Total (sub)		10/04/2020 10:10	EPA 904.0 Radium Sum	C
Radium 226 (sub)		10/04/2020 10:10	EPA 903.1	
Sample ID: 0041376-06	Water	Sampled:04/06/2020 14:20	Specific Method	
Radium 226 (sub)		10/03/2020 14:20	EPA 903.1	
Radium 228 (sub)		10/03/2020 14:20	EPA 904.0 Radium Sum	C
Radium Total (sub)		10/03/2020 14:20	EPA 904.0 Radium Sum	C
Non Year	. 14	1-09-20 Date		
Released By		Date	Received By	Date
Released By		Date	Received By	Date

#### SUBCONTRACT ORDER

# Pace Analytical Services, LLC Kentucky 0041376

# 30358430

Analysis		Expires	Laboratory ID	Comments	
Sample ID: 0041376-07	Water	Sampled:04/07/2020 10:20	Specific Method		
Radium 226 (sub)	······	10/04/2020 10:20	EPA 903.1		
Radium 228 (sub)		10/04/2020 10:20	EPA 904.0 Radium S	um C	
Radium Total (sub)		10/04/2020 10:20	EPA 904.0 Radium S	um (	
Sample ID: 0041376-08	Water	Sampled:04/07/2020 11:50	Specific Method		
Radium Total (sub)		10/04/2020 11:50	EPA 904.0 Radium S	um (	
Radium 226 (sub)		10/04/2020 11:50	EPA 903.1		
Radium 228 (sub)		10/04/2020 11:50	EPA 904,0 Radium S	um (	

No Year	04.09.20	·		
Released By	Date	Received By	Date	
Released By	Date	Received By	Date	

#### Sample Custody

# 30350430

#### By Nancy Yeager Printed 04/09/2020 09:05

Lab ID	Container	Cooler	Last	OwnBepartmebbcationHome	LocatRitatus Dispositic@ustody Date
0041376-0	1 Elastic 1L pH<2 w/HNO3 Rad 226	(Seta)ult Coo	eNDY	Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	1 Plastic 1L pH<2 w/HNO3 Rad 228			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	2 Plastic 1L pH<2 w/HNO3 Rad 226			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
0041376-0	2 Plastic 1L pH<2 w/HNO3 Rad 228	(Beta)ult Coo	IeNDY	Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	3 Elastic 1L pH<2 w/HNO3 Rad 226			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	3 Plastic 1L pH<2 w/HNO3 Rad 228			Wet Chem in-Transit	Batched Active (Out)04/09/2020 09:05
	4 Plastic 1L pH<2 w/HNO3 Rad 226	•		Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	4 Plastic 1L pH<2 w/HNO3 Rad 228			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	5 Elastic 1L pH<2 w/HNO3 Rad 226			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	5 Plastic 1L pH<2 w/HNO3 Rad 228			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	6 Elastic 1L pH<2 w/HNO3 Rad 226			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
-	6 Plastic 1L pH<2 w/HNO3 Rad 228			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	7 Elastic 1L pH<2 w/HNO3 Rad 226			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	7 Plastic 1L pH<2 w/HNO3 Rad 228			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	8 Elastic 1L pH<2 w/HNO3 Rad 226			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05
	8 Plastic 1L pH<2 w/HNO3 Rad 228			Wet Chem In-Transit	Batched Active (Out)04/09/2020 09:05

•			
Relinquished By	Date	Received By	Date
· · · · · · · · · · · · · · · · · · ·		Described On	Date
Refinquished By	Date	Received By	Date

Pittsburgh La	b Sample Condit	ion l	Jpon	Re	ceipt	
Page Analytical	Client Name:	<u>M</u>	C	μ°	+ McCay Project # # 30350	430
Courier: Fed Ex Tracking #: 110 13	UPS DUSPS Client			$\mathbf{\nabla}$	Pace Other Label QUM	
			-		LiMS Login WM	I
Custody Seal on Cooler	/Box Present:	<b>V</b> n		$\sim$	intact: []yes []no	
Thermometer Used	<del></del>			Wet	Blue None	
Cooler Temperature	Observed Temp 5	1	°C	Corre	ection Factor: 0.4 °C Final Temp: 4.7 °C	
Temp should be above freez	ing to 6°C				pH paper Lot# Date and initials of person examining contents: NWW 4/10/2020	
Comments:		Yes	No	N/A	10 DO301 contents: NMR 4/10/2010	
Chain of Custody Present	+				1.	
Chain of Custody Filled O					2.	
Chain of Custody Relinqu					3.	
Sampler Name & Signatu		[			4.	
Sample Labels match CO	· · · · · · · · · · · · · · · · · · ·				5. NO clate of time	
-Includes date/time/ID	Matrix: \\\	7			on labels	
Samples Arrived within Ho	old Time:				6.	
Short Held Time Analysi	is (<72hr remaining):				7.	
Rush Turn Around Time	Requested:				8.	
Sufficient Volume:					9.	
Correct Containers Used:					10.	
-Pace Containers Used	<del>d</del> :	ļ				
Containers Intact:			ļ		11.	
Orthophosphate field filter	ed	ļ	<u> </u>		12.	<b>.</b>
Hex Cr Aqueous sample f	ield filtered				13.	
Organic Samples check	ced for dechlorination:				14.	
Filtered volume received f			<u> </u>	_	15,	
All containers have been che					16 added 5.0ml HNO3 to	
exceptions; VOA, colifor Non-aqueous matrix	n, TOC, O&G, Phenolics, I	₹adon,			each sample	
All containers meet metho requirements.	d preservation		/		Initial when NWV Date/time of 410 1000 1610 preservation	
					Initial when NWV Date/firms of 410/2020 1610 Lot # of added preservative DL20-0362	
Headspace In VOA Vials (	>6mm):				17.	
Trip Blank Present:					18.	
Trip Blank Custody Seals				_		•
Rad Samples Screened	< 0.5 mrem/hr				Initial when MMP Date: 410 20 20	
Client Notification/ Reso	lution:					•
Person-Contacted:				Date/	Fime:Contacted By:	<u></u>
Comments/Resolution:						
	<u></u>					

A check in this box indicates that additional information has been stored in ereports.

Note: Whenever there is a discrepancy affecting North Carolina compliance samples, a copy of this form will be sent to the North Carolina DEHNR Certification Office (i.e. out of hold, incorrect preservative, out of temp, incorrect containers)

\*PM review is documented electronically in LIMS. When the Project Manager closes the SRF Review schedule in LIMS. The review is in the Status section of the Workorder Edit Screen.

# **Appendix E Remedy Selection Evaluation Criteria**

**TABLE E-1. Summary of Evaluation Criteria**Groundwater Remedy Selection
Big Rivers Electric Corporation - Green Landfill

40 CFR 257.97	Corrective Measure	Corre	ctive Mea	sure Alter	native
Reference	Evaluation Criteria under 40 CFR 257.97	Alt 2a	Alt 3	Alt 4	Alt 5
	Threshold Criteria				
(b)(1)	Be protective of human health and the environment	1	3	3	3
(b)(2)	Attain the Groundwater Protection Standards	1	3.5	2	3.5
(b)(3)	Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into	_	_	0	
	the environment	1	3	2	4
(b)(4)					
	Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors	1	3	2	4
	such as avoiding inappropriate disturbance of sensitive ecosystems				
(b)(5)	Comply with standards for management of wastes as specified in Section	0.5	0.5	0.5	0.5
. , , ,	257.98(d)	2.5	2.5	2.5	2.5
( ) ( )	Balancing Criteria				
(c)(1)	The long and short-term effectiveness of the potential remedy(s), along with the degree of certainty that the remedy will prove successful based on				
	a consideration of the following:				
(c)(1)(i)	Magnitude of reduction of existing risks	1	4	3	2
(c)(1)(ii)	Magnitude of residual risks in terms of likelihood of further releases due to	1	3.5	3.5	2
(c)(1)(iii)	CCR remaining following implementation of a remedy The type and degree of long-term management required, including				_
(6)(1)(111)	monitoring, operation, and maintenance	1	2.5	2.5	4
(c)(1)(iv)	Short-term risks that might be posed to the community or the environment				
	during implementation of such a remedy, including potential threats to human health and the environment associated with excavation,	1	3	2	4
	transportation, and re-disposal of contaminant				
(c)(1)(v)	Time until full protection is achieved	1	3	2	4
(c)(1)(vi)	Potential for exposure of humans and environmental receptors to				
	remaining wastes, considering the potential threat to human health and the environment associated with excavation, transportation, re-disposal, or	1	3	2	4
	containment;				
(c)(1)(vii)	Long-term reliability of the engineering and institutional controls	1	3	2	4
(c)(1)(viii)	Potential need for replacement of the remedy	4	2	1	3
(c)(2)	The effectiveness of the remedy in controlling the source to reduce further				
(c)(2)(i)	releases based on consideration of the following factors: The extent to which containment practices will reduce further releases	1	l a	Ιο	Iα
(c)(2)(ii)	The extent to which treatment technologies may be used	1 1	3 4	2 3	4 2
(c)(3)	The ease or difficulty of implementing a potential remedy(s) based on	•		,	, <del>-</del>
	consideration of the following types of factors		T		,
(c)(3)(i)	Degree of difficulty associated with constructing the technology	4	2	1	3
(c)(3)(ii)	Expected operational reliability of the technologies  Need to coordinate with and obtain necessary approvals and permits from	4	2	1	3
(c)(3)(iii)	other agencies	2.5	2.5	2.5	2.5
(c)(3)(iv)	Availability of necessary equipment and specialists	4	2	1	3
(c)(3)(v)	Available capacity and location of needed treatment, storage, and disposal	1	2	3	4
	services Modifying Critoria	<u> </u>			L '
(c)(1)	Modifying Criteria  The degree to which community concerns are addressed by a potential				
(c)(4)	remedy(s)				
NA (Agreed Order)	State Acceptance	1	3.5	3.5	2
(c)(4)	Community Acceptance	1	3.5	3.5	2
	Total Score =	37	63.5	50	69.5

#### **TABLE E-2. Threshold Criteria Evaluation**

Groundwater Remedy Selection
Big Rivers Electric Corporation - Green Landfill

40 CFR 257.97	Corrective Measure	Corre	ective Mea	sure Alter	native								
Reference	Evaluation Criteria under 40 CFR 257.97	Alt 2a	Alt 3	Alt 4	Alt 5	Benefit Analysis							
	Threshold Criteria												
(b)(1)	Be protective of human health and the environment (HH&E)	1	3	3	3	All 4 alternatives are expected to be protective of HH&E. <b>Alt 2a</b> is considered to be the minimum corrective action that would be required to achieve the CAOs, with the other 3 alternatives building to some degree upon Alt 2a. However Alt 2a relies upon natural attenuation to achieve and ultimately meet the CAOs and therefore has been scored lower for this criteria. The other 3 alternatives are expected to be protective of HH&E to the same degree and have been scored equally.							
(b)(2)	Attain the Groundwater Protection Standards (GWPS)	1	3.5	2	3.5	All 4 alternatives are expected to meet the GWPS, however the time frame for attainment is expected to vary based upon the degree to which the alternative employs an active component and how long the active component will take to design and implement. Alt 2a employs no active remedial component and has been scored lowest. Implementation of other source control measures (included with Alt 3 and Alt 5) is viewed as the corrective measure likely to provide a benefit in the shortest time frame. Addition of hydraulic/physical containment technologies combined with ex-situ treatment associated with Alt 3 and Alt 4 will required additional engineering and pilot testing, likely extending the time required for implementation. Alt 4 would require enhanced engineering and testing compared to Alt 3 so it was ranked lower than Alt 3. The Alt 3 and Alt 5 alternatives are likely to attain the GWPS in the shortest time frame and have been scored highest.							
(b)(3)	Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into the environment	1	3	2	4	All 4 alternatives are expected to reduce or eliminate further releases of Appendix IV constituents. Alt 2a is considered to be the minimum corrective action that would be required to achieve the CAOs, with the other 3 alternatives building to some degree upon Alt 2a. However Alt 2a relies upon natural attenuation to achieve ultimately meet the CAOs and therefore has been scored lowest for this criteria. Alt 3 and Alt 4 incorporate active remedial components to remove COCs from the environment. Given that Alt 3 and Alt 4 incorporate an ex-situ component, both represent slightly higher potential for furthers releases into the environment compared with Alt 5. Given that Alt 3 contains a source control component it scores higher than Alt 4. Alt 5 will prevent further releases by removing source material from the South Sediment Basin and is not seen to represent as much of a environmental risk via a release to surface water receptors as Alt 3 and Alt 4.							
(b)(4)	Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors such as avoiding inappropriate disturbance of sensitive ecosystems	1	3	2	4	All 4 alternatives are expected to remove contamination from the environment. Alt 2a employs no active remedial component and has been scored lowest of all. Both Alt 3 and Alt 4 incorporate an active remedial component to remove COCs from the environment, but incorporate engineering and ex-situ components, representing a slight probability of impacting sensitive ecosystems and have been scored lower compared to Alt 5. Both Alt 3 and Alt 5 incorporate removing source material from the South Sediment Basin and other source control measures, in addition to addressing groundwater impacts. Due to the lack of an ex-situ component, Alt 5 has been scored highest of all.							
(b)(5)	Comply with standards for management of wastes as specified in Section 257.98(d) [See Notes]	2.5	2.5	2.5	2.5	All 4 alternatives are expected to comply with waste management standards to the same degree and have been scored equally.							
	SUBTOTALS	6.5	15	11.5	17								

#### notes

- 1) Alternative #2a (A2a): CiP, ICs, and Groundwater Monitoring
- 2) Alternative #3 (A3): CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- 3) Alternative #4 (A4): CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- 4) Alternative #5 (A5): CiP, Other Source Control, ICs, and Groundwater Monitoring
- 5) Ranking scores range from 1 to 4; 1 = lowest ranking score; 4 = highest ranking score
- 6) When alternatives are all equivalent the ranking is assigned as the average value of all possible ranking (i.e., (1+2+3+4)/4 = 2.5)

**TABLE E-3. Balancing Criteria Evaluation**Groundwater Remedy Selection Big Rivers Electric Corporation - Green Landfill

40 CFR 257.97	Corrective Measure	Corre	ctive Mea	sure Alte	rnative					
Reference	Evaluation Criteria under 40 CFR 257.97	Alt 2a	Alt 3	Alt 4	Alt 5	Benefit Analysis				
Balancing Criteria										
(c)(1)	The long and short-term effectiveness of the potential remedy(s), along with the degree of certainty that the remedy will prove successful based on a consideration of the following:									
(c)(1)(i)	Magnitude of reduction of existing risks	1	4	3	2	All 4 alternatives are expected to result in a reduction of existing risks. Alt 2a is considered to be the minimum corrective action that would be required to achieve the CAOs, with the other 3 alternatives building to some degree upon Alt 2a. However Alt 2a relies upon natural attenuation to ultimately achieve the CAOs and therefore has been scored lowest for this criteria. Alt 3 and Alt 4 incorporate an active remedial component to remove COCs from the environment, which is considered to be effective at reducing existing risks. Given that Alt 4 incorporates an ex-situ component, it does represent slightly higher existing risk than Alt 3. Alt 5 on itis own provides for some reduction of existing risks by removing source material from the South Sediment Basin, but scores lower than Alt 3 and Alt 4.				
(c)(1)(ii)	Magnitude of residual risks in terms of likelihood of further releases due to CCR remaining following implementation of a remedy [See Note]	1	3.5	3.5	2	All 4 alternatives are expected to result in a reduction of residual risks due to further releases but allow for CCR to remain in place indefinitely. Alt 2a employs no active component for containing further releases and has been scored lowest of all. Alt 3 will reduce further releases due to the hydraulic containment provided by a groundwater extraction system and the ability of treatment to remove COCs from the environment. Alt 4 will reduce further releases due to the implementation of physical containment and treatment of groundwater to remove COCs from the environment. Alt 5 would also reduce further releases to the environment, but due to the uncertainty with regard to the impacts observed at MW-3A scored slightly lower. Alt 3 and Alt 4 are considered to be equal with regard to this criteria.				
(c)(1)(iii)	The type and degree of long-term management required, including monitoring, operation, and maintenance [See Note]	1	2.5	2.5	4	Alt 2a will only achieve the established CAO at the end of the Unit operational lifecycle after cap construction, which estimated to be at least 100 years after CiP construction. As a result, Alt 2a will require the most long-term management and has been scored lowest of all. Although the source control component included with Alt 5 will require some longer term maintenance, both Alt 3 and Alt 4 incorporate treatment components requiring considerable expenditure of resources and energy during construction, implementation, and long-term operation. Therefore, Alt 5 has been scored highest of all the alternatives. Alt 3 and Alt 4 are considered to be equal with regard to this criteria.				
(c)(1)(iv)	Short-term risks that might be posed to the community or the environment during implementation of such a remedy, including potential threats to human health and the environment associated with excavation, transportation, and re-disposal of contaminant	1	3	2	4	All 4 alternatives contain some level of short-term risk. Alt 2a employs no active remedial component and has been scored lowest of all. Given that Alt 3 and Alt 4 incorporate an ex-situ component, both represent slightly higher potential for furthes releases into the environment compared with Alt 5. Given that Alt 3 contains a source control component it scores higher than Alt 4. Alt 5 does require removing source material from the South Sediment Basin but is not seen to represent as much risk to the environment during excavation compared to Alt 3 and Alt 4.				
(c)(1)(v)	Time until full protection is achieved	1	3	2	4	Alt 2a will achieve the established CAO at the end of the Unit operational lifecycle after cap construction, which would hault source loading to groundwater, and further allow unimpacted groundwater to flush through the aquifer. The time period for attainment of Alt 2a is estimated to be at least 100 years after CiP construction. Alt 3 would attain the established CAO for the Unit after hydraulic containment eliminates the offsite migration of impacted groundwater, thereby eliminating the exposure pathway. The time period for attainment is relatively short (i.e., <30 years). In the long term, Alt 3 will maintain compliance with the established CAO after cap construction at the end of the Unit operational lifecycle, and removing sourcce material of the South Sediment Basin which will end the source loading to groundwater, as unimpacted groundwater flushes through the aquifer. Alt 4 would attain the established CAO for the landfill after physical containment and extraction eliminates the offsite migration of impacted groundwater, thereby eliminating the exposure pathway. The time period for attainment is based on construction of the grout curtain and groundwater extraction system and is expected to be protracted. In the long term, Alt 4 will maintain compliance with the established CAO after cap construction at the end of the Unit operational lifecycle, which will end the source loading to the groundwater, as unimpacted groundwater flushes through the aquifer. Alt 5 would attain the established CAO for the Unit after removing sourcce material from the South Sediment Basin which will end the source loading to groundwater, as unimpacted groundwater flushes through the aquifer, thereby eliminating the exposure pathway. The time period for attainment via Alt 5 is relatively short. In the long term, Alt 5 will maintain compliance with the established CAO after cap construction at the end of the Unit operational lifecycle. Alt 5 has been scored higher than Alt 3, as design of the source control measures is underway as				

**TABLE E-3. Balancing Criteria Evaluation**Groundwater Remedy Selection Big Rivers Electric Corporation - Green Landfill

40 CFR 257.97	Corrective Measure	Corre	ctive Mea	sure Alte	rnative	
Reference	Evaluation Criteria under 40 CFR 257.97	Alt 2a	Alt 3	Alt 4	Alt 5	Benefit Analysis
				ncing Crite	ria	
(c)(1)(vi)	Potential for exposure of humans and environmental receptors to remaining wastes, considering the potential threat to human health and the environment associated with excavation, transportation, re-disposal, or containment;	1	3	2	4	All 4 alternatives allow for CCR to remain in place indefinitely. Alt 2a employs no active remedial component and has been scored lowest of all. Given that Alt 3 and Alt 4 incorporate an ex-situ component, both represent slightly higher potential for furthers releases into the environment compared with Alt 5. Given that Alt 3 contains a source control component it scores higher than Alt 4. Alt 5 does require removing source material from the South Sediment Basin but is not seen to represent as much risk to the environment during excavation compared to Alt 3 and Alt 4.
(c)(1)(vii)	Long-term reliability of the engineering and institutional controls	1	3	2	4	All 4 alternatives incorporate institutional controls. <b>Alt 2a</b> is considered to be the minimum corrective action that would be required to achieve the CAOs, relying upon natural attenuation to achieve ultimately meet the CAOs and therefore has been scored lowest for this criteria. Given that <b>Alt 3 and Alt 4</b> incorporate an engineering component, both represent slightly higher reliability concerns compared with <b>Alt 5</b> . Given that <b>Alt 3</b> contains a source control component it scores higher than <b>Alt 4</b> .
(c)(1)(viii)	Potential need for replacement of the remedy	4	2	1	3	With the exception of <b>Alt 2a</b> , each alternative employs treatment technologies. <b>Alt 2a</b> employs no active remedial component requiring replacement, and has been scored highest of all. Both <b>Alt 3</b> and <b>Alt 4</b> incorporate an active remedial component to remove COCs from the environment, including engineering and ex-situ components, and have been scored lower than <b>Alt 5</b> . <b>Alt 3</b> incorporates source control measures, and has been scored higher than <b>Alt 4</b> .
(c)(2)	The effectiveness of the remedy in controlling the source to reduce further releases based on consideration of the following factors:					
(c)(2)(i)	The extent to which containment practices will reduce further releases	1	3	2	4	All 4 alternatives are expected to reduce or eliminate further releases of Appendix IV constituents. Alt 2a is considered to be the minimum corrective action that would be required to achieve the CAOs, with the other 3 alternatives building to some degree upon Alt 2a. However Alt 2a relies upon natural attenuation to achieve ultimately meet the CAOs and therefore has been scored lowest for this criteria. Alt 3 and Alt 4 incorporate active remedial components to remove COCs from the environment. Given that Alt 3 and Alt 4 incorporate an Ex-Situ component, both represent slightly higher potential for furthers releases into the environment than Alt 5. Given that Alt 3 contains a source control component it scores higher than Alt 4. Alt 5 will prevent further releases by removing source material from the South Sediment Basin and is not seen to represent as much risk to the environment as Alt 3 and Alt 4.
(c)(2)(ii)	The extent to which treatment technologies may be used	1	4	3	2	With the exception of <b>Alt 2a</b> , each alternative employs treatment technologies. <b>Alt 2a</b> employs no active remedial component and has been scored lowest of all. Both <b>Alt 3</b> and <b>Alt 4</b> incorporate an active remedial component to remove COCs from the environment, including engineering and ex-situ components, and have been scored higher than <b>Alt 5</b> . <b>Alt 3</b> incorporates source control measures, and has been scored highest of all.
(c)(3)	The ease or difficulty of implementing a potential remedy(s) based on consideration of the following types of factors					
(c)(3)(i)	Degree of difficulty associated with constructing the technology	4	2	1	3	With the exception of Alt 2a, each alternative employs treatment technologies. Alt 2a employs no active remedial component and has been scored highest of all. Alt 3 would pose some challenges to the installation and operation of the extraction wells. The proximity to the river will require substantially higher extraction rates in order to provide hydraulic containment. The proximity to the river may pose accessibility issues and result in inflated costs. Alt 4 would be very difficult to implement and is expected to pose some challenges with respect to the installation of the grout curtain and extraction system along the perimeter of the Landfill. The proximity to the river may pose accessibility issues and result in inflated costs. Trenching equipment may be able to meet the depth required for an effective Physical Containment barrier. Alt 4 has been scored lowest of all options with regard to the criteria. Draining and lining the South Sediment Basin requires nominal engineering and construction efforts. Both Alt 3 and Alt 4 incorporate an active remedial component to remove COCs from the environment, including engineering and ex-situ components, and have been scored lower than Alt 5.
(c)(3)(ii)	Expected operational reliability of the technologies	4	2	1	3	With the exception of <b>Alt 2a</b> , each alternative employs treatment technologies. <b>Alt 2a</b> employs no active remedial component requiring operation, and has been scored highest of all. Both <b>Alt 3</b> and <b>Alt 4</b> incorporate an active remedial component to remove COCs from the environment, including engineering and ex-situ components, and have been scored lower than <b>Alt 5</b> . <b>Alt 3</b> incorporates source control measures, and has been scored higher than <b>Alt 4</b> .
(c)(3)(iii)	Need to coordinate with and obtain necessary approvals and permits from other agencies [See Note]	2.5	2.5	2.5	2.5	All 4 alternatives are expected to require permitting and approval from KDWM to the same degree and have been scored equally.

#### **TABLE E-3. Balancing Criteria Evaluation**

Groundwater Remedy Selection
Big Rivers Electric Corporation - Green Landfill

40 CFR 257.97	Corrective Measure	Corrective Measure Alternative			native	
Reference	Evaluation Criteria under 40 CFR 257.97	Alt 2a	Alt 3	Alt 4	Alt 5	Benefit Analysis
				Balan	cing Crite	ria
(c)(3)(iv)	Availability of necessary equipment and specialists	4	2	1	3	With the exception of <b>Alt 2a</b> , each alternative employs treatment technologies. <b>Alt 2a</b> employs no active remedial component requiring operation, and has been scored highest of all. Both <b>Alt 3</b> and <b>Alt 4</b> incorporate an active remedial component to remove COCs from the environment, including engineering and ex-situ components, and have been scored lower than <b>Alt 5</b> . <b>Alt 4</b> would be very difficult to implement and is expected to pose some challenges with respect to the installation of the grout curtain and extraction system along the perimeter of the Landfill. <b>Alt 4</b> is expected to require the most equipment and specialists and has been scored lowest of all.
(c)(3)(v)	Available capacity and location of needed treatment, storage, and disposal services	1	2	3	4	With the exception of <b>Alt 2a</b> , each alternative employs treatment technologies. <b>Alt 2a</b> employs no active remedial component requiring operation, and has been scored lowest of all. Both <b>Alt 3</b> and <b>Alt 4</b> incorporate an active remedial component to remove COCs from the environment, including engineering and ex-situ components, and have been scored lower than <b>Alt 5</b> due to the need for treatment. <b>Alt 3</b> is expected to require the most treatment requirements and has been scored lower than <b>Alt 4</b> .
	SUBTOTALS	28.5	41.5	31.5	48.5	

#### notes:

- 1) Alternative #2a (A2a): CiP, ICs, and Groundwater Monitoring
- 2) Alternative #3 (A3): CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- 3) Alternative #4 (A4): CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- 4) Alternative #5 (A5): CiP, Other Source Control, ICs, and Groundwater Monitoring
- 5) Ranking scores range from 1 to 4; 1 = lowest ranking score; 4 = highest ranking score
- 6) When alternatives are all equivalent the ranking is assigned as the average value of all possible ranking (i.e., (1+2+3+4)/4 = 2.5)

#### TABLE E-4. Modifying Criteria Evaluation

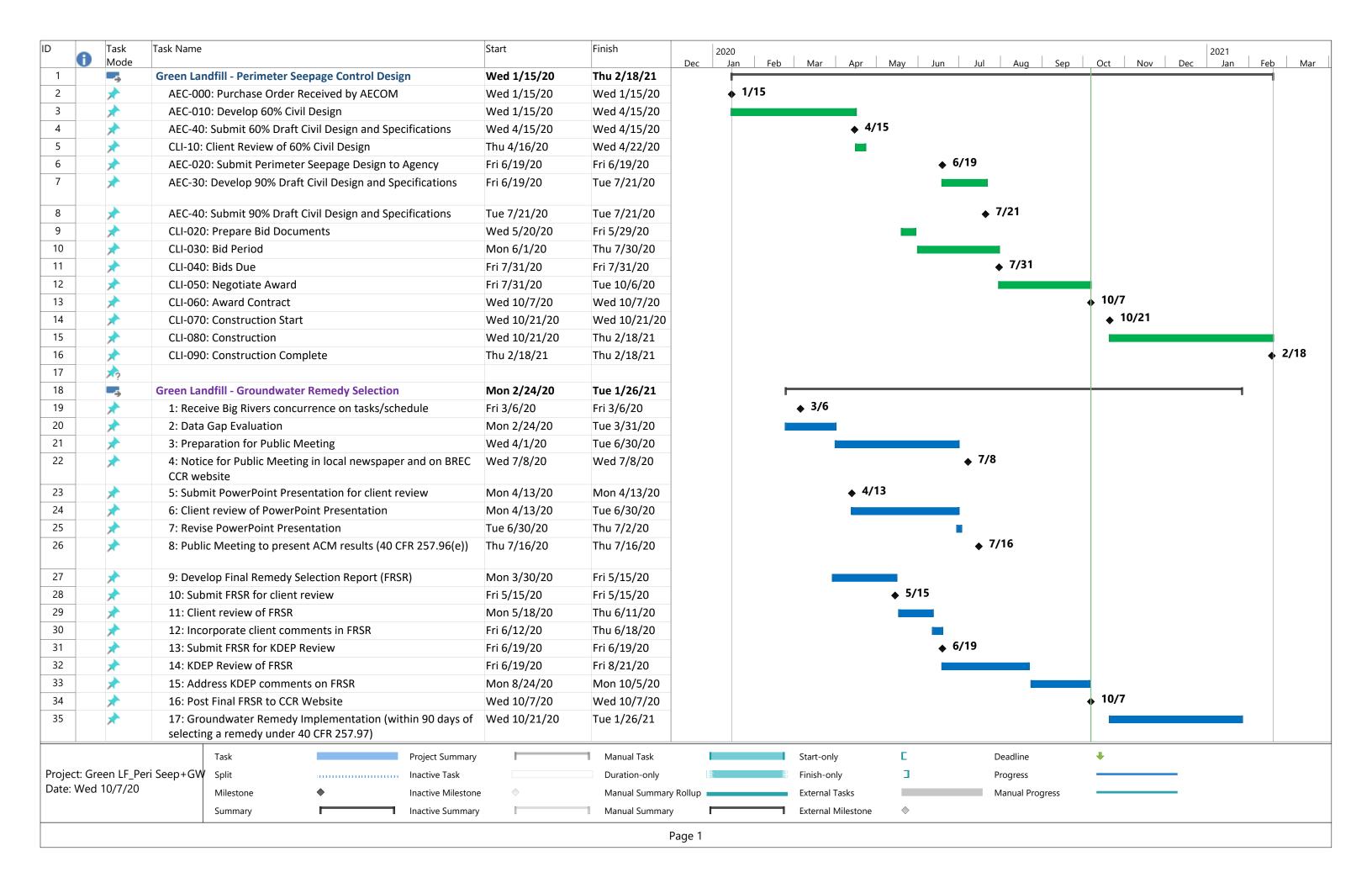
**Groundwater Remedy Selection** Big Rivers Electric Corporation - Green Landfill

40 CFR 257.97	Corrective Measure	Corre	Corrective Measure Alternative			
Reference	Evaluation Criteria under 40 CFR 257.97	Alt 2a	Alt 3	Alt 4	Alt 5	Benefit Analysis
				Modif	ying Crite	ria
(c)(4)	The degree to which community concerns are addressed by a potential remedy(s)					
NA (Agreed Order)	State Acceptance [See Notes]	1	3.5	3.5		Alt 2a is expected to be met with limited state acceptance due to the protracted remedy time frame. Alt 3 and Alt 4 will both minimize the potential impacts to the receptors upon implementation of the extraction system, and the potential for permitting would be relatively straightforward following the completion of the design, thus increasing the regulatory acceptance of the overall remedy. Alt 5 is expected to receive moderate acceptance from the state with respect to additional control of other potential sources of groundwater contamination.
(c)(4)	Community Acceptance [See Notes]	1	3.5	3.5	2	Alt 2a is expected to be met with limited community acceptance due to the protracted remedy time frame. Alt 3 leaves waste in place but provides for active, short-term effective measures that would likely meet with moderate acceptance from the community. Alt 4 would likely meet with moderate acceptance from the community with respect to the established CAO and the addition of the grout curtain and extraction system; however, the remedy timeframe and the discharge of treated groundwater may be an issue. Alt 5 would potentially meet with limited acceptance from the community due to the remedy time frame, which will be complete only after completion of the Landfill's operational lifecycle. However Alt 5 is expected to be more acceptable to the community compared to Alt 2a due to the inclusion of an active corrective measure component.

SUBTOTALS 2

- 1) Alternative #2a (A2a): CiP, ICs, and Groundwater Monitoring
- 2) Alternative #3 (A3): CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- 3) Alternative #4 (A4): CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- 4) Alternative #5 (A5): CiP, Other Source Control, ICs, and Groundwater Monitoring
- 5) Ranking scores range from 1 to 4; 1 = lowest ranking score; 4 = highest ranking score
- 6) When alternatives are all equivalent the ranking is assigned as the average value of all possible ranking (i.e., (1+2+3+4)/4 = 2.5)

# **Appendix F Remedy Implementation Schedule**



AECOM 525 Vine Street Cincinnati, OH 45202 www.aecom.com



# Semi-Annual Remedy Selection Progress Report

Reid/HMP&L Surface Impoundment Sebree Generating Station Webster County, Kentucky

Prepared for:



Big Rivers Electric Corporation Sebree Generating Station 9000 Highway 2096 Robards, KY 42452

Prepared by:

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AECOM PN 60619822

June 2020

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#### 1. Introduction

In accordance with provisions of the United States Environmental Protection Agency's (USEPA) coal combustion residual (CCR) rule, Title 40 of the Code of Federal Regulations (CFR) Part 257.97, Big Rivers Electric Cooperation (BREC) is in the process of selecting a remedy for groundwater impacts at the Reid/Henderson Municipal Power & Light (Reid/HMP&L) Surface Impoundment (the Unit) at the Sebree Generating Station located in Webster County, Robards, Kentucky (Figure 1).

Assessment monitoring results indicate the presence of lithium at a Statistically Significant Level (SSL) above the Ground Water Protection Standard (GWPS) in one monitoring well (MW-10) at the Unit. A map illustrating the site with location of all program monitoring wells is presented as **Figure 2**.

In response to the SSL exceedance, BREC evaluated the nature and extent of groundwater impacts as required by Title 40 CFR Part 257.95(g) for characterization monitoring. In addition, BREC performed an Assessment of Corrective Measures (ACM), to identify applicable remedial technologies to address lithium impacts in groundwater pursuant to Tile 40 CFR Part 257.96. A notice of ACM initiation dated January 14, 2019 was posted to BREC's publicly-accessible CCR reporting website. A report summarizing the results of the ACM (AECOM, June 2019) was posted to BREC's publicly-accessible CCR reporting website on June 14, 2019.

Title 40 CFR Part 257.97(a) requires that progress reports be prepared on a semi-annual basis describing progress made in selecting and designing a remedy. The first semi-annual *Remedy Selection Progress Report* (AECOM, December 2019) was posted to BREC's publicly-accessible CCR reporting website on December 9, 2019. In alignment with the CCR rule requirement, the following sections included within this semi-annual progress report provide an overview of BREC's activities previously performed, currently underway, and planned in the future to select a remedy that meets the requirement of Title 40 CFR Part 257.97 (b) as follows:

- Be protective of human health and the environment;
- (2) Attain the GWPS as specified pursuant to Section 257.95(h);
- (3) Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into the environment;
- (4) Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors such as avoiding inappropriate disturbance of sensitive ecosystems;
- (5) Comply with standards for management of wastes as specified in Section 257.98(d).

#### 2. Site Background

#### 2.1 Site Description

BREC operates the Sebree Station, which is a coal-fired power generating facility located on the Green River northeast of Sebree, Kentucky. Sebree Station is composed of Green Station and Reid/HMP&L Station. BREC owns Green and Reid Stations, while the City of Henderson owns HMP&L Station 2. The Sebree Station is bounded by Interstate-69 to the west and the Green River to the east (see **Figure 1**). Reid Unit 1 (65 Megawatts [MW]) began commercial operation in 1966 and is scheduled to be retired in 2020 pending regulatory approval from the Kentucky Public Service Commission and Rural Utilities Service. The Reid Combustion Turbine (65 MW) was commercialized in 1976. HMP&L Station 2, Units 1 (167 MW) and 2 (168 MW) began commercial operation in 1973 and 1974 respectively. Both HMP&L units were retired as of February 1, 2019. Green Station Units 1 (250 MW) and 2 (242 MW) began commercial operation in 1979 and 1981, respectively.

The location of the Reid/HMP&L Station Surface Impoundment is illustrated in **Figure 2**. The Surface Impoundment has been in place for more than 40 years and was used previously for the placement of CCR material. As stated in the published CCR monitoring well network certification, available on the BREC website, the Reid/HMP&L Station Surface Impoundment is a combined incised/dike earthen embankment structure. It is diked on the west, south and east sides, while the north side is incised. The south dike has the greatest height, reaching approximately 20 feet. Most of the central portion of the south dike was constructed on a subdued ridge.

#### 2.2 Groundwater Investigation Summary

Monitoring wells were installed around the perimeter of the Unit in December 2015 prior to the implementation of the CCR Rule. These wells meet the requirements of Title 40 CFR Part 257.90 of the CCR Rule for installation of a groundwater monitoring system. Under these requirements monitoring wells must adequately represent the quality of background groundwater and groundwater representing the downgradient waste boundary. The existing wells are located along the perimeter of the footprint for the Unit. One upgradient monitoring well (MW-7) and three downgradient monitoring wells (MW-8, MW-9, and MW-10) were installed adjacent to the Unit to determine the general direction of groundwater movement and to monitor groundwater impacts. The monitoring wells were installed in the uppermost saturated portion of the sandstone bedrock aquifer.

Nine rounds of Baseline groundwater sampling for Appendix III constituents was conducted between March 2016 and October 2017. Statistical evaluation of Appendix III constituents monitored for Detection monitoring indicated that statistically significant increases (SSIs) over background have occurred, and therefore, Assessment monitoring was triggered. Detection monitoring activities and data are presented in the annual reports that have been prepared to date (AECOM 2018, 2019, and 2020).

As part of Assessment monitoring, upgradient and downgradient wells for the Unit were sampled for Appendix IV constituents in April, July, and September 2018. GWPS were established for the Appendix IV constituents occurring at SSIs (lithium only), and statistical evaluation of the lithium concentrations indicated exceedances of GWPSs at SSLs, as detailed in **Table 1** below

Table 1 - Reid/HMP&L Surface Impoundment Constituents of Concern

Monitoring Well (Date)	Parameter Lithium GWPS 0.04 (mg/L)
MW-10 (Apr 2018)	0.694
MW-10 (Jul 2018)	0.630
MW-10 (Sep 2018)	0.570

GWPSs are the greater of the site-specific background concentrations, the USEPA primary drinking water standard maximum contaminant limits (MCL), or GWPS provided in 40 CFR 257.95(3)(h)(2)

An additional characterization well, MW-110, was subsequently installed to estimate the downgradient extent of impacted groundwater. Sample collection for Appendix III and IV parameters took place in March and April 2019. The analytical results for lithium in MW-110 were below the GWPS. The additional characterization data are summarized in **Table 2** below.

Table 2 – Reid/HMP&L Surface Impoundment Characterization Sample Results

	Parameter
Monitoring Well (Date)	Lithium GWPS 0.04
	(mg/L)
MW-110 (March 2019)	0.0299
MW-110 (April 2019)	0.0303

The results from both characterization sampling events helped to confirm the downgradient (southwestern) extent of COC impacts above GWPS at the Unit.

Semi-annual Assessment monitoring continued at the Unit in 2019 and 2020 in accordance with 40 CFR Part 257.95.

#### 2.3 Conceptual Site Model

Development and refinement of a Conceptual Site Model (CSM) is necessary to support remedy selection for the Unit. A CSM is based on a set of working hypotheses regarding how contaminants of concern (COCs) entered the environment at a site, how they were and continue to be transported to various media, what the potential routes of exposure are, and who may be exposed, including both human and ecological receptors. As such, the CSM is a "living" model. As new data become available or site conditions change, a CSM should be evaluated and updated as necessary.

The CSM for the Unit was first provided in the June 2019 ACM for the Unit (AECOM 2019). The CSM presents the physical setting of the Unit (adjacent to the Green River), the unconsolidated and bedrock geologic strata underlying the Unit, the occurrence and movement of groundwater, the distribution of COCs in groundwater, and the potential receptors (or lack thereof) for impacted groundwater. These elements are described in detail below and have been updated with new information for this report as appropriate.

#### 2.3.1 Physical Setting

The Unit is located within the Interior Low Plateaus physiographic province. The province is part of the Interior Plains division of the United States. Characteristic features of the province include unglaciated

rolling limestone plains with alluvial valleys and entrenched rivers and streams. Several large rivers are in the region, including the Green, Ohio, Kentucky, Tennessee, and the Cumberland Rivers. The geology underlying the Unit consists of unconsolidated materials, including loess and alluvial deposits, underlain by Upper to Middle Pennsylvanian-age clastic and carbonate bedrock consisting primarily of sandstone and shale. The unconsolidated materials also include fill, silty and clayey residuum, and minor amounts of sandy, clayey channel fill alluvium.

The Unit is located on upland area near the west bank of the Green River. The uppermost edge of the earthen embankment is situated at an elevation of approximately 429 feet above mean sea level (amsl). Although the Green River is located less than 0.5 miles from the site, the structure does not extend significantly into the floodplain. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands (Associated Engineers 2016, Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan). The immediate watershed that drains to the unit, and in which the unit is considered to be located, is unnamed and 25.45 acres in size. The unnamed watershed discharges from the Unit outflow structure and is routed, under a Kentucky Pollution Discharge and Elimination System permit, to the Green River.

#### 2.3.2 Geology

The Unit lies in the Western Kentucky Coalfields section, characterized by rolling uplands underlain by coal-bearing bedrock of the Pennsylvanian Period. Near the Unit, maximum topographic relief is on the order of 80 feet. The geologic quadrangle (Geologic map of the Robards quadrangle, Henderson and Webster Counties, Kentucky, 1973) for the area published by the Kentucky Geologic Survey (KGS) shows the surficial material in portions of the western half of the Unit to be unconsolidated loess representing the Pleistocene geologic epoch. The loess consists of sandy and clayey silt. Underlying the loess deposits and exposed at the surface on the eastern half of the Unit are broadly distributed Pleistocene and Holocene alluvium deposits consisting of intermixed and interlensing clay, silt, sand, and gravel. In close proximity to the Unit, the alluvium is generally a low permeability unit that forms terraces along the Green River at elevations of roughly 380 and 395 ft., amsl. The unconsolidated surficial materials range from approximately 24 feet (MW-7) to 47 feet (MW-110) in thickness surrounding the Unit.

The unconsolidated materials are underlain by bedrock of the Upper Pennsylvanian Shelburn Formation [formerly identified as the Lisman Formation (Fairer, 1973)] and the Middle Pennsylvanian Carbondale Formation. At the base of the Shelburn Formation is the Providence Limestone Member, consisting of two distinct limestone beds separated by a sandy shale. The member is exposed in a streambed near the northwest corner of the Unit but is absent beneath much of the Unit footprint due to erosional channeling. The underlying Carbondale Formation consists of cyclic sequences of sandstones, shales, siltstones and coals. The Carbondale sediments were deposited in a fluvial-deltaic system. As a result of this depositional environment, the lithologic units of the Carbondale tend to be lenticular bodies rather than continuous sheet-like strata. Gradational and abrupt horizontal changes in lithology are often encountered.

#### 2.3.3 Groundwater Hydrogeology

For purposes of compliance with the CCR Rule groundwater monitoring requirements, the interbedded sandstone and shale of the Carbondale Formation is considered the uppermost aquifer underlying the Unit. The uppermost aquifer is hydraulically confined and first encountered at an elevation of approximately 425 ft., amsl at the northeast end (at MW-7), and 388 ft. amsl at the west end of the Unit (at MW-9).

Groundwater elevation data collected in October 2019 are summarized on **Table 3** below. These data were utilized to construct a piezometric surface map illustrating groundwater flow conditions for the uppermost aquifer (see **Figure 3**). Flow direction beneath the Unit is to the southwest towards an unnamed tributary to Groves Creek located west-southwest of the impoundment.

Table 3. Reid/HMP&L Surface Impoundment – October 2019 Groundwater Elevation Data

Monitoring Well	Top of Casing Elevation (ft) <sup>1</sup>	Depth to Groundwater (ft)	Groundwater Elevation (ft, amsl)
MW-7	444.43	18.59	425.84
MW-8	394.29	5.20	389.09
MW-9	395.40	7.35	388.05
MW-10	422.27	33.28	388.99

Reference elevation of monitoring wells surveyed by Associated Engineers, Inc., Madisonville, Kentucky, January 2015. Survey coordinates were based on the Kentucky State Plane, Kentucky Southern Zone, NAD27 datum.

Slug tests were performed between April 24, 2019 and April 25, 2019 at monitoring wells MW-10, and MW-110 to assess the hydraulic characteristics of the uppermost aquifer. The estimated hydraulic conductivity of the monitoring wells tested ranged from 3 x  $10^{-6}$  to 5 x  $10^{-4}$  centimeters per second (cm/sec).

Although previous site-specific investigations have noted the presence of perched zones of saturation in the overlying unconsolidated materials, these discontinuous zones do not qualify as an uppermost aquifer under the CCR Rule because they do not produce usable quantities of groundwater.

#### 2.3.4 Constituents of Concern

Current groundwater analytical data and statistical analysis indicate that the only COC detected at SSLs above its GWPS in groundwater at the Unit is lithium. Lithium has been detected at SSLs in the monitoring well MW-10 southwest of the Unit.

#### 2.3.5 Impacted Media

Groundwater is the single impacted media of concern identified as requiring corrective measures at the Unit.

#### 2.3.6 Distribution of COCs

Groundwater sampling was performed at the Unit most recently in April 2020. The additional lithium data collected during this event are summarized below in **Table 4**.

Table 4. Reid/HMP&L Surface Impoundment - April 2020 Lithium Analytical Results

	Parameter	
Monitoring Well (Date)	Lithium GWPS 0.04	
	(mg/L)	
MW-7	0.007	
MW-8	0.03	
MW-9	0.01	
MW-10	0.49	
MW-110	0.02	

**Figure 4** illustrates the distribution of COCs and other groundwater quality constituents in groundwater at the Unit. This distribution of COCs in groundwater suggests that impacts to groundwater likely originate as seepage from beneath the surface impoundment, however there is currently no feasible means of directly tracing that potential under the footprint of the Unit.

#### 2.3.7 Potential Receptors/Exposure Pathways

Contact with water (e.g., shallow groundwater or surface water) impacted by COCs at levels above GWPS or Water Quality Criteria is regarded as the potential pathway for exposure of potential receptors. Based on data published by KGS, there are no known groundwater wells used for drinking water within a 1-mile radius of the Unit, thus limiting the potential receptors to the surface water, i.e., the Green River and its tributary, Groves Creek. The pathways to these receptors include seepage of water from the Unit through manmade and natural hydraulic conduits.

Other potential exposure pathways (e.g., soil or vapor) are not considered complete as the CCR material is isolated in the Unit. This isolation prevents direct access by individuals that might result in direct contact or ingestion. In addition, the inherent non-volatile nature of the Unit-specific COCs eliminates the potential for a complete vapor pathway (i.e., vapor intrusion to indoor air).

#### 2.4 Interim Corrective Measures

No interim corrective measures have been performed at the Unit for groundwater impacts.

#### 2.5 Assessment of Corrective Measures Summary

In June 2019, BREC performed an ACM for the Unit to identify remedial alternatives to address groundwater impacts. Title 40 CFR Part 257.96(c) requires that the ACM include an analysis of the effectiveness of potential corrective measures in meeting the objectives for remedies identified under Section 257.97(b), by addressing at least the following:

- The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- The time required to begin and complete the remedy; and
- 3) The institutional requirements, such as state or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the remedy(s).

As part of the groundwater ACM, several potential corrective measures technologies were evaluated to identify which ones could be carried forward as components of corrective measures alternatives. The results of the corrective measures technology evaluation are presented below in **Table 5**.

Table 5 - Potential Corrective Measures Options for Groundwater Impacts

Potentially Applicable Technology	Status	Description/Overview
No Action	Not retained as standalone technology, but carried forward for baseline comparisons	This technology has been included in the preliminary evaluation/screening but is not retained because it will not meet the established CAOs.
Institutional Controls (ICs)	Retained as supplement to corrective measures alternatives	The use of ICs (i.e., Environmental Covenants, groundwater use restrictions, etc.) is retained as a useful technology. However, it is noted the ICs are not anticipated to be used as a stand-alone technology. Environmental Covenants, groundwater use restrictions, etc., are expected to be combined with other applicable technologies as part of corrective measures alternatives.

Potentially Applicable Technology	Status	Description/Overview
Groundwater Monitoring (Assessment and Detection modes)	Retained as supplement to corrective measures alternatives	The use of groundwater monitoring (Assessment and/or Detection modes as appropriate) when combined with other applicable technologies as part of any proposed corrective measures alternative is retained to address the CAO and to track the effectiveness of the overall remedy. However, it is not retained as a stand-alone technology.
Hydraulic Containment	Retained	The use of hydraulic containment is retained because it is an effective means of preventing off-site migration of soluble contaminants. Hydraulic containment requires management and potential ex-situ treatment of extracted groundwater, so it is not a stand-alone technology. The CSM will guide the design of any groundwater extraction system to optimize the total discharge of groundwater needed to provide hydraulic containment.
Physical Containment	Retained	The use of physical containment is retained because it can be an effective means of managing groundwater flow. Physical containment often requires pairing with hydraulic containment and/or in-situ treatment (funnel and gate style) to manage the flux of groundwater flow into the system. The CSM will guide the design of any physical barrier system, but technology limitations may increase implementation difficulty with scale.
Ex-situ Physical/Chemical/Biological Treatment	Retained	Ex-situ treatment technologies are retained as a way of removing contaminants from extracted groundwater from a hydraulic containment system. Ex-situ treatment may be paired with wastewater treatment, non-groundwater release treatment systems, or with permitted discharge to manage groundwater contamination. The CSM and data gaps investigations will guide the design of any ex-situ treatment.
Closure in Place (CiP) (of the regulated unit)	Retained	The use of CiP as a source control technology and is amenable with respect to CAO attainment.
Closure by Removal (CbR) (of the regulated unit)	Retained	The use of CbR as a source control technology is amenable with respect to CAO attainment.
Other Source Control Technologies  Note: Technologies that were retain	Retained	Control of source area non-groundwater related releases. For the purposes of this groundwater ACM, management of non-groundwater releases are not included in the alternatives evaluation. Engineering measures, including leachate collection, lining of trenches and/or ponds, and other isolation methods are regarded as part of closure technologies selected by other means.

Technologies that were retained may be used as components of a corrective action alternative, but when evaluated in conjunction with other available technologies any single technology may not be utilized.

Preliminary assembly of corrective measures alternatives was performed based on site-specific and regional geology and groundwater conditions. For the Reid/HMP&L Station Surface Impoundment, five corrective measures alternatives were developed from this list of applicable corrective measures technologies:

Alternative #1 – No Action, and Groundwater Monitoring

- Alternative #2a Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring
- Alternative #2b Closure by Removal (CbR), ICs, and Groundwater Monitoring
- Alternative #3 CiP, ICs, Hydraulic Containment, Other Source Control, Ex-Situ Treatment, and Groundwater Monitoring
- Alternative #4 CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

The assembly of corrective measures alternatives presented in the ACM was considered preliminary and subject to revision following additional evaluation during the remedy selection process and/or following comment from the regulatory community and public. Further evaluation of the alternatives is discussed in the following sections.

#### 3. Remedy Selection Progress

The ACM performed for the Unit in June 2019 identified a total of five (5) corrective measures alternatives to be carried forward into the remedy selection process. In December 2019, BREC provided a *Semi-annual Remedy Selection Progress Report* (AECOM, December 2019) as required under 40 CFR Part 257.97(a). As part of this submittal, two (2) corrective measures alternatives were eliminated from further consideration, including:

- Alternative #1 (No Action and Groundwater Monitoring) This alternative does not control or remove COCs from the environment and therefore does not achieve the RAOs.
- Alternative #2b (CbR, ICs, and Groundwater Monitoring) Implementing a CbR approach is
  considered cost prohibitive. In addition, any CbR approach would require relocating waste to an
  existing disposal unit or construction of a new waste disposal unit, which does not align with the
  one of the fundamental goals of RCRA (conserving energy and natural resources).

Three (3) potential corrective measures alternatives have been identified by BREC as viable options to address lithium impacts in groundwater at the Unit, including:

- Alternative #2a: CiP, ICs, and Groundwater Monitoring
- Alternative #3: CiP, ICs, Hydraulic Containment, Ex-Situ Treatment, and Groundwater Monitoring
- Alternative #4: CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

Each of the remaining 3 corrective measures alternatives is discussed in more detail below.

#### 3.1 Potential Corrective Action Alternatives

#### 3.1.1 Alternative #2a – CiP, ICs, and Groundwater Monitoring

Alternative #2a as currently envisioned would employ a combination of three corrective measures technologies:

- CiP source control, which consists of planned Reid/HMP&L Surface Impoundment closure activities;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater monitoring (Assessment) to document the effectiveness of the corrective measures.

Alternative #2a is recommended for further evaluation.

## 3.1.2 Alternative #3 – CiP, ICs, Hydraulic Containment, Ex-Situ Treatment, and Groundwater Monitoring

Alternative #3 builds on Alternative #2a to also include the addition of Hydraulic Containment and Ex-Situ Treatment of groundwater:

- CiP source control, which consists of planned Surface Impoundment closure activities;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes;
- Hydraulic Containment using one or more vertical wells designed to prevent the movement of impacted groundwater past the limits of the unit to the downgradient groundwater environment and potential points of exposure;

- Ex-Situ Treatment of groundwater extracted for hydraulic containment, which involves aboveground physical/chemical treatment methods and/or permitted discharge until the CAOs are achieved;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater Monitoring (Assessment mode) to track the effectiveness of the corrective measures
  and to identify conditions that allow the return to Detection-mode monitoring and ultimately to
  cessation of corrective measures.

Alternative #3 is recommended for further evaluation.

## 3.1.3 Alternative #4 – CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

Alternative #4 consists of BREC's planned unit closure activities, physical containment of impacted groundwater via installation of a funnel-gate system, and ex-situ treatment of contained groundwater via an extraction well installed at the containment gate. Impacted groundwater would be contained by grout curtain constructed in a funnel-and-gate arrangement that directs the flow of groundwater to an extraction point. The grout curtain would be installed by drilling two lines of grout injection points that extend northwestward and northeastward from the southeast corner of the unit. The length of each limb of the barrier would be 500 feet, and the target depth would be approximately 325 ft-amsl. A single extraction well would be installed at the "gate" with a screened interval of 50 to 100 ft-bgs and a pumping capacity of up to 20 gpm. Groundwater will be pumped and conveyed to an existing surface water impoundment at the Sebree Station, which will allow for compliance with discharge permits through an established NPDES outfall.

CiP via ash stabilization and capping would control the source of COCs and thereby reduce contaminant loading to the extraction system. Concentrations downgradient of the physical barrier would be expected to decrease over time through several natural attenuation mechanisms including advection, dilution, and dispersion. Groundwater Monitoring (Assessment) would continue to track the effectiveness of the corrective measures and to identify conditions that allow the return to Detection monitoring and ultimately closure.

Alternative #4 is recommended for further evaluation.

#### 3.2 Remedy Evaluation

Currently BREC considers the (3) potential corrective action alternatives as viable options to address groundwater impacts at the Unit, including:

- Alternative #2a;
- Alternative #3; and
- Alternative #4

To evaluate each alternative, additional data collection will likely be required. BREC is currently evaluating data collection needs in the following areas to assist with remedy selection:

- Nature and Extent groundwater trends, influence of non-groundwater remedies, etc.
- 2) Physical Characteristics available data on the physical characteristics of the landfill and retention pond
- 3) Performance Modeling data needed to develop digital models demonstrating the effectiveness of potential alternatives
- 4) Engineering feasibility, cost estimates, etc.

BREC is working to establish a comprehensive list of data collection needs to proceed forward with remedy evaluation and anticipates providing additional data in future semi-annual remedy selection progress reports.

#### 3.3 Public Meeting

At the beginning of 2020, BREC had initiated preparation to conduct a public meeting to discuss the results of the Groundwater ACM as required by 40 CFR 257.96(e). However, due to the onset of the COVID-19 pandemic, BREC has been prevented from holding the public meeting so far in 2020. BREC plans to hold a public meeting once the mass gathering restrictions related to COVID-19 are lifted in Kentucky.

#### 4. Conclusion

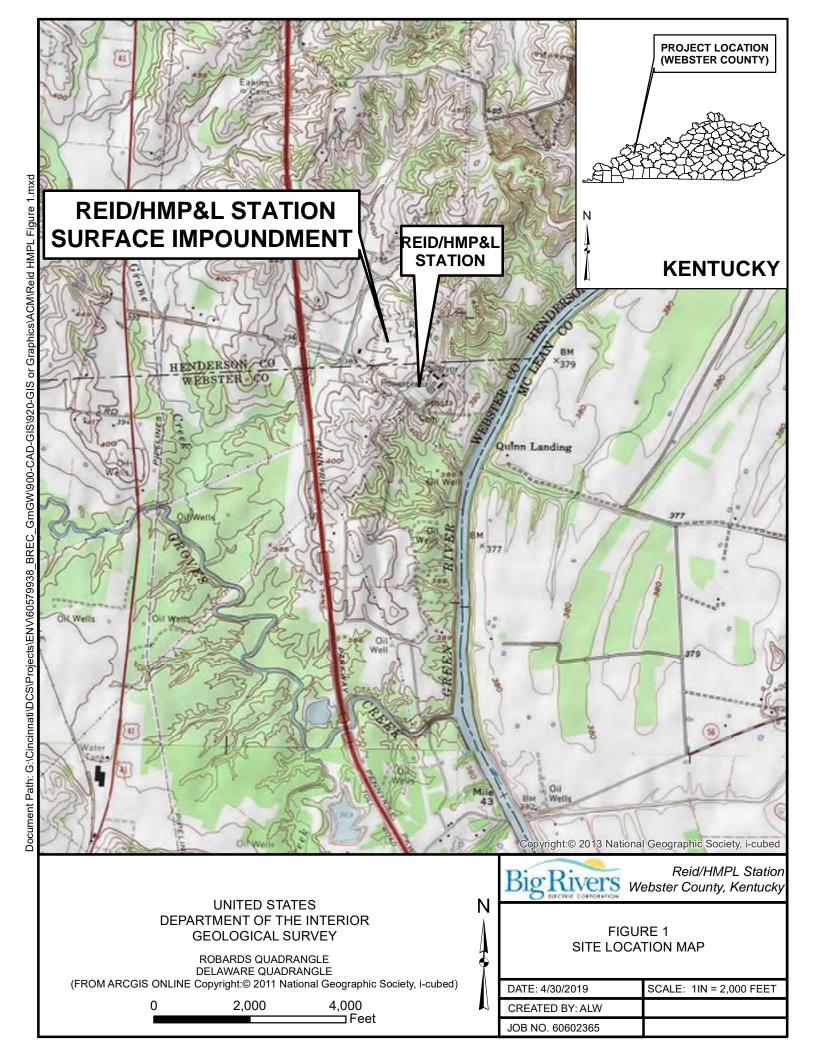
Additional updates regarding remedy selection, including any additional corrective measures being considered, will be presented twice a year in future remedy selection progress reports. Once sufficient data has been collected to select an effective comprehensive remedy for the Unit, a public meeting will be held 30 days prior to formal remedy selection, followed by a detailed Remedy Selection Report describing the remedy and proposed schedule for implementation.

If needed, the next remedy selection progress report for the Unit is expected in December 2020.

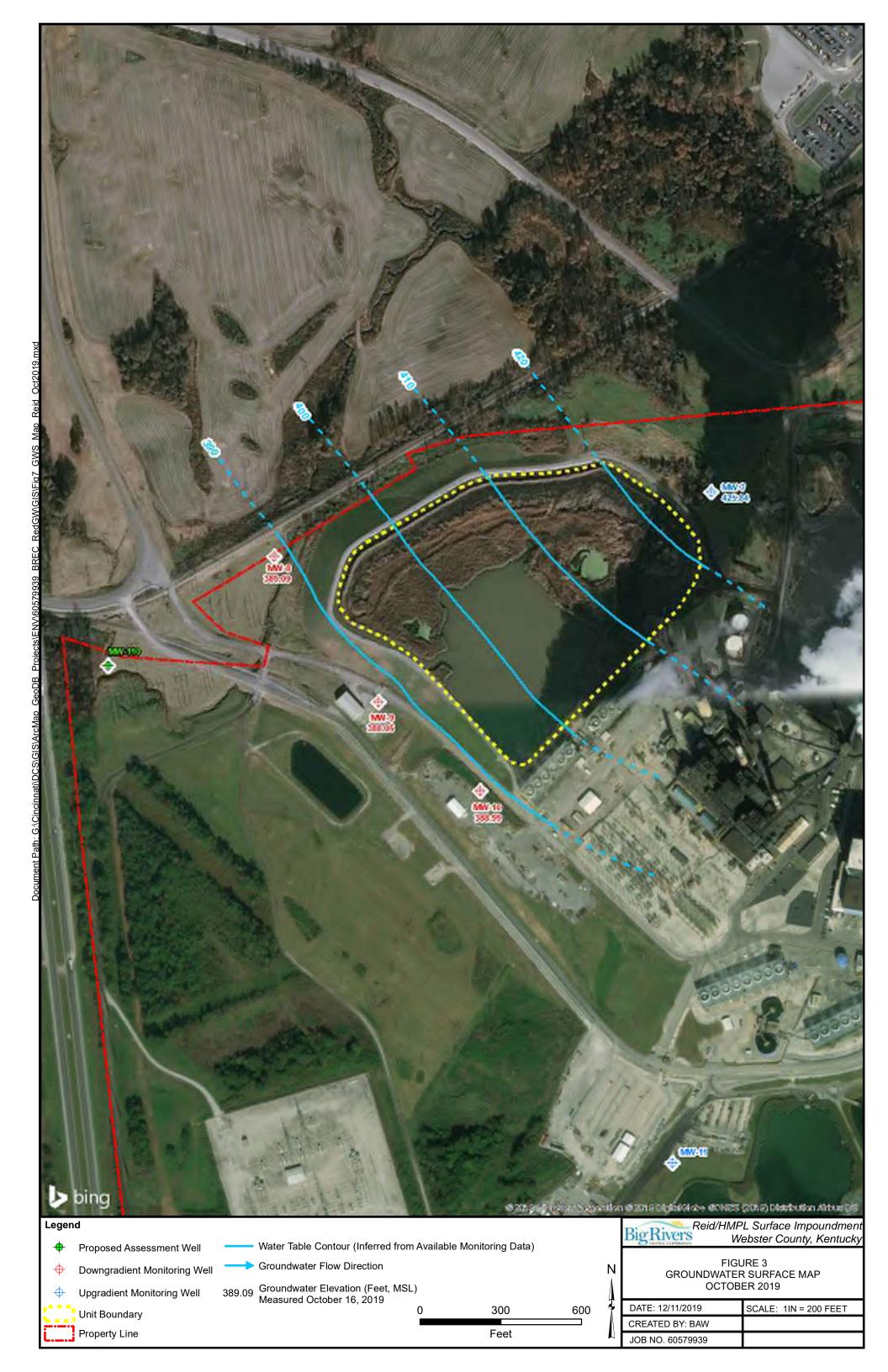
#### 5. References

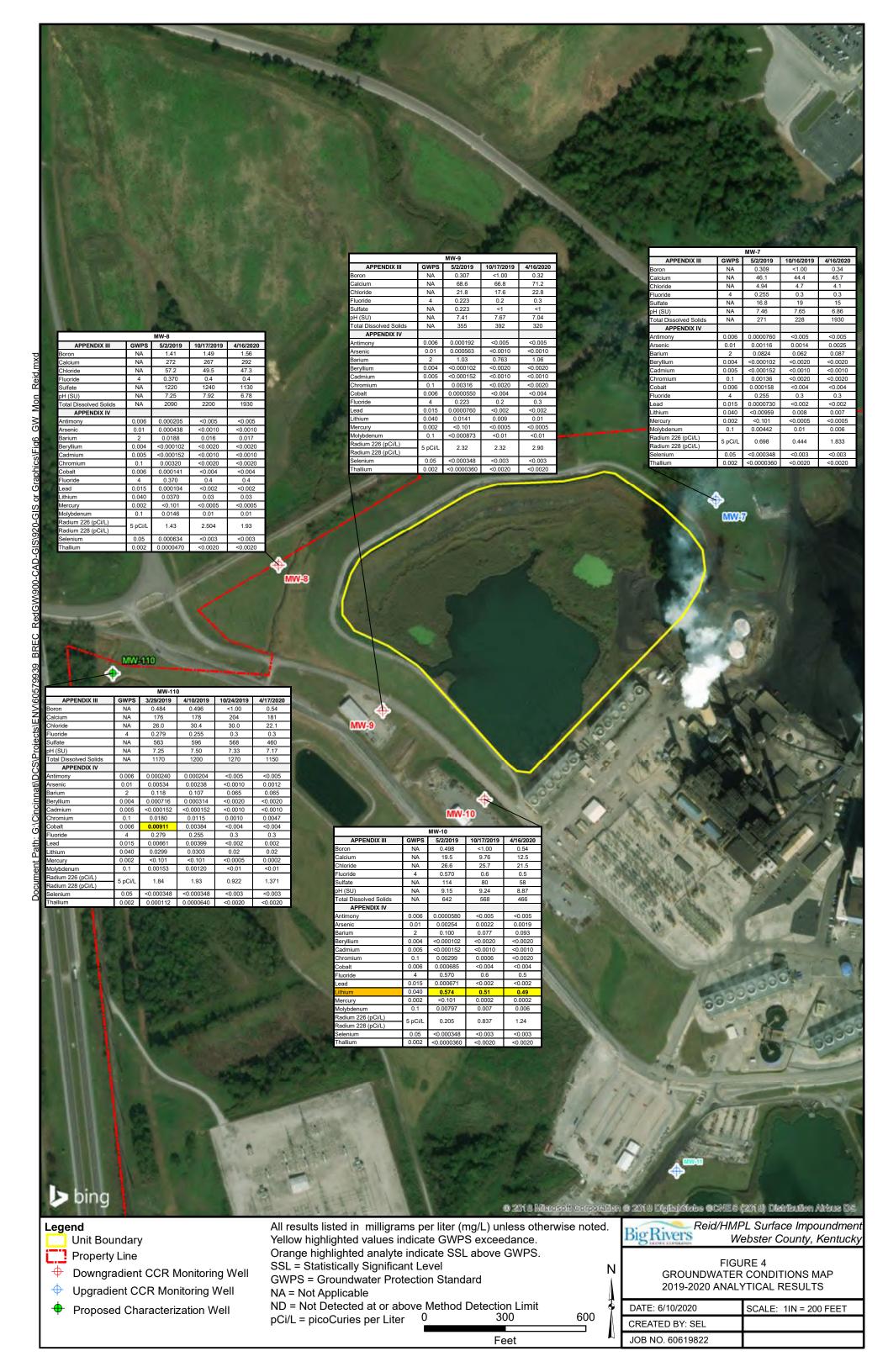
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## **Figures**









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# Semi-Annual Remedy Selection Progress Report

Green Landfill Sebree Station Webster County, Kentucky

Prepared for:



Big Rivers Electric Corporation Sebree Generating Station 9000 Highway 2096 Robards, KY 42452

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June 8, 2020

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	1	Big Rivers Electric Corporation

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### 1. Introduction

In accordance with provisions of the United States Environmental Protection Agency's (USEPA) coal combustion residual (CCR) rule, Title 40 of the Code of Federal Regulations (CFR) Part 257.97, Big Rivers Electric Cooperation (BREC) is in the process of selecting a remedy for groundwater impacts at the Green Station CCR Landfill (the Unit) at the Sebree Generating Station located in Webster County, Robards, Kentucky (Figure 1).

Assessment monitoring results indicate the presence of lithium at a Statistically Significant Level (SSL) above the Ground Water Protection Standard (GWPS) in four monitoring wells (MW-3A, MW-4, MW-5, and MW-6) at the Unit. A map illustrating the site with location of all program monitoring wells is presented as **Figure 2**.

In response to the SSL exceedance, BREC evaluated the nature and extent of groundwater impacts as required by Title 40 CFR Part 257.95(g) for characterization monitoring. In addition, BREC performed an Assessment of Corrective Measures (ACM), to identify applicable remedial technologies to address lithium impacts in groundwater pursuant to Tile 40 CFR Part 257.96. A notice of ACM initiation dated January 14, 2019 was posted to BREC's publicly-accessible CCR reporting website. A report summarizing the results of the ACM (AECOM, June 2019) was posted to BREC's publicly-accessible CCR reporting website on June 14, 2019.

Title 40 CFR Part 257.97(a) requires that progress reports be prepared on a semi-annual basis describing progress made in selecting and designing a remedy. The first semi-annual *Remedy Selection Progress Report* (AECOM, December 2019) was posted to BREC's publicly-accessible CCR reporting website on December 9, 2019. In alignment with the CCR rule requirement, the following sections included within this semi-annual progress report provide an overview of BREC's activities previously performed, currently underway, and planned in the future to select a remedy that meets the requirement of Title 40 CFR Section 257.97 (b) as follows:

- (1) Be protective of human health and the environment;
- (2) Attain the GWPS as specified pursuant to Section 257.95(h);
- (3) Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into the environment;
- (4) Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors such as avoiding inappropriate disturbance of sensitive ecosystems;
- (5) Comply with standards for management of wastes as specified in Section 257.98(d).

### 2. Site Background

### 2.1 Site Description

BREC owns and operates Sebree Station, which is a coal-fired power generating facility located on the Green River northeast of Sebree, Kentucky. Sebree Station is composed of Green Station and Reid/Henderson Municipal Power & Light (HMP&L) Station. The Sebree Station is bounded by Interstate-69 to the west and the Green River to the east (see **Figure 1**). Reid Unit 1 (66 Megawatts [MW]) began commercial operation in 1966 and is scheduled to be retired in 2020 pending regulatory approval from the Kentucky Public Service Commission and Rural Utilities Service. The Reid Combustion Turbine (72 MW) was commercialized in 1976. HMP&L Station 2, Units 1 (167 MW) and 2 (168 MW) began commercial operation in 1973 and 1974 respectively. Both HMP&L units were retired as of February 1, 2019. Green Station Units 1 (250 MW) and 2 (242 MW) began commercial operation in 1979 and 1981, respectively.

The location of the Green Landfill is illustrated on **Figure 1**. The Green Landfill is located directly south of Sebree Station, situated south of the Green Station CCR Surface Impoundment. The Green Landfill is a Kentucky permitted landfill (Permit No. SW11700007) that currently receives special waste generated by burning coal (CCRs) from Green Stations. The Reid and HMP&L stations historically disposed special waste in the Green Landfill. The landfill began receiving CCR wastes in 1980. The current Green Landfill footprint is approximately 170 acres.

As stated in the published CCR monitoring well network certification, available on the BREC website (http://www.bigrivers.com/), the original ground surface within the landfill footprint was irregular and the dominant features were small stream valleys draining towards the Green River, which is located just east of the landfill; and towards Groves Creek, which is located just south of the landfill. There was also historic oil and gas production at and in the immediate vicinity of the Green Landfill. A review of the records from the Kentucky Geological Survey (KGS) showed that at or immediately adjacent to the Site, there were a number of dry exploratory oil/gas exploration holes, oil production wells, one gas production well, and one secondary recovery injection well. There were also former brine ponds at the Site. Most of these wells were abandoned in accordance with applicable regulations by BREC in 1997 and 1998. The last existing oil well was decommissioned in 2019.

### 2.2 Groundwater Investigation Summary

Monitoring wells were installed at the Unit beginning in November 1996 prior to the implementation of the CCR Rule. However, the existing wells meet the requirements of Title 40 CFR Section 257.90 of the CCR Rule for installation of a groundwater monitoring system. These regulations require that monitoring wells adequately represent the quality of background groundwater and groundwater representing the downgradient waste boundary. The existing wells are located along the perimeter of the landfill footprint. One upgradient monitoring well (MW-1) and five downgradient monitoring wells (MW-2, MW-3A, MW-4, MW-5 and MW-6) were installed at the Unit to determine the general direction of groundwater movement and to monitor groundwater impacts. One additional characterization monitoring well (MW-104) was installed downgradient of the Unit in 2018. All monitoring wells were installed in the uppermost saturated portion of the sandstone bedrock aquifer. A map illustrating the location of all program monitoring wells is presented as **Figure 2**.

Nine rounds of Baseline groundwater sampling for Appendix III constituents were conducted between March 2016 and October 2017. Statistical evaluation for Detection monitoring indicated that statistically significant increases (SSIs) over background had occurred, and therefore, Assessment monitoring was triggered. Detection monitoring activities and data are presented in the annual reports that have been prepared to date, (AECOM 2018, 2019, and 2020).

As part of Assessment monitoring, upgradient and downgradient wells for the Unit were sampled for Appendix IV constituents in June, July, and September 2018. GWPSs were established for the Appendix

IV constituents occurring at SSIs (lithium only), and statistical evaluation of the lithium concentrations indicated exceedances of GWPSs at SSLs, as detailed in **Table 1** below.

**Table 1. Green Landfill Constituents of Concern** 

Monitoring Well (Date)	Parameter Lithium GWPS 0.04 (mg/L)
MW-3A (Jun 2018)	0.699
MW-3A (Jul 2018)	0.790
MW-3A (Sep 2018)	0.766
MW-4 (Jun 2018)	1.81
MW-4 (Jul 2018)	1.91
MW-4(Sep 2018)	1.81
MW-5(Jun 2018)	0.459
MW-5 (Jul 2018)	0.481
MW-5 (Sep 2018)	0.425
MW-6 (Jun 2018)	0.0650
MW-6 (Jul 2018)	0.0590
MW-6 (Sep 2018)	0.0558

GWPSs are the greater of the site-specific background concentrations, the USEPA primary drinking water standard maximum contaminant limits (MCL), or GWPS provided in 40 CFR 257.95(3)(h)(2)

An additional characterization well, MW-104, was subsequently installed to estimate the downgradient extent of impacted groundwater. Sample collection from MW-104 for Appendix III and IV parameters took place in March and April 2019. The analytical results for lithium were below the GWPS. The additional characterization data are summarized in **Table 2** below.

Table 2. Green Landfill -2019 Characterization Sample Results

	Parameter
Monitoring Well (Date)	Lithium GWPS 0.04 <sup>a</sup> (mg/L)
MW-104 (March 2019)	0.0281
MW-104 (April 2019)	0.0288

a The Upper Prediction Limit for lithium was calculated as 0.008 mg/L.

The results from both characterization sampling events helped to confirm the downgradient (southwestern) extent of constituent of concern (COC) impacts above GWPS at the Unit. However, it should be stated that downgradient characterization is limited due to the presence of the Green River immediately adjacent to the Unit.

Semi-annual Assessment monitoring continued at the Unit in 2019 and 2020 in accordance with 40 CFR 257.95.

### 2.3 Conceptual Site Model

Development and refinement of a Conceptual Site Model (CSM) is necessary to support remedy selection for the Unit. A CSM is based on a set of working hypotheses regarding how contaminants of concern (COCs) entered the environment at a site, how they were and continue to be transported to various media, what the potential routes of exposure are, and who may be exposed, including both human and ecological receptors. As such, the CSM is a "living" model. As new data become available or site conditions change, a CSM should be evaluated and updated as necessary.

The CSM for the Unit was first provided in the June 2019 ACM for the Unit (AECOM 2019). The CSM presents the physical setting of the Unit (adjacent to the Green River), the unconsolidated and bedrock geologic strata underlying the Unit, the occurrence and movement of groundwater, the distribution of COCs in groundwater, and the potential receptors (or lack thereof) for impacted groundwater. These elements are described in detail below and have been updated with new information for this report as appropriate.

### 2.3.1 Physical Setting

The Unit is located within the Interior Low Plateaus physiographic province. The province is part of the Interior Plains division of the United States. Characteristic features of the province include unglaciated rolling limestone plains with alluvial valleys and entrenched rivers and streams. Several large rivers are in the region, including the Green, Ohio, Kentucky, Tennessee, and the Cumberland Rivers. The geology underlying the Unit consists of unconsolidated materials, including loess and alluvial deposits, underlain by Upper to Middle Pennsylvanian-age clastic and carbonate bedrock consisting primarily of sandstone and shale. The unconsolidated materials also include fill, silty and clayey residuum, and minor amounts of sandy, clayey channel fill alluvium.

The Unit is located on an upland adjacent to the west bank of the Green River at an elevation of approximately 436 feet, above mean sea level [ft., amsl] (at the north end of the landfill) and 397 ft., amsl (at the south end of the landfill), with a maximum elevation of 608 ft., amsl at the landfill crest. Precipitation falling within the Green Landfill is directed to ponds on the north and south sides of the Unit and then to the river under Kentucky Pollution Discharge and Elimination System (KPDES) permit No. KY0001929. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands (Associated Engineers 2016, Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan).

### 2.3.2 Geology

The Unit lies in the Western Kentucky Coalfields section, characterized by rolling uplands underlain by coal-bearing bedrock of the Pennsylvanian Period. Near the Unit, maximum topographic relief is on the order of 80 feet. The geologic quadrangle (Geologic map of the Robards quadrangle, Henderson and Webster Counties, Kentucky, 1973) for the area published by the Kentucky Geologic Survey (KGS) shows the surficial material in portions of the western half of the Unit to be unconsolidated loess representing the Pleistocene geologic epoch. The loess consists of sandy and clayey silt. Underlying the loess deposits and exposed at the surface on the eastern half of the Unit are broadly distributed Pleistocene and Holocene alluvium deposits consisting of intermixed and interlensing clay, silt, sand, and gravel. In close proximity to the Unit, the alluvium is generally a low permeability unit that forms terraces along the Green River at elevations of roughly 380 and 395 ft., amsl. The unconsolidated surficial materials range from approximately 10 feet (MW-5) to 52 feet (MW-104) in thickness surrounding the Unit. Figure 3 provides an excerpt from the geologic quadrangle for the immediate area surrounding the Unit.

The unconsolidated materials are underlain by bedrock of the Upper Pennsylvanian Shelburn Formation [formerly identified as the Lisman Formation (Fairer, 1973)] and the Middle Pennsylvanian Carbondale Formation. At the base of the Shelburn Formation is the Providence Limestone Member, consisting of two distinct limestone beds separated by a sandy shale. The member is exposed in a streambed near the

northwest corner of the Unit but is absent beneath much of the Unit footprint due to erosional channeling. The underlying Carbondale Formation consists of cyclic sequences of sandstones, shales, siltstones and coals. The Carbondale sediments were deposited in a fluvial-deltaic system. As a result of this depositional environment, the lithologic units of the Carbondale tend to be lenticular bodies rather than continuous sheet-like strata. Gradational and abrupt horizontal changes in lithology are often encountered.

### 2.3.3 Groundwater Hydrogeology

For purposes of compliance with the CCR Rule groundwater monitoring requirements, the interbedded sandstone and shale of the Carbondale Formation is considered the uppermost aquifer underlying the Unit. The uppermost aquifer is hydraulically confined and first encountered at an elevation of approximately 401 ft., amsl at the northwest end of the landfill, and 367 ft., amsl at the southeast end of the landfill (AECOM, 2019).

Groundwater elevation data collected in April 2020 are summarized on **Table 3** below. These data were utilized to construct a piezometric surface map illustrating groundwater flow conditions for the uppermost aquifer (see **Figure 3**). Overall groundwater flow beneath the footprint of the Unit is to the east towards the Green River and south-southeast towards Groves Creek.

Monitoring Well	Top of Casing Elevation (ft) <sup>1</sup>	Depth to Groundwater (ft)	Groundwater Elevation (ft, amsl)
MW-1	423.23	19.52	403.71
MW-2	392.37	16.24	376.13
MW-3A	386.48	12.08	374.40
MW-4	391.33	17.90	373.43
MW-5	390.18	17.62	372.56
MW-6	388.17	15.62	372.55
MW-12 <sup>2</sup>	395.54	22.15	373.39

Table 3. Green Landfill -April 2020 Groundwater Elevation Data

Slug tests were performed on April 25, 2019 at monitoring wells MW-3A, MW-4, MW-6, and MW-104 to assess the hydraulic characteristics of the uppermost aquifer. The estimated hydraulic conductivity of the monitoring wells tested ranged from  $2 \times 10^{-5}$  to  $3 \times 10^{-3}$  centimeters per second (cm/sec).

Although previous site-specific investigations have noted the presence of perched zones of saturation in the overlying unconsolidated materials, these discontinuous zones do not qualify as an uppermost aquifer under the CCR Rule because they do not produce usable quantities of groundwater.

### 2.3.4 Constituents of Concern

Current groundwater analytical data and statistical analysis indicate that the only COC detected at SSLs above its GWPS in groundwater at the Unit is lithium. Lithium has been detected at SSLs in the wells MW-4, MW-5, and MW-6 surrounding the South Pond and in MW-3A located north (downstream on the Green River) of MW-4.

<sup>1</sup> Reference elevation of monitoring wells surveyed by Fuller, Mossbarger, Scott and May, Civil Engineers, Inc., Lexington, Kentucky, December 1996, December 1999. Survey coordinates were based on the Kentucky State Plane, Kentucky Southern Zone, NAD27 datum.

MW-12 is utilized for collection of piezometric data only and is not part of the CCR monitoring well network for the Green Landfill.

### 2.3.5 Impacted Media

Both groundwater and surface water have been identified as impacted media of concern requiring corrective measures at the Unit.

#### 2.3.6 Distribution of COCs

Groundwater sampling was performed at the Unit most recently in April 2020. The additional lithium data collected during this event are summarized below in **Table 4**.

Table 4. Green Landfill - April 2020 Lithium Analytical Results

	Parameter
Monitoring Well (Date)	Lithium GWPS 0.04
	(mg/L)
MW-1	0.03
MW-2	0.007
MW-3A	0.68
MW-4	0.82
MW-5	0.38
MW-6	0.05

**Figure 4** illustrates the distribution of COCs and other groundwater quality constituents in groundwater at the Unit. This distribution of COCs in groundwater suggests that impacts to groundwater likely originate from two primary source area. Impacts observed at MW-4, MW-5 and MW-6 likely originated as infiltration from the South Pond where storm water and landfill seepage accumulate on the south side of the landfill before being pumped to the Green Surface Impoundment. Data from characterization well MW-104 indicate that MW-3A may be effectively separated from the South Pond by a buried valley in the bedrock aquifer where groundwater does not appear to be impacted. This suggests that the impact observed at MW-3A may have instead originated from a different source, potentially from localized landfill seepage, which is now captured by the Deep Seep Collection Trench (see Section 2.4). It is possible that the MW-3A impact originates from the western end of the South Pond, but there is currently no feasible means of directly tracing that potential under the footprint of the landfill. It is, however, possible to evaluate this potential by monitoring MW-3A over time after the South Pond is rehabilitated as is currently planned. Ongoing monitoring of MW-3A also has the potential to demonstrate whether the landfill seepage intercepted by the Deep Seep Collection Trench is the source of impact.

### 2.3.7 Potential Receptors/Exposure Pathways

Contact with water (e.g., shallow groundwater or surface water) impacted by COCs at levels above GWPS or Water Quality Criteria is regarded as the potential pathway for exposure of potential receptors. Based on data published by KGS, there are no known groundwater wells used for drinking water within a 1-mile radius of the Unit, thus limiting the potential receptors to the surface water, i.e., the Green River and its tributary, Groves Creek. The pathways to these receptors include seepage of water from the Unit through manmade and natural hydraulic conduits.

Other potential exposure pathways (e.g., soil or vapor) are not considered complete as the CCR material is isolated in the Unit. This isolation prevents direct access by individuals that might result in direct contact or ingestion. In addition, the inherent non-volatile nature of the Unit-specific COCs eliminates the potential for a complete vapor pathway (i.e., vapor intrusion to indoor air).

### 2.4 Interim Corrective Measures

In September and October 2019 BREC initiated design and construction of two containment systems intended as an interim corrective measure to reduce and prevent non-groundwater releases at the Unit from reaching the Green River. The containment systems are identified as the Deep Seep Collection Trench (also known as the Eastern Collection Trench) and the Northwest Seep Collection Trench.

No formal interim corrective measures have been performed at the Green Landfill to address groundwater impacts. However, the interim corrective measures for known non-groundwater releases completed at the Unit are expected to benefit corrective action for groundwater impacts. The compatibility of those corrective measures with potential groundwater remedies is currently being evaluated as part of the Unit's assessment monitoring and will continued to be evaluated in the future as part of systematic performance reviews (see Section 5.2).

### 2.4.1 Deep Seep Collection Trench

BREC began construction of the Deep Seep Collection Trench on October 7, 2019. The installation of four partially overlapping trenches and an individual sump was completed on November 11, 2019. This completion allowed removal of collected seepage using temporary pumping and piping until the permanent system components were completed. The system became fully operational on May 29, 2020.

The Deep Seep Collection Trench is located on the eastern side of the landfill, adjacent to the Green River. This collection system consists of 1,065 lineal feet of perforated (HDPE) pipe and four (4) stainless steel sumps. The HDPE perforated pipe is surrounded by a washed river gravel, with profiles set at a 0.5% slope toward the associated pumping (sump) station. Each section of HDPE pipe overlaps at the sump interconnection to prevent seepage bypass and to ensure all deep seeps are properly captured. Each sump was set at an elevation of 352 ft., amsl.

Liquids collected within the Deep Seep Collection Trench are conveyed to a series of pumping stations/ponds that eventually discharge to the plant's main KPDES Outfall (#001).

#### 2.4.2 Northwest Seep Collection Trench

BREC began construction of the Northwest Seep Collection Trench on September 3, 2019. The construction of the collection trench was completed on January 22, 2020. The system is located in the northwest corner of the landfill and consists of 357 lineal feet of HDPE perforated pipe within the primary collection trench installed at an elevation of 391.4 ft, amsl. The HDPE perforated pipe is surrounded by a washed river gravel, with profiles set at a 0.5% slope toward the associated pumping (sump) station. Since the installation of the primary trench, BREC has installed two relay stations to ensure all possible seeps are captured and pumped to a permitted KPDES outfall. The Northwest Seep Collection Trench is configured to pump the incoming flow to a target manhole, which is located on the northeast corner of the landfill. The target manhole subsequently discharges to KPDES permitted outfall #009.

### 2.5 Assessment of Corrective Measures Summary

In June 2019, BREC performed an ACM for the Unit to identify remedial alternatives to address groundwater impacts. Title 40 CFR Section 257.96(c) requires that the ACM include an analysis of the effectiveness of potential corrective measures in meeting the objectives for remedies identified under Section 257.97(b), by addressing at least the following:

- The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- The time required to begin and complete the remedy; and
- 3) The institutional requirements, such as state or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the remedy(s).

As part of the groundwater ACM, several potential corrective measures technologies were evaluated to identify which ones could be carried forward as components of corrective measures alternatives. The results of the corrective measures technology evaluation are presented below in **Table 5**.

Table 5 – Potential Corrective Measures Options for Groundwater Impacts

Potentially Applicable Technology	Status	Description/Overview
No Action	Not retained as standalone technology, but carried forward for baseline comparisons	This technology has been included in the preliminary evaluation/screening but is not retained because it will not meet the established Corrective Action Objectives (CAOs).
Institutional Controls (ICs)	Retained as supplement to corrective measures alternatives	The use of ICs (i.e., Environmental Covenant, groundwater use restrictions, etc.) is retained as a useful technology. However, it is noted the ICs are not anticipated to be used as a stand-alone technology. Environmental Covenants, groundwater use restrictions, etc., are expected to be combined with other applicable technologies as part of corrective measures alternatives.
Groundwater Monitoring (Assessment and Detection mode)	Retained as supplement to corrective measures alternatives	The use of groundwater monitoring (Assessment and/or Detection modes as appropriate) when combined with other applicable technologies as part of any proposed corrective measures alternative is retained to address the CAO and to track the effectiveness of the overall remedy. However, it is not retained as a standalone technology.
Hydraulic Containment	Retained	The use of hydraulic containment is retained because it is an effective means of preventing offsite migration of soluble contaminants. Hydraulic containment requires management and potential ex-situ treatment of extracted groundwater, so it is not a stand-alone technology. The CSM will guide the design of any groundwater extraction system to optimize the total discharge of groundwater needed to provide hydraulic containment.
Physical Containment	Retained	The use of physical containment is retained because it can be an effective means of managing groundwater flow. Physical containment often requires pairing with hydraulic containment and/or in-situ treatment (funnel and gate style) to manage the flux of groundwater flow into the system. The CSM will guide the design of any physical barrier system, but technology limitations increase implementation difficulty with scale.
Ex-situ Treatment (Physical, Chemical or Biological)	Retained	Ex-situ treatment technologies are retained as a way of removing contaminants from extracted groundwater from a hydraulic containment system. Ex-situ treatment may be paired with wastewater treatment, nongroundwater release treatment systems, or with permitted discharge to manage groundwater contamination. The CSM and data gaps investigations will guide the design of any ex-situ treatment
Closure in Place (CiP) (of the regulated unit)	Retained	The use of CiP as a source control technology and is amenable with respect to CAO attainment.
Closure by Removal (CbR) (of the regulated unit)	Retained	The use of CbR as a source control technology is amenable with respect to CAO attainment.

Potentially Applicable Technology	Status	Description/Overview
Other Source Control Technologies	Retained	Control of source area non-groundwater related releases. For the purposes of this groundwater ACM, management of non-groundwater releases are not included in the alternatives evaluation. Engineering measures, including leachate collection, lining of trenches and/or ponds, and other isolation methods are regarded as part of closure technologies selected by other means.

Note: Technologies that were retained may be used as components of a corrective action alternative, but when evaluated in conjunction with other available technologies any single technology may not be utilized.

Preliminary assembly of corrective measures alternatives was performed based on site-specific and regional geology and groundwater conditions. For the Unit, six corrective measures alternatives were developed from this list of applicable corrective measures technologies during the ACM screening process:

- Alternative #1 No Action and Groundwater Monitoring
- Alternative #2a Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring
- Alternative #2b Closure by Removal (CbR), ICs, and Groundwater Monitoring
- Alternative #3 CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #4 CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #5 CiP, Other Source Control, ICs, and Groundwater Monitoring

The assembly of corrective measures alternatives presented in the ACM was considered preliminary and subject to revision following additional evaluation during the remedy selection process and/or following comment from the regulatory community and public. Further evaluation of the alternatives is discussed in the following sections.

### 3. Remedy Selection Progress

The groundwater ACM performed for the Unit in June 2019 identified a total of six (6) corrective measures alternatives to be carried forward into the remedy selection process. In December 2019, BREC provided a *Semi-annual Remedy Selection Progress Report* (AECOM, December 2019) as required under 40 CFR 257.97(a). As part of this submittal, two (2) corrective measures alternatives were eliminated from further consideration, including:

- Alternative #1 (No Action and Groundwater Monitoring) This alternative does not control or remove COCs from the environment and therefore does not achieve the RAOs.
- Alternative #2b (CbR, ICs, and Groundwater Monitoring) Implementing a CbR approach is
  considered cost prohibitive. In addition, any CbR approach would require relocating waste to an
  existing disposal unit or construction of a new waste disposal unit, which does not align with the
  one of the fundamental goals of RCRA (conserving energy and natural resources).

Four (4) potential corrective measures alternatives have been identified by BREC as viable options to address lithium impacts in groundwater and non-groundwater releases at the Unit, including:

- Alternative #2a: CiP, ICs, and Groundwater Monitoring
- Alternative #3: CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #4: CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #5: CiP, Other Source Control, ICs, and Groundwater Monitoring

Each of the remaining 4 corrective measures alternatives is discussed in more detail below.

#### 3.1 Potential Corrective Action Alternatives

### 3.1.1 Alternative #2a – CiP, ICs, and Groundwater Monitoring

Alternative #2a as currently envisioned would employ a combination of three corrective measures technologies:

- CiP source control, which consists of which consists of routine cover management during landfill operation, and planned closure activities for the Green Landfill;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater monitoring (Assessment) to document the effectiveness of the corrective measures.

Alternative #2a is recommended for further evaluation.

### 3.1.2 Alternative #3 – CiP, ICs, Hydraulic Containment, Other Source Control, Ex-Situ Treatment, and Groundwater Monitoring

Alternative #3 as currently envisioned would build upon Alternative #2a to also include the addition of hydraulic containment, using vertical groundwater recovery wells, other source control (i.e., rehabilitation of the South Pond, and managing existing non-groundwater seeps), and ex-situ treatment of groundwater, which involves above-ground physical/chemical treatment methods and/or permitted discharge until CAOs are achieved.

Alternative #3 is recommended for further evaluation.

### 3.1.3 Alternative #4 – CiP, ICs, Physical Containment (Funnel-Gate), Permeable Reactive Barrier, and Groundwater Monitoring

Alternative #4 as currently envisioned would consist of CiP (BREC's planned unit closure activities), physical containment of impacted groundwater via installation of a grout curtain with an extraction well at the gate, and ex-situ treatment of extracted groundwater by physical/chemical treatment methods and/or permitted discharge.

Alternative #4 is recommended for further evaluation.

### 3.1.4 Alternative #5 - CiP, ICs, Other Source Control, and Groundwater Monitoring

Alternative #5 is similar to Alternative #2a except for the addition of other source control, in the form of draining and lining the South Pond and managing existing non-groundwater seeps.

Alternative #5 is recommended for further evaluation.

### 3.2 Remedy Evaluation

Currently BREC considers four (4) potential corrective action alternatives as viable options to address groundwater impacts at the Unit, including:

- Alternative #2a;
- Alternative #3;
- Alternative #4: and
- Alternative #5.

To evaluate each alternative, additional data collection will likely be required. BREC is currently evaluating data collection needs in the following areas to assist with remedy selection:

- 1) Nature and Extent groundwater trends, influence of non-groundwater remedies, etc.
- 2) Physical Characteristics available data on the physical characteristics of the landfill and retention pond
- Performance Modeling data needed to develop digital models demonstrating the effectiveness of potential alternatives
- 4) Engineering feasibility, cost estimates, etc.

BREC is working to establish a comprehensive list of data collection needs to proceed forward with remedy evaluation and anticipates providing additional data in future semi-annual remedy selection progress reports.

In Fall 2019, BREC constructed a series of collection trenches around the perimeter of the Unit to address non-groundwater releases. The 2020 groundwater monitoring program will assist in evaluating the success of the non-groundwater release remedies and provide relevant and important information to be considered in the final groundwater remedy selection.

### 3.3 Public Meeting

At the beginning of 2020, BREC had initiated preparation to conduct a public meeting to discuss the results of the Groundwater ACM as required by 40 CFR 257.96(e). However, due to the onset of the COVID-19 pandemic, BREC has been prevented from holding the public meeting so far in 2020. BREC plans to hold a public meeting once the mass gathering restrictions related to COVID-19 are lifted in Kentucky.

### 4. Conclusion

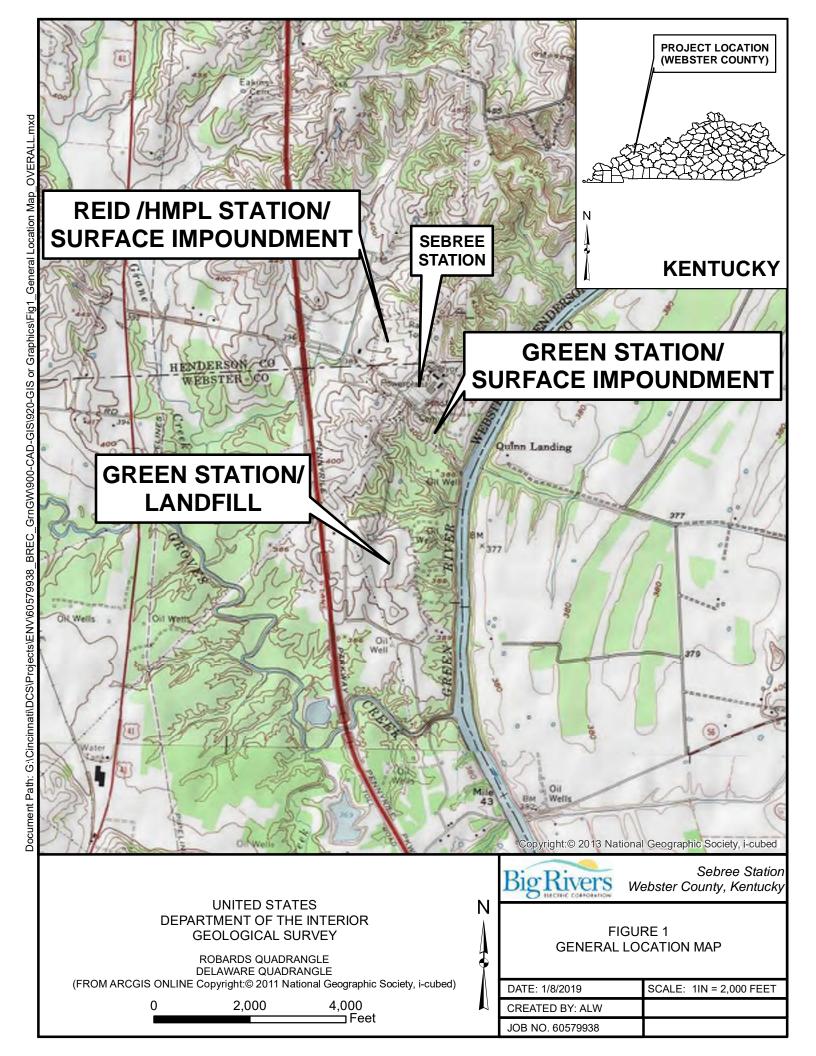
Additional updates regarding remedy selection, including any additional corrective measures being considered, will be presented twice a year in future remedy selection progress reports. Once sufficient data has been collected to select an effective comprehensive remedy for the Unit, a public meeting will be held 30 days prior to formal remedy selection, followed by a detailed Remedy Selection Report describing the remedy and proposed schedule for implementation.

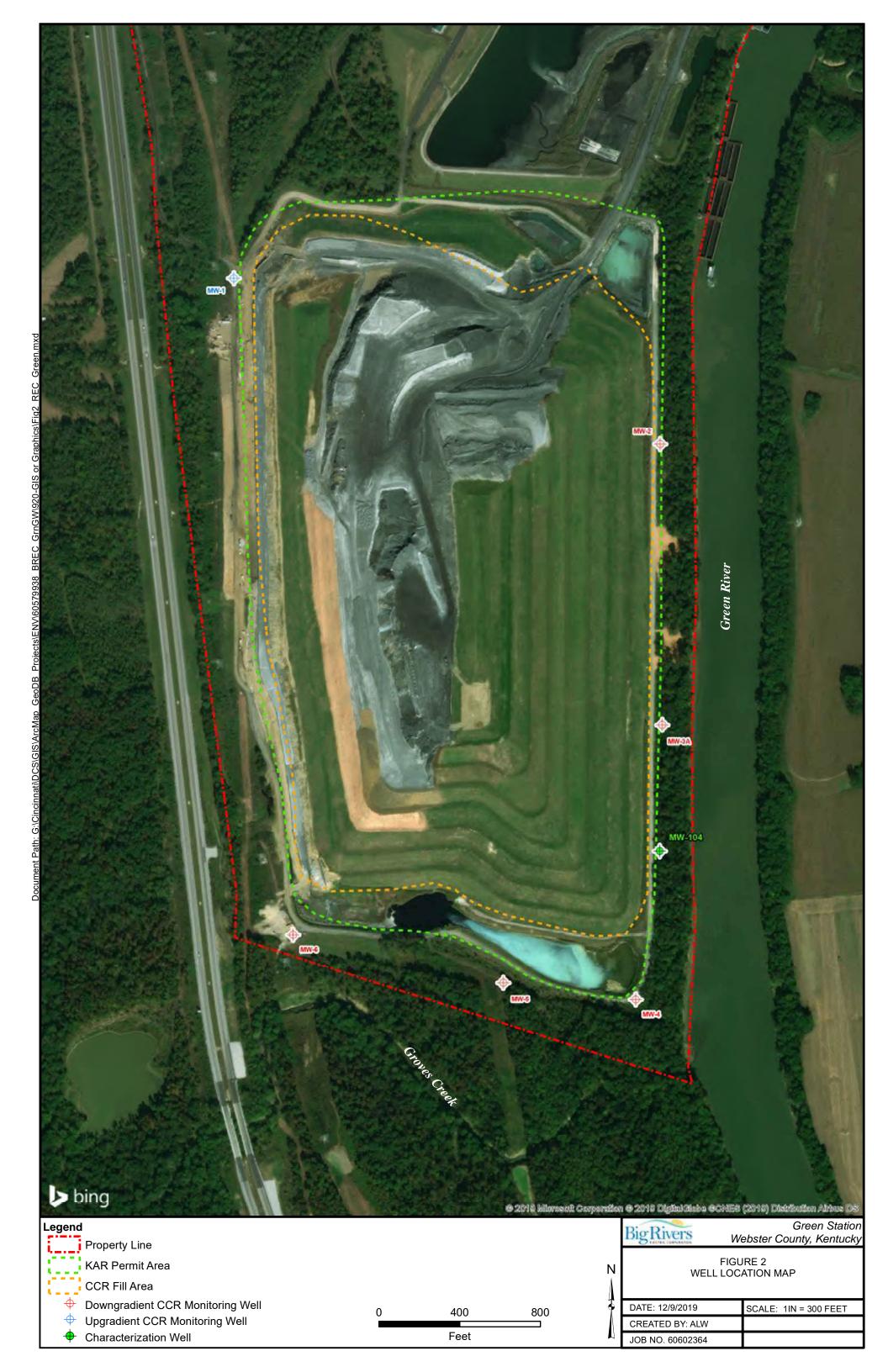
If needed, the next remedy selection progress report for the Unit is expected in December 2020.

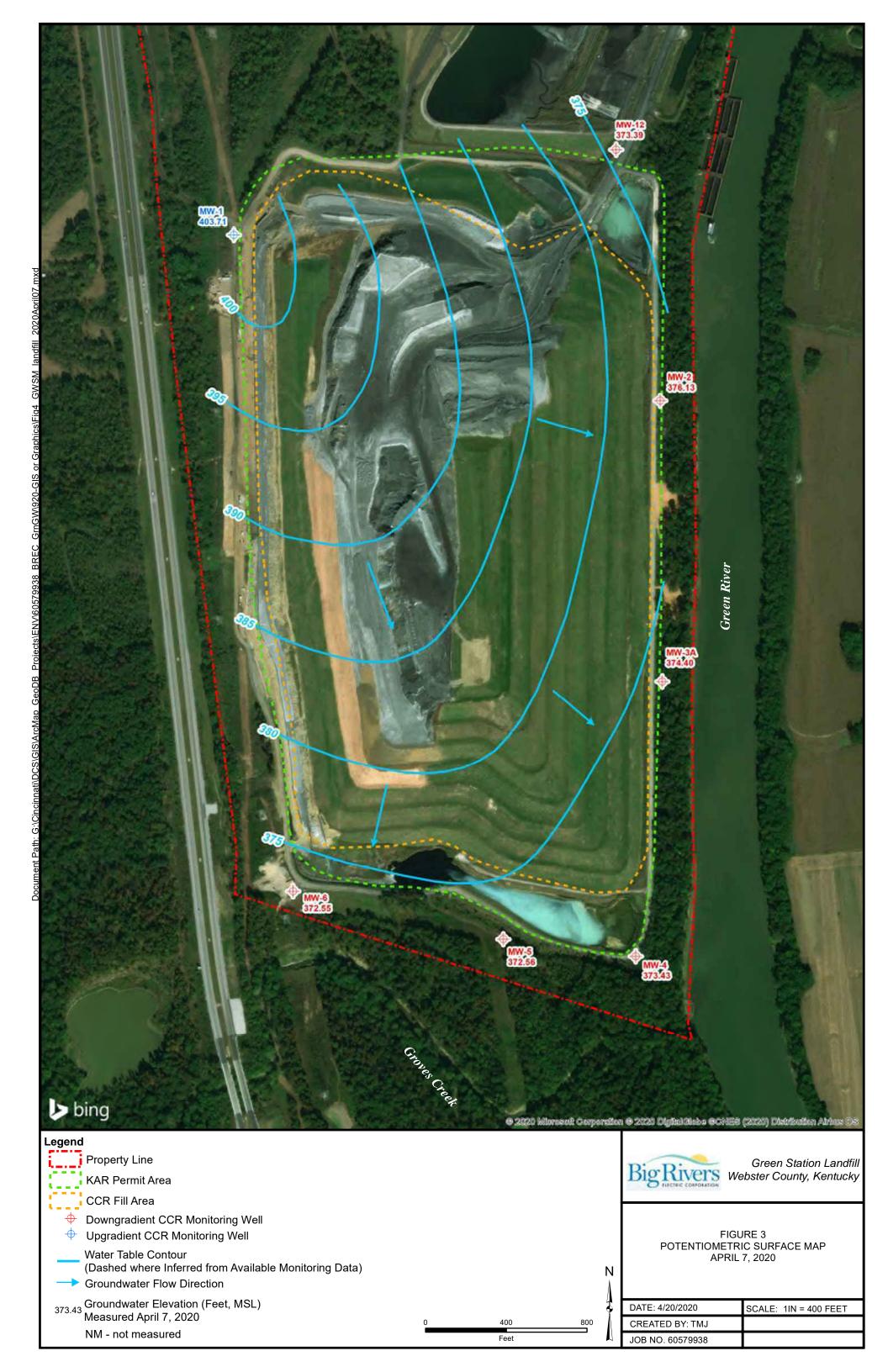
### 5. References

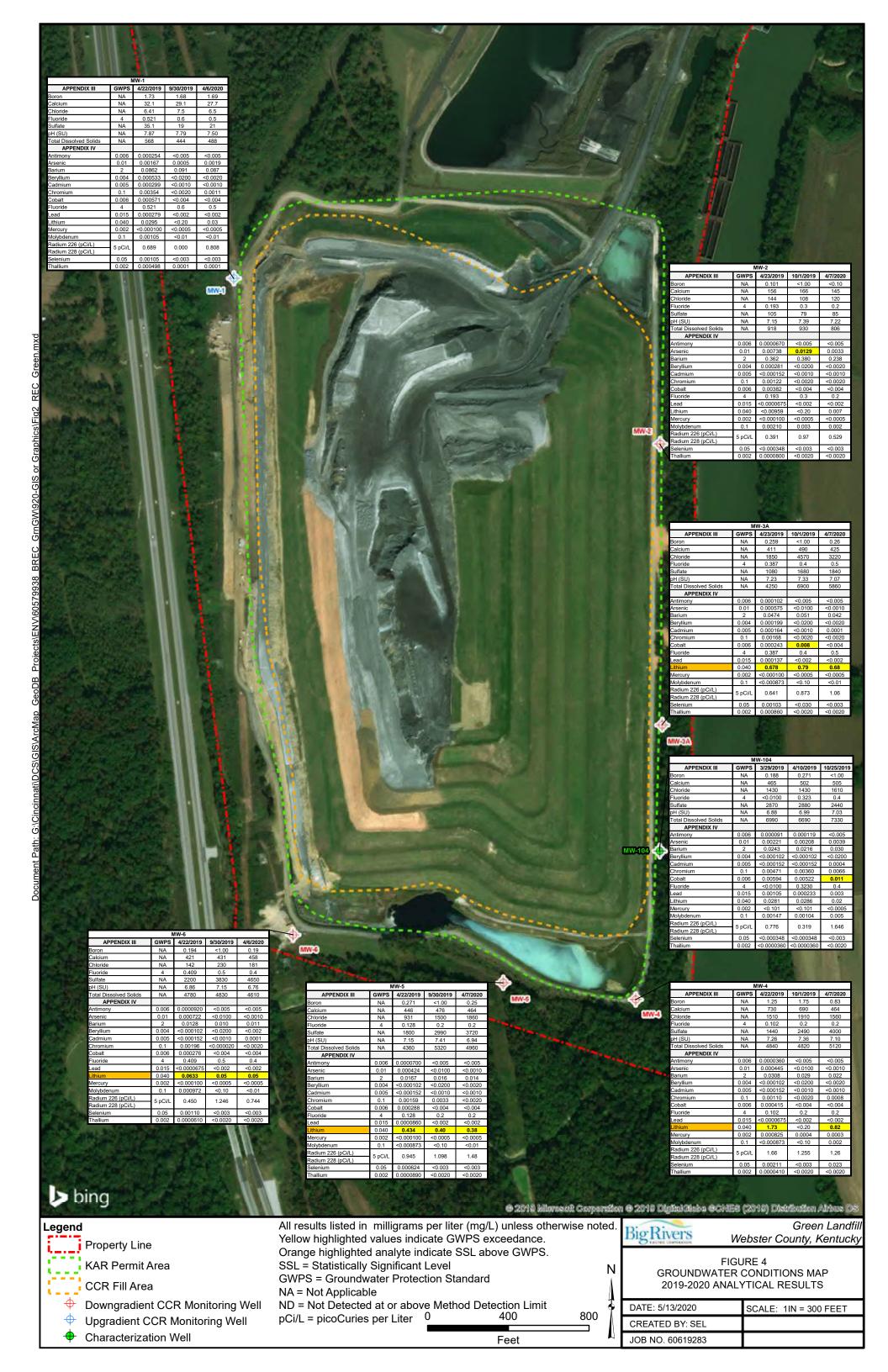
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### **Figures**









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## REMEDY SELECTION PROGRESS REPORT

### GREEN LANDFILL SEBREE STATION WEBSTER COUNTY, KENTUCKY

**December 9, 2019** 

Prepared For:



Big Rivers Electric Corporation Sebree Generating Station 9000 Highway 2096 Robards, KY 42452

Prepared by:



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### 1.0 INTRODUCTION

In accordance with provisions of the United States Environmental Protection Agency's (USEPA) coal combustion residual (CCR) rule, Title 40 of the Code of Federal Regulations (CFR) Section 257.97, Big Rivers Electric Cooperation (BREC) is in the process of selecting a remedy for groundwater impacts at the Green Station CCR Landfill (the Unit) at the Sebree Generating Station located in Webster County, Robards, Kentucky (Figure 1).

Assessment monitoring results indicate the presence of lithium at a Statistically Significant Level (SSL) above the Ground Water Protection Standard (GWPS) in four monitoring wells (MW-3A, MW-4, MW-5, and MW-6) at the Unit. A map illustrating the site with location of all program monitoring wells is presented as **Figure 2**.

In response to the SSL exceedance, BREC evaluated the nature and extent of groundwater impacts as required by Title 40 CFR Section 257.95(g) for characterization monitoring. In addition, BREC performed an Assessment of Corrective Measures (ACM), to identify applicable remedial technologies to address lithium impacts in groundwater pursuant to Tile 40 CFR Section 257.96. A notice of ACM initiation dated January 14, 2019 was posted to BREC's publicly-accessible CCR reporting website. A report summarizing the results of the ACM (AECOM, June 2019) was posted to BREC's publicly-accessible CCR reporting website on June 14, 2019.

Title 40 CFR Section 257.97(a) requires that progress reports be prepared on a semi-annual basis describing progress made in selecting and designing a remedy. The following sections provide an overview of BREC's activities previously performed, currently underway, and planned in the future to select a remedy that meets the requirement of Title 40 CFR Section 257.97 (b) as follows:

- (1) Be protective of human health and the environment;
- (2) Attain the GWPS as specified pursuant to Section 257.95(h);
- (3) Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into the environment;
- (4) Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors such as avoiding inappropriate disturbance of sensitive ecosystems;
- (5) Comply with standards for management of wastes as specified in Section 257.98(d).

### 2.0 SITE BACKGROUND

### 2.1 Site Description

BREC owns and operates Sebree Station, which is a coal-fired power generating facility located on the Green River northeast of Sebree, Kentucky. Sebree Station is composed of Green Station and Reid/Henderson Municipal Power & Light (HMP&L) Station. The Sebree Station is bounded by Interstate-69 to the west and the Green River to the east (see **Figure 1**). Reid Unit 1 (66 Megawatts [MW]) began commercial operation in 1966 and it will be converted from coal to natural gas in the future. The Reid Combustion Turbine (72 MW) was commercialized in 1976. HMP&L Station 2, Units 1 (167 MW) and 2 (168 MW) began commercial operation in 1973 and 1974 respectively. Both HMP&L units were retired as of February 1, 2019. Green Station Units 1 (242 MW) and 2 (242 MW) began commercial operation in 1979 and 1981, respectively.

The location of the Green Landfill is illustrated on **Figure 1**. The Green Landfill is located directly south of Sebree Station, situated south of the Green Station CCR Surface Impoundment. The Green Landfill is a Kentucky permitted landfill (Permit No. SW11700007) that receives special wastes generated by burning coal (CCRs) from Green and Reid/HMP&L Stations. The landfill began receiving CCR wastes in 1980. The current Green Landfill footprint is approximately 170 acres.

As stated in the published CCR monitoring well network certification, available on the BREC website (http://www.bigrivers.com/), the original ground surface within the landfill footprint was irregular and the dominant features were small stream valleys draining towards the Green River, which is located just east of the landfill; and towards Groves Creek, which is located just south of the landfill. There was also historic oil and gas production at and in the immediate vicinity of the Green Landfill. A review of the records from the Kentucky Geological Survey (KGS) showed that at or immediately adjacent to the Site, there were a number of dry exploratory oil/gas exploration holes, oil production wells, one gas production well, and one secondary recovery injection well. There were also former brine ponds at the Site. Most of these wells were abandoned in accordance with applicable regulations by BREC in 1997 and 1998. The last existing oil well was decommissioned in 2019.

### 2.2 Groundwater Investigation Summary

Monitoring wells were installed at the Unit beginning in November 1996 prior to the implementation of the CCR Rule. However, the existing wells meet the requirements of Title 40 CFR Section 257.90 of the CCR Rule for installation of a groundwater monitoring system. These requirements are that wells must adequately represent the quality of background groundwater and groundwater representing the downgradient waste boundary. The existing wells are located along the perimeter of the landfill footprint (**Figure 2**). One upgradient monitoring well (MW-1) and five downgradient monitoring wells (MW-2, MW-3A, MW-4, MW-5 and MW-6) were installed at the Unit to determine the general direction of groundwater movement and to monitor groundwater impacts. The monitoring wells were installed in the uppermost saturated portion of the sandstone bedrock aquifer.

Nine rounds of Baseline groundwater sampling for Appendix III constituents were conducted between March 2016 and October 2017. Statistical evaluation for Detection monitoring indicated that SSIs over background had occurred, and therefore, Assessment monitoring was triggered. Detection monitoring activities and data are presented in the annual reports that have been prepared to date, (AECOM 2018 and 2019).

As part of Assessment monitoring, upgradient and downgradient wells for the Unit were sampled for Appendix IV constituents in June, July, and September 2018. GWPS were established for Assessment

monitoring of the Appendix IV constituents, and statistical evaluation indicated exceedances of GWPSs at SSLs, as detailed in **Table 1** below.

Table 1 Green Station CCR Landfill Constituents of Concern

Monitoring Well (Date)	Parameter  Lithium  GWPS 0.04 (mg/L)
MW-3A (Jun 2018)	0.699
MW-3A (Jul 2018)	0.790
MW-3A (Sep 2018)	0.766
MW-4 (Jun 2018)	1.81
MW-4 (Jul 2018)	1.91
MW-4(Sep 2018)	1.81
MW-5(Jun 2018)	0.459
MW-5 (Jul 2018)	0.481
MW-5 (Sep 2018)	0.425
MW-6 (Jun 2018)	0.0650
MW-6 (Jul 2018)	0.0590
MW-6 (Sep 2018)	0.0558

GWPSs are the greater of the site-specific background concentrations, the USEPA primary drinking water standard maximum contaminant limits (MCL), or GWPS provided in 40 CFR 257.95(3)(h)(2)

An additional characterization well, MW-104, was subsequently installed to estimate the downgradient extent of impacted groundwater. Sample collection for Appendix III and IV parameters took place in March and April 2019. The analytical results for lithium were below the GWPS. The additional characterization data are summarized in **Table 2** below.

Table 2 - Green Station CCR Landfill Characterization Sample Results

	Parameter
Monitoring Well (Date)	Lithium UPL 0.008 GWPS 0.04 (mg/L)
MW-104 (March 2019)	0.0281
MW-104 (April 2019)	0.0288

The results from both characterization sampling events helped to confirm the downgradient (southwestern) extent of COC impacts above GWPS at the Green Landfill. However, downgradient characterization is limited due to the presence of the Green River immediately adjacent to the Unit.

### 2.3 Conceptual Site Model

A Conceptual Site Model (CSM) has been developed to support the remedy selection process for groundwater corrective action at the Unit.

### 2.3.1 Physical Setting

The Unit is located within the Interior Low Plateaus physiographic province. The province is part of the Interior Plains division of the United States. Characteristic features of the province include unglaciated rolling limestone plains with alluvial valleys and entrenched rivers and streams. Several large rivers are in the region, including the Green, the Ohio, the Kentucky, the Tennessee, and the Cumberland Rivers. The geology underlying the Unit consists of unconsolidated materials, including loess and alluvial deposits, underlain by Upper to Middle Pennsylvanian-age clastics and carbonates consisting primarily of sandstone and shale. The unconsolidated material also include fill, silty and clayey residuum, and minor amounts of sandy, clayey channel fill alluvium.

The Unit is located on an upland adjacent to the west bank of the Green River at an elevation of approximately 436 feet, above mean sea level [ft., amsl] (at the north end of the landfill) and 397 ft., amsl (at the south end of the landfill), with a maximum elevation of 608 ft., amsl at the landfill crest. Precipitation falling within the Green Landfill is directed to ponds on the north and south sides of the Unit and then to the river under Kentucky Pollution Discharge and Elimination System permit. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands (Associated Engineers 2016, Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan). The Unit does not have a leachate collection and management system, although systems are being constructed as part of Interim Corrective Measures for non-groundwater impacts.

#### 2.3.2 Geology

The Unit lies in the Western Kentucky Coalfields section, characterized by rolling uplands underlain by coalbearing bedrock of the Pennsylvanian Period. Near the Unit, maximum topographic relief is on the order of 80 feet. The geologic quadrangle (Geologic map of the Robards quadrangle, Henderson and Webster Counties, Kentucky, 1973) for the area published by the Kentucky Geologic Survey (KGS) shows the surficial material in portions of the western half of the Unit to be unconsolidated loess representing the Pleistocene geologic epoch. The loess consists of sandy and clayey silt. Underlying the loess deposits and exposed at the surface on the eastern half of the Unit are broadly distributed Pleistocene and Holocene alluvium deposits consisting of intermixed and interlensing clay, silt, sand, and gravel. In close proximity to the Unit, the alluvium is generally a low permeability unit that forms terraces along the Green River at elevations of roughly 380 and 395 ft., amsl. The unconsolidated surficial materials range from approximately 10 feet (MW-5) to 52 feet (MW-104) in thickness surrounding the Unit.

The unconsolidated materials are underlain by bedrock of the Upper Pennsylvanian Shelburn Formation [formerly identified as the Lisman Formation (Fairer, 1973)] and the Middle Pennsylvanian Carbondale Formation. At the base of the Shelburn Formation is the Providence Limestone Member, consisting of two distinct limestone beds separated by a sandy shale. The member is exposed in a streambed near the northwest corner of the Unit but is absent over much of the Unit footprint due to erosional channeling. The underlying Carbondale Formation consists of cyclic sequences of sandstones, shales, siltstones and coals. The Carbondale sediments were deposited in a fluvial-deltaic system. As a result of this depositional environment, the lithologic units of the Carbondale tend to be lenticular bodies rather than continuous sheet-like strata. Gradational and abrupt horizontal changes in lithology are often encountered.

### 2.3.3 Hydrogeology

For purposes of compliance with the CCR Rule groundwater monitoring requirements, the interbedded sandstone and shale of the Carbondale Formation is considered the uppermost aquifer underlying the Unit. The uppermost aquifer is hydraulically confined and first encountered at an elevation of approximately 401 ft., amsl at the northwest end of the landfill, and 367 ft., amsl at the southeast end of the landfill (AECOM, 2019). Flow direction beneath the Unit is typically southeast towards the Green River.

Slug tests were performed on April 25, 2019 at monitoring wells MW-3A, MW-4, MW-6, and MW-104 to assess the hydraulic characteristics of the uppermost aquifer. The estimated hydraulic conductivity of the monitoring wells tested ranged from 2x10<sup>-5</sup> to 3x10<sup>-3</sup> centimeters per second (cm/sec).

Although previous site-specific investigations have noted the presence of perched zones of saturation in the overlying unconsolidated materials, these discontinuous zones do not qualify as an uppermost aquifer under the CCR Rule because they do not produce usable quantities of groundwater.

### 2.3.4 Constituents of Concern (COC)

As discussed above, a single Appendix IV COC (lithium) was detected at concentrations exceeding GWPS in multiple monitoring well locations. As a result, the corrective measure evaluation is confined to the area between and adjacent to the wells in which the exceedances were identified (MW-3A, MW-4, MW-5, and MW-6).

### 2.3.5 Potential Receptors/Exposure Pathways

Contact with water (e.g., shallow groundwater or surface water) impacted by COCs at levels above GWPS is regarded as the potential pathway for exposure of potential receptors. Based on data published by KGS, there are no known groundwater wells used for drinking water within a 1-mile radius of the Unit, thus limiting the potential receptors to the surface water, i.e., the Green River and its tributary, Groves Creek. The pathways to these receptors include seepage of water from the Unit through manmade and natural hydraulic barriers.

Other potential exposure pathways (e.g., soil or vapor) are not considered complete as the CCR material is isolated in the Unit. This isolation prevents direct access by individuals that might result in direct contact or ingestion. In addition, the inherent non-volatile nature of the unit-specific COCs eliminates the potential for a complete vapor pathway (i.e., vapor intrusion to indoor air).

#### 2.4 Interim Corrective Measures

No formal interim corrective measures have been performed at the Green Landfill for groundwater, but corrective measures for known non-groundwater releases are underway. The compatibility of those corrective measures with potential groundwater remedies is being evaluated as part of the remedy selection process.

### 2.5 Assessment of Corrective Measures Summary

Title 40 CFR Section 257.96(c) requires that the ACM include an analysis of the effectiveness of potential corrective measures in meeting the objectives for remedies identified under Section 257.97(b), by addressing at least the following:

- The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- 2) The time required to begin and complete the remedy; and
- 3) The institutional requirements, such as state or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the remedy(s).

Several potential corrective measures technologies were evaluated to identify which ones could be carried forward as components of corrective measures alternatives. The results of the corrective measures technology evaluation are presented in **Table 3** below.

Table 3 - Potential Corrective Measures Options Technology Description/Overview

Potentially Applicable Technology	Status	Description/Overview	
No Action	Not retained as standalone technology, but carried forward for baseline comparisons	This technology has been included in the preliminary evaluation/screening but is not retained because it will not meet the established Corrective Action Objectives (CAOs).	
Institutional Controls (ICs)	Retained as supplement to corrective measures alternatives	The use of ICs (i.e., Environmental Covenant, groundwater use restrictions, etc.) is retained as a useful technology. However, it is noted the ICs are not anticipated to be used as a stand-alone technology. Environmental Covenants, groundwater use restrictions, etc., are expected to be combined with other applicable technologies as part of corrective measures alternatives.	
Groundwater Monitoring (Assessment and Detection mode)	Retained as supplement to corrective measures alternatives	The use of groundwater monitoring (Assessment and/or Detection modes as appropriate) when combined with other applicable technologies as part of any proposed corrective measures alternative is retained to address the CAO and to track the effectiveness of the overall remedy. However, it is not retained as a standalone technology.	
Hydraulic Containment	Retained	The use of hydraulic containment is retained because it is an effective means of preventing offsite migration of soluble contaminants. Hydraulic containment requires management and potential ex-situ treatment of extracted groundwater, so it is not a stand-alone technology. The CSM will guide the design of any groundwater extraction system to optimize the total discharge of groundwater needed to provide hydraulic containment.	
Physical Containment	Retained	The use of physical containment is retained because it can be an effective means of managing groundwater flow. Physical containment often requires pairing with hydraulic containment and/or in-situ treatment (funnel and gate style) to manage the flux of groundwater flow into the system. The CSM will guide the design of any physical barrier system, but technology limitations increase implementation difficulty with scale.	

Potentially Applicable Technology	Status	Description/Overview
Ex-situ Physical/Chemical/Biological Treatment	Retained	Ex-situ treatment technologies are retained as a way of removing contaminants from extracted groundwater from a hydraulic containment system. Ex-situ treatment may be paired with wastewater treatment, non-groundwater release treatment systems, or with permitted discharge to manage groundwater contamination. The CSM and data gaps investigations will guide the design of any ex-situ treatment
Closure in Place (CiP) (of the regulated unit)	Retained	The use of CiP as a source control technology and is amenable with respect to CAO attainment.
Closure by Removal (CbR) (of the regulated unit)	Retained	The use of CbR as a source control technology is amenable with respect to CAO attainment.
Other Source Control Technologies	Retained	Control of source area non-groundwater related releases. For the purposes of this groundwater ACM, management of non-groundwater releases are not included in the alternatives evaluation. Engineering measures, including leachate collection, lining of trenches and/or ponds, and other isolation methods are regarded as part of closure technologies selected by other means.

Note: Technologies that were retained may be used as components of a corrective action alternative, but when evaluated in conjunction with other available technologies any single technology may not be utilized.

Preliminary assembly of corrective measures alternatives was performed based on site-specific and regional geology and groundwater conditions. For the Unit, six corrective measures alternatives were developed from this list of applicable corrective measures technologies:

- Alternative #1 No Action and Groundwater Monitoring
- Alternative #2a Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring
- Alternative #2b Closure by Removal (CbR), ICs, and Groundwater Monitoring
- Alternative #3 CiP, Hydraulic Containment, Other Source Control (consisting of seepage collection and treatment), Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #4 CiP, Physical Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring
- Alternative #5 CiP, Other Source Control, ICs, and Groundwater Monitoring

The assembly of corrective measures alternatives presented in the ACM is considered preliminary and could be revised at a later date following detailed analysis during the remedy selection process and/or following comment from the regulatory community and public.

### 3.0 REMEDY SELECTION PROGRESS

Six corrective measure alternatives were identified during the ACM process for potential implementation at the Unit to address groundwater impacts. Each corrective measure alternative consists of one or more corrective measures technologies assembled into a strategy for the groundwater remedy. Each alternative is discussed in more detail below.

#### 3.1 Potential Corrective Action Alternatives

### 3.1.1 Alternative #1 - No Action and Groundwater Monitoring

Alternative #1 consists of taking no action to address groundwater impacts at the Unit. Under the No Action alternative, no corrective action would be implemented to remove, control, mitigate, or minimize exposure to impacted groundwater. The No Action alternative establishes a baseline or reference point against which each of the corrective measure alternatives is compared.

Since Alternative #1 would not attain the CAOs for the Unit, this alternative would not likely be acceptable to stakeholders. Therefore, Alternative #1 is not recommended for further consideration.

### 3.1.2 Alternative #2a - Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring

Alternative #2a as currently envisioned would employ a combination of three corrective measures technologies:

- CiP source control, which consists of which consists of routine cover management during landfill operation, and planned closure activities for the Green Landfill;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater monitoring (Assessment) to document the effectiveness of the corrective measures.

Alternative #2a is recommended for further evaluation.

### 3.1.3 Alternative #2b - Closure by Removal (CbR), ICs, and Groundwater Monitoring

Alternative #2b as currently envisioned would be similar to Alternative #2a except that CiP is replaced by CbR, which consists of excavation and removal of the Unit. Given that Alternative #2b is likely cost prohibitive, this alternative is <u>not</u> recommended for further consideration.

### 3.1.4 Alternative #3 - CiP, ICs, Hydraulic Containment, Other Source Control, Ex-Situ Treatment, and Groundwater Monitoring

Alternative #3 as currently envisioned would build upon Alternative #2a to also include the addition of hydraulic containment, using vertical groundwater recovery wells, other source control (i.e., draining and lining the South Pond, and managing existing non-groundwater seeps), and ex-situ treatment of groundwater, which involves above-ground physical/chemical treatment methods and/or permitted discharge until CAOs are achieved.

Alternative #3 is recommended for further evaluation.

### 3.1.5 Alternative #4 – CiP, ICs, Physical Containment (Funnel-Gate), Permeable Reactive Barrier, and Groundwater Monitoring

Alternative #4 as currently envisioned would consist of CiP (BREC's planned unit closure activities), physical containment of impacted groundwater via installation of a grout curtain with an extraction well at the gate, and ex-situ treatment of extracted groundwater by physical/chemical treatment methods and/or permitted discharge.

Alternative #4 is recommended for further evaluation.

### 3.1.6 Alternative #5 - CiP, ICs, Other Source Control, and Groundwater Monitoring

Alternative #5 is similar to Alternative #2a except for the addition of other source control, in the form of draining and lining the South Pond and managing existing non-groundwater seeps.

Alternative #5 is recommended for further evaluation.

### 3.2 Remedy Evaluation

Currently BREC considers four (4) potential corrective action alternatives as viable options to address groundwater impacts at the Unit, including:

- Alternative #2a;
- Alternative #3;
- Alternative #4; and
- Alternative #5.

To evaluate each alternative, additional data collection will likely be required. BREC is currently evaluating data collection needs in the following areas to assist with remedy selection:

- 1) Nature and Extent groundwater trends, influence of non-groundwater remedies, etc.
- 2) Physical Characteristics available data on the physical characteristics of the landfill and retention pond
- 3) Performance Modeling data needed to develop digital models demonstrating the effectiveness of potential alternatives
- 4) Engineering feasibility, cost estimates, etc.

BREC is working to establish a comprehensive list of data collection needs to proceed forward with remedy evaluation and anticipates providing additional data in future semi-annual remedy selection progress reports.

In Fall 2019, BREC constructed a series of collection trenches around the perimeter of the Unit to address non-groundwater releases. The 2020 groundwater monitoring program will assist in evaluating the success of the non-groundwater release remedies and provide relevant and important information to be considered in the final groundwater remedy selection.

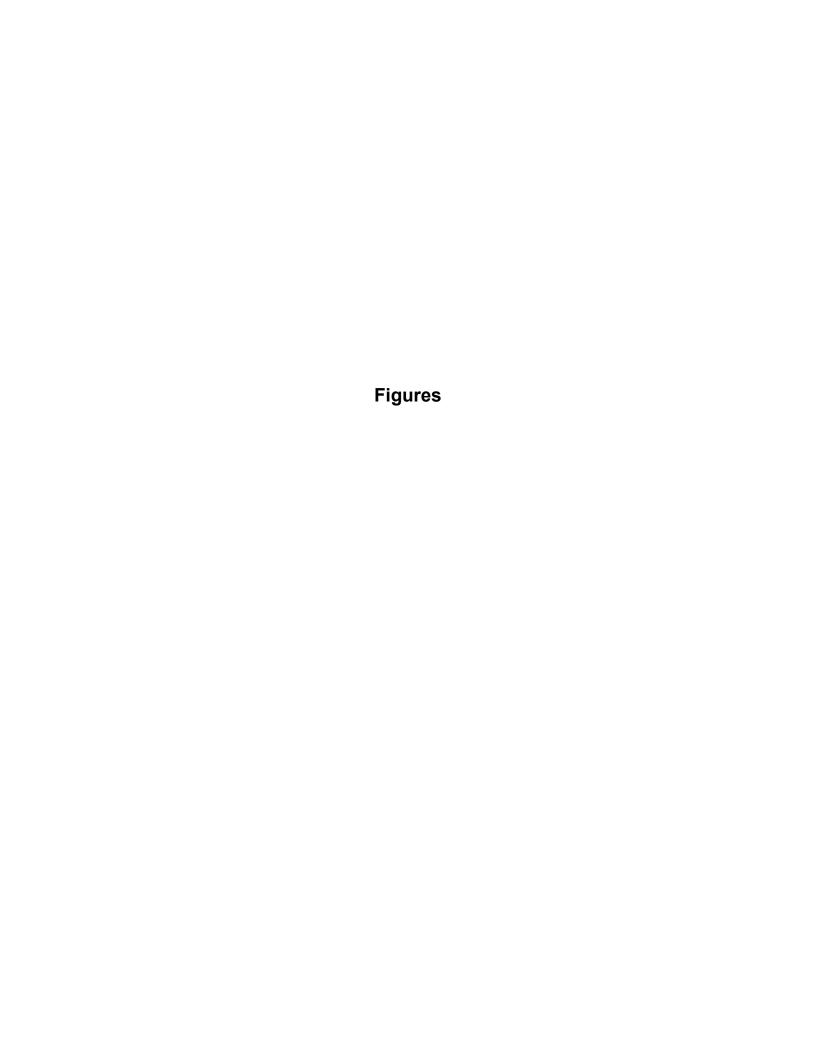
### 4.0 CONCLUSION

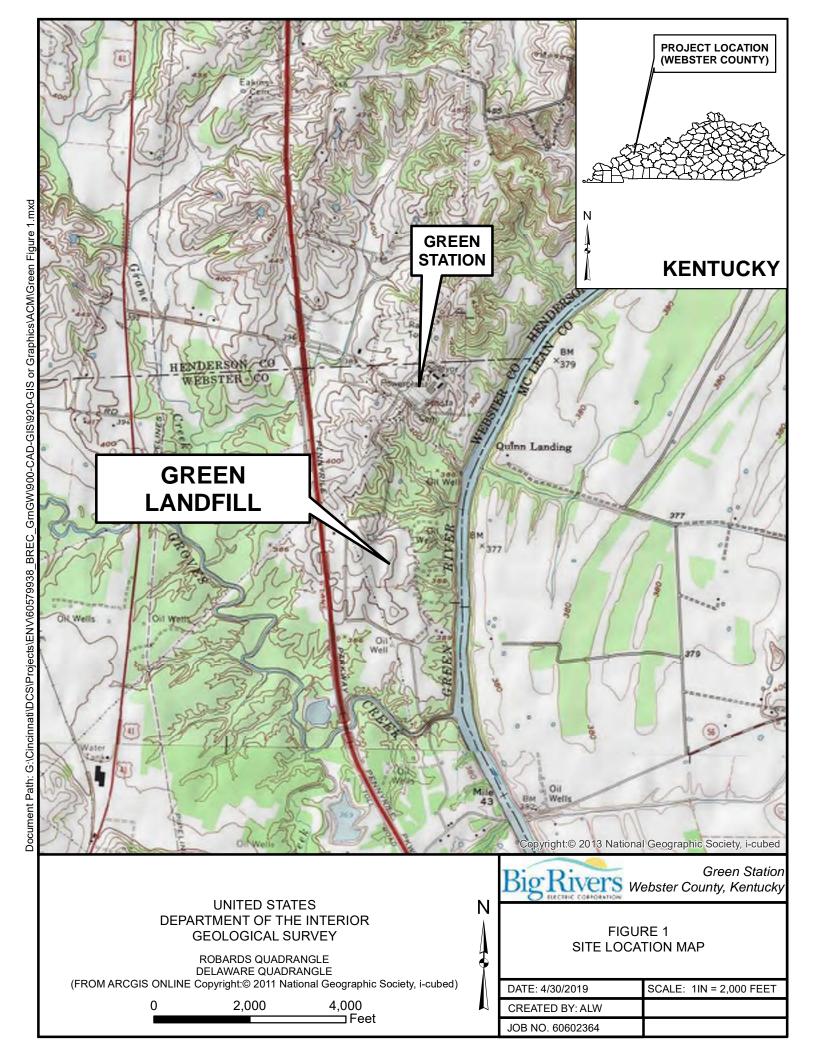
Additional updates regarding remedy selection, including any additional corrective measures being considered, will be presented twice a year in future remedy selection progress reports. Once sufficient data has been collected to select an effective comprehensive remedy for the Unit, a public meeting will be held 30 days prior to formal remedy selection, followed by a detailed Remedy Selection Report describing the remedy and proposed schedule for implementation.

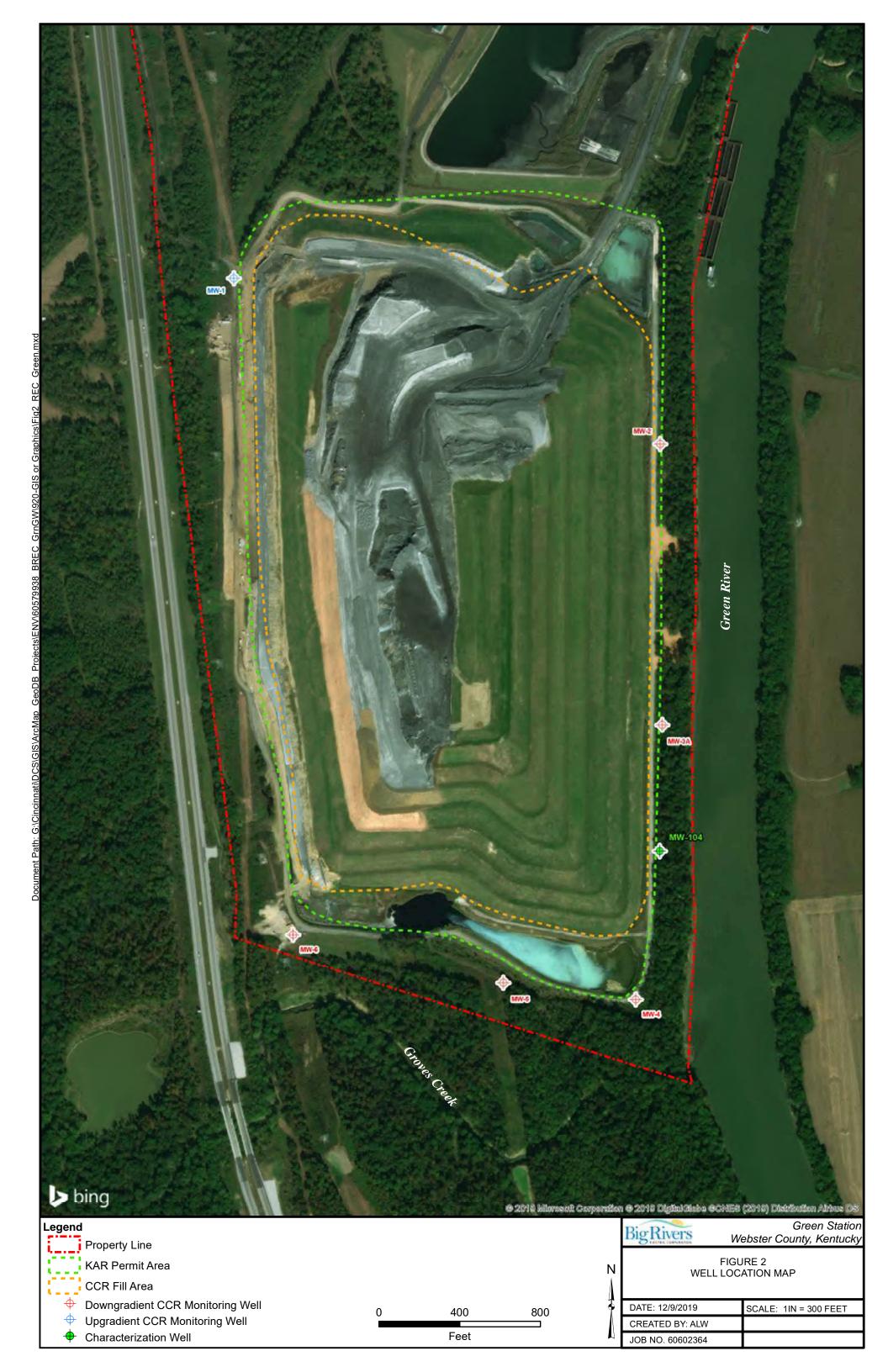
If needed, the next remedy selection progress report for the Unit is expected in June 2020.

#### 5.0 REFERENCES

- AECOM, 2018. Annual Groundwater Monitoring and Corrective Action Report, 2016-2017; Green Station CCR Landfill, Webster County, Kentucky.
- AECOM, 2019. Annual Groundwater Monitoring and Corrective Action Report, 2018; Green Station CCR Landfill, Webster County, Kentucky.
- EPA, 40 CFR Part 257. [EPA-HQ-RCRA-2015-0331; FRL-9928-44-OSWER]. RIN-2050-AE81. Technical Amendments to the Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals From Electric Utilities—Correction of the Effective Date. Federal Register / Vol. 80, No. 127 / Thursday, July 2, 2015 / Rules and Regulations.
- Fairer, G.M., Geologic Map of the Robards Quadrangle, Henderson and Webster Counties, Kentucky, U.S. Geological Survey, 1973.









# COMMONWEALTH OF KENTUCKY ENERGY AND ENVIRONMENT CABINET DIVISION OF WASTE MANAGEMENT Permit No. SW11700007 AI No. 4196 FILE NO. 18-3-0138

FILED DEC 1 6 2019

Office of Administrative Hearing

IN RE:

R.D. GREEN LANDFILL

BIG RIVERS ELECTRIC CORPORATION 201 3<sup>rd</sup> Street P.O. Box 24 Henderson, KY 42420

Facility/Violation Location:
Big Rivers Electric Corp. – Sebree Station
9000 Highway 2096
Robards, KY 42452

#### AGREED ORDER

\*\*\*\*\*\*\*\*

WHEREAS, the parties to this Agreed Order, the Energy and Environment Cabinet (hereinafter "Cabinet") and Big Rivers Electric Corporation (hereinafter "BREC"), state:

#### STATEMENTS OF FACT

- 1. The Cabinet is charged with the statutory duty of enforcing KRS Chapter 224, and the regulations promulgated pursuant thereto.
- KRS 224.50-760 governs the disposal of special waste, including utility wastes.
   The Cabinet promulgated regulations at 401 KAR Chapters 45 and 46 to implement its duty to regulate the disposal of special wastes.
- 3. In 2015, the United States Environmental Protection Agency ("EPA") promulgated 40 CFR 257.50 257.107 establishing national standards to govern the location, design,

construction and operation of landfills and surface impoundments for the disposal of utility wastes known as coal combustion residuals ("CCR") (hereinafter "Federal CCR Rule"). As promulgated, the Federal CCR Rule is self-implementing. In 2017, the Cabinet promulgated 401 KAR 46:110 to incorporate the federal standards into Kentucky regulations. 401 KAR 46:110 Sections 5 and 8 incorporate inspection, control, assessment, and corrective action requirements set forth in 40 CFR 257.81, 257.84, and 257.90 that apply to surface run-off and unauthorized surface releases from existing CCR landfills. Under the Federal CCR Rule, existing CCR landfills, as defined at 40 CFR 257.53, were authorized to continue operation without installing leachate collection systems. 80 Fed. Reg. 21302, 21370 (April 17, 2015).

- 4. BREC owns and operates Sebree Station, an electric power generating station located in Henderson and Webster Counties, Kentucky (the "Facility"). BREC owns and operates a special waste landfill at the Facility for the disposal of utility wastes including CCR (hereinafter the "Green Landfill") generated at Sebree Station. The site has been assigned AI ID No. 4196. To operate the Green Landfill the Cabinet's Division of Waste Management, Solid Waste Branch (DWM), issued BREC Special Waste Permit No. SW11700007 pursuant to 401 KAR Chapter 45. Operating authorization was received on September 13, 1987.
- 5. Green Landfill is an existing CCR landfill under the Federal CCR Rule and is therefore subject to the operating criteria and corrective action standards of 401 KAR 46:110, that incorporate the 40 CFR Part 257 standards. Those standards apply independent of the terms of the 401 KAR Chapter 45 permit.
- BREC holds Kentucky Pollution Discharge Elimination System ("KPDES") Permit
   No. KY0001929 issued by the Cabinet's Division of Water ("DOW") on June 15, 2018

(hereinafter "KPDES Permit") regulating discharges from point sources at the Facility into Waters of the Commonwealth pursuant to 401 KAR 5:055. Outfalls 001, 009, 012 and 014 are controlled by sedimentation basins and are authorized to discharge CCR landfill runoff and leachate from the Green Landfill.

- 7. In June 2017, authorized representatives of DWM conducted an inspection of the Green Landfill and observed leachate outbreaks and leachate flowing in unlined ditches from the landfill toward the sedimentation basins at Outfalls 012 and 014. Additionally, surface seeps were identified by DWM along the eastern side of the Green Landfill on the natural ground slope between the haul road and the Green River that, if not controlled, have the potential to flow to the Green River.
- 8. On or about July 12 and 13, 2018, BREC conducted water sampling along the Green River to determine whether any seeps or other impacts to the surface water could be attributed to leachate outbreaks from the Green Landfill. Results from in-river samples and a majority of seep samples generally fell within regulatory limits.
- 9. On or about December 10, 2018, DWM and BREC conducted a joint leachate and sampling event of four (4) locations adjacent to the Green Landfill. Sampling results produced by both DWM and BREC indicated chlorides were present in elevated levels at all four sampling locations. An inspection report detailing the sampling event was issued on May 20, 2019.
- 10. On April 2, 2019, DWM and DOW conducted a joint inspection of the Green Landfill and observed seepage and stormwater from below the northwestern side of the landfill flowing in an unlined ditch toward the Green River. Analysis of the seepage and stormwater indicates it contains CCR-related constituents but the concentrations did not exceed Kentucky's water quality standards.

- 11. On May 2, 2019, DOW issued a Notice of Violation ("NOV") to BREC for an unpermitted discharge based upon the April 2, 2019 inspection. The remedial measures required that the discharge be added to the KPDES permit for the facility and addressed through the best management practices required under the permit. BREC responded to the NOV in July 2019 and submitted a KPDES permit modification application for the stormwater runoff in the culvert identified during the April 2, 2019 inspection.
- 12. On May 20, 2019, DWM issued an NOV to BREC for the seep identified during the April 2, 2019 inspection. The remedial measures required that BREC address the seep pursuant to 401 KAR 46:110. BREC responded to the NOV on July 30, 2019 and identified its remedial measures.
- BREC's leachate management activities and remedial plans at the Facility. BREC noted that it had implemented procedures for control of leachate outbreaks and had authorized a consulting firm to evaluate and design remedies for the seeps on the northwestern side of the Green Landfill and those located below the eastern side of the landfill along the bank of the Green River. BREC revised its run-off plan for leachate that is required by 40 CFR 257.81 and has developed standard operating procedures ("SOPs") for leachate outbreaks that are intended to ensure compliance with 40 CFR 257.81, 257.84(b)(5), and 257.90(d), as incorporated in 401 KAR 46:110, and KPDES permit requirements for leachate management.
- 14. The Cabinet and BREC acknowledge that EPA is in the process of reconsidering the scope and applicability of response requirements for non-groundwater releases from CCR landfills. Any final amendments to those standards will automatically become effective under 401 KAR 46:110 due to the federal standards being incorporated by reference, and in such an event

the SOPs set forth in this Agreed Order would be subject to change or amendment. The Cabinet is also in the process of developing amended regulations at 401 KAR Chapter 46 to address procedures for permitting for groundwater and non-groundwater release corrective action at CCR units, which, when effective, will apply to the Green Landfill and may, as appropriate, result in BREC's reconsideration of the remedies implemented under this Agreed Order.

- ensure compliance with surface water standards, as reflected in the Facility's KPDES permit, to prevent degradation of the waters of the Commonwealth from uncontrolled surface seeps, and to reduce the risk of impacts to groundwater. As set forth herein, the parties have agreed upon remedial measures intended to ensure leachate and seeps are managed at the Green Landfill in a manner to comply with the facility's KPDES permit, the special waste landfill permit, and applicable CCR landfill regulations as incorporated at 401 KAR 46:110 Section 8 for non-groundwater releases.
- SOPs, and the remedial assessment design plans and schedules for the surface seeps along the slopes below the Green Landfill. The Cabinet reviewed the plans and determined the proposed SOPs and BREC's remedial assessment plans and milestones for completing remedial measures for the seeps to be acceptable response actions to address concerns for potential operational deficiencies and releases associated with the leachate outbreaks and seeps. While negotiating this Agreed Order, BREC initiated contracting of the engineered containment controls developed as remedies under 40 CFR 257.90(d) and 257.84(b)(5) that are intended to reduce and prevent impacts from the Green Landfill surface seeps to the Green River based upon the design plans that were submitted to the Cabinet for review and comment. BREC is now proceeding with

construction of those remedial projects. The construction schedule and post construction performance monitoring for those projects are set forth in Exhibits 2 and 3.

- 17. In January 2019, BREC posted a notice on its CCR Rule compliance website that it had commenced an assessment of corrective measures (ACM) for lithium, a CCR constituent, detected in groundwater beneath the Green Landfill. That notice also stated that BREC had initiated an ACM for the non-groundwater release surface seeps. Both ACMs have been completed and were posted to BREC's CCR Rule compliance website in July 2019.
- Agreed Order resolves the DWM and DOW NOVs and concerns of potential deficiencies related to seep and leachate outbreak non-groundwater release response protocols and remedial measures. This Agreed Order also provides the process, as set out in Exhibit 4, for BREC to complete the evaluation of groundwater corrective action remedies at the Green Landfill pursuant to 401 KAR 46:110 and to obtain Cabinet review and comment of the corrective action remedy design, schedule for implementation, and post-construction performance monitoring.
- 19. Big Rivers neither admits nor denies the assertions of the Cabinet set forth above, but agrees to resolve the DWM and DOW claims regarding non-groundwater leachate releases and seeps at Green Landfill through the development and implementation of remedial measures set forth herein to address any threat or potential threat to human health and the environment associated with management of CCR and leachate at its Facility, to ensure compliance with 401 KAR 46:110 and the Federal CCR Rule as incorporated thereby for non-groundwater and groundwater releases.

NOW THEREFORE, in the interest of settling all civil claims and controversies involving the alleged deficiencies described above, the parties hereby consent to the entry of this Agreed Order and agree as follows:

#### REMEDIAL MEASURES

- 20. BREC shall implement the SOPs set forth in Exhibit 1 to address leachate and seep releases to the surface at the Green Landfill. The SOPs, which have been incorporated into BREC's run on and run off control plans on its CCR Rule compliance website, may be amended as circumstances warrant with written notice provided thirty days (30) in advance to Director of DWM, 300 Sower Blvd., 2nd Floor, Frankfort, KY 40601.
- 3 for the surface seeps along and below the northwestern and eastern sides of Green Landfill to prevent uncontrolled migration of the surface seeps to the Green River. On or about August 23 and September 19, 2019, BREC provided notice to DWM that it was commencing construction of the projects as specified in Exhibits 2 and 3. BREC shall provide written notice to the Solid Waste Branch Manager five (5) business days prior to commencing additional construction projects and within five (5) business days of completing construction of the projects as set forth in Exhibits 2 and 3. For purposes of this section, written notice may be sent to the Solid Waste Branch Manager, 300 Sower Blvd., 1st Floor, Frankfort, KY 40601, and/or electronic mail directed to Danny Anderson, PE, at Danny Anderson@ky.gov or his successor or designee.
- 22. BREC may request an amendment of the accepted seep remedial action plans and schedules set forth in Exhibits 2 and 3 in writing sent to the Directors of DWM and the Division of Enforcement at 300 Sower Blvd., 3rd Floor, Frankfort, KY 40601. The request shall state the reasons therefore and include any proposed changes to plans and specifications. The Cabinet shall

review proposed amendments and may, in whole or part, 1) approve or 2) disapprove and provide comments identifying deficiencies. If granted, the Amended Exhibit(s) shall not affect any provision of this Agreed Order unless expressly provided for in the amendment. Amendment under this section does not require an amendment request pursuant to paragraph 32 below.

23. BREC shall follow the process and schedule set out in Exhibit 4 in selecting and implementing a corrective action remedy for the statistically significant increase in lithium in groundwater referenced in its CCR Rule compliance website notification.

#### **PENALTIES**

24. BREC shall pay the Cabinet a civil penalty in the amount of twenty thousand dollars (\$20,000) for the violations alleged above. The amount of the civil penalty shall be due no later than thirty (30) days after this Agreed Order is entered by the Secretary or his designee.

#### STIPULATED PENALTIES

- 25. BREC shall pay the Cabinet a stipulated penalty in the amount of five hundred (\$500) per day, within fifteen (15) days of mailing of written notice from the Cabinet for failure to timely meet any remedial milestones required by Exhibits 2 and 3 to this Agreed Order. This penalty is in addition to, and not in lieu of, any other penalty the Cabinet could assess. The Cabinet may, in its discretion, waive stipulated penalties that would otherwise be due.
- 26. Within fifteen (15) days of receipt of written demand for payment of a stipulated penalty, BREC shall submit payment of the stipulated penalty. The stipulated penalties are in addition to and not in lieu of, any other penalty that could be assessed. The payment of stipulated penalties shall not alter in any way BREC's obligation to complete the performance of the actions described in this Agreed Order.

- 27. If BREC believes the request for payment of a stipulated penalty is erroneous or contrary to law, BREC may request a hearing in accordance with KRS 224.10-420(2). The request for hearing does not excuse timely payment of the penalty. If an order is entered pursuant to KRS 224.10-440 that excuses payment, the Cabinet will refund the payment. Failure to make timely payment shall constitute an additional violation.
- 28. Payment of civil and any stipulated penalties shall be by cashier's check, certified check, or money order, made payable to "Kentucky State Treasurer" and sent to the attention of Director, Division of Enforcement, Department for Environmental Protection, 300 Sower Blvd., Frankfort, Kentucky 40601.

#### MISCELLANEOUS PROVISIONS

- 29. This Agreed Order only resolves those claims, NOVs, and alleged deficiencies specifically described above. Other than those matters resolved by entry of this Agreed Order nothing contained herein shall be construed to waive or to limit any remedy or cause of action by the Cabinet based on statutes or regulations under its jurisdiction and BREC reserves its defenses thereto. The Cabinet expressly reserves its right at any time to issue administrative orders and to take any other action it deems necessary that is not inconsistent with this Agreed Order, including the right to order all necessary remedial measures, assess penalties for violations, or recover all response costs incurred, and BREC reserves its defenses thereto.
- 30. This Agreed Order shall not prevent the Cabinet from issuing, reissuing, renewing, modifying, revoking, suspending, denying, terminating, or reopening any permit to BREC. BREC reserves its defenses thereto, except that BREC shall not use this Agreed Order as a defense to those permitting actions.
  - 31. BREC waives its right to any hearing on the matters resolved herein. However,

failure by BREC to comply strictly with any or all of the terms of this Agreed Order shall be grounds for the Cabinet to seek enforcement of this Agreed Order in Franklin Circuit Court and to pursue any other appropriate administrative or judicial action under KRS Chapter 224 and the regulations promulgated pursuant thereto.

- 32. The Agreed Order may not be amended except by a written order of the Cabinet's Secretary or his designee. BREC may request an amendment by writing the Director of Division of Enforcement at 300 Sower Blvd., Frankfort, Kentucky 40601 and stating the reasons for the request. If granted, the amended Agreed Order shall not affect any provision of this Agreed Order unless expressly provided in the amended Agreed Order. The Cabinet and BREC agree that the obligations of this Agreed Order may be modified by final promulgation of EPA's Federal CCR Rule reconsideration rule setting requirements for addressing surface releases, including leachate management at existing CCR landfills, and agree obligations of this Agreed Order shall be superseded and amended by any such final rule, or the Cabinet's amendment of its regulations at 401 KAR Chapter 46, to the extent such rules are inconsistent with this Agreed Order.
- 33. Unless otherwise stated in this Agreed Order, all submittals required of BREC by this Agreed Order shall be sent to: Director, Division of Enforcement, 300 Sower Blvd., Frankfort, Kentucky 40601.
- 34. The Cabinet does not, by its consent to the entry of this Agreed Order, warrant or aver in any manner that BREC's complete compliance with this Agreed Order will result in compliance with the provisions of KRS Chapter 224; 401 KAR Chapters 30, 45, and 46; or the Federal CCR Rule. Notwithstanding the Cabinet's review and approval of any plans formulated pursuant to this Agreed Order, BREC shall remain solely responsible for compliance with the

terms of KRS Chapter 224; 401 KAR Chapters 30, 45, and 46; or the Federal CCR Rule, this Agreed Order and any permit and compliance schedule requirements.

- 35. BREC shall give notice of this Agreed Order to any purchaser, lessee or successor in interest prior to the transfer of ownership and/or operation of any part of its now-existing facility occurring prior to termination of this Agreed Order, shall notify the Cabinet that such notice has been given, and shall follow all statutory and regulatory requirements for a transfer. Whether or not a transfer takes place, BREC shall remain fully responsible for payment of all stipulated penalties and response costs and for performance of all remedial measures identified in this Agreed Order.
- 36. The Cabinet agrees to allow the performance of the above-listed remedial measures by BREC to satisfy its obligations to the Cabinet generated by the alleged deficiencies described above.
- 37. The Cabinet and BREC agree that the remedial measures agreed to herein are facility-specific and designed to comply with the statutes and regulations cited herein. This Agreed Order applies specifically and exclusively to the unique facility referenced herein and is inapplicable to any other site or facility.
- 38. This Agreed Order shall be of no force and effect unless and until it is entered by the Secretary or his designee as evidenced by his signature thereon.

#### **TERMINATION**

39. This Agreed Order shall terminate upon BREC's completion of all requirements described in this Agreed Order. BREC may submit written notice to the Cabinet when it believes all requirements have been performed. The Cabinet will notify BREC in writing of whether it intends to agree with or object to termination. The Cabinet reserves its right to enforce this Agreed

Order, and BREC reserves its right to file a petition for hearing pursuant to KRS 224.10-420(2) contesting the Cabinet's determination.

AGREED TO BY:

Mike W. Chambliss, Vice President System Operations

Big Rivers Electric Corporation

12-6-2019 Date

12/4/2019 Date

HAVE SEEN:

Jack Bender, Attorney for Big Rivers Electric Corporation

Dinsmore & Shohl LLP

#### APPROVAL RECOMMENDED BY:

Michael Kroeger, Director
Division of Enforcement

12/6/19 Date

Jon Maybriar, Director

Division of Waste Management

/2-6-19 Date

John S. Horne, II, Executive Director

Office of Legal Services

#### **ORDER**

Wherefore, the foregoing Agreed Order is entered as the final Order of the Energy and Environment Cabinet this 6 day of December, 2019.

**ENERGY AND ENVIRONMENT CABINET** 

R. BRUCE SCOTT, DEPUTY SECRETARY ENERGY AND ENVIRONMENT CABINET

#### **CERTIFICATE OF SERVICE**

I hereby certify that a true and accurate copy of the foregoing AGREED ORDER was mailed, postage prepaid, to the following this was of December 2016.

Hon. Jack Bender
Dinsmore & Shohl LLP
Lexington Financial Center
250 West Main Street
Suite 1400
Lexington, KY 40507

and mailed, messenger to:

Daniel Cleveland Office of Legal Services 300 Sower BLVD, 3<sup>rd</sup> Floor

Michael Kroeger, Director Division of Enforcement 300 Sower BLVD, 3<sup>rd</sup> Floor

DOCKET COORDINATOR

DWM BGD SH

#### Leachate Management Standard Operating Procedures

#### Green Landfill and Green Surface Impoundment

#### Subject: Surface Seep and Leachate Outbreaks Repair

To ensure compliance with 40 CFR 257 Subpart D and 401 KAR Chapters 45 and 46, the following procedure will be utilized for identification and repair of seeps and leachate outbreaks at CCR landfills and surface impoundments. For purposes of this SOP, a leachate outbreak is wastewater/seepage flowing directly from the covered CCR that has passed through or emerged from solid waste and contains soluble, suspended or miscible materials removed from such wastes. Seeps are flows that emerge from the ground immediately below the actual waste disposal area and that may contain leachate that is mixed with water from saturated soils or surface water infiltration.

- An inspection by a qualified person will be conducted once per week to identify any seeps and leachate outbreaks at CCR landfills and CCR surface impoundments. The inspection will include the entire perimeter of both the Green Landfill and Green Surface impoundment as weather conditions allow at the time of the inspection. The weather conditions at the time of the inspection must be documented on the inspection form.
- Identified seeps and leachate outbreaks must be located and documented by Global Positioning Satellite (GPS) and digital photography.
- Identified seeps and leachate outbreaks must be quantified as to the amount of standing
  or flowing water in gallons per minute. Measurements or estimates of the impacted area
  in square feet must be included. Other information relevant to remediation of the
  outbreak or seep shall be included on the BREC inspection form.
- All information fields on the BREC inspection form shall be completed.
- Categorize the seep or leachate outbreak into one of three categories:
  - Category 1 Leachate/seep flow is contained within a drainage ditch and pond system that flows to a KPDES permitted outfall and the outbreak or seep is readily repairable by removing the impacted area and replacing the cover dirt with compacted clay, seeded and mulched, when the soil conditions are not too wet to preclude typical construction activities or the ambient temperature is not too low to preclude typical construction activities. For purposes of this determination, readily repairable is an outbreak or seep that can reasonably be believed to be remediated by removing the impacted area and replacing the cover with compacted clay. This determination requires the judgment of the inspector based upon the size, flow, and any repeat history of the outbreak or seep. For any area where there is no visible flow and no rutting/erosion of the soil from prior flow(s), but only saturated soil, then such an area will not be

- identified as a seep/leachate outbreak but will be identified and recorded as "saturated soil" in the log and monitored during subsequent weekly inspections.
- Category 2 Leachate/seep is contained within a drainage ditch and pond system
  that flows to a KPDES permitted outfall but requires further investigation and
  evaluation prior to any attempt at remediation or if initial remediation efforts
  prove to be unsuccessful.
- Category 3 Leachate/seep is not contained within the KPDES permitted ditch and pond system. Any areas of leachate/seep discharges that are identified must be remediated, contained or routed to the KPDES permitted ditch and pond system if the seep displays a visible flow. Actions must begin immediately to prevent an unpermitted point source discharge to a water of the United States by remediating the outbreak or seep.
- Steps to take if a Category 1 seep/leachate outbreak reappears:
  - o If a Category 1 seep/leachate outbreak reappears more than 30 days after a previous repair and the flow from the seep/leachate outbreak has been reduced or the extent of the impact is reduced from the initial identification of the seep/leachate outbreak, then Big Rivers may classify the reappearance of the seep/leachate outbreak as a Category 1 seep/leachate outbreak and commence repairs per the Agreed Order (excavate, compact, seed, and mulch.)
  - For any area where there is no visible flow and no rutting/erosion of the soil from prior flow(s), but only saturated soil, then such an area will not be identified as a seep/leachate outbreak but will be identified and recorded as "saturated soil" in the log and monitored during subsequent weekly inspections.
  - Seeps/leachate outbreaks that reappear less than 30 days after a repair or that reappear at a later date with increased flow or impact area will be classified as a Category 2.
- Collect water samples for constituents listed in Table 1. A water sample will only be collected for analysis when a sufficient amount of water is flowing on the surface to collect a sample without disturbing the underlying soil. Samples will be collected once for each categorization unless there are visual changes such as color in the leachate. Seep/leachate water samples will be collected once when identified as a Category 1 and again if reclassified as a Category 2. The analysis will be performed by a laboratory certified in the State of Kentucky. The analysis must contain the chain of custody and complete analysis with QA/QC results. Results will be maintained in the Landfill operating log on-site.
- Place categorized information in the Landfill operating log.
- Corrective actions for readily repairable seeps and leachate outbreaks must begin as soon
  as reasonably feasible with consideration given to inclement weather patterns and soil
  moisture conditions.
- Remediation areas outside the KPDES permitted ditch and pond system must include the
  installation of sedimentation controls as found in the Storm Water Pollution Prevention
  Plan/Best Management Plan guidance document published by the Kentucky Division of
  Water. Water samples from seeps containing a visible flow shall be taken for impacted

areas outside the KPDES permitted ditch and pond system and analyzed for the constituents found in Table 1.

- Cover soil and/or special waste removed during the remediation process must be placed in an active area of a CCR landfill or reused during the remediation of the unit if practicable.
- Replacement soil must be compacted, seeded and mulched.
- Environmental Services shall evaluate and determine remediation plans for a Category 2 seep/leachate outbreak that is deemed not readily repairable based upon flow and landfill conditions. Until remediation occurs, the seep/leachate flow shall be visually monitored, conveyed to a KPDES permitted outfall, and treated as necessary to ensure compliance with KPDES discharge limits and applicable water quality standards in the receiving stream. Remediation activities required for a Category 2 outbreak will be sent to the Division of Waste Management, 300 Sower Boulevard, Frankfort, Kentucky 40601 within five (5) business days of finalizing the report.
- Category 3 seeps displaying a visual flow will be reported to the Kentucky Division of Water Surface Water Permits Branch in Frankfort, Kentucky and the Madisonville Field Office consistent with the Section 2.12 reporting provisions of the KPDES permit for leachate/seep outbreaks. Category 3 seeps with a visual flow will also be reported to the Kentucky Division of Waste Management Field Operations Branch in Frankfort, Kentucky and the Madisonville Field Office. Reporting of the seeps shall occur as soon as feasible after discovery of such a seep, but no later than ten (10) days after discovery. Environmental Services shall evaluate and determine remediation plans for a Category 3 seep that is deemed not readily repairable based upon flow and landfill conditions.

These protocols shall be followed at CCR units subject to the federal CCR Rule and 401 KAR Chapter 46.

#### • Table 1

- o From 40 CFR 257 App. III
  - Boron
  - Calcium
  - Chloride
  - Fluoride
  - pH
  - Sulfate
  - Total Dissolved Solids
- From 40 CFR 257 App. IV
  - Antimony
  - Arsenic
  - Barium
  - Beryllium
  - Cadmium
  - Chromium
  - Cobalt

- Fluoride
- Lead
- Lithium
- Mercury
- Molybdenum
- Selenium
- Thallium
- Radium 226 and 228 combined
- o From 401 KAR 45:160
  - \*Chemical Oxygen Demand
  - \*Total Organic Carbon
  - \*Specific Conductance
  - \*Copper
  - \*Nickel
  - \*Zinc
  - \*iron
  - \*Sodium
  - \*Magnesium
  - \*Potassium
  - \*Bicarbonate
  - \*Carbonate

## Northwestern Seep Collection Trench Remedy Construction Schedule Green Station Existing CCR Landfill

#### **Construction Schedule**

- 1. BREC shall initiate construction of the Northwestern Seep Collection Project (the "Project") consistent with the design plans previously reviewed by DWM no later than 90 days after entry of the Agreed Order. Notice of initiation of construction shall be provided to the Director of DWM a minimum of five (5) days prior to on-site construction activities. For purposes of this paragraph, initiation of construction occurs when earthmoving activities are commenced on the trench, and shall not include mobilization of equipment to the site, initial access work, or staging of materials. BREC agrees to provide such notice even if construction is initiated before entry of this Agreed Order.
- BREC shall complete construction of the Project within 270 days of entry of the Agreed Order. Notice of completion of construction shall be provided to the Director of DWM within 30 days following completion of construction of the collection trench and ancillary facilities to support its operation. The notice shall conform to 40 CFR 257.98(e).

#### Post-construction Performance Monitoring

- 1. BREC shall implement a post-construction performance monitoring program as follows.
  - a. The Northwestern seep shall be inspected quarterly and any flow conditions in the collection system and at the subject seep shall be recorded in the inspection log.
  - b. Any visibly flowing seeps shall be sampled and analyzed for the constituents listed in the seep/leachate SOP (Exhibit 1) as needed to evaluate whether impact (above permit levels) is present or absent.
  - c. A report on the operation of the collection system during each calendar quarter shall be submitted to the Director of DWM. The report shall be submitted within 30 days of the end of each calendar quarter, and placed in the landfill operating record. The report shall report on the effectiveness of the remedy.
  - d. A final report on the completion of the remedy shall be submitted to the Director of DWM and placed in the operating record consistent with 40 CFR 257.98(e) for review and comment by DWM. The final report will be submitted to DWM within 60 days following the completion of four (4) consecutive quarters without identifying seepage in the targeted Northwestern seep collection area.
  - e. The collection system shall be operated until seepage at the surface is not observed for four consecutive quarters and seepage into the collection system diminishes below levels that can result in a non-groundwater release. Prior to ceasing operation and reporting, concurrence shall be obtained in writing from DWM.

### Eastern Seep Collection Trench Remedy Construction and Post-Construction Schedule Green Station Existing CCR Landfill

#### **Construction Schedule**

- 1. BREC shall initiate construction of the Eastern Seep Collection Project (the "Project") consistent with the design plans previously reviewed by DWM no later than 120 days after entry of the Agreed Order. Notice of initiation of construction shall be provided to the Director of DWM a minimum of five (5) days prior to on-site construction activities. For purposes of this paragraph, initiation of construction occurs when earthmoving activities are commenced on the trench, and shall not include mobilization of equipment to the site, initial access work, or staging of materials. BREC agrees to provide such notice even if construction is initiated before entry of this Agreed Order.
- BREC shall complete construction of the Project within 365 days of entry of the Agreed Order. Notice of completion of construction shall be provided to the Director of DWM within 30 days following completion of construction of the collection trench and ancillary facilities to support its operation. The notice shall conform to 40 CFR 257.98(e).

#### Post-construction Performance Monitoring

- BREC shall implement a post-construction performance monitoring program as follows.
  - a. The Eastern river bank seeps and collection system shall be inspected quarterly for operational conditions and for indications of continuing seepage flow. Any visibly flowing seeps shall be sampled and analyzed for the constituents listed in the seep/leachate SOP (Exhibit 1) unless prior concentrations of constituents are demonstrated to be below the applicable Water Quality Criteria.
  - b. A report on the operation of the collection system during each calendar quarter shall be submitted to the Director of DWM. The quarterly system operation and seep inspection monitoring data shall be included in the report. The report shall be submitted within 30 days after the end of each calendar quarter, and placed in the operating record. The report shall address the effectiveness of the remedy.
  - c. A final report on the completion of the remedy shall be submitted to the Director of DWM and placed in the operating record consistent with 40 CFR 257.98(e) for review and comment by DWM. The final report will be submitted to DWM within 60 days following the completion of four (4) consecutive quarters without identifying a flowing seep in the targeted Eastern river bank seep collection area. The final report will be placed in the operating record 30 days after approval from DWM.
  - d. The collection system shall be operated until seepage at the surface is not observed for four consecutive quarters and seepage into the collection system diminishes below levels that can result in a non-groundwater release. Prior to ceasing operation and reporting, concurrence shall be obtained in writing from DWM.

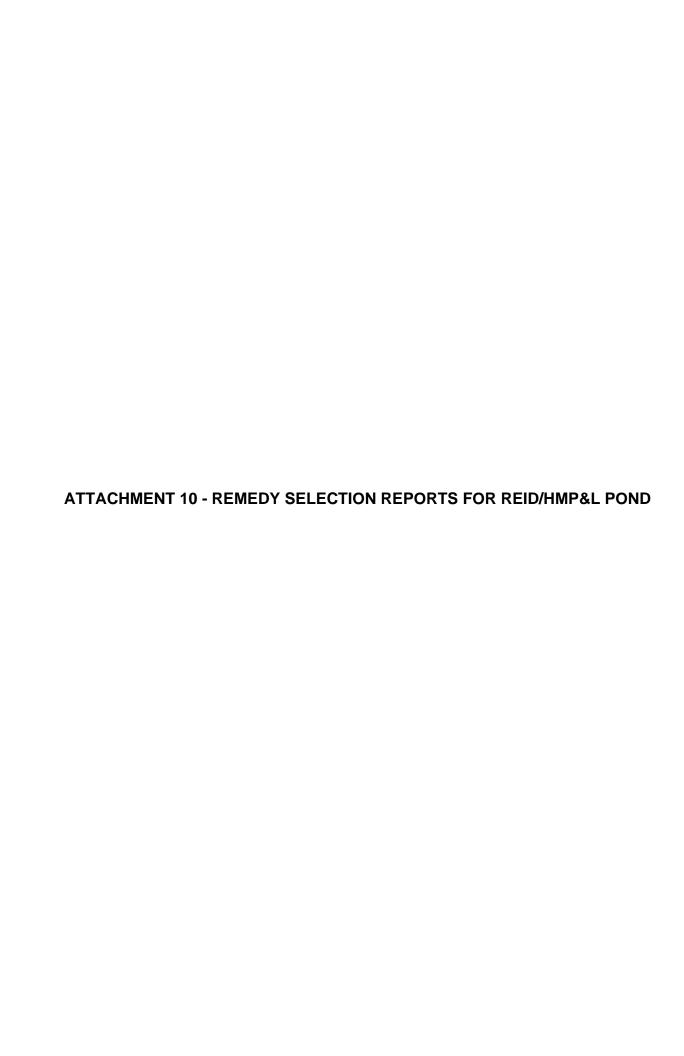
## Groundwater Corrective Action Remedy Selection Process Green Station Existing CCR Landfill

#### Selection of Remedy

- 1. BREC shall conduct the public meeting required by 40 CFR 257.96(e) and 401 KAR 46:110 Section 8 regarding the groundwater and supplemental non-groundwater assessment of corrective measures within 180 days of entry of this Agreed Order. Public notice of the meeting shall be posted on BREC's CCR Rule compliance website and published in the Henderson Gleaner newspaper seven (7) to twenty-one (21) days prior to the meeting. The publication shall state the time, place and purpose of the public meeting and that the assessment of corrective measures is available for review on the BREC CCR Rule compliance website. At the public meeting, BREC shall reference the remedy selection process as set forth in this Agreed Order.
- 2. BREC shall evaluate the comments from the public meeting and other factors as required by 40 CFR 257.97 and 401 KAR 46:110 Section 8 and prepare a draft groundwater remedy selection report for submittal to DWM for a 30 day review and comment period. The draft report shall be submitted to DWM within 90 days following the public meeting, unless DWM approves a longer period in order for BREC to complete is evaluation.
- BREC shall select the final groundwater corrective action remedy as soon as feasible after receipt of any DWM comments on the proposed remedy during the DWM review period.
- 4. The final groundwater and supplemental non-groundwater corrective action remedy selection reports shall be posted to BREC's CCR Rule compliance website in accordance with 40 CFR 257.97 and 257.107.

#### **Design and Construction of Remedy**

 BREC shall design and construct the selected groundwater remedy and conduct postconstruction performance monitoring pursuant to the implementation schedule set forth in the final corrective action remedy selection report.





## Semi-Annual Remedy Selection Progress Report

Reid/HMP&L Surface Impoundment Sebree Generating Station Webster County, Kentucky

Prepared for:



Big Rivers Electric Corporation Sebree Generating Station 9000 Highway 2096 Robards, KY 42452

Prepared by:

AECOM Technical Services 525 Vine Street, Suite 1800 Cincinnati, Ohio 45202

AECOM PN 60619822

June 2020

#### Quality information

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#### 1. Introduction

In accordance with provisions of the United States Environmental Protection Agency's (USEPA) coal combustion residual (CCR) rule, Title 40 of the Code of Federal Regulations (CFR) Part 257.97, Big Rivers Electric Cooperation (BREC) is in the process of selecting a remedy for groundwater impacts at the Reid/Henderson Municipal Power & Light (Reid/HMP&L) Surface Impoundment (the Unit) at the Sebree Generating Station located in Webster County, Robards, Kentucky (Figure 1).

Assessment monitoring results indicate the presence of lithium at a Statistically Significant Level (SSL) above the Ground Water Protection Standard (GWPS) in one monitoring well (MW-10) at the Unit. A map illustrating the site with location of all program monitoring wells is presented as **Figure 2**.

In response to the SSL exceedance, BREC evaluated the nature and extent of groundwater impacts as required by Title 40 CFR Part 257.95(g) for characterization monitoring. In addition, BREC performed an Assessment of Corrective Measures (ACM), to identify applicable remedial technologies to address lithium impacts in groundwater pursuant to Tile 40 CFR Part 257.96. A notice of ACM initiation dated January 14, 2019 was posted to BREC's publicly-accessible CCR reporting website. A report summarizing the results of the ACM (AECOM, June 2019) was posted to BREC's publicly-accessible CCR reporting website on June 14, 2019.

Title 40 CFR Part 257.97(a) requires that progress reports be prepared on a semi-annual basis describing progress made in selecting and designing a remedy. The first semi-annual *Remedy Selection Progress Report* (AECOM, December 2019) was posted to BREC's publicly-accessible CCR reporting website on December 9, 2019. In alignment with the CCR rule requirement, the following sections included within this semi-annual progress report provide an overview of BREC's activities previously performed, currently underway, and planned in the future to select a remedy that meets the requirement of Title 40 CFR Part 257.97 (b) as follows:

- Be protective of human health and the environment;
- (2) Attain the GWPS as specified pursuant to Section 257.95(h);
- (3) Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into the environment;
- (4) Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors such as avoiding inappropriate disturbance of sensitive ecosystems;
- (5) Comply with standards for management of wastes as specified in Section 257.98(d).

#### 2. Site Background

#### 2.1 Site Description

BREC operates the Sebree Station, which is a coal-fired power generating facility located on the Green River northeast of Sebree, Kentucky. Sebree Station is composed of Green Station and Reid/HMP&L Station. BREC owns Green and Reid Stations, while the City of Henderson owns HMP&L Station 2. The Sebree Station is bounded by Interstate-69 to the west and the Green River to the east (see **Figure 1**). Reid Unit 1 (65 Megawatts [MW]) began commercial operation in 1966 and is scheduled to be retired in 2020 pending regulatory approval from the Kentucky Public Service Commission and Rural Utilities Service. The Reid Combustion Turbine (65 MW) was commercialized in 1976. HMP&L Station 2, Units 1 (167 MW) and 2 (168 MW) began commercial operation in 1973 and 1974 respectively. Both HMP&L units were retired as of February 1, 2019. Green Station Units 1 (250 MW) and 2 (242 MW) began commercial operation in 1979 and 1981, respectively.

The location of the Reid/HMP&L Station Surface Impoundment is illustrated in **Figure 2**. The Surface Impoundment has been in place for more than 40 years and was used previously for the placement of CCR material. As stated in the published CCR monitoring well network certification, available on the BREC website, the Reid/HMP&L Station Surface Impoundment is a combined incised/dike earthen embankment structure. It is diked on the west, south and east sides, while the north side is incised. The south dike has the greatest height, reaching approximately 20 feet. Most of the central portion of the south dike was constructed on a subdued ridge.

#### 2.2 Groundwater Investigation Summary

Monitoring wells were installed around the perimeter of the Unit in December 2015 prior to the implementation of the CCR Rule. These wells meet the requirements of Title 40 CFR Part 257.90 of the CCR Rule for installation of a groundwater monitoring system. Under these requirements monitoring wells must adequately represent the quality of background groundwater and groundwater representing the downgradient waste boundary. The existing wells are located along the perimeter of the footprint for the Unit. One upgradient monitoring well (MW-7) and three downgradient monitoring wells (MW-8, MW-9, and MW-10) were installed adjacent to the Unit to determine the general direction of groundwater movement and to monitor groundwater impacts. The monitoring wells were installed in the uppermost saturated portion of the sandstone bedrock aquifer.

Nine rounds of Baseline groundwater sampling for Appendix III constituents was conducted between March 2016 and October 2017. Statistical evaluation of Appendix III constituents monitored for Detection monitoring indicated that statistically significant increases (SSIs) over background have occurred, and therefore, Assessment monitoring was triggered. Detection monitoring activities and data are presented in the annual reports that have been prepared to date (AECOM 2018, 2019, and 2020).

As part of Assessment monitoring, upgradient and downgradient wells for the Unit were sampled for Appendix IV constituents in April, July, and September 2018. GWPS were established for the Appendix IV constituents occurring at SSIs (lithium only), and statistical evaluation of the lithium concentrations indicated exceedances of GWPSs at SSLs, as detailed in **Table 1** below

Table 1 - Reid/HMP&L Surface Impoundment Constituents of Concern

Monitoring Well (Date)	Parameter Lithium GWPS 0.04 (mg/L)
MW-10 (Apr 2018)	0.694
MW-10 (Jul 2018)	0.630
MW-10 (Sep 2018)	0.570

GWPSs are the greater of the site-specific background concentrations, the USEPA primary drinking water standard maximum contaminant limits (MCL), or GWPS provided in 40 CFR 257.95(3)(h)(2)

An additional characterization well, MW-110, was subsequently installed to estimate the downgradient extent of impacted groundwater. Sample collection for Appendix III and IV parameters took place in March and April 2019. The analytical results for lithium in MW-110 were below the GWPS. The additional characterization data are summarized in **Table 2** below.

Table 2 – Reid/HMP&L Surface Impoundment Characterization Sample Results

	Parameter	
Monitoring Well (Date)	Lithium GWPS 0.04	
	(mg/L)	
MW-110 (March 2019)	0.0299	
MW-110 (April 2019)	0.0303	

The results from both characterization sampling events helped to confirm the downgradient (southwestern) extent of COC impacts above GWPS at the Unit.

Semi-annual Assessment monitoring continued at the Unit in 2019 and 2020 in accordance with 40 CFR Part 257.95.

#### 2.3 Conceptual Site Model

Development and refinement of a Conceptual Site Model (CSM) is necessary to support remedy selection for the Unit. A CSM is based on a set of working hypotheses regarding how contaminants of concern (COCs) entered the environment at a site, how they were and continue to be transported to various media, what the potential routes of exposure are, and who may be exposed, including both human and ecological receptors. As such, the CSM is a "living" model. As new data become available or site conditions change, a CSM should be evaluated and updated as necessary.

The CSM for the Unit was first provided in the June 2019 ACM for the Unit (AECOM 2019). The CSM presents the physical setting of the Unit (adjacent to the Green River), the unconsolidated and bedrock geologic strata underlying the Unit, the occurrence and movement of groundwater, the distribution of COCs in groundwater, and the potential receptors (or lack thereof) for impacted groundwater. These elements are described in detail below and have been updated with new information for this report as appropriate.

#### 2.3.1 Physical Setting

The Unit is located within the Interior Low Plateaus physiographic province. The province is part of the Interior Plains division of the United States. Characteristic features of the province include unglaciated

rolling limestone plains with alluvial valleys and entrenched rivers and streams. Several large rivers are in the region, including the Green, Ohio, Kentucky, Tennessee, and the Cumberland Rivers. The geology underlying the Unit consists of unconsolidated materials, including loess and alluvial deposits, underlain by Upper to Middle Pennsylvanian-age clastic and carbonate bedrock consisting primarily of sandstone and shale. The unconsolidated materials also include fill, silty and clayey residuum, and minor amounts of sandy, clayey channel fill alluvium.

The Unit is located on upland area near the west bank of the Green River. The uppermost edge of the earthen embankment is situated at an elevation of approximately 429 feet above mean sea level (amsl). Although the Green River is located less than 0.5 miles from the site, the structure does not extend significantly into the floodplain. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands (Associated Engineers 2016, Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan). The immediate watershed that drains to the unit, and in which the unit is considered to be located, is unnamed and 25.45 acres in size. The unnamed watershed discharges from the Unit outflow structure and is routed, under a Kentucky Pollution Discharge and Elimination System permit, to the Green River.

#### 2.3.2 Geology

The Unit lies in the Western Kentucky Coalfields section, characterized by rolling uplands underlain by coal-bearing bedrock of the Pennsylvanian Period. Near the Unit, maximum topographic relief is on the order of 80 feet. The geologic quadrangle (Geologic map of the Robards quadrangle, Henderson and Webster Counties, Kentucky, 1973) for the area published by the Kentucky Geologic Survey (KGS) shows the surficial material in portions of the western half of the Unit to be unconsolidated loess representing the Pleistocene geologic epoch. The loess consists of sandy and clayey silt. Underlying the loess deposits and exposed at the surface on the eastern half of the Unit are broadly distributed Pleistocene and Holocene alluvium deposits consisting of intermixed and interlensing clay, silt, sand, and gravel. In close proximity to the Unit, the alluvium is generally a low permeability unit that forms terraces along the Green River at elevations of roughly 380 and 395 ft., amsl. The unconsolidated surficial materials range from approximately 24 feet (MW-7) to 47 feet (MW-110) in thickness surrounding the Unit.

The unconsolidated materials are underlain by bedrock of the Upper Pennsylvanian Shelburn Formation [formerly identified as the Lisman Formation (Fairer, 1973)] and the Middle Pennsylvanian Carbondale Formation. At the base of the Shelburn Formation is the Providence Limestone Member, consisting of two distinct limestone beds separated by a sandy shale. The member is exposed in a streambed near the northwest corner of the Unit but is absent beneath much of the Unit footprint due to erosional channeling. The underlying Carbondale Formation consists of cyclic sequences of sandstones, shales, siltstones and coals. The Carbondale sediments were deposited in a fluvial-deltaic system. As a result of this depositional environment, the lithologic units of the Carbondale tend to be lenticular bodies rather than continuous sheet-like strata. Gradational and abrupt horizontal changes in lithology are often encountered.

#### 2.3.3 Groundwater Hydrogeology

For purposes of compliance with the CCR Rule groundwater monitoring requirements, the interbedded sandstone and shale of the Carbondale Formation is considered the uppermost aquifer underlying the Unit. The uppermost aquifer is hydraulically confined and first encountered at an elevation of approximately 425 ft., amsl at the northeast end (at MW-7), and 388 ft. amsl at the west end of the Unit (at MW-9).

Groundwater elevation data collected in October 2019 are summarized on **Table 3** below. These data were utilized to construct a piezometric surface map illustrating groundwater flow conditions for the uppermost aquifer (see **Figure 3**). Flow direction beneath the Unit is to the southwest towards an unnamed tributary to Groves Creek located west-southwest of the impoundment.

Table 3. Reid/HMP&L Surface Impoundment – October 2019 Groundwater Elevation Data

Monitoring Well	Top of Casing Elevation (ft) <sup>1</sup>	Depth to Groundwater (ft)	Groundwater Elevation (ft, amsl)
MW-7	444.43	18.59	425.84
MW-8	394.29	5.20	389.09
MW-9	395.40	7.35	388.05
MW-10	422.27	33.28	388.99

Reference elevation of monitoring wells surveyed by Associated Engineers, Inc., Madisonville, Kentucky, January 2015. Survey coordinates were based on the Kentucky State Plane, Kentucky Southern Zone, NAD27 datum.

Slug tests were performed between April 24, 2019 and April 25, 2019 at monitoring wells MW-10, and MW-110 to assess the hydraulic characteristics of the uppermost aquifer. The estimated hydraulic conductivity of the monitoring wells tested ranged from  $3 \times 10^{-6}$  to  $5 \times 10^{-4}$  centimeters per second (cm/sec).

Although previous site-specific investigations have noted the presence of perched zones of saturation in the overlying unconsolidated materials, these discontinuous zones do not qualify as an uppermost aquifer under the CCR Rule because they do not produce usable quantities of groundwater.

#### 2.3.4 Constituents of Concern

Current groundwater analytical data and statistical analysis indicate that the only COC detected at SSLs above its GWPS in groundwater at the Unit is lithium. Lithium has been detected at SSLs in the monitoring well MW-10 southwest of the Unit.

#### 2.3.5 Impacted Media

Groundwater is the single impacted media of concern identified as requiring corrective measures at the Unit.

#### 2.3.6 Distribution of COCs

Groundwater sampling was performed at the Unit most recently in April 2020. The additional lithium data collected during this event are summarized below in **Table 4**.

Table 4. Reid/HMP&L Surface Impoundment - April 2020 Lithium Analytical Results

	Parameter		
Monitoring Well (Date)	Lithium GWPS 0.04		
	(mg/L)		
MW-7	0.007		
MW-8	0.03		
MW-9	0.01		
MW-10	0.49		
MW-110	0.02		

**Figure 4** illustrates the distribution of COCs and other groundwater quality constituents in groundwater at the Unit. This distribution of COCs in groundwater suggests that impacts to groundwater likely originate as seepage from beneath the surface impoundment, however there is currently no feasible means of directly tracing that potential under the footprint of the Unit.

#### 2.3.7 Potential Receptors/Exposure Pathways

Contact with water (e.g., shallow groundwater or surface water) impacted by COCs at levels above GWPS or Water Quality Criteria is regarded as the potential pathway for exposure of potential receptors. Based on data published by KGS, there are no known groundwater wells used for drinking water within a 1-mile radius of the Unit, thus limiting the potential receptors to the surface water, i.e., the Green River and its tributary, Groves Creek. The pathways to these receptors include seepage of water from the Unit through manmade and natural hydraulic conduits.

Other potential exposure pathways (e.g., soil or vapor) are not considered complete as the CCR material is isolated in the Unit. This isolation prevents direct access by individuals that might result in direct contact or ingestion. In addition, the inherent non-volatile nature of the Unit-specific COCs eliminates the potential for a complete vapor pathway (i.e., vapor intrusion to indoor air).

#### 2.4 Interim Corrective Measures

No interim corrective measures have been performed at the Unit for groundwater impacts.

#### 2.5 Assessment of Corrective Measures Summary

In June 2019, BREC performed an ACM for the Unit to identify remedial alternatives to address groundwater impacts. Title 40 CFR Part 257.96(c) requires that the ACM include an analysis of the effectiveness of potential corrective measures in meeting the objectives for remedies identified under Section 257.97(b), by addressing at least the following:

- The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- 2) The time required to begin and complete the remedy; and
- 3) The institutional requirements, such as state or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the remedy(s).

As part of the groundwater ACM, several potential corrective measures technologies were evaluated to identify which ones could be carried forward as components of corrective measures alternatives. The results of the corrective measures technology evaluation are presented below in **Table 5**.

Table 5 - Potential Corrective Measures Options for Groundwater Impacts

Potentially Applicable Technology	Status	Description/Overview
No Action	Not retained as standalone technology, but carried forward for baseline comparisons	This technology has been included in the preliminary evaluation/screening but is not retained because it will not meet the established CAOs.
Institutional Controls (ICs)	Retained as supplement to corrective measures alternatives	The use of ICs (i.e., Environmental Covenants, groundwater use restrictions, etc.) is retained as a useful technology. However, it is noted the ICs are not anticipated to be used as a stand-alone technology. Environmental Covenants, groundwater use restrictions, etc., are expected to be combined with other applicable technologies as part of corrective measures alternatives.

Potentially Applicable Technology	Status	Description/Overview
Groundwater Monitoring (Assessment and Detection modes)	Retained as supplement to corrective measures alternatives	The use of groundwater monitoring (Assessment and/or Detection modes as appropriate) when combined with other applicable technologies as part of any proposed corrective measures alternative is retained to address the CAO and to track the effectiveness of the overall remedy. However, it is not retained as a stand-alone technology.
Hydraulic Containment	Retained	The use of hydraulic containment is retained because it is an effective means of preventing off-site migration of soluble contaminants. Hydraulic containment requires management and potential ex-situ treatment of extracted groundwater, so it is not a stand-alone technology. The CSM will guide the design of any groundwater extraction system to optimize the total discharge of groundwater needed to provide hydraulic containment.
Physical Containment	Retained	The use of physical containment is retained because it can be an effective means of managing groundwater flow. Physical containment often requires pairing with hydraulic containment and/or in-situ treatment (funnel and gate style) to manage the flux of groundwater flow into the system. The CSM will guide the design of any physical barrier system, but technology limitations may increase implementation difficulty with scale.
Ex-situ Physical/Chemical/Biological Treatment	Retained	Ex-situ treatment technologies are retained as a way of removing contaminants from extracted groundwater from a hydraulic containment system. Ex-situ treatment may be paired with wastewater treatment, non-groundwater release treatment systems, or with permitted discharge to manage groundwater contamination. The CSM and data gaps investigations will guide the design of any ex-situ treatment.
Closure in Place (CiP) (of the regulated unit)	Retained	The use of CiP as a source control technology and is amenable with respect to CAO attainment.
Closure by Removal (CbR) (of the regulated unit)	Retained	The use of CbR as a source control technology is amenable with respect to CAO attainment.
Other Source Control Technologies  Note: Technologies that were retain	Retained	Control of source area non-groundwater related releases. For the purposes of this groundwater ACM, management of non-groundwater releases are not included in the alternatives evaluation. Engineering measures, including leachate collection, lining of trenches and/or ponds, and other isolation methods are regarded as part of closure technologies selected by other means.

Technologies that were retained may be used as components of a corrective action alternative, but when evaluated in conjunction with other available technologies any single technology may not be utilized.

Preliminary assembly of corrective measures alternatives was performed based on site-specific and regional geology and groundwater conditions. For the Reid/HMP&L Station Surface Impoundment, five corrective measures alternatives were developed from this list of applicable corrective measures technologies:

Alternative #1 – No Action, and Groundwater Monitoring

- Alternative #2a Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring
- Alternative #2b Closure by Removal (CbR), ICs, and Groundwater Monitoring
- Alternative #3 CiP, ICs, Hydraulic Containment, Other Source Control, Ex-Situ Treatment, and Groundwater Monitoring
- Alternative #4 CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

The assembly of corrective measures alternatives presented in the ACM was considered preliminary and subject to revision following additional evaluation during the remedy selection process and/or following comment from the regulatory community and public. Further evaluation of the alternatives is discussed in the following sections.

#### 3. Remedy Selection Progress

The ACM performed for the Unit in June 2019 identified a total of five (5) corrective measures alternatives to be carried forward into the remedy selection process. In December 2019, BREC provided a *Semi-annual Remedy Selection Progress Report* (AECOM, December 2019) as required under 40 CFR Part 257.97(a). As part of this submittal, two (2) corrective measures alternatives were eliminated from further consideration, including:

- Alternative #1 (No Action and Groundwater Monitoring) This alternative does not control or remove COCs from the environment and therefore does not achieve the RAOs.
- Alternative #2b (CbR, ICs, and Groundwater Monitoring) Implementing a CbR approach is considered cost prohibitive. In addition, any CbR approach would require relocating waste to an existing disposal unit or construction of a new waste disposal unit, which does not align with the one of the fundamental goals of RCRA (conserving energy and natural resources).

Three (3) potential corrective measures alternatives have been identified by BREC as viable options to address lithium impacts in groundwater at the Unit, including:

- Alternative #2a: CiP, ICs, and Groundwater Monitoring
- Alternative #3: CiP, ICs, Hydraulic Containment, Ex-Situ Treatment, and Groundwater Monitoring
- Alternative #4: CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

Each of the remaining 3 corrective measures alternatives is discussed in more detail below.

#### 3.1 Potential Corrective Action Alternatives

#### 3.1.1 Alternative #2a – CiP, ICs, and Groundwater Monitoring

Alternative #2a as currently envisioned would employ a combination of three corrective measures technologies:

- CiP source control, which consists of planned Reid/HMP&L Surface Impoundment closure activities;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater monitoring (Assessment) to document the effectiveness of the corrective measures.

Alternative #2a is recommended for further evaluation.

## 3.1.2 Alternative #3 – CiP, ICs, Hydraulic Containment, Ex-Situ Treatment, and Groundwater Monitoring

Alternative #3 builds on Alternative #2a to also include the addition of Hydraulic Containment and Ex-Situ Treatment of groundwater:

- CiP source control, which consists of planned Surface Impoundment closure activities;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes;
- Hydraulic Containment using one or more vertical wells designed to prevent the movement of impacted groundwater past the limits of the unit to the downgradient groundwater environment and potential points of exposure;

- Ex-Situ Treatment of groundwater extracted for hydraulic containment, which involves aboveground physical/chemical treatment methods and/or permitted discharge until the CAOs are achieved;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater Monitoring (Assessment mode) to track the effectiveness of the corrective measures
  and to identify conditions that allow the return to Detection-mode monitoring and ultimately to
  cessation of corrective measures.

Alternative #3 is recommended for further evaluation.

## 3.1.3 Alternative #4 – CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

Alternative #4 consists of BREC's planned unit closure activities, physical containment of impacted groundwater via installation of a funnel-gate system, and ex-situ treatment of contained groundwater via an extraction well installed at the containment gate. Impacted groundwater would be contained by grout curtain constructed in a funnel-and-gate arrangement that directs the flow of groundwater to an extraction point. The grout curtain would be installed by drilling two lines of grout injection points that extend northwestward and northeastward from the southeast corner of the unit. The length of each limb of the barrier would be 500 feet, and the target depth would be approximately 325 ft-amsl. A single extraction well would be installed at the "gate" with a screened interval of 50 to 100 ft-bgs and a pumping capacity of up to 20 gpm. Groundwater will be pumped and conveyed to an existing surface water impoundment at the Sebree Station, which will allow for compliance with discharge permits through an established NPDES outfall.

CiP via ash stabilization and capping would control the source of COCs and thereby reduce contaminant loading to the extraction system. Concentrations downgradient of the physical barrier would be expected to decrease over time through several natural attenuation mechanisms including advection, dilution, and dispersion. Groundwater Monitoring (Assessment) would continue to track the effectiveness of the corrective measures and to identify conditions that allow the return to Detection monitoring and ultimately closure.

Alternative #4 is recommended for further evaluation.

#### 3.2 Remedy Evaluation

Currently BREC considers the (3) potential corrective action alternatives as viable options to address groundwater impacts at the Unit, including:

- Alternative #2a;
- Alternative #3; and
- Alternative #4

To evaluate each alternative, additional data collection will likely be required. BREC is currently evaluating data collection needs in the following areas to assist with remedy selection:

- Nature and Extent groundwater trends, influence of non-groundwater remedies, etc.
- 2) Physical Characteristics available data on the physical characteristics of the landfill and retention pond
- 3) Performance Modeling data needed to develop digital models demonstrating the effectiveness of potential alternatives
- 4) Engineering feasibility, cost estimates, etc.

BREC is working to establish a comprehensive list of data collection needs to proceed forward with remedy evaluation and anticipates providing additional data in future semi-annual remedy selection progress reports.

#### 3.3 Public Meeting

At the beginning of 2020, BREC had initiated preparation to conduct a public meeting to discuss the results of the Groundwater ACM as required by 40 CFR 257.96(e). However, due to the onset of the COVID-19 pandemic, BREC has been prevented from holding the public meeting so far in 2020. BREC plans to hold a public meeting once the mass gathering restrictions related to COVID-19 are lifted in Kentucky.

#### 4. Conclusion

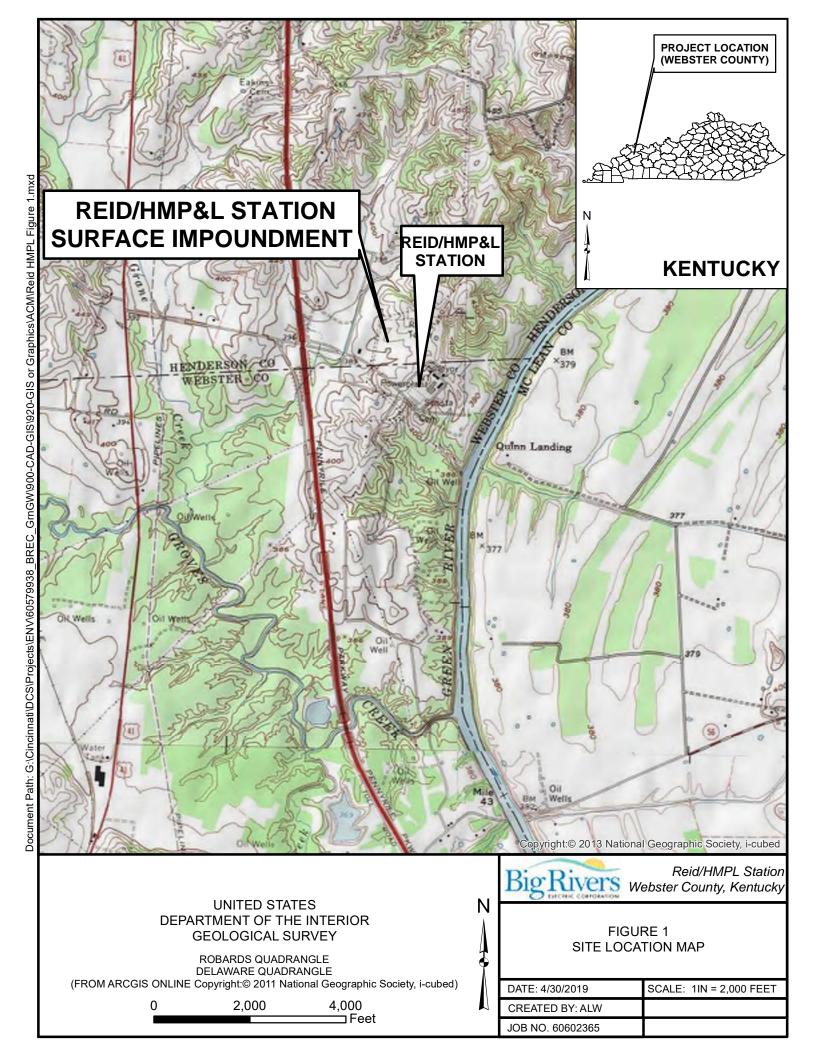
Additional updates regarding remedy selection, including any additional corrective measures being considered, will be presented twice a year in future remedy selection progress reports. Once sufficient data has been collected to select an effective comprehensive remedy for the Unit, a public meeting will be held 30 days prior to formal remedy selection, followed by a detailed Remedy Selection Report describing the remedy and proposed schedule for implementation.

If needed, the next remedy selection progress report for the Unit is expected in December 2020.

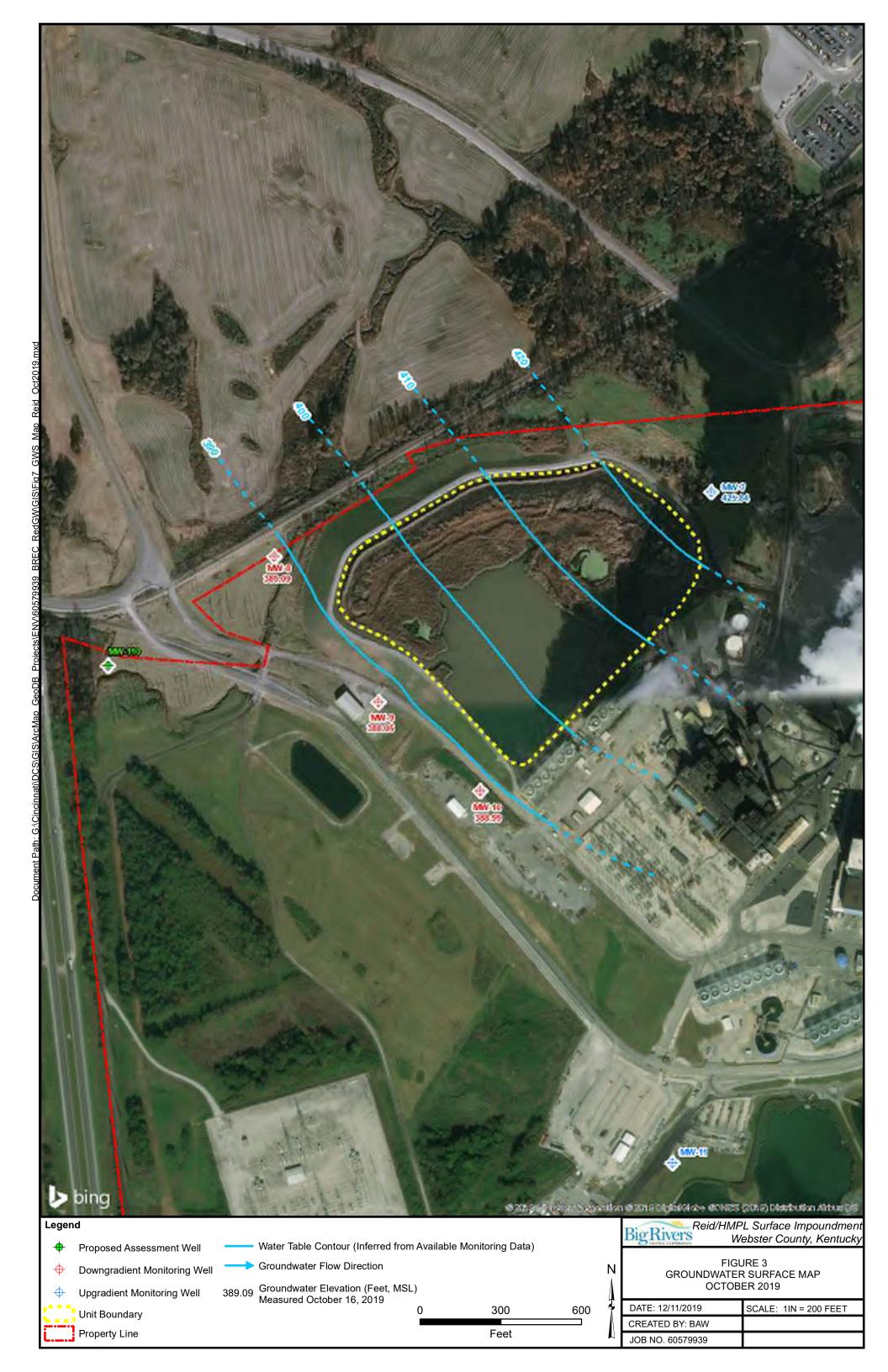
#### 5. References

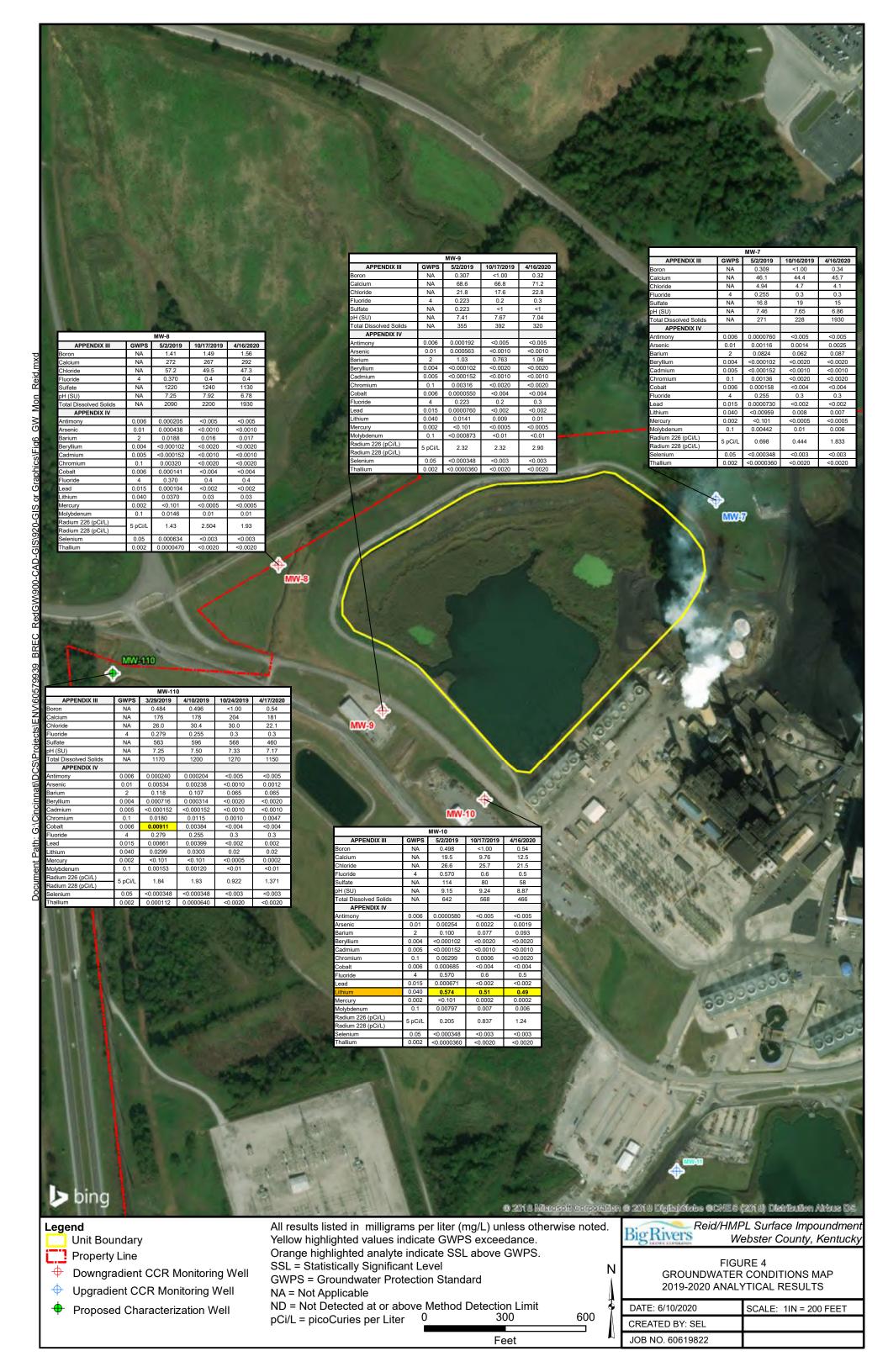
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### **Figures**









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## REMEDY SELECTION PROGRESS REPORT

#### REID/HMP&L SURFACE IMPOUNDMENT SEBREE GENERATING STATION WEBSTER COUNTY, KENTUCKY

December 9, 2019

Prepared For:



Big Rivers Electric Corporation Sebree Generating Station 9000 Highway 2096 Robards, KY 42452

Prepared by:



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#### 1.0 INTRODUCTION

In accordance with provisions of the United States Environmental Protection Agency's (USEPA) coal combustion residual (CCR) rule, Title 40 of the Code of Federal Regulations (CFR) Section 257.97, Big Rivers Electric Cooperation (BREC) is in the process of selecting a remedy for groundwater impacts at the Reid/Henderson Municipal Power & Light (Reid/HMP&L) Surface Impoundment (the Unit) at the Sebree Generating Station located in Webster County, Robards, Kentucky (Figure 1).

Assessment monitoring results indicate the presence of lithium at a Statistically Significant Level (SSL) above the Ground Water Protection Standard (GWPS) in one monitoring well (MW-10) at the Unit. A map illustrating the site with location of all program monitoring wells is presented as **Figure 2**.

In response to the SSL exceedance, BREC evaluated the nature and extent of groundwater impacts as required by Title 40 CFR Section 257.95(g) for characterization monitoring. In addition, BREC performed an Assessment of Corrective Measures (ACM), to identify applicable remedial technologies to address lithium impacts in groundwater pursuant to Tile 40 CFR Section 257.96. A notice of ACM initiation dated January 14, 2019 was posted to BREC's publicly-accessible CCR reporting website. A report summarizing the results of the ACM (AECOM, June 2019) was posted to BREC's publicly-accessible CCR reporting website on June 14, 2019.

Title 40 CFR Section 257.97(a) requires that progress reports be prepared on a semi-annual basis describing progress made in selecting and designing a remedy. The following sections provide an overview of BREC's activities previously performed, currently underway, and planned in the future to select a remedy that meets the requirement of Title 40 CFR Section 257.97 (b) as follows:

- (1) Be protective of human health and the environment;
- (2) Attain the GWPS as specified pursuant to Section 257.95(h);
- (3) Control the source(s) of releases so as to reduce or eliminate, to the maximum extent feasible, further releases of Appendix IV constituents into the environment;
- (4) Remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible, taking into account factors such as avoiding inappropriate disturbance of sensitive ecosystems;
- (5) Comply with standards for management of wastes as specified in Section 257.98(d).

#### 2.0 SITE BACKGROUND

#### 2.1 Site Description

BREC operates the Sebree Station, which is a coal-fired power generating facility located on the Green River northeast of Sebree, Kentucky. Sebree Station is composed of Green Station and Reid/HMP&L Station. BREC owns Green and Reid Stations, while the City of Henderson owns HMP&L Station 2. The Sebree Station is bounded by Interstate-69 to the west and the Green River to the east (see **Figure 1**). Reid Unit 1 (66 Megawatts) began commercial operation in 1966 and it will be converted from coal to natural gas in the future. The Reid Combustion Turbine (72 MW) was commercialized in 1976. HMP&L Station 2, Units 1 (167 MW) and 2 (168 MW) began commercial operation in 1973 and 1974 respectively. Both HMP&L units were retired as of February 1, 2019. Green Station Units 1 (242 MW) and 2 (242 MW) began commercial operation in 1979 and 1981, respectively.

The location of the Reid/HMP&L Station Surface Impoundment is illustrated in **Figure 2**. The Surface Impoundment has been in place for more than 40 years and is used for the placement of CCR material. As stated in the published CCR monitoring well network certification, available on the BREC website, the Reid/HMP&L Station Surface Impoundment is a combined incised/dike earthen embankment structure. It is diked on the west, south and east sides, while the north side is incised. The south dike has the greatest height, reaching approximately 20 feet. Most of the central portion of the south dike was constructed on a subdued ridge.

#### 2.2 Groundwater Investigation Summary

Monitoring wells were installed around the perimeter of the Unit in December 2015 prior to the implementation of the CCR Rule. These wells meet the requirements of §257.90 of the CCR Rule for installation of a groundwater monitoring system. Under these requirements monitoring wells must adequately represent the quality of background groundwater and groundwater representing the downgradient waste boundary. The existing wells are located along the perimeter of the footprint for the Unit. One upgradient monitoring well (MW-7) and three downgradient monitoring wells (MW-8, MW-9, and MW-10) were installed adjacent to the Unit to determine the general direction of groundwater movement and to monitor groundwater impacts. The monitoring wells were installed in the uppermost saturated portion of the sandstone bedrock aquifer.

Nine rounds of Baseline groundwater sampling was conducted between March 2016 and October 2017. Statistical evaluation of Appendix III constituents monitored for Detection monitoring indicated that statistically significant increases (SSIs) over background have occurred, and therefore, Assessment monitoring was triggered. Detection monitoring activities and data are presented in the annual reports that have been prepared to date (AECOM 2018 and 2019).

As part of Assessment monitoring, upgradient and downgradient wells for the Unit were sampled for Appendix IV constituents in April, July, and September 2018. GWPS were established for Assessment monitoring of the Appendix IV constituents, and statistical evaluation indicated exceedances of GWPSs at SSLs, as detailed in **Table 1** below.

Table 1 - Reid/HMP&L Surface Impoundment Constituents of Concern

Monitoring Well (Date)	Parameter Lithium GWPS 0.04 (mg/L)
MW-10 (Apr 2018)	0.694
MW-10 (Jul 2018)	0.630
MW-10 (Sep 2018)	0.570

GWPSs are the greater of the site-specific background concentrations, the USEPA primary drinking water standard maximum contaminant limits (MCL), or GWPS provided in 40 CFR 257.95(3)(h)(2)

An additional characterization well, MW-110, was subsequently installed to estimate the downgradient extent of impacted groundwater. Sample collection for Appendix III and IV parameters took place in March and April 2019. The analytical results for lithium in MW-110 were below the GWPS. The additional characterization data are summarized in **Table 2** below.

Table 2 - Reid/HMP&L Surface Impoundment Characterization Sample Results

	Parameter	
Monitoring Well (Date)	Lithium GWPS 0.04	
	(mg/L)	
MW-110 (March 2019)	0.0299	
MW-110 (April 2019)	0.0303	

The results from both characterization sampling events helped to confirm the downgradient (southwestern) extent of COC impacts above GWPS at the Unit.

#### 2.3 Conceptual Site Model

A Conceptual Site Model (CSM) has been developed to support the remedy selection process for groundwater corrective action at the Unit.

#### 2.3.1 Physical Setting

The Unit is located within the Interior Low Plateaus physiographic province. The province is part of the Interior Plains division of the United States. Characteristic features of the province include unglaciated rolling limestone plains with alluvial valleys and entrenched rivers and streams. Several large rivers are in the region, including the Green, the Ohio, the Kentucky, the Tennessee, and the Cumberland Rivers. The geology underlying the Unit consists of unconsolidated materials, including loess and alluvial deposits, underlain by Upper to Middle Pennsylvanian-age clastics and carbonates consisting primarily of sandstone and shale. The unconsolidated materials also include fill, silty and clayey residuum, and minor amounts of sandy, and clayey channel fill alluvium.

The Unit is located on upland area near the west bank of the Green River. The uppermost edge of the earthen embankment is situated at an elevation of approximately 429 feet above mean sea level (amsl). Although the Green River is located less than 0.5 miles from the site, the structure does not extend significantly into the floodplain. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands (Associated Engineers 2016, Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan). The immediate watershed that drains to the unit, and in which the unit is considered to be located, is unnamed and 25.45 acres in size. The unnamed watershed discharges from the Unit outflow structure and is routed, under a Kentucky Pollution Discharge and Elimination System permit, to the Green River.

#### 2.3.2 Geology

The Unit lies in the Western Kentucky Coalfields section of the Interior Low Plateaus physiographic province, characterized by rolling uplands underlain by coal-bearing bedrock of the Pennsylvanian Period. Near the Unit, maximum topographic relief is on the order of 80 feet. The geologic quadrangle (Geologic map of the Robards quadrangle, Henderson and Webster Counties, Kentucky, 1973) for the area published by the Kentucky Geologic Survey (KGS) shows the surficial material in portions of the western half of the Unit to be unconsolidated loess representing the Pleistocene geologic epoch. The loess consists of sandy and clayey silt. Underlying the loess deposits in places are broadly distributed Pleistocene and Holocene alluvium deposits consisting of intermixed and interlensing clay, silt, sand, and gravel. In close proximity to the Unit, the alluvium is generally a low permeability unit that forms terraces along the Green River at elevations of roughly 380 and 395 ft., amsl. The unconsolidated surficial materials range from approximately 24 feet (MW-7) to 47 feet (MW-110) in thickness surrounding the Unit.

The unconsolidated materials are underlain by bedrock of the Upper Pennsylvanian Shelburn Formation [formerly identified as the Lisman Formation (Fairer, 1973)] and the Middle Pennsylvanian Carbondale Formation. At the base of the Shelburn Formation is the Providence Limestone Member, consisting of two distinct limestone beds separated by a sandy shale, but this member is absent in much of the area due to erosional channeling. The underlying Carbondale Formation consists of cyclic sequences of sandstones, shales, siltstones and coals. The Carbondale sediments were deposited in a fluvial-deltaic system. As a result of this depositional environment, the lithologic units of the Carbondale tend to be lenticular bodies rather than continuous sheet-like strata. Gradational and abrupt horizontal changes in lithology are often encountered.

#### 2.3.3 Hydrogeology

For purposes of compliance with the CCR Rule groundwater monitoring requirements, the interbedded sandstone and shale of the Carbondale Formation is considered the uppermost aquifer underlying the Unit. The uppermost aquifer is hydraulically confined and first encountered at an elevation of approximately 413.4 ft., amsl at the northeast end (at MW-7), and 341.6 ft. amsl at the west end of the Unit (at MW-8). Flow direction beneath the Unit is typically to the southwest towards an unnamed tributary to Groves Creek located west/southwest of the impoundment.

Slug tests were performed between April 24, 2019 and April 25, 2019 at monitoring wells MW-10, and MW-110 to assess the hydraulic characteristics of the uppermost aquifer. The estimated hydraulic conductivity of the monitoring wells tested ranged from  $3 \times 10^{-6}$  to  $5 \times 10^{-4}$  centimeters per second (cm/sec).

Although previous site-specific investigations have noted the presence of perched zones of saturation in the overlying unconsolidated materials, these discontinuous zones do not qualify as an uppermost aquifer under the CCR Rule because they do not produce usable quantities of groundwater.

#### 2.3.4 Constituents of Concern (COC)

As discussed above, a single Appendix IV COC (lithium) was detected at concentrations exceeding GWPS in one monitoring well location (MW-10). As a result, the corrective measure evaluation is confined to the area adjacent to this monitoring well.

#### 2.3.5 Potential Receptors/Exposure Pathways

Contact with water (e.g., shallow groundwater or surface water) impacted by COCs at levels above GWPS is regarded as the potential pathway for exposure of potential receptors. Based on data published by KGS, there are no known groundwater wells used for drinking water within a 1-mile radius of the Unit, thus limiting the potential receptors to the surface water, i.e., the Green River and its tributary, Groves Creek. The pathways to these receptors include seepage of water from the Unit through manmade and natural hydraulic barriers.

Other potential exposure pathways (e.g., soil or vapor) are not considered complete as the CCR material is isolated in the Unit. This isolation prevents direct access by individuals that might result in direct contact or ingestion. In addition, the inherent non-volatile nature of the unit-specific COCs eliminates the potential for a complete vapor pathway (i.e., vapor intrusion to indoor air).

#### 2.4 Interim Corrective Measures

No interim corrective measures have been performed at the Unit for groundwater impacts.

#### 2.5 Assessment of Corrective Measures Summary

Title 40 CFR Section 257.96(c) requires that the ACM include an analysis of the effectiveness of potential corrective measures in meeting the objectives for remedies identified under Section 257.97(b), by addressing at least the following:

- The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- 2) The time required to begin and complete the remedy; and
- 3) The institutional requirements, such as state or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the remedy(s).

Several potential corrective measures technologies were evaluated to identify which ones could be carried forward as components of corrective measures alternatives. The results of the corrective measures technology evaluation are presented in **Table 3** below.

Table 3 – Potential Corrective Measures Options Technology Description/Overview

Potentially Applicable Technology	Status	Description/Overview
No Action	Not retained as standalone technology, but carried forward for baseline comparisons	This technology has been included in the preliminary evaluation/screening but is not retained because it will not meet the established CAOs.
Institutional Controls (ICs)	Retained as supplement to corrective measures alternatives	The use of ICs (i.e., Environmental Covenants, groundwater use restrictions, etc.) is retained as a useful technology. However, it is noted the ICs are not anticipated to be used as a stand-alone technology. Environmental Covenants, groundwater use restrictions, etc., are expected to be combined with other applicable technologies as part of corrective measures alternatives.
Groundwater Monitoring (Assessment and Detection modes)	Retained as supplement to corrective measures alternatives	The use of groundwater monitoring (Assessment and/or Detection modes as appropriate) when combined with other applicable technologies as part of any proposed corrective measures alternative is retained to address the CAO and to track the effectiveness of the overall remedy. However, it is not retained as a stand-alone technology.
Hydraulic Containment	Retained	The use of hydraulic containment is retained because it is an effective means of preventing off-site migration of soluble contaminants. Hydraulic containment requires management and potential ex-situ treatment of extracted groundwater, so it is not a stand-alone technology. The CSM will guide the design of any groundwater extraction system to optimize the total discharge of groundwater needed to provide hydraulic containment.
Physical Containment	Retained	The use of physical containment is retained because it can be an effective means of managing groundwater flow. Physical containment often requires pairing with hydraulic containment and/or in-situ treatment (funnel and gate style) to manage the flux of groundwater flow into the system. The CSM will guide the design of any physical barrier system, but technology limitations may increase implementation difficulty with scale.
Ex-situ Physical/Chemical/Biological Treatment	Retained	Ex-situ treatment technologies are retained as a way of removing contaminants from extracted groundwater from a hydraulic containment system. Ex-situ treatment may be paired with wastewater treatment, non-groundwater release treatment systems, or with permitted discharge to manage groundwater contamination. The CSM and data gaps investigations will guide the design of any ex-situ treatment.
Closure in Place (CiP) (of the regulated unit)	Retained	The use of CiP as a source control technology and is amenable with respect to CAO attainment.

Potentially Applicable Technology	Status	Description/Overview
Closure by Removal (CbR) (of the regulated unit)	Retained	The use of CbR as a source control technology is amenable with respect to CAO attainment.
Other Source Control Technologies	Retained	Control of source area non-groundwater related releases. For the purposes of this groundwater ACM, management of non-groundwater releases are not included in the alternatives evaluation. Engineering measures, including leachate collection, lining of trenches and/or ponds, and other isolation methods are regarded as part of closure technologies selected by other means.

Technologies that were retained may be used as components of a corrective action alternative, but when evaluated in conjunction with other available technologies any single technology may not be utilized.

Preliminary assembly of corrective measures alternatives was performed based on site-specific and regional geology and groundwater conditions. For the Reid/HMP&L Station Surface Impoundment, five corrective measures alternatives were developed from this list of applicable corrective measures technologies:

- Alternative #1 No Action, and Groundwater Monitoring
- Alternative #2a Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring
- Alternative #2b Closure by Removal (CbR), ICs, and Groundwater Monitoring
- Alternative #3 CiP, ICs, Hydraulic Containment, Other Source Control, Ex-Situ Treatment, and Groundwater Monitoring
- Alternative #4 CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

The assembly of corrective measures alternatives presented in the ACM is considered preliminary and could be revised at a later date following detailed analysis during the remedy selection process and/or following comment from the regulatory community and public.

#### 3.0 REMEDY SELECTION PROGRESS

Five corrective measure alternatives were identified during the ACM process for potential implementation at the Unit to address groundwater impacts. Each corrective measure alternative consists of one or more corrective measures technologies assembled into a strategy for the groundwater remedy. Each alternative is discussed in more detail below.

#### 3.1 Potential Corrective Action Alternatives

#### 3.1.1 Alternative #1 - No Action and Groundwater Monitoring

Alternative #1 consists of taking no action to address groundwater impacts at the Unit. Under the No Action alternative, no corrective action would be implemented to remove, control, mitigate, or minimize exposure to impacted groundwater. The No Action alternative establishes a baseline or reference point against which each of the corrective measure alternatives is compared.

Since Alternative #1 would not attain the CAOs for the Unit, this alternative would not likely be acceptable to stakeholders. Therefore, Alternative #1 is not recommended for further consideration.

## 3.1.2 Alternative #2a - Closure in Place (CiP), Institutional Controls (ICs), and Groundwater Monitoring

Alternative #2a as currently envisioned would employ a combination of three corrective measures technologies:

- CiP source control, which consists of planned Reid/HMP&L Surface Impoundment closure activities;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater monitoring (Assessment) to document the effectiveness of the corrective measures.

Alternative #2a is recommended for further evaluation.

#### 3.1.3 Alternative #2b - Closure by Removal (CbR), ICs, and Groundwater Monitoring

Alternative #2b as currently envisioned would be similar to Alternative #2a except that CiP is replaced by CbR, which consists of excavation and removal of the Unit. Given that Alternative #2b is likely cost prohibitive, this alternative is <u>not</u> recommended for further consideration.

## 3.1.4 Alternative #3 - CiP, Hydraulic Containment, Ex-Situ Treatment, ICs, and Groundwater Monitoring

Alternative #3 builds on Alternative #2a to also include the addition of Hydraulic Containment and Ex-Situ Treatment of groundwater:

- CiP source control, which consists of planned Surface Impoundment closure activities;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes;

- Hydraulic Containment using one or more vertical wells designed to prevent the movement of impacted groundwater past the limits of the unit to the downgradient groundwater environment and potential points of exposure;
- Ex-Situ Treatment of groundwater extracted for hydraulic containment, which involves aboveground physical/chemical treatment methods and/or permitted discharge until the CAOs are achieved;
- Implementation of ICs designed to restrict the property to industrial use and to prohibit groundwater use for potable purposes; and
- Groundwater Monitoring (Assessment mode) to track the effectiveness of the corrective measures and to identify conditions that allow the return to Detection-mode monitoring and ultimately to cessation of corrective measures.

Alternative #3 is recommended for further evaluation.

## 3.1.5 Alternative #4 - CiP, ICs, Physical Containment, Ex-Situ Treatment, and Groundwater Monitoring

Alternative #4 consists of BREC's planned unit closure activities, physical containment of impacted groundwater via installation of a funnel-gate system, and ex-situ treatment of contained groundwater via an extraction well installed at the containment gate. Impacted groundwater would be contained by grout curtain constructed in a funnel-and-gate arrangement that directs the flow of groundwater to an extraction point. The grout curtain would be installed by drilling two lines of grout injection points that extend northwestward and northeastward from the southeast corner of the unit. The length of each limb of the barrier would be 500 feet, and the target depth would be approximately 325 ft-amsl. A single extraction well would be installed at the "gate" with a screened interval of 50 to 100 ft-bgs and a pumping capacity of up to 20 gpm. Groundwater will be pumped and conveyed to an existing surface water impoundment at the Sebree Station, which will allow for compliance with discharge permits through an established NPDES outfall.

CiP via ash stabilization and capping would control the source of COCs and thereby reduce contaminant loading to the extraction system. Concentrations downgradient of the physical barrier would be expected to decrease over time through several natural attenuation mechanisms including advection, dilution, and dispersion. Groundwater Monitoring (Assessment) would continue to track the effectiveness of the corrective measures and to identify conditions that allow the return to Detection monitoring and ultimately

Alternative #4 is recommended for further evaluation.

#### 3.2 Remedy Evaluation

Currently BREC considers three (3) potential corrective action alternatives as viable options to address groundwater impacts at the Unit, including:

- Alternative #2a;
- Alternative #3; and
- Alternative #4;

To evaluate each alternative, additional data collection will likely be required. BREC is currently evaluating data collection needs in the following areas to assist with remedy selection:

1) Nature and Extent – groundwater trends, influence of non-groundwater remedies, etc.

- 2) Physical Characteristics available data on the physical characteristics of the landfill and retention bond
- 3) Performance Modeling data needed to develop digital models demonstrating the effectiveness of potential alternatives
- 4) Engineering feasibility, cost estimates, etc.

BREC is working to establish a comprehensive list of data collection needs to proceed forward with remedy evaluation and anticipates providing additional data in future semi-annual remedy selection progress reports.

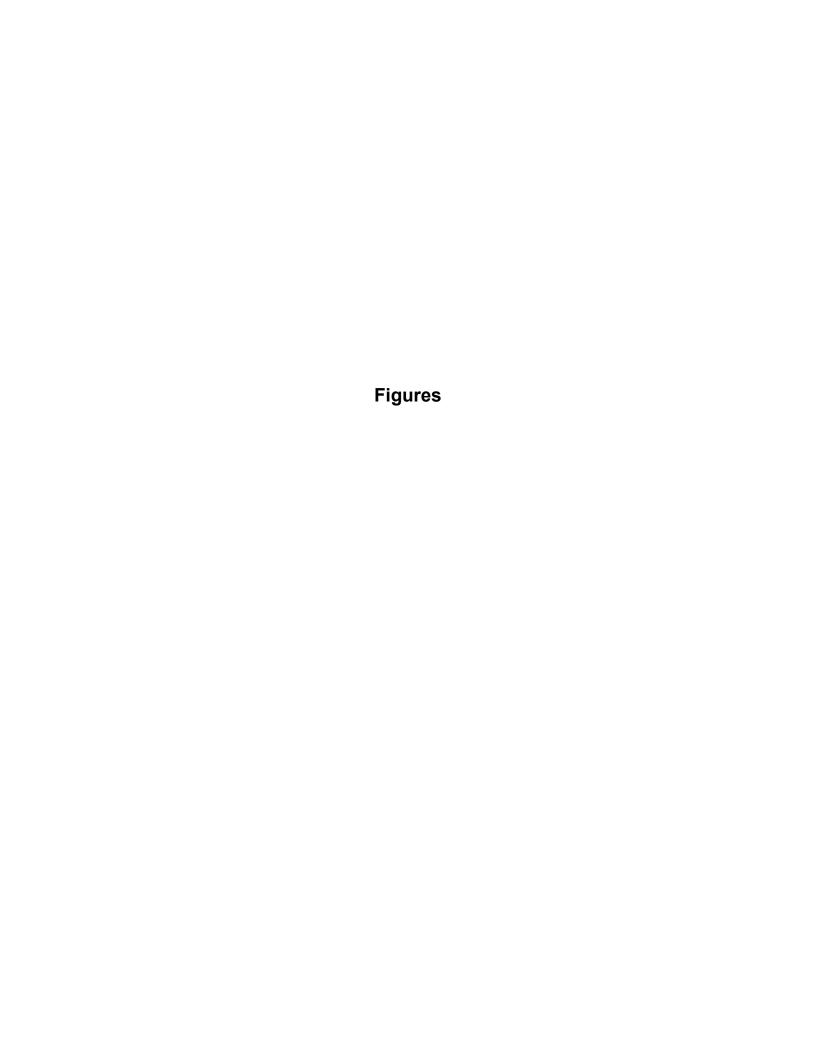
#### 4.0 CONCLUSION

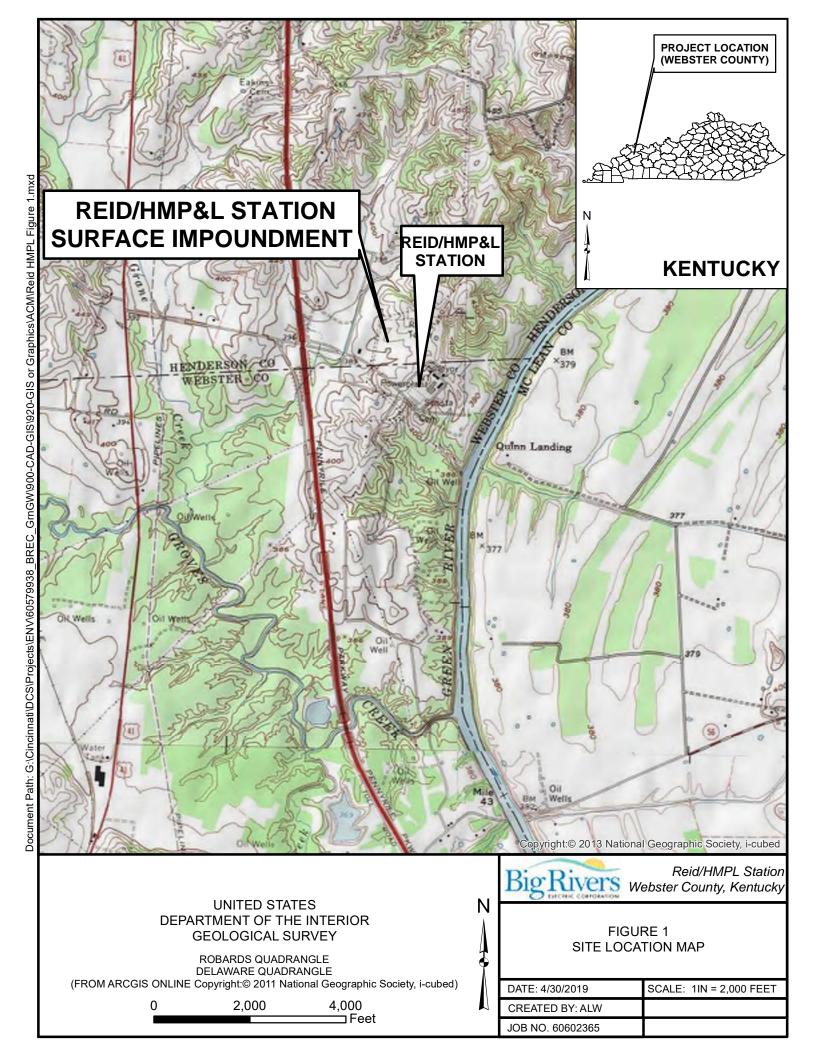
Additional updates regarding remedy selection, including any additional corrective measures being considered, will be presented twice a year in future remedy selection progress reports. Once sufficient data has been collected to select an effective comprehensive remedy for the Unit, a public meeting will be held 30 days prior to formal remedy selection, followed by a detailed Remedy Selection Report describing the remedy and proposed schedule for implementation.

If needed, the next remedy selection progress report for the Unit is expected in June 2020.

#### 5.0 REFERENCES

- AECOM, 2018. Annual Groundwater Monitoring and Corrective Action Report, 2016-2017; Reid/HMP&L Station Surface Impoundment, Webster County, Kentucky.
- AECOM, 2019. Annual Groundwater Monitoring and Corrective Action Report, 2018; Reid/HMP&L Station Surface Impoundment, Webster County, Kentucky.
- Associated Engineers 2016. Hydrologic and Hydraulic Capacity Assessment and Initial Inflow Design Flood Control System Plan.
- EPA, 40 CFR Part 257. [EPA-HQ-RCRA-2015-0331; FRL-9928-44-OSWER]. RIN-2050-AE81. Technical Amendments to the Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals From Electric Utilities—Correction of the Effective Date. Federal Register / Vol. 80, No. 127 / Thursday, July 2, 2015 / Rules and Regulations.
- Fairer, G.M., Geologic Map of the Robards Quadrangle, Henderson and Webster Counties, Kentucky, U.S. Geological Survey, 1973.











#### **Green Station CCR Surface Impoundment**

Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Structural Stability Assessment

October 11, 2016

#### Prepared By:



Project ID: 160028A

# Big Rivers Electric Corporation Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Structural Stability Assessment

#### **CCR Surface Impoundment Information**

Name: Green Station CCR Surface Impoundment

Operator: Sebree Generating Station

Address: 9000 Highway 2096

Robards, Kentucky 42452

CCR Unit Identification Number: Kentucky State Dam Inventory System ID No. 0980

#### **Qualified Professional Engineer**

Name: David A. Lamb

Company: Associated Engineers, Inc.

Kentucky P.E. Number: 17822

#### **Regulatory Applicability**

As part of the § 257.73 Structural integrity criteria for existing CCR surface impoundments requirements, an owner or operator of an existing CCR surface impoundment must no later than October 17, 2016:

Conduct initial structural stability assessments and document whether the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices for the maximum volume of CCR and CCR wastewater which can be impounded therein. The assessment must, at a minimum, document whether the CCR unit has been designed, constructed, operated, and maintained with:

- 1. Stable foundations and abutments:
- 2. Adequate slope protection to protect against surface erosion, wave action, and adverse effects of sudden drawdown:
- 3. Dikes mechanically compacted to a density sufficient to withstand the range of loading conditions in the CCR unit;
- 4. Vegetated slopes of dikes and surrounding areas not to exceed a height of six inches above the slope of the dike, except for slopes which have an alternate form or forms

of slope protection;

- 5. A single spillway or a combination of spillways configured as specified in the final rule. The combined capacity of all spillways must be designed, constructed, operated, and maintained to adequately manage flow during and following the peak discharge from the event specified in the final rule and all spillways must be either of non-erodible construction and designed to carry sustained flows; or earth- or grass-lined and designed to carry short-term, infrequent flows at non-erosive velocities where sustained flows are not expected. The combined capacity of all spillways must adequately manage flow during and following the peak discharge from a probable maximum flood (PMF) for a high hazard potential CCR surface impoundment; or 1000-year flood for a significant hazard potential CCR surface impoundment; or 100-year flood for a low hazard potential CCR surface impoundment;
- 6. Hydraulic structures underlying the base of the CCR unit or passing through the dike of the CCR unit that maintain structural integrity and are free of significant deterioration, deformation, distortion, bedding deficiencies, sedimentation, and debris which may negatively affect the operation of the hydraulic structure; and
- 7. For CCR units with downstream slopes which can be inundated by the pool of an adjacent water body, such as a river, stream or lake, downstream slopes that maintain structural stability during low pool of the adjacent water body or sudden drawdown of the adjacent water body.

#### From: VI. Development of the Final Rule - Technical Requirements

In order to ensure the proper upkeep and operation of the CCR unit, the owner or operator must demonstrate that the CCR surface impoundment has been designed, constructed, operated and maintained to provide structural stability. Specifically, the final rule requires the owner or operator to demonstrate that the design, construction, operation, and maintenance of the CCR surface impoundment is consistent with recognized and generally accepted good engineering practices for the maximum volume of CCR and water that can be impounded therein. Specifically, the final rule focuses on the critical structural aspects of the CCR surface impoundment that EPA identified in the proposed rule, and identifies the minimum elements that a professional engineer must provide engineering details on or otherwise address. Consistent with the proposal, these demonstrations must be certified by a qualified professional engineer.

In addition to implementing adequate slope protection against erosion, which is a structural stability requirement applicable to all CCR units, the owner or operator of a CCR surface impoundment exceeding the specified size threshold (height of five feet or more and a storage volume of 20 acre-feet or more; or a height of 20 feet or more) must demonstrate that the unit, including any vertical and lateral expansions, is constructed with "stable foundations and abutments." A stable foundation is an essential element of surface impoundment construction and prevents differential settlement of the embankment which can result in adverse internal stresses with the embankment cross-section.

Consistent with general engineering construction methodologies, the structural stability assessment also requires the owner or operator to determine whether the CCR surface impoundment has been mechanically compacted to a density sufficient to withstand the range of loading conditions in the CCR unit. Compaction of a dike or embankment is considered essential, as the compaction of soils leads to an increase in density and subsequently strength. Soil mechanics theory has established that the density of a soil corresponds to the moisture content and strength of the soil. The rule requires the owner or operator make this determination for all dikes of a CCR surface impoundment.

The owner or operator must also design, construct, operate, and maintain the CCR surface impoundment spillway or spillways with appropriate material so as to prevent the degradation of the spillway, as well as to ensure that the CCR surface impoundment has adequate spillway capacity to manage the outflow from a specific inflow design flood. In addition, a demonstration must be made that the CCR surface impoundment has been designed, constructed, operated, and maintained with inflow design flood controls and/or spillway capacity to manage peak discharge during and following inflow design floods. This demonstration is required to ensure the CCR surface impoundments will have adequate hydrologic and hydraulic capacity to prevent such failures as overtopping and excessive internal seepage and erosion. Spillways must be designed to withstand discharge from the inflow design flood without losing their structural form and leading to discharge issues, such as erosion or overtopping of the embankment. This requirement is covered in more detail in the hydrologic and hydraulic capacity requirements for CCR surface impoundments section of this rule.

EPA is not requiring a facility to include any demonstration relating to the potential for rapid, or sudden, drawdown loading condition. Rapid or sudden drawdown is a condition in earthen embankments in which the embankment becomes saturated through seepage in an extended high pool elevation in the reservoir. A threat to the embankment emerges when the reservoir pool is drawn down or lowered at a rate significantly higher than the excess poor water pressure within the embankment can diminish. Typically, rapid drawdown scenarios are considered for embankments with reservoirs used for water supply and management, emergency reservoirs, or agricultural supply, in which the reservoir is rapidly discharged from the structure.

A second consideration regarding rapid drawdown, however, is the rapid drawdown of a water body adjacent to the slope of the CCR surface impoundment which may periodically inundate the slope. Many CCR surface impoundments are located in areas in which the downstream slope of the CCR surface impoundment runs down to a lake, stream, or river. In such instances, rapid drawdown must be considered for the stability of the downstream slope of the embankment in the event of a rapid drawdown in the lake, stream, or river pool elevation or stage. Because the water ponded against the downstream slope of the CCR surface impoundment provides a stabilizing load on the slope of the CCR surface impoundment, the rapid or gradual loss of this stabilizing force must be considered in the analysis of the CCR surface impoundment. The rule, therefore, requires that existing and new CCR surface impoundments and any lateral expansions of such units with a downstream slope that can be inundated by an adjacent water body, such as rivers, streams, or lakes, be

constructed with downstream slopes that will maintain structural integrity in events of low pool or rapid drawdown of the adjacent water body. This ensures that the structural integrity of the downstream slope of the CCR surface impoundment will be maintained, even though the conditions of an adjacent surface water body may be outside the owner or operator's control.

#### **Description of Impoundment**

An aerial photo of the CCR unit is provided as Attachment A and an excerpt from U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps showing the location of the CCR unit is provided as Attachment B.

The CCR unit has been in place for 40 plus years and is used for the placement of coal combustion residual material; currently slurried bottom ash. The immediate watershed that drains to the CCR unit, and in which the CCR unit is considered to be located, is unnamed and 54.13 acres in size. The unnamed watershed discharges from the CCR impoundment outflow structure and is routed to the Green River.

The CCR unit is a combined incised/earthen embankment structure. Embankments form the west, south and east sides of the impoundment and the north side is incised. The Green River is located approximately 400 feet east of the structure. Due to surface relief, only the toe area of the south dike is potentially subject to flooding. The predominant features were small stream valleys draining eastward to the Green River. Most of the central portion of the south dike was constructed on a subdued ridge. The toe of the outboard slope intersected a lower drainage area. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands.

The west dike is generally less than five feet in height and the south dike reaches a maximum height of 19.5 feet. The east dike reaches a maximum height of approximately eight feet and is buttressed with a secondary parallel embankment that serves as a 40-foot wide roadway. The Burns and Roe, Inc. Engineering and Consultants June 30, 1978 site grading plans show the original construction layout and ground contours for the impoundment site. Bottom ash has been placed above the normal pool along the inboard side, essentially creating reclaimed land

Depth of impounded water and CCR is 16 feet and 46 feet (at respective locations of maximum impounded water and CCR depths). Elevation of impounded water and CCR is 394 feet and 408 feet, respectively, above mean sea level. These approximate depths and respective elevations are based on the most recent (December 2015) flight derived topographic contours and bathymetric survey data.

The remaining storage capacity is approximately 172,000 cubic yards (if CCR can be placed to the elevation of the current water surface). This volume was calculated based on the

maximum allowable storage volume and the current volume of CCR stored in the facility based on the most recent bathymetric survey.

The approximate volume of impounded water and CCR is 981,000 cubic yards (approximate water volume is 172,000 cubic yards and approximate CCR volume is 809,000 cubic yards). This volume was calculated based on the maximum storage capacity, the current amount of CCR stored in the facility based on the most recent bathymetric survey, and the best available as-built data for the structure construction prior to placement of CCR.

The impoundment discharge consists of two corrugated steel pipes, each 24 inches in diameter. The pipe intakes are through a concrete common headwall collection structure with a variable height steel debris deflector on each pipe intake.

#### **Results of the Initial Structural Stability Assessment**

The initial structural stability assessment has been completed and documents whether the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices for the maximum volume of CCR and CCR wastewater which can be impounded therein. Slope stability analyses were performed using Rocscience Inc. Slide geotechnical software. Slide provides numerical tools to analyze the stability of embankments using limit equilibrium methods.

The assessment documents whether the CCR unit has been designed, constructed, operated, and maintained with:

1. Stable foundations and abutments;

The 2015 Annual Inspection indicates that the Green CCR impoundment exhibits stable foundations and abutments. No related deficiencies were observed during the annual inspection.

2. Adequate slope protection to protect against surface erosion, wave action, and adverse effects of sudden drawdown;

The 2015 Annual Inspection indicates the Green CCR impoundment exhibits mostly adequate slope protection from erosion, wave action and any effects if sudden drawdown could occur. No related deficiencies were observed during the annual inspection.

3. Dikes mechanically compacted to a density sufficient to withstand the range of loading conditions in the CCR unit;

Quality assurance and/or testing reports describing the compaction methods and results during embankment construction are not available. A geotechnical exploration was performed to meet the requirements of 40 CFR §257.73(e); the exploration included Standard Penetration Testing (SPT) and acquisition of undisturbed soil samples. Based

on the field results and laboratory analyses, the materials within the embankment are sufficient to withstand the anticipated loading conditions.

4. Vegetated slopes of dikes and surrounding areas not to exceed a height of six inches above the slope of the dike, except for slopes which have an alternate form or forms of slope protection;

The 2015 Annual Inspection indicates the Green CCR impoundment embankment exhibits vegetation mostly greater a height of six inches above the slope of the dike. The Utility Solid Waste Activities Group, et al. has petitioned the USEPA to remand this requirement from the final rule because it is not practical and in remanding the provision, there is no reasonable probability of adverse effects on human health or the environment. USEPA has agreed that the requirement should be remanded.

5. A single spillway or a combination of spillways configured as specified in the final rule. The combined capacity of all spillways must be designed, constructed, operated, and maintained to adequately manage flow during and following the peak discharge from the event specified in the final rule and all spillways must be either of non-erodible construction and designed to carry sustained flows; or earth- or grass-lined and designed to carry short-term, infrequent flows at non-erosive velocities where sustained flows are not expected. The combined capacity of all spillways must adequately manage flow during and following the peak discharge from a probable maximum flood (PMF) for a high hazard potential CCR surface impoundment; or 1000-year flood for a significant hazard potential CCR surface impoundment; or 100-year flood for a low hazard potential CCR surface impoundment;

The impoundment has a single spillway structure. The spillway is comprised of two 30-inch corrugated metal pipes with a dual-pipe concrete headwall. The impoundment was analyzed for a 1000-year/24-hour storm event using SCS methodologies and a Type II rainfall distribution. Precipitation depth during the design storm was acquired from the National Oceanic and Atmospheric Administration (NOAA) Atlas 14 Point Precipitation Frequency Estimates database. Based on the analysis, the spillway structure can manage the flow from the design storm without overtopping the embankment. The analysis was based on the current impoundment configuration, storm water flows, process water flows, and contents volume.

6. Hydraulic structures underlying the base of the CCR unit or passing through the dike of the CCR unit that maintain structural integrity and are free of significant deterioration, deformation, distortion, bedding deficiencies, sedimentation, and debris which may negatively affect the operation of the hydraulic structure; and

The 2015 Annual Inspection indicates the hydraulic structures underlying the base of the Green impoundment or passing through the dike of the CCR unit that maintain structural integrity and are free of significant deterioration, deformation, distortion, bedding deficiencies, sedimentation, and debris which may negatively affect the operation of the hydraulic structure. No related deficiencies were observed during the annual inspection.

7. For CCR units with downstream slopes which can be inundated by the pool of an adjacent water body, such as a river, stream or lake, downstream slopes that maintain structural stability during low pool of the adjacent water body or sudden drawdown of the adjacent water body.

Due to the configuration and location of the impoundment, stability is not anticipated to be significantly affected by low river pool conditions. Although a portion of the downstream slope of the impoundment embankment is located below the 100-year flood elevation of the Green River and may experience encroachment of the adjacent water body during a flood event, stability is not anticipated to be affected by sudden drawdown. A rapid drawdown analysis was completed to assess the downstream slope of the impoundment during such an event and the analysis resulted in an acceptable factor of safety demonstrating that the slope will maintain structural stability during a sudden drawdown.

#### **Sources of Information**

Geotechnical and other information provided by Associated Engineers, Inc.

Engineering design drawings and other information provided by Big Rivers Electric Corporation

United States Geological Survey U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps

# Professional Engineer Certification [Per 40 CFR § 257.73] Green CCR Impoundment Initial Structural Stability Assessment

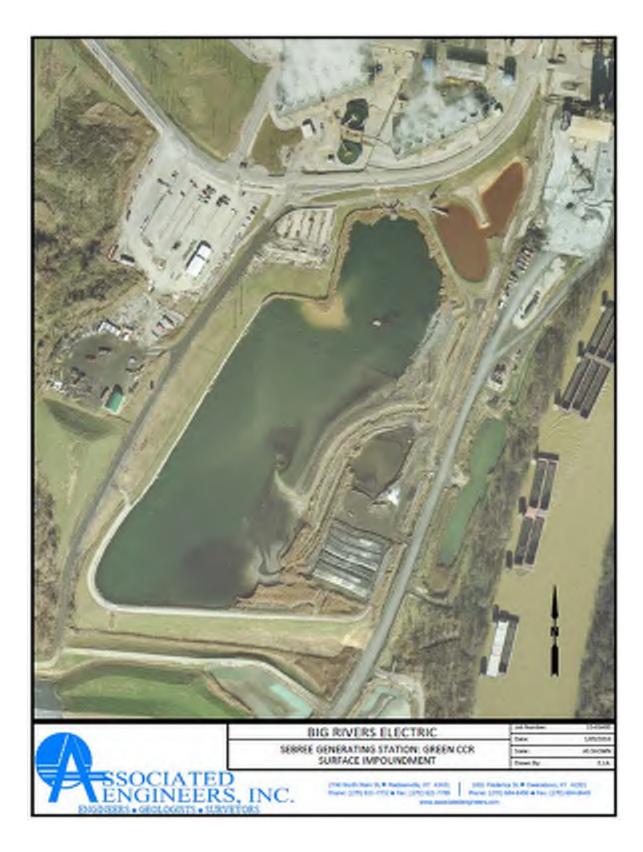
I hereby certify that myself or an agent under my review has prepared this Initial Structural Stability Assessment (Assessment), and being familiar with the provisions of the final rule to regulate the disposal of coal combustion residuals (CCR) as solid waste under subtitle D of the Resource Conservation and Recovery Act (RCRA), attest that this Assessment has been prepared in accordance with good engineering practices and meets the intent of 40 CFR Part 257.73. To the best of my knowledge and belief, the information contained in this Assessment is true, complete, and accurate.

David A

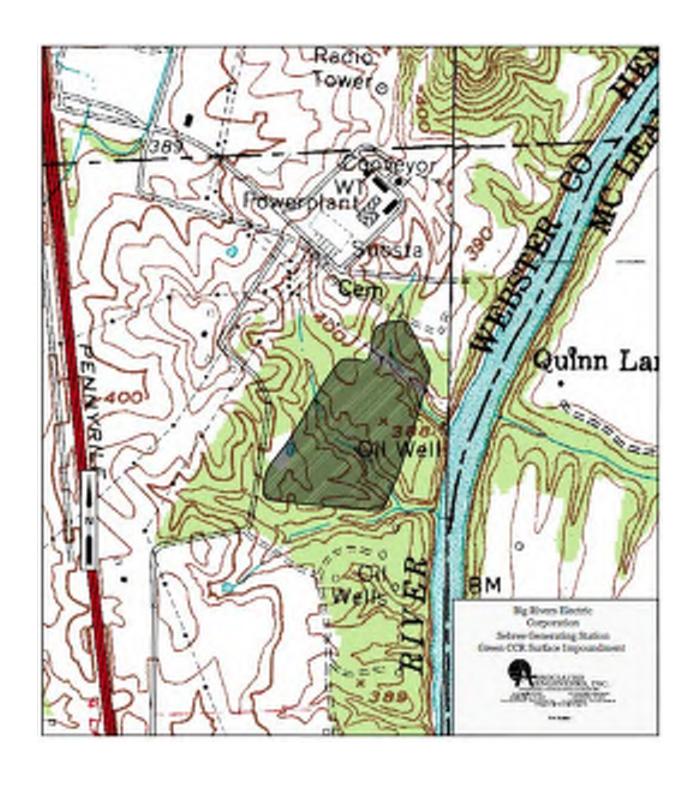
State of Rentuck 19

<u>5</u>. 17822

Date:



Attachment A. Aerial Photo of the Green CCR Surface Impoundment



Attachment B. Topographic Map showing the Green CCR Surface Impoundment



# **Green Station CCR Surface Impoundment**

Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

October 11, 2016

#### **Prepared By:**



Project ID: 160028A

# Big Rivers Electric Corporation Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

#### **CCR Surface Impoundment Information**

Name: Green Station CCR Surface Impoundment

Operator: Sebree Generating Station

Address: 9000 Highway 2096

Robards, Kentucky 42452

CCR Unit Identification Number: Kentucky State Dam Inventory System ID No. 0980

#### **Qualified Professional Engineer**

Name: David A. Lamb

Company: Associated Engineers, Inc.

Kentucky P.E. Number: 17822

#### **Regulatory Applicability**

As part of the § 257.73 Structural integrity criteria for existing CCR surface impoundments requirements, an owner or operator of an existing CCR surface impoundment must no later than October 17, 2016:

Conduct an initial safety factor assessment for each CCR unit and document whether the calculated factors of safety for each CCR unit achieve the minimum safety factors specified below for the critical cross section of the embankment. The critical cross section is the cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments must be supported by appropriate engineering calculations, documenting whether the CCR unit achieves the following minimum factors of safety:

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.40.
- 3. The calculated seismic factor of safety must equal or exceed 1.00.

4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

### From: VI. Development of the Final Rule - Technical Requirements

#### General Safety Factor Assessment Considerations

Generally accepted engineering methodologies specify that the determination of the structural stability factors of safety specified above is to be calculated by the qualified professional engineer using conventional analysis procedures or, if necessary, special analysis procedures. Conventional analysis procedures include, but are not limited to, limit equilibrium methods of slope stability analysis, whereas, special analysis procedures include, but are not limited to, finite element methods, finite difference methods, three-dimensional methods, or probabilistic methods. Whichever methodology is used to determine the factors of safety of the CCR surface impoundment, the qualified professional engineer must document the methodology used, as well as the basis for using that methodology, and the analysis must be supported by appropriate engineering calculations.

# The Calculated Static Factor of Safety Under the Long-Term, Maximum Storage Pool Loading Condition

It is generally accepted practice to analyze the stability of the downstream slope of the dam embankment for steady-state seepage (or steady seepage) conditions with the reservoir at its normal operating pool elevation (usually the spillway crest elevation) since this is the loading condition the embankment will experience most. This condition is called steady seepage with maximum storage pool. The maximum storage pool loading is the maximum water level that can be maintained that will result in the full development of a steady-state seepage condition. Maximum storage pool loading conditions need to be calculated to ensure that the CCR surface impoundment can withstand a maximum expected pool elevation with full development of saturation in the embankment under long-term loading. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum storage pool loading condition meet or exceed 1.5.

### The Calculated Static Factor of Safety Under the Maximum Surcharge Pool Loading Condition

The maximum surcharge pool loading condition is calculated to evaluate the effect of a raised level (e.g., flood surcharge) on the stability of the downstream slope. This ensures that the CCR surface impoundment can withstand a temporary rise in pool elevation above the maximum storage pool elevation for which the CCR surface impoundment may normally be subject under inflow design flood stage, for a short-term until the inflow design flood is passed through the CCR surface impoundment. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum surcharge pool loading condition meet or exceed 1.4.

#### The Calculated Seismic Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold must meet a seismic factor of safety equal to or greater than 1.0. All CCR surface

impoundments must also be capable of withstanding a design earthquake without damage to the foundation or embankment that would cause a discharge of its contents. To further support the location criteria established in this rule, CCR surface impoundments and any lateral expansion exceeding a specific height and/or volume threshold must be assessed under seismic loading conditions for a seismic loading event with a 2% probability of exceedance in 50 years, equivalent to a return period of approximately 2,500 years, based on the USGS seismic hazard maps for seismic events with this return period for the region where the CCR unit is located. EPA chose the 2% exceedance probability in 50 years event based on its common use in seismic design criteria throughout engineering.

### The Calculated Liquefaction Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold and have been determined to contain soils susceptible to liquefaction must meet a liquefaction factor of safety equal to or greater than 1.20. A prudent engineering analysis of structural stability also includes a liquefaction potential analysis and analysis of post-liquefaction static factors of safety. As discussed previously, liquefaction is a phenomenon which typically occurs in loose, saturated or partially-saturated soils in which the effective stress of the soils reduces to zero, corresponding to a total loss of shear strength of the soil. The most common occurrence of liquefaction is in loose soils, typically sands. The liquefaction FOS determination in the final rule is used to determine if a CCR unit would remain stable if the soils of the embankment of the CCR unit were to experience liquefaction. Liquefaction analysis is only necessary in instances where CCR surface impoundments show, through representative soil sampling, construction documentation, or anecdotal evidence from personnel with knowledge of the CCR unit's construction, that soils of the embankment are susceptible to liquefaction.

# <u>Failure To Demonstrate Minimum Safety Factors or Failure To Complete a Timely Safety Factor Assessment</u>

As previously discussed, the rule requires an owner or operator to document that the calculated factors of safety for each CCR surface impoundment achieve the minimum safety factors specified in the rule. For any CCR surface impoundment that does not meet these requirements, the owner or operator must either take any engineering measure necessary to ensure that the unit meets the requirements by the rule's deadlines, or cease placement of CCR and non-CCR waste into the unit and initiate closure of such CCR unit as provided in section 257.102 within six months. Similarly, if an owner or operator fails to complete the initial safety factor assessment or any subsequent periodic factor safety assessment by the deadlines established in the rule, the owner or operator must cease placing CCR and non-CCR waste into the unit and initiate closure within six months.

#### **Description of Impoundment**

An aerial photo of the CCR unit is provided as Attachment A and an excerpt from U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps showing the location of the CCR unit is provided as Attachment B.

The CCR unit has been in place for 40 plus years The CCR unit operator has general maintenance and repair procedures in place as they determine necessary. There are no known occurrences of structural instability of the CCR unit.

The CCR unit has been in place for 40 plus years. The CCR unit is used for the placement of coal combustion residual material; currently slurried bottom ash. The immediate watershed that drains to the CCR unit, and in which the CCR unit is considered to be located, is unnamed and 54.13 acres in size. The unnamed watershed discharges from the CCR impoundment outflow structure and is routed to the Green River.

The CCR unit is a combined incised/earthen embankment structure. Embankments form the west, south and east sides of the impoundment and the north side is incised. The Green River is located approximately 400 feet east of the structure. Due to surface relief, only the toe area of the south dike is potentially subject to flooding. The predominant features were small stream valleys draining eastward to the Green River. Most of the central portion of the south dike was constructed on a subdued ridge. The toe of the outboard slope intersected a lower drainage area. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands.

The west dike is generally less than five feet in height and the south dike reaches a maximum height of 19.5 feet. The east dike reaches a maximum height of approximately eight feet and is buttressed with a secondary parallel embankment that serves as a 40-foot wide roadway. The Burns and Roe, Inc. Engineering and Consultants June 30, 1978 site grading plans show the original construction layout and ground contours for the impoundment site. Bottom ash has been placed above the normal pool along the inboard side, essentially creating reclaimed land

Depth of impounded water and CCR is 16 feet and 46 feet (at respective locations of maximum impounded water and CCR depths). Elevation of impounded water and CCR is 394 feet and 408 feet, respectively, above mean sea level. These approximate depths and respective elevations are based on the most recent (December 2015) flight derived topographic contours and bathymetric survey data.

The remaining storage capacity is approximately 172,000 cubic yards (if CCR can be placed to the elevation of the current water surface). This volume was calculated based on the maximum allowable storage volume and the current volume of CCR stored in the facility based on the most recent bathymetric survey.

The approximate volume of impounded water and CCR is 981,000 cubic yards (approximate water volume is 172,000 cubic yards and approximate CCR volume is 809,000 cubic yards). This volume was calculated based on the maximum storage capacity, the current amount of CCR stored in the facility based on the most recent bathymetric survey, and the best available as-built data for the structure construction prior to placement of CCR.

The impoundment discharge consists of two corrugated steel pipes, each 30 inches in diameter. The pipe intakes are through a concrete common headwall collection structure with a variable height steel debris deflector on each pipe intake.

#### **Calculated Safety Factors**

Results of the initial safety factor assessment for the critical cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments are supported by appropriate engineering calculations.

The safety factor analysis was conducted using the Rocscience Inc. Slide geotechnical software by evaluating four cross sections along the embankment. Slide provides numerical tools to analyze the stability of embankments using limit equilibrium methods. At each cross section, drilling, surveying, laboratory testing, and a slope stability analysis were performed. Based on the four analysis scenarios, the lowest factor of safety for all scenarios was not found at a single cross section; therefore, the results listed below are the lowest factor of safety realized from all analyzed cross sections for each scenario. The safety factor assessments are supported by appropriate engineering calculations and the Slide modeling results for the Green CCR impoundment are attached to this report.

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition equals: 1.800
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition equals: 1.800
- 3. The calculated seismic factor of safety equals: 1.002
- 4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety equals: 1.800

#### **Sources of Information**

Geotechnical and other information provided by Associated Engineers, Inc.

Engineering design drawings and other information provided by Big Rivers Electric Corporation

United States Geological Survey U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps

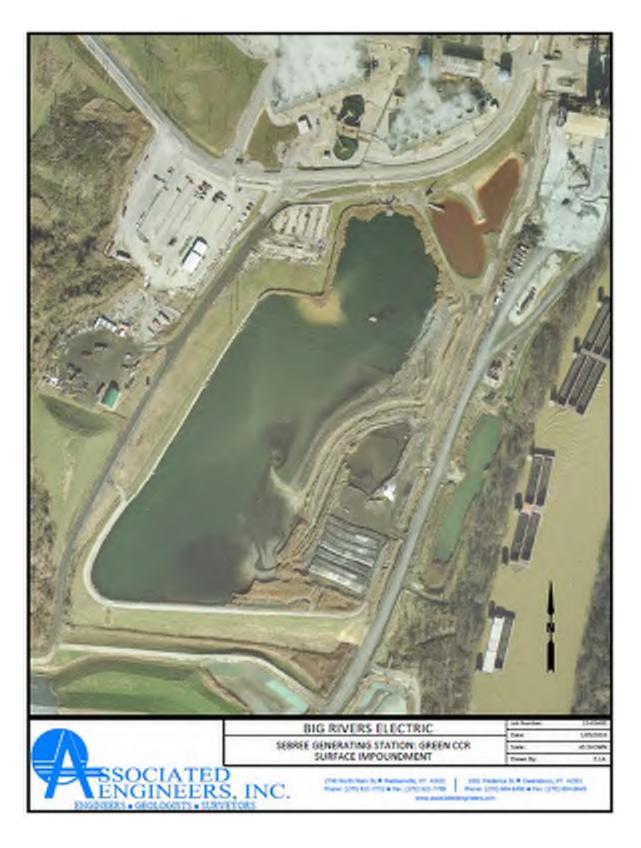
# Professional Engineer Certification [Per 40 CFR § 257.73] Green CCR Impoundment Initial Safety Factor Assessment

I hereby certify that myself or an agent under my review has prepared this Initial Safety Factor Assessment (Assessment), and being familiar with the provisions of the final rule to regulate the disposal of coal combustion residuals (CCR) as solid waste under subtitle D of the Resource Conservation and Recovery Act (RCRA), attest that this Assessment has been prepared in accordance with good engineering practices and meets the intent of 40 CFR Part 257.73. To the best of my knowledge and belief, the information contained in this Assessment is true, complete, and accurate.

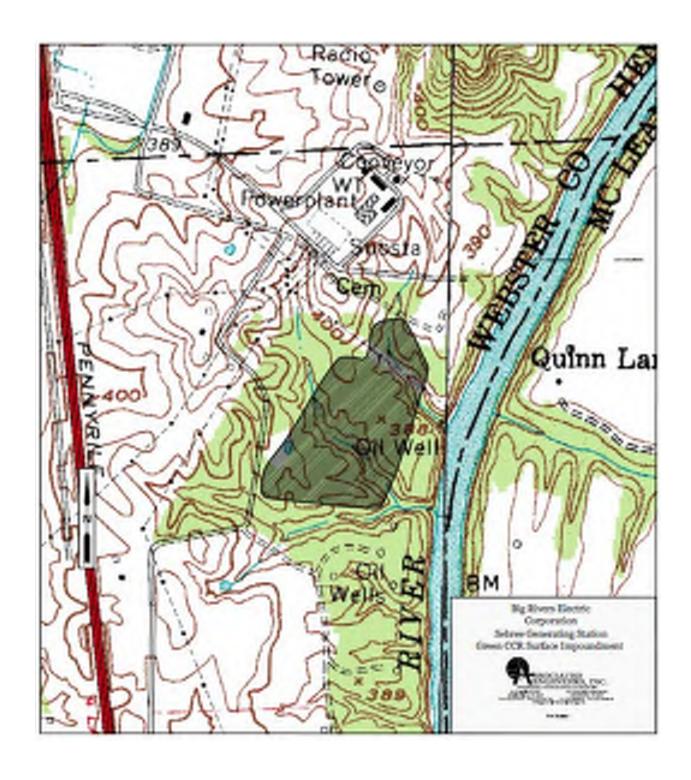
David A. Lamb

State of Kamercky Liconse 16. 17822

Date: 10/11/16



Attachment A. Aerial Photo of the Green CCR Surface Impoundment



Attachment B. Topographic Map showing the Green CCR Surface Impoundment

# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-1

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



	BREC Green Station CCR Surface Impoundment					
Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Lo					e Pool Loading Condition	
e	Drawn By Scale		Company	Associated Engineers, Inc		
	<sup>Date</sup> 9/5/2016, 3:43:18 PM		File Name	GR-1.slim		

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Sandy Lean Clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Υ	
379.185	
379.172	
377.695	
376.221	
375.6	
376.406	
377.906	
378.317	
378.983	

	Projec	t
-		
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sisien	Drawn	7 E
SLIDE 6 030	Date	

	BREC Green Station CCR Surface Impoundment					
	Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition					
Drawn By Scale Company Associated Engineers, Inc					Associated Engineers, Inc	
	Date	0/5/2016 3:43:18 D	М	File Name	GP_1 clim	

Date 9/5/2016, 3:43:18 PM GR-1.slim

85.7592	379.391
140.9	382.48
170.48	393.92

# **External Boundary**

Х	Υ
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
170.48	376.396
170.48	393.967
161.556	396.809
153.137	396.9
143.057	396.719
119.757	389.859
96.35	382.029
84.12	378.983
76.996	378.317
70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

Х	Y	
0	353.7	
55	353.7	
77	356.8	
153	364.9	

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition			Pool Loading Condition	
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date	9/5/2016, 3:43:18 Pf	М	File Name	GR-1.slim

170.48 366.698

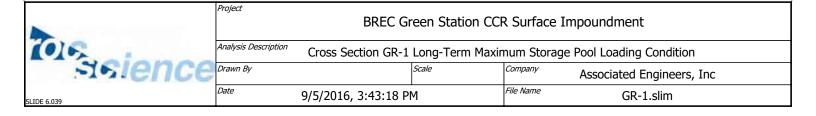
### **Material Boundary**

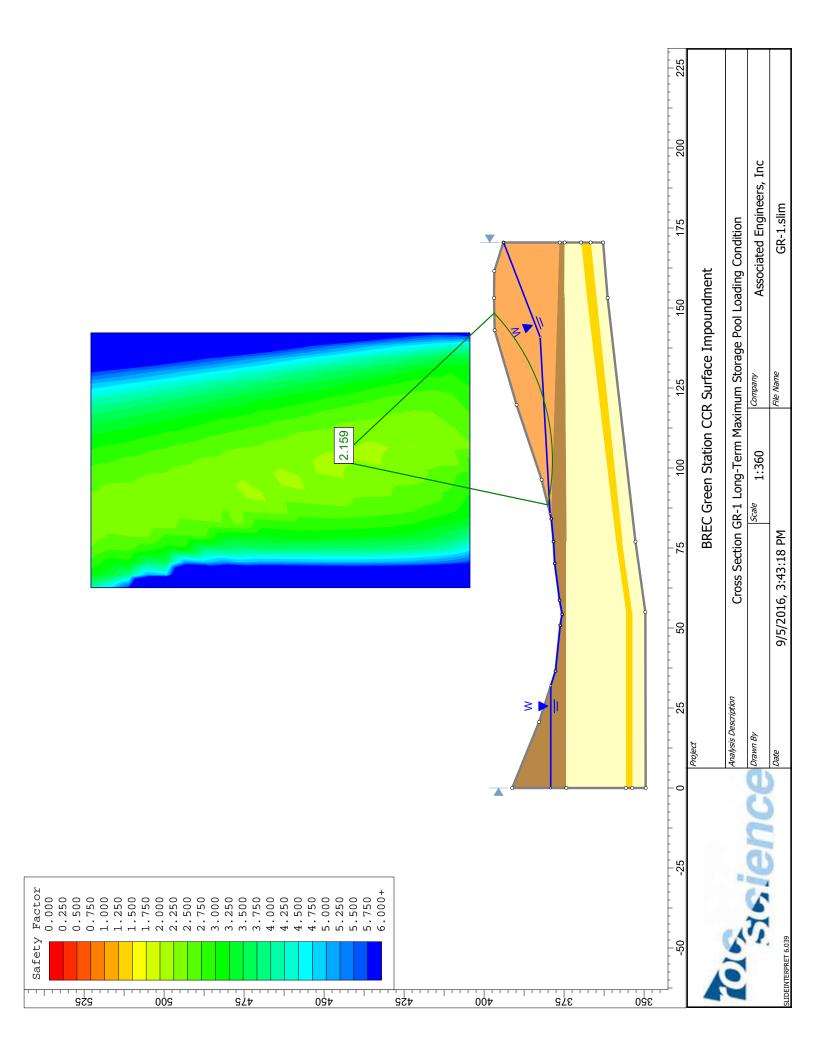
Х	Υ		
0	355.7		
55	355.7		
77	358.8		
153	367.9		
170.48	369.698		

#### **Material Boundary**

Х	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

X	Υ
84.12	378.983
153	376.9
170.48	376.396





# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-1 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-1 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



		BREC Green Station CCR Surface Impoundment					
	Analysis Description	Cross Section (	GR-1 Maximum Sı	Surcharge Pool Loading Condition			
9	Drawn By	wn By Scale			Associated Engineers, Inc		
	Date	9/5/2016, 3:43:18 PI	М	File Name	GR-1 Surcharge.slim		

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean clay (CL)	Sandy lean clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Y
0	379.185
31.955	379.172
36.551	377.695
50.832	376.221
54.305	375.6
58.684	376.406
70.171	377.906
76.996	378.317
84.12	378.983

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(0)6	Analysis Description Cross Section GR-1 Maximum Surcharge Pool Loading Condition				
sience	Drawn By	Scale	Company	Associated Engineers, Inc	
SLIDE 6.039	<sup>Date</sup> 9/5/2016, 3:43:18 PM		File Name	GR-1 Surcharge.slim	

85.7592 379.391 140.9 382.48 165.719 395.483 170.48 395.48

# **External Boundary**

х	Y
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
170.48	376.396
170.48	393.967
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70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

X	Υ
0	353.7
55	353.7
77	356.8



		BREC Green Station CCR Surface Impoundment			
Analysis Description Cross Section GR-1 Maximum Surcharge Pool Loading Condition					Pool Loading Condition
e	Drawn By		Scale	Company	Associated Engineers, Inc
	Date	9/5/2016, 3:43:18 P	 М	File Name	GR-1 Surcharge.slim

153 364.9 170.48 366.698

# **Material Boundary**

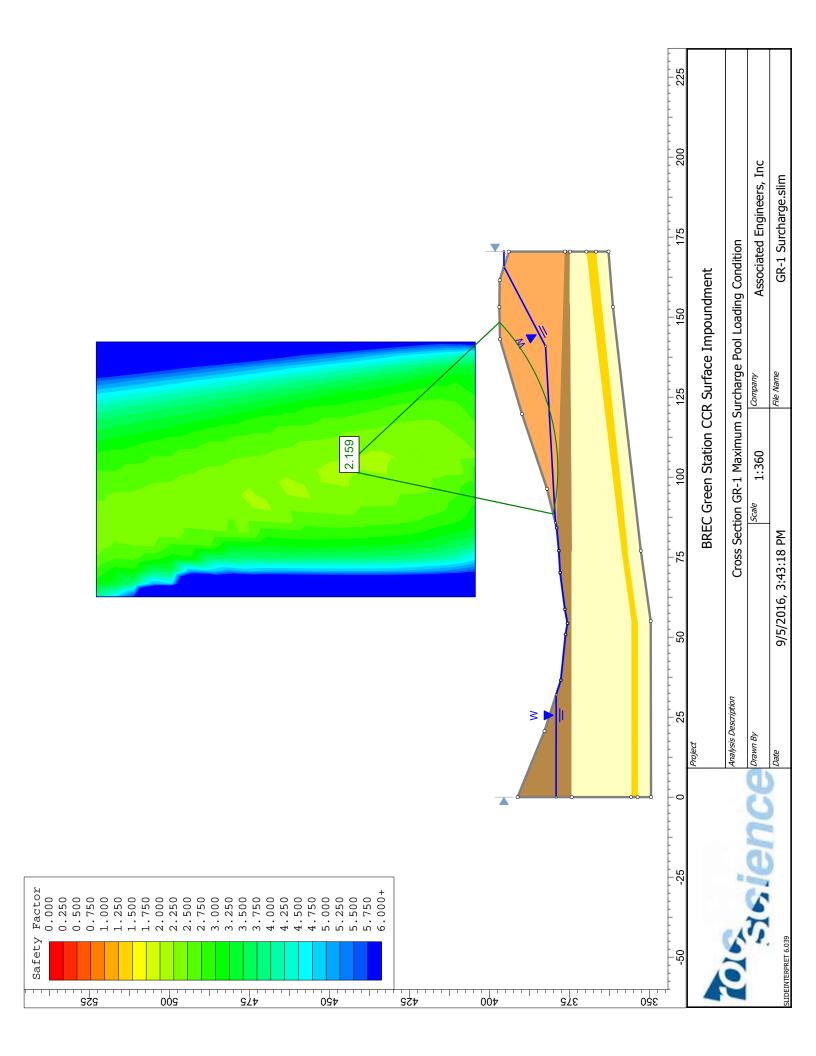
X	Υ
0	355.7
55	355.7
77	358.8
153	367.9
170.48	369.698

# **Material Boundary**

Х	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

Х	Υ
84.12	378.983
153	376.9
170.48	376.396

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-1 Maximum Surcharge Pool Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date	9/5/2016, 3:43:18 P	М	File Name	GR-1 Surcharge.slim



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-1 Seis

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment Analysis: Cross Section GR-1 Seismic Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

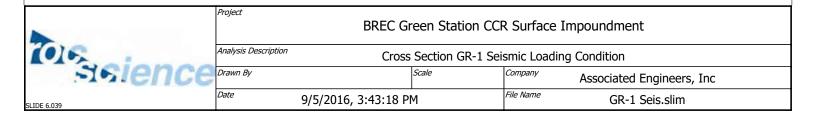
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

# **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# Loading

Seismic Load Coefficient (Horizontal): 0.2364

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean clay (CL)	Sandy Lean Clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
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50.832	376.221		
54.305	375.6		

	7.0,000	BREC Gr	een Station CO	CR Surface I	mpoundment
(0)6	Analysis Description Cross Section GR-1 Seismic Loading Condition				
sience	Drawn By		Associated Engineers, Inc		
SLIDE 6.039	Date (	9/5/2016, 3:43:18 PN	М	File Name	GR-1 Seis.slim

58.684 376.406 70.171 377.906 76.996 378.317 84.12 378.983 85.7592 379.391 140.9 382.48 170.48 393.92

#### **External Boundary**

Х	Y
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
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76.996	378.317
70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

# **Material Boundary**

X Y



	BREC Green Station CCR Surface Impoundment					
	Analysis Description Cross Section GR-1 Seismic Loading Condition					
e	Drawn By	Scale Company Associated Engineers, Inc				
	Date	9/5/2016, 3:43:18 Pf	М	File Name	GR-1 Seis.slim	

170.48	366.698
153	364.9
77	356.8
55	353.7
0	353.7

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Х	Υ
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55	355.7
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153	367.9
170.48	369.698

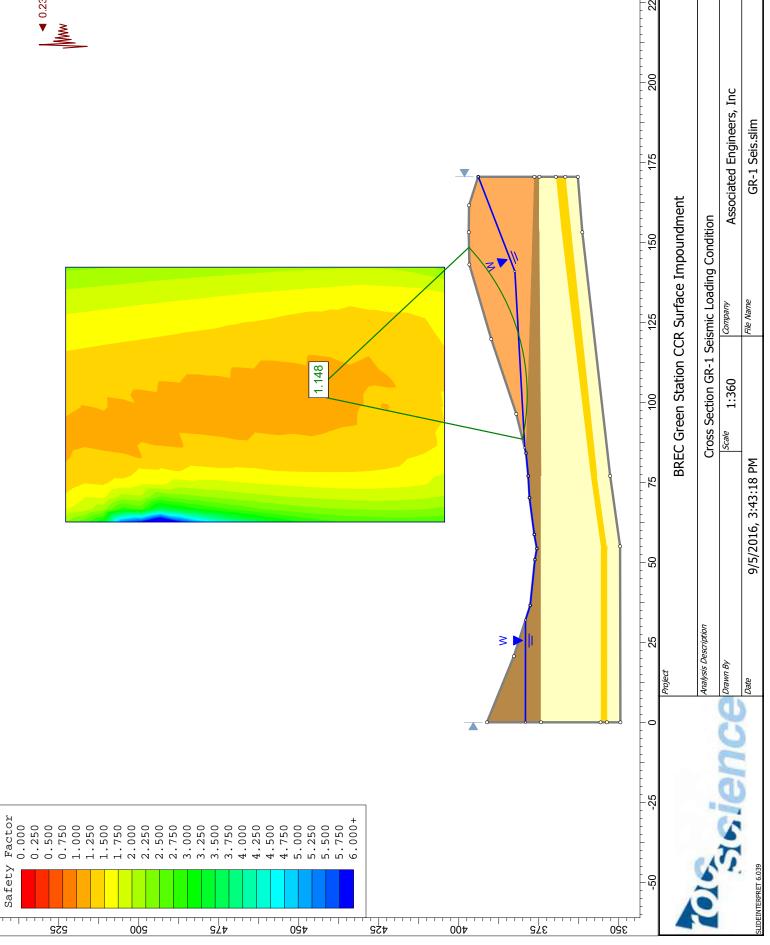
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X	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

Х	Υ
84.12	378.983
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	Project	BREC Gr	een Station CCI	R Surface In	npoundment		
(0)6	Analysis Description Cross Section GR-1 Seismic Loading Condition						
sience	Scale Company Associated Engineers, Inc						
SLIDE 6.039	Date	9/5/2016, 3:43:18 P	М	File Name	GR-1 Seis.slim		

Safety



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-2

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

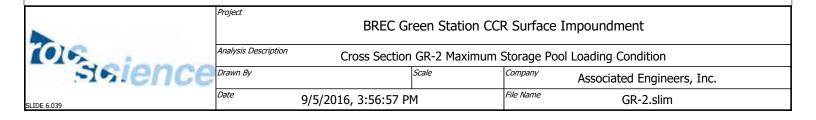
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

X	Υ
0	373.571
10.818	375.37
27.54	376.571
42.515	377.335
44.31	377.146
48.515	378.327
102.8	382.2

	Project BREC Green Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section GR-2 Maximum Storage Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	<i>Date</i> 9	/5/2016, 3:56:57 Pi	М	File Name	GR-2.slim	

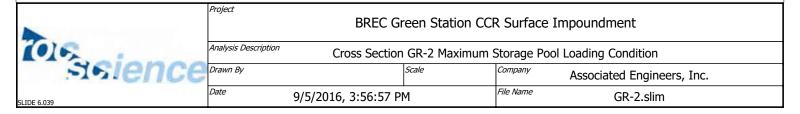
128.457 393.92 130 393.92

# **External Boundary**

V	V	
X	Y	
3.298e-013	348.3	
7.98633	347.116	
42.51	342	
112.29	334.7	
130	331.5	
130	338	
130	343	
130	351.773	
130	382.006	
130	382.1	
130	393.439	
119.451	396.731	
112.288	397.189	
101.806	396.285	
83.858	389.332	
67.857	383.758	
44.31	377.146	
42.515	377.335	
27.54	376.571	
10.818	375.37	
8.27071	374.946	
0	373.571	
0	371.3	
0	363	
0	359	
3.10558e-013	349.774	

# **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ	
3.10558e-013	349.774	
42.5	347.3	
79	345.5	
112.2	344.7	
130	343	

# **Material Boundary**

Х	Υ	
0	359	
42.5	353.3	
79	345.5	

# **Material Boundary**

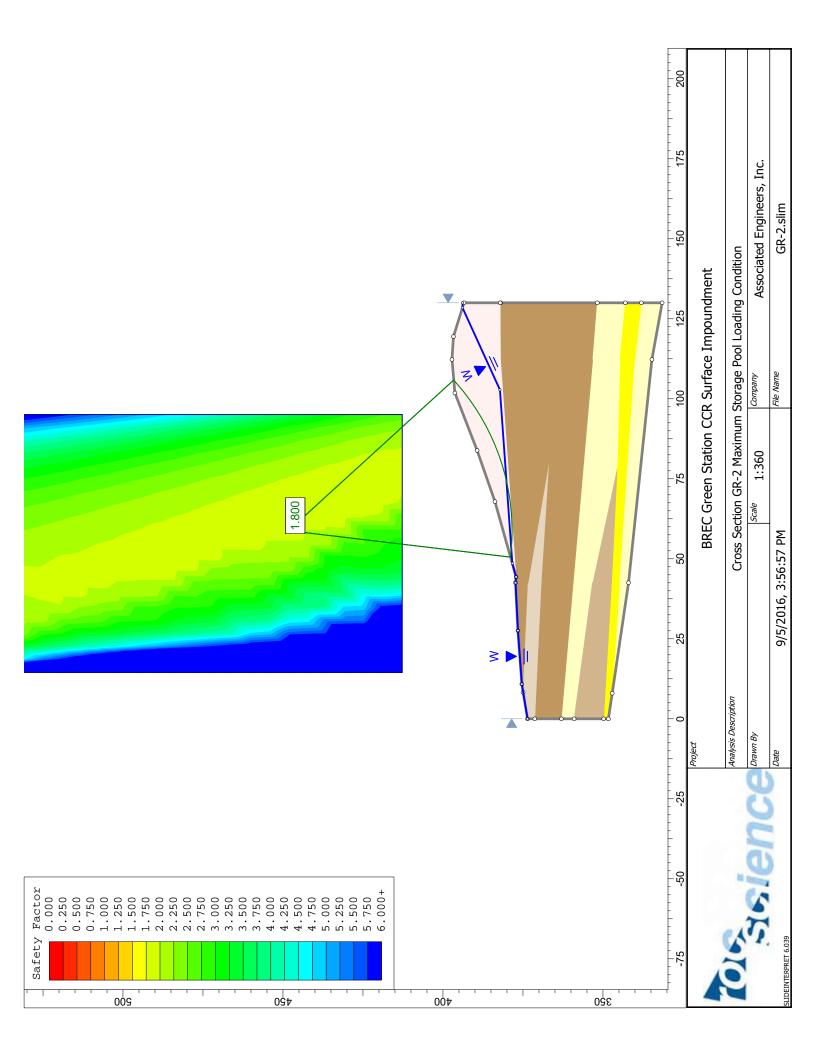
Х	Υ	
0	363	
42.5	359.3	
112.2	353.2	
130	351.773	

# **Material Boundary**

х	Y		
0	371.3		
42.5	369.3		
80	366.9		
42.5	373.4		
8.27071	374.946		

Х	Υ	
44.31	377.146	
112.2	382	
130	382.1	

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Maximum Storage Pool Loading Condition				Loading Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	<sup>Date</sup> 9/5/2016, 3:56:57 PM		File Name	GR-2.slim	



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

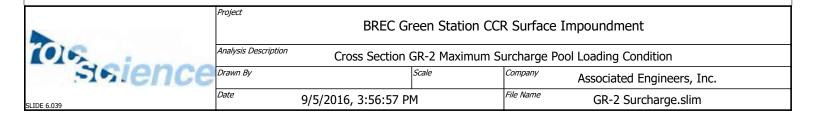
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

X	Y
0	373.571
10.818	375.37
27.54	376.571
42.515	377.335
44.31	377.146
48.515	378.327
102.8	382.2

	BREC Green Station CCR Surface Impoundment				
7016	Analysis Description	Cross Section	GR-2 Maximum S	urcharge Poo	l Loading Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date 9/	5/2016, 3:56:57 Pl	M	File Name	GR-2 Surcharge.slim

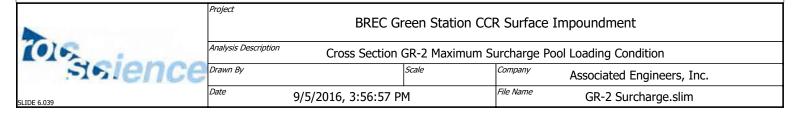
123.45 395.483 130 395.48

#### **External Boundary**

Х	Υ
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

#### **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

Х	Υ		
0	359		
42.5	353.3		
79	345.5		

## **Material Boundary**

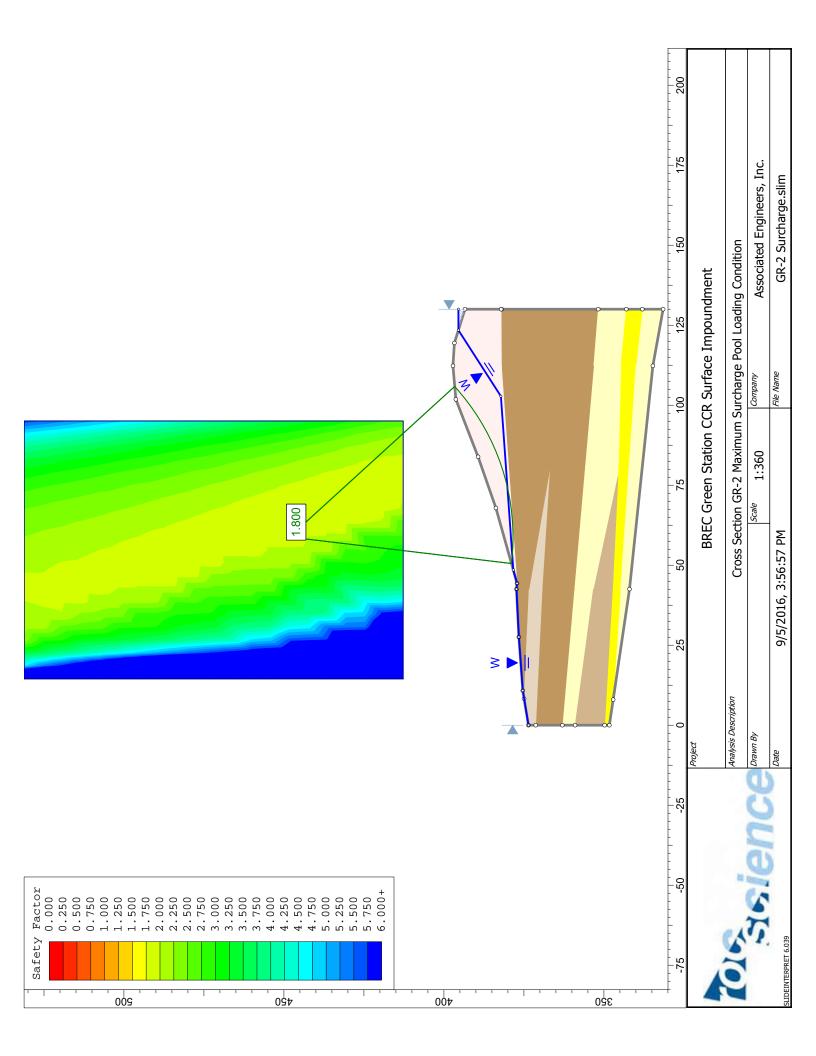
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

## **Material Boundary**

х	Υ	
0	371.3	
42.5	369.3	
80	366.9	
42.5	373.4	
8.27071	374.946	

Х	Y			
44.31	377.146			
112.2	382			
130	382.1			

	Project	BREC Gr	een Station CCI	R Surface I	mpoundment
(0)6	Analysis Description Cross Section GR-2 Maximum Surcharge Pool Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	М	File Name	GR-2 Surcharge.slim



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Seis 2

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment Analysis: Cross Section GR-2 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

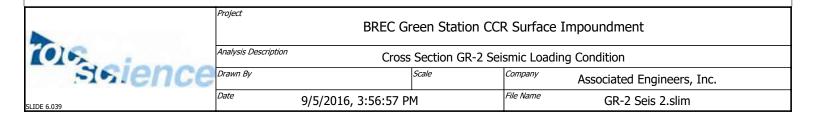
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

## **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.2364

## **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

х	Υ
0	373.571
10.818	375.37

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Seismic Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	<sup>Date</sup> 9/5/2016, 3	3:56:57 PM	File Name	GR-2 Seis 2.slim	

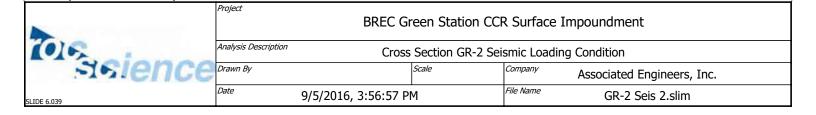
27.54 376.571 42.515 377.335 44.31 377.146 48.515 378.327 102.8 382.2 128.457 393.92 130 393.92

## **External Boundary**

Х	Υ
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

## **Material Boundary**

X Y 7.98633 347.116



42.5	344.8
112.2	340
130	338

Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

## **Material Boundary**

Х	Υ
0	359
42.5	353.3
79	345.5

## **Material Boundary**

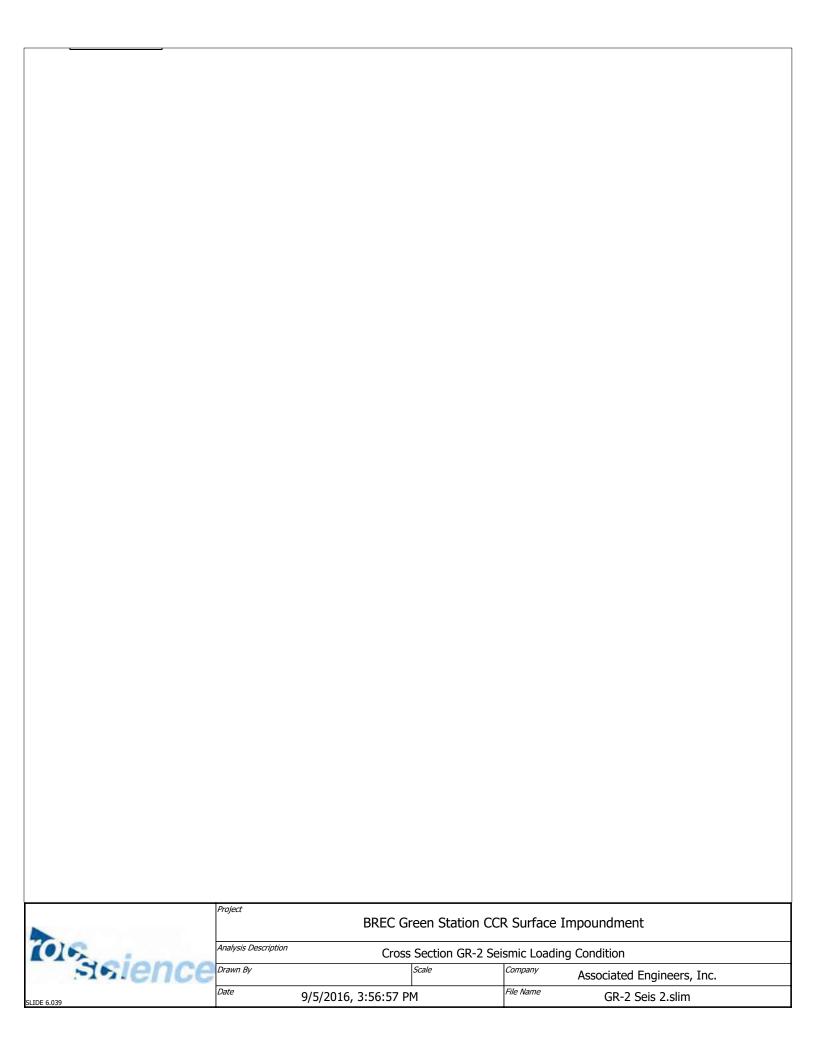
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

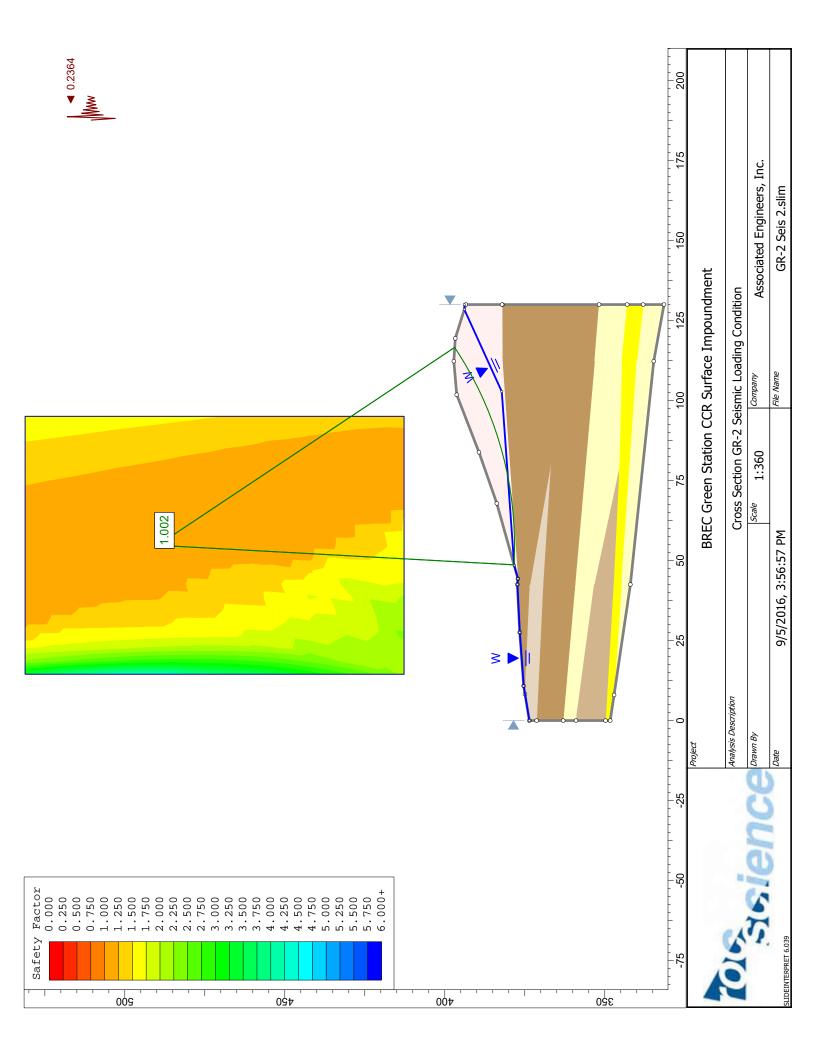
## **Material Boundary**

х	Υ
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Seismic Loading Condition			g Condition	
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	4	File Name	GR-2 Seis 2.slim





# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Liq

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Liquefaction Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

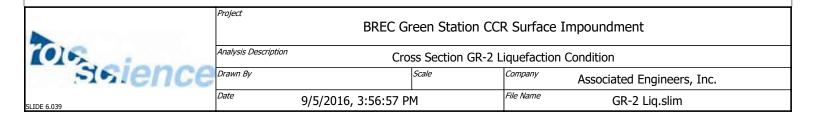
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	0	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

х	Υ
0	373.571
10.818	375.37
27.54	376.571
42.515	377.335
44.31	377.146
48.515	378.327
102.8	382.2

	BREC Green Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section GR-2 Liquefaction Condition					
sience	Drawn By		Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	Date	9/5/2016, 3:56:57 PI	М	File Name	GR-2 Liq.slim	

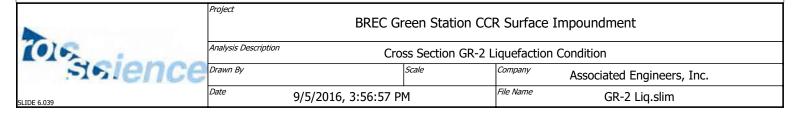
128.457 393.92 130 393.92

## **External Boundary**

х	Y
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

## **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

Х	Υ
0	359
42.5	353.3
79	345.5

## **Material Boundary**

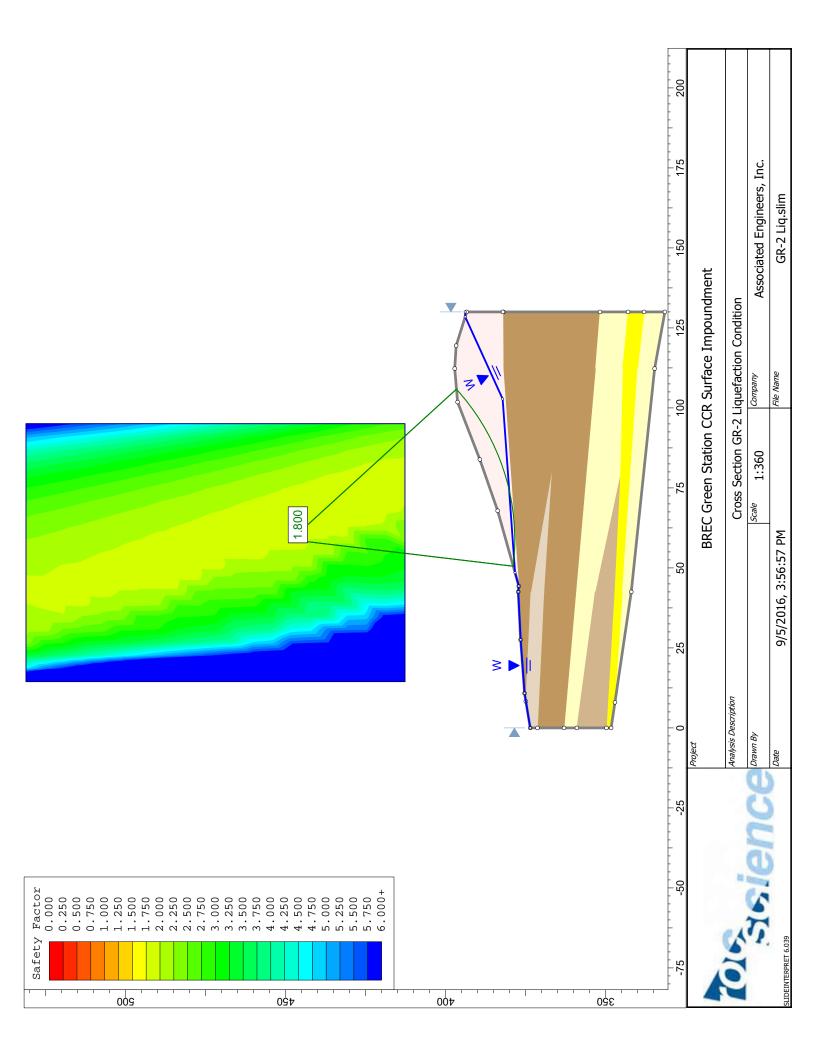
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

## **Material Boundary**

х	Y
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section GR-2 Liquefaction Condition					
sience	Drawn By		Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	М	File Name	GR-2 Liq.slim	





## **Reid/HMPL Station CCR Surface Impoundment**

Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

October 11, 2016

#### **Prepared By:**



Project ID: 160027A

# Big Rivers Electric Corporation Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

#### **CCR Surface Impoundment Information**

Name: Reid/HMPL Station CCR Surface Impoundment

Operator: Sebree Generating Station

Address: 9000 Highway 2096

Robards, Kentucky 42452

CCR Unit Identification Number: Kentucky State Dam Inventory System ID No. 0855

#### **Qualified Professional Engineer**

Name: David A. Lamb

Company: Associated Engineers, Inc.

Kentucky P.E. Number: 17822

#### **Regulatory Applicability**

As part of the § 257.73 Structural integrity criteria for existing CCR surface impoundments requirements, an owner or operator of an existing CCR surface impoundment must no later than October 17, 2016:

Conduct an initial safety factor assessment for each CCR unit and document whether the calculated factors of safety for each CCR unit achieve the minimum safety factors specified below for the critical cross section of the embankment. The critical cross section is the cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments must be supported by appropriate engineering calculations, documenting whether the CCR unit achieves the following minimum factors of safety:

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.40.
- 3. The calculated seismic factor of safety must equal or exceed 1.00.

4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

#### From: VI. Development of the Final Rule - Technical Requirements

#### **General Safety Factor Assessment Considerations**

Generally accepted engineering methodologies specify that the determination of the structural stability factors of safety specified above is to be calculated by the qualified professional engineer using conventional analysis procedures or, if necessary, special analysis procedures. Conventional analysis procedures include, but are not limited to, limit equilibrium methods of slope stability analysis, whereas, special analysis procedures include, but are not limited to, finite element methods, finite difference methods, three-dimensional methods, or probabilistic methods. Whichever methodology is used to determine the factors of safety of the CCR surface impoundment, the qualified professional engineer must document the methodology used, as well as the basis for using that methodology, and the analysis must be supported by appropriate engineering calculations.

## The Calculated Static Factor of Safety Under the Long-Term, Maximum Storage Pool Loading Condition

It is generally accepted practice to analyze the stability of the downstream slope of the dam embankment for steady-state seepage (or steady seepage) conditions with the reservoir at its normal operating pool elevation (usually the spillway crest elevation) since this is the loading condition the embankment will experience most. This condition is called steady seepage with maximum storage pool. The maximum storage pool loading is the maximum water level that can be maintained that will result in the full development of a steady-state seepage condition. Maximum storage pool loading conditions need to be calculated to ensure that the CCR surface impoundment can withstand a maximum expected pool elevation with full development of saturation in the embankment under long-term loading. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum storage pool loading condition meet or exceed 1.5.

#### The Calculated Static Factor of Safety Under the Maximum Surcharge Pool Loading Condition

The maximum surcharge pool loading condition is calculated to evaluate the effect of a raised level (e.g., flood surcharge) on the stability of the downstream slope. This ensures that the CCR surface impoundment can withstand a temporary rise in pool elevation above the maximum storage pool elevation for which the CCR surface impoundment may normally be subject under inflow design flood stage, for a short-term until the inflow design flood is passed through the CCR surface impoundment. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum surcharge pool loading condition meet or exceed 1.4.

#### The Calculated Seismic Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold must meet a seismic factor of safety equal to or greater than 1.0. All CCR surface impoundments must also be capable of withstanding a design earthquake without damage to

the foundation or embankment that would cause a discharge of its contents. To further support the location criteria established in this rule, CCR surface impoundments and any lateral expansion exceeding a specific height and/or volume threshold must be assessed under seismic loading conditions for a seismic loading event with a 2% probability of exceedance in 50 years, equivalent to a return period of approximately 2,500 years, based on the USGS seismic hazard maps for seismic events with this return period for the region where the CCR unit is located. EPA chose the 2% exceedance probability in 50 years event based on its common use in seismic design criteria throughout engineering.

#### The Calculated Liquefaction Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold and have been determined to contain soils susceptible to liquefaction must meet a liquefaction factor of safety equal to or greater than 1.20. A prudent engineering analysis of structural stability also includes a liquefaction potential analysis and analysis of post-liquefaction static factors of safety. As discussed previously, liquefaction is a phenomenon which typically occurs in loose, saturated or partially-saturated soils in which the effective stress of the soils reduces to zero, corresponding to a total loss of shear strength of the soil. The most common occurrence of liquefaction is in loose soils, typically sands. The liquefaction FOS determination in the final rule is used to determine if a CCR unit would remain stable if the soils of the embankment of the CCR unit were to experience liquefaction. Liquefaction analysis is only necessary in instances where CCR surface impoundments show, through representative soil sampling, construction documentation, or anecdotal evidence from personnel with knowledge of the CCR unit's construction, that soils of the embankment are susceptible to liquefaction.

#### <u>Failure To Demonstrate Minimum Safety Factors or Failure To Complete a Timely</u> Safety Factor Assessment

As previously discussed, the rule requires an owner or operator to document that the calculated factors of safety for each CCR surface impoundment achieve the minimum safety factors specified in the rule. For any CCR surface impoundment that does not meet these requirements, the owner or operator must either take any engineering measure necessary to ensure that the unit meets the requirements by the rule's deadlines, or cease placement of CCR and non-CCR waste into the unit and initiate closure of such CCR unit as provided in section 257.102 within six months. Similarly, if an owner or operator fails to complete the initial safety factor assessment or any subsequent periodic factor safety assessment by the deadlines established in the rule, the owner or operator must cease placing CCR and non-CCR waste into the unit and initiate closure within six months.

#### **Description of Impoundment**

An aerial photo of the CCR unit is provided as Attachment A and an excerpt from U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps showing the location of the CCR unit is provided as Attachment B.

The CCR unit has been in place for 40 plus years The CCR unit operator has general maintenance and repair procedures in place as they determine necessary. There are no known occurrences of structural instability of the CCR unit.

The CCR unit is used for the placement of coal combustion residual material; currently slurried bottom ash. The immediate watershed that drains to the CCR unit, and in which the CCR unit is considered to be located, is unnamed and 25.45 acres in size. The unnamed watershed discharges from the CCR impoundment outflow structure and is routed to the Green River.

The CCR unit is a combined incised/earthen embankment structure. Embankments form the west, south and east sides of the impoundment and the north side is incised. The original terrain on which the pond was constructed generally sloped toward the west. Although the Green River is located less than 0.5 miles from the site, the structure does not extend significantly into the floodplain. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands.

The embankment reaches its greatest relief of approximately 42 feet on the west side. The Burns & McDonnell Engineering Co. October 8, 1971 design drawings show the inboard slope and central core portion of the dike to be constructed of compacted soil fill and the outboard slope to be consisted of sand fill. A sand blanket drain was designed for the outboard third of the base of the dike for the majority of the length and the plans show a crushed limestone drainage layer with a minimum thickness of 18 inches topped with a minimum six inches thick sand layer which extends across the entire width of the dike cross section in the southwest corner. The plans also show a cut-off trench in the original ground below dike crest and extending for the entire length of the dike.

Depth of impounded water and CCR is 16 feet and 39 feet (at respective locations of maximum impounded water and CCR depths). Elevation of impounded water and CCR is 426 feet and 440 feet, respectively, above mean sea level. These approximate depths and respective elevations are based on the most recent (December 2015) flight derived topographic contours and bathymetric survey data.

The remaining storage capacity is approximately 85,000 cubic yards (if CCR can be placed to the elevation of the current water surface). This volume was calculated based on the maximum allowable storage volume and the current volume of CCR stored in the facility based on the most recent bathymetric survey.

The approximate volume of impounded water and CCR is 767,000 cubic yards (approximate water volume is 85,000 cubic yards and approximate CCR volume is 682,000 cubic yards). This volume was calculated based on the maximum storage capacity, the current amount of CCR stored in the facility based on the most recent bathymetric survey, and the best available as-built data for the structure construction prior to placement of CCR.

The impoundment discharge consists of a rectangular concrete drop structure with a variable

height steel debris skimmer. The pool elevation can be controlled by adding or removing stop logs. The discharge structure connects to a 24-inch diameter smooth walled metal pipe underground conveyance.

#### **Calculated Safety Factors**

Results of the initial safety factor assessment for the critical cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments are supported by appropriate engineering calculations.

The safety factor analysis was conducted using the Rocscience Inc. Slide geotechnical software by evaluating four cross sections along the embankment. Slide provides numerical tools to analyze the stability of embankments using limit equilibrium methods. At each cross section, drilling, surveying, laboratory testing, and a slope stability analysis were performed. Based on the four analysis scenarios, the lowest factor of safety for all scenarios was not found at a single cross section; therefore, the results listed below are the lowest factor of safety realized from all analyzed cross sections for each scenario. The safety factor assessments are supported by appropriate engineering calculations and the Slide modeling results for the Reid/HMPL CCR impoundment are attached to this report.

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition equals: 2.053
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition equals: 2.052
- 3. The calculated seismic factor of safety equals: 1.075
- 4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety equals: 1.585

#### **Sources of Information**

Geotechnical and other information provided by Associated Engineers, Inc.

Engineering design drawings and other information provided by Big Rivers Electric Corporation

United States Geological Survey U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps

## Professional Engineer Certification [Per 40 CFR § 257.73] Reid/HMPL CCR Impoundment Initial Safety Factor Assessment

I hereby certify that myself or an agent under my review has prepared this Initial Safety Factor Assessment (Assessment), and being familiar with the provisions of the final rule to regulate the disposal of coal combustion residuals (CCR) as solid waste under subtitle D of the Resource Conservation and Recovery Act (RCRA), attest that this Assessment has been prepared in accordance with good engineering practices and meets the intent of 40 CFR Part 257.73. To the best of my knowledge and belief, the information contained in this Assessment is true, complete, and accurate.

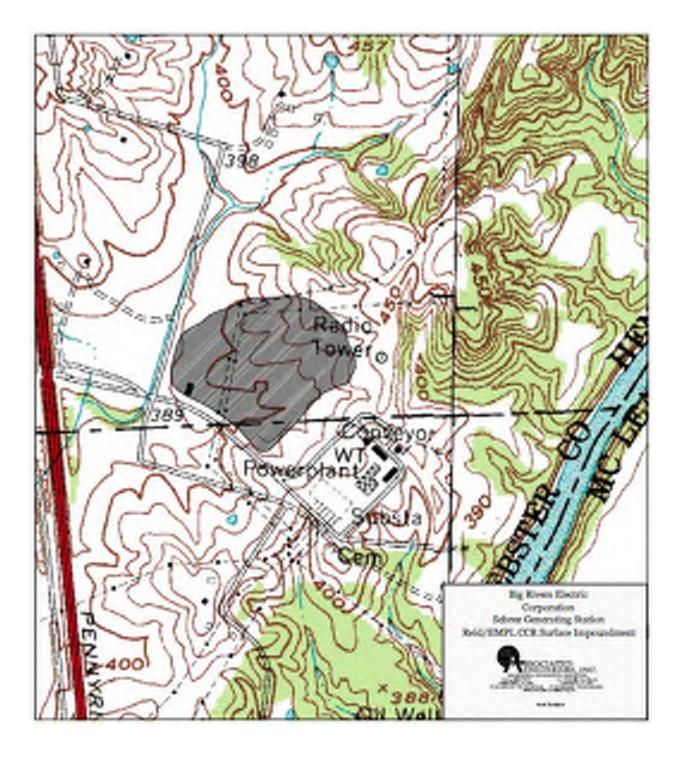
David A Lapibolino A.

State of Reglucky/bacense ₩6. 17822

Date: 10/11/16



Attachment A. Aerial Photo of the Reid/HMPL CCR Surface Impoundment



Attachment B. Topographic Map showing the Reid/HMPL CCR Surface Impoundment

## Slide Analysis Information Big Rivers Electric Corporation

#### **Project Summary**

File Name: RH-1

Last saved with Slide version: 6.039
Project Title: Big Rivers Electric Corporation

Analysis: Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	Big Rivers Electric Corporation					
(0)6	Analysis Description	Analysis Description Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition				
sience	Drawn By	Scale Company Associated Engineers, Inc.			Associated Engineers, Inc.	
SLIDE 6.039	Date	9/5/2016, 2:31:54 PM	М	File Name	RH-1.slim	

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	72	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	31	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

## **List Of Coordinates**

#### **Water Table**

х	Υ
0	392.5
30.9	392.24
85.846	396.532

	Big Rivers Electric Corporation			
(0)6	Analysis Description Rei	d/HMPL Pond RH-1 Maxi	mum Storage Po	ool Loading Condition
sience	Drawn By	Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date 9/5/2016	, 2:31:54 PM	File Name	RH-1.slim

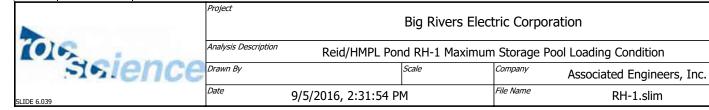
153	397.3
187.5	416.24
197.467	426.28
210	426.28

## **External Boundary**

Х	Υ
0	371.5
43.37	372.6
180.89	373.8
210	374.1
210	381.6
210	384.6
210	388.7
210	395.8
210	397.8
210	419.644
206.306	421.6
198.751	425.6
191.136	429.632
180.794	429.579
169.909	425.6
148.327	417.711
116.257	406.72
99.1875	401.002
91.5524	398.444
85.846	396.532
83.24	397.044
77.457	398.595
71.748	398.856
47.962	398.89
43.367	398.519
32.528	396.852
18.716	394.794
0	395.066
0	393.5
0	392.5
0	384.5
0	381.5
0	372

#### **Material Boundary**

X Y



0	372
40.0	275.5
43.3	375.5
140	381.6
180.9	381.6
210	381.6

Х	Y
0	381.5
43.3	381.5
140	381.6

#### **Material Boundary**

х	Υ
0	384.5
43.3	384.5
102.292	384.543
180	384.6
210	384.6

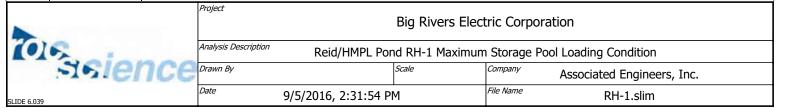
#### **Material Boundary**

х	Υ
0	392.5
43.3	392.5
102.672	391.249
180.9	389.6
210	388.7

#### **Material Boundary**

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

Х	Υ
85.846	396.532
153	397.3
180.9	397.6



х	Υ
99.1875	401.002
153	401.5
153	401.333
153	401.333
153	397.3

#### **Material Boundary**

х	Υ
153	401.333
171.018	419.644
172.943	421.6
176.879	425.6
180.794	429.579

#### **Material Boundary**

х	Υ
180.9	425.6
198.751	425.6

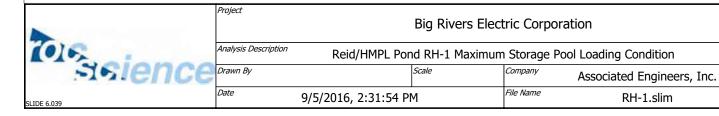
#### **Material Boundary**

х	Υ
172.943	421.6
180.9	421.6

#### **Material Boundary**

Х	Υ
180.9	421.6
206.306	421.6

х	Υ
171.018	419.644
210	419.644
180.9	419.6



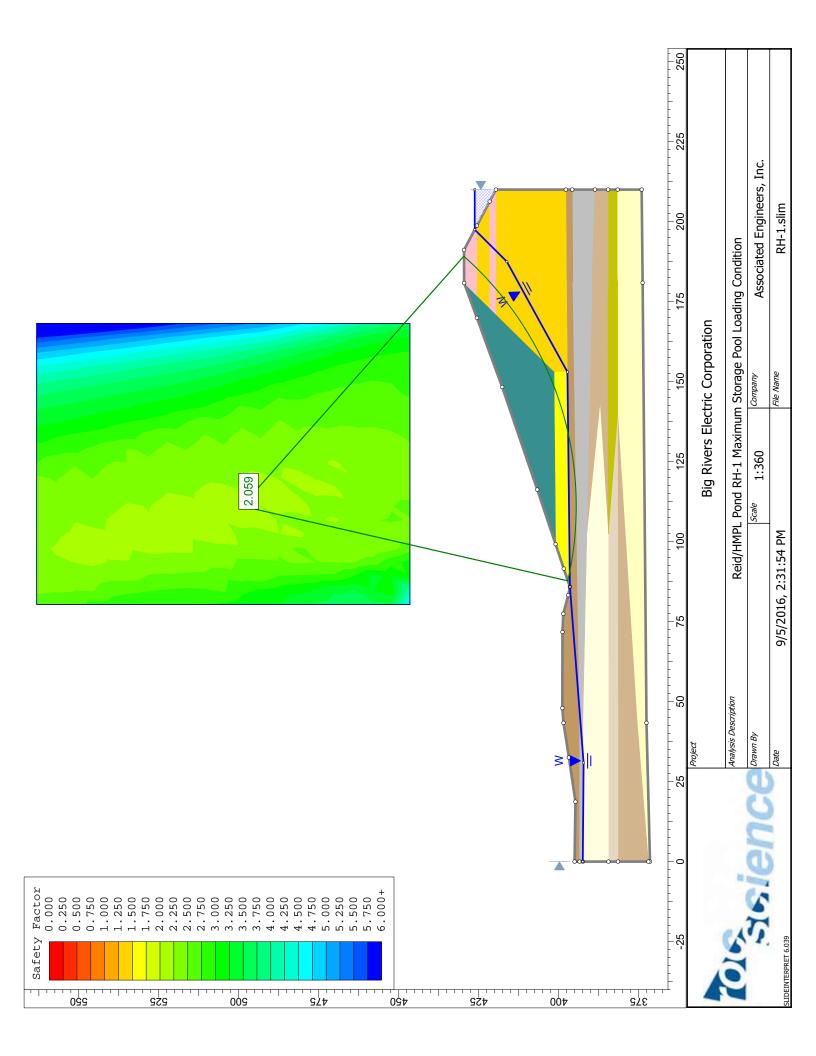
Х	Y
102.292	384.543
140	381.6

## **Material Boundary**

х	Υ
102.292	384.543
142.735	387.164
102.672	391.249

х	Υ
176.879	425.6
180.9	425.6

	Project		Big Rivers Elec	tric Corpora	ation
(0)6	Analysis Description Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition				ool Loading Condition
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date	9/5/2016, 2:31:54 PM	М	File Name	RH-1.slim



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

#### **Project Summary**

File Name: RH-1 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-1 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	Project	BREC Reid,	/HMPL Station (	CCR Surface	Impoundment
(0)6	Analysis Description Cross Section RH-1 Maximum Surcharge Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date 9	9/5/2016, 2:31:54 PM		File Name	RH-1 Surcharge.slim

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	72	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	31	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

## **List Of Coordinates**

#### **Water Table**

Х	Υ
0	392.5
30.9	392.24
85.846	396.532

	BREC Reid/HMPL Station CCR Surface Impoundment			
(0)6	Analysis Description Cross Section RH-1 Maximum Surcharge Pool Loading Condition			ol Loading Condition
sience	Drawn By	Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	9/5/2016, 2:31:54 F	PM	File Name	RH-1 Surcharge.slim

153	397.3
187.5	416.24
194.956	427.61
210	427.61

## **External Boundary**

Х	Υ
0	371.5
43.37	372.6
180.89	373.8
210	374.1
210	381.6
210	384.6
210	388.7
210	395.8
210	397.8
210	419.644
206.306	421.6
198.751	425.6
191.136	429.632
180.794	429.579
169.909	425.6
148.327	417.711
116.257	406.72
99.1875	401.002
91.5524	398.444
85.846	396.532
83.24	397.044
77.457	398.595
71.748	398.856
47.962	398.89
43.367	398.519
32.528	396.852
18.716	394.794
0	395.066
0	393.5
0	392.5
0	384.5
0	381.5
0	372

## **Material Boundary**

X Y



	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-1 Maximum Surcharge Pool Loading Condition				
9	Drawn By		Scale	Company	Associated Engineers, Inc.
	Date	9/5/2016, 2:31:54 Pl	 М	File Name	RH-1 Surcharge.slim

0	372
43.3	375.5
140	381.6
180.9	381.6
210	381.6

Х	Y
0	381.5
43.3	381.5
140	381.6

#### **Material Boundary**

х	Υ
0	384.5
43.3	384.5
102.292	384.543
180	384.6
210	384.6

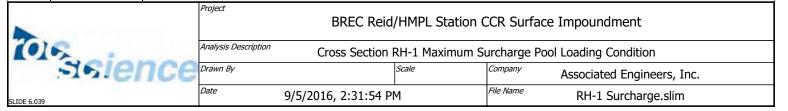
#### **Material Boundary**

х	Υ
0	392.5
43.3	392.5
102.672	391.249
180.9	389.6
210	388.7

#### **Material Boundary**

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

X	Υ
85.846	396.532
153	397.3
180.9	397.6



Х	Y		
99.1875	401.002		
153	401.5		
153	401.333		
153	401.333		
153	397.3		

#### **Material Boundary**

х	Υ		
153	401.333		
171.018	419.644		
172.943	421.6		
176.879	425.6		
180.794	429.579		

#### **Material Boundary**

Х	Υ
180.9	425.6
198.751	425.6

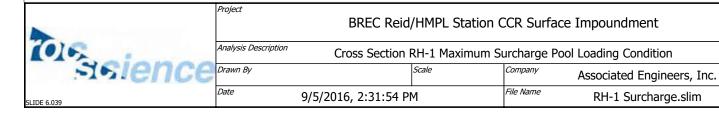
#### **Material Boundary**

Х	Y
172.943	421.6
180.9	421.6

#### **Material Boundary**

Х	Y
180.9	421.6
206.306	421.6

х	Y		
171.018	419.644		
210	419.644		
180.9	419.6		



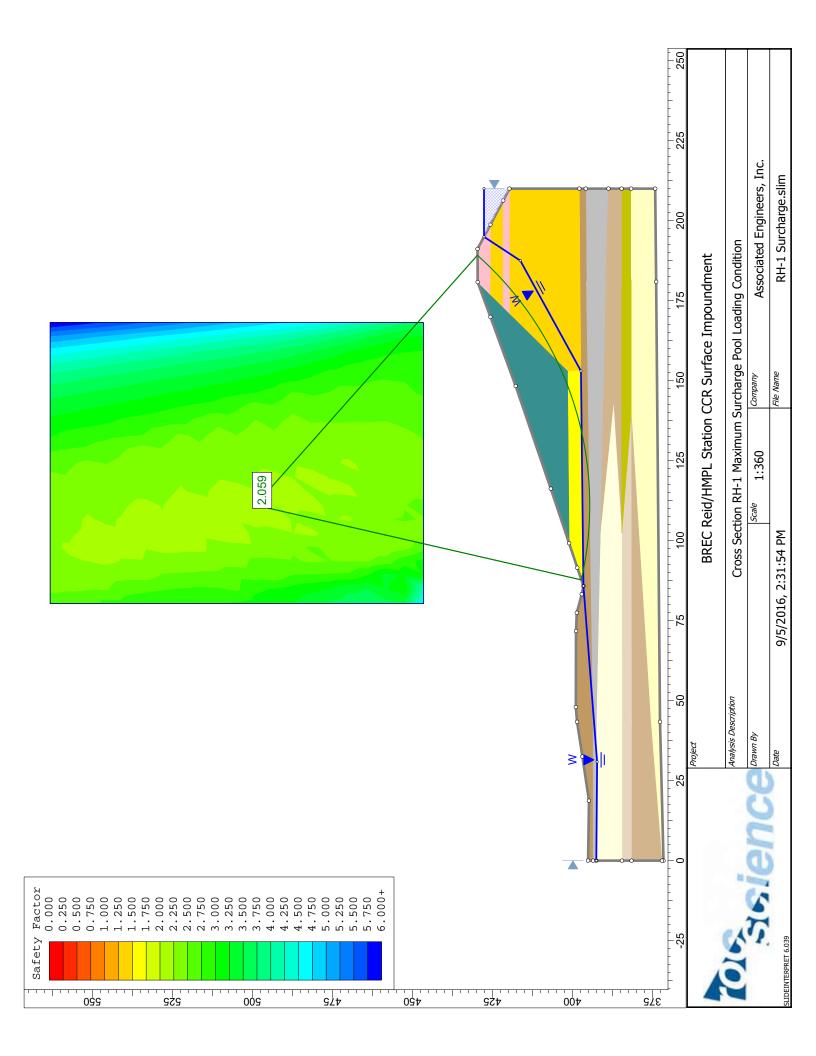
Х	Y
102.292	384.543
140	381.6

## **Material Boundary**

х	Υ				
102.292	384.543				
142.735	387.164				
102.672	391.249				

х	Υ
176.879	425.6
180.9	425.6

	BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description	Cross Section RH-1 Maximum Surcharge Pool Loading Condition					
sience	Orawn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	9/5/2016, 2:31:54 PI	М	File Name	RH-1 Surcharge.slim		



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-1 Seis

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-1 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment							
(0)6	Analysis Description	Cross Section RH-1 Seismic Loading Condition						
sience	Drawn By	wn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	9/5/2016, 2:31:54 P	М	File Name	RH-1 Seis.slim			

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

#### **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disable

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## Loading

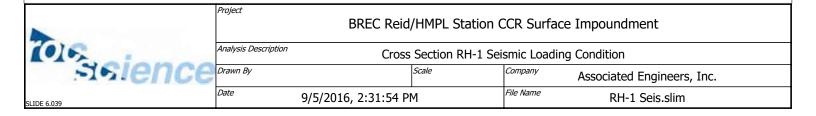
Seismic Load Coefficient (Horizontal): 0.2377

## **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	72	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	31	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

## **List Of Coordinates**



## Water Table

х	Υ		
0	392.5		
30.9	392.24		
85.846	396.532		
153	397.3		
187.5	416.24		
197.467	426.28		
210	426.28		

## **External Boundary**

Х	Υ	
0	371.5	
43.37	372.6	
180.89	373.8	
210	374.1	
210	381.6	
210	384.6	
210	388.7	
210	395.8	
210	397.8	
210	419.644	
206.306	421.6	
198.751	425.6	
191.136	429.632	
180.794	429.579	
169.909	425.6	
148.327	417.711	
116.257	406.72	
99.1875	401.002	
91.5524	398.444	
85.846	396.532	
83.24	397.044	
77.457	398.595	
71.748	398.856	
47.962	398.89	
43.367	398.519	
32.528	396.852	
18.716	394.794	
0	395.066	
0	393.5	
0	392.5	
0	384.5	

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	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-1 Seismic Loading Condition				
ì	Drawn By Scale Company Associated Engineers, Inc.			Associated Engineers, Inc.	
	Date 9/5/2016, 2:31:54 PM		File Name	RH-1 Seis.slim	

0	381.5
0	372

Х	Υ	
0	372	
43.3	375.5	
140	381.6	
180.9	381.6	
210	381.6	

## **Material Boundary**

Х	Υ
0	381.5
43.3	381.5
140	381.6

## **Material Boundary**

X	Y	
0	384.5	
43.3	384.5	
102.292	384.543	
180	384.6	
210	384.6	

# **Material Boundary**

х	Υ		
0	392.5		
43.3	392.5		
102.672	391.249		
180.9	389.6		
210	388.7		

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-1 Seismic Loading Condition				g Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	9/5/2016, 2:31:54 PM		File Name	RH-1 Seis.slim	

X	Υ
85.846	396.532
153	397.3
180.9	397.6
210	397.8

#### **Material Boundary**

х	Y		
99.1875	401.002		
153	401.5		
153	401.333		
153	401.333		
153	397.3		

#### **Material Boundary**

х	Y		
153	401.333		
171.018	419.644		
172.943	421.6		
176.879	425.6		
180.794	429.579		

#### **Material Boundary**

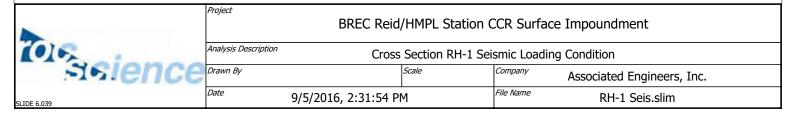
х	Υ	
180.9	425.6	
198.751	425.6	

#### **Material Boundary**

х	Υ	
172.943	421.6	
180.9	421.6	

## **Material Boundary**

Х	Υ		
180.9	421.6		
206.306	421.6		



Х	Υ		
171.018	419.644		
210	419.644		
180.9	419.6		

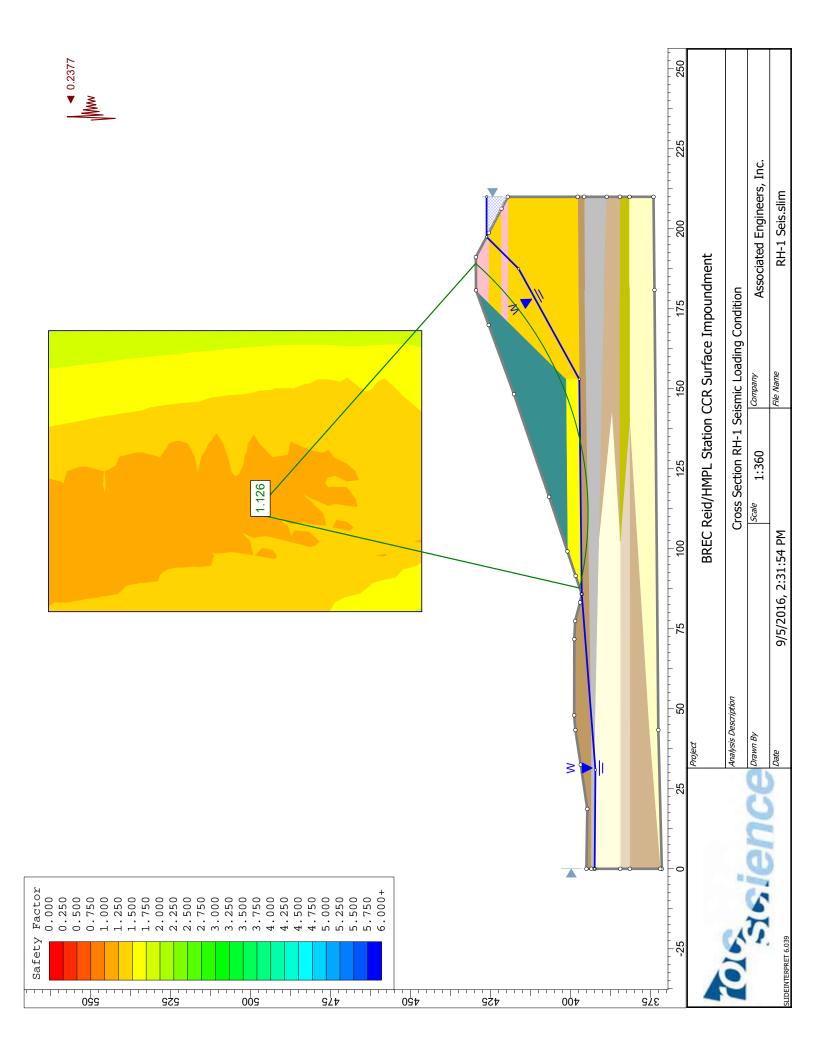
х	Υ		
102.292	384.543		
140	381.6		

## **Material Boundary**

х	Y		
102.292	384.543		
142.735	387.164		
102.672	391.249		

X	Υ		
176.879	425.6		
180.9	425.6		

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-1 Seismic Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date	9/5/2016, 2:31:54 PM		File Name	RH-1 Seis.slim	



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-1 Seis Liq

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-1 Liquefaction Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

## **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-1 Liquefaction Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	9/5/2016, 2:31:54 PM		File Name	RH-1 Seis Liq.slim		

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	0	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	0	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

# **List Of Coordinates**

#### **Water Table**

х	Υ
0	392.5
30.9	392.24
85.846	396.532

	Project BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description	Palysis Description Cross Section RH-1 Liquefaction Condition			
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 2:31:54 P	М	File Name	RH-1 Seis Liq.slim

153	397.3
187.5	416.24
197.467	426.28
210	426.28

## **External Boundary**

Х	Υ
0	371.5
43.37	372.6
180.89	373.8
210	374.1
210	381.6
210	384.6
210	388.7
210	395.8
210	397.8
210	419.644
206.306	421.6
198.751	425.6
191.136	429.632
180.794	429.579
169.909	425.6
148.327	417.711
116.257	406.72
99.1875	401.002
91.5524	398.444
85.846	396.532
83.24	397.044
77.457	398.595
71.748	398.856
47.962	398.89
43.367	398.519
32.528	396.852
18.716	394.794
0	395.066
0	393.5
0	392.5
0	384.5
0	381.5
0	372

## **Material Boundary**

X Y



	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-1 Liquefaction Condition				
e	Drawn By	Scale Company Associated Engineers, Inc.			ated Engineers, Inc.
	<sup>Date</sup> 9/5/2016, 2:31:54 PM		М	ile Name RI	H-1 Seis Liq.slim

0 372 43.3 375.5 140 381.6 180.9 381.6 210 381.6

#### **Material Boundary**

Х	Υ
0	381.5
43.3	381.5
140	381.6

#### **Material Boundary**

х	Υ
0	384.5
43.3	384.5
102.292	384.543
180	384.6
210	384.6

## **Material Boundary**

х	Υ
0	392.5
43.3	392.5
102.672	391.249
180.9	389.6
210	388.7

## **Material Boundary**

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

Х	Υ
85.846	396.532
153	397.3
180.9	397.6



	BREC Reid/HMPL Station CCR Surface Impoundment					
	Analysis Description	Cross Section RH-1 Liquefaction Condition				
e	Drawn By		Scale	Company	Associated Engineers, Inc.	
	<sup>Date</sup> 9/5/2016, 2:31:54 PM		File Name	RH-1 Seis Liq.slim		

х	Y			
99.1875	401.002			
153	401.5			
153	401.333			
153	401.333			
153	397.3			

#### **Material Boundary**

х	Υ			
153	401.333			
171.018	419.644			
172.943	421.6			
176.879	425.6			
180.794	429.579			

#### **Material Boundary**

Х	Y		
180.9	425.6		
198.751	425.6		

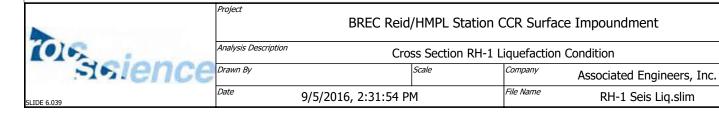
#### **Material Boundary**

Х	Υ		
172.943	421.6		
180.9	421.6		

#### **Material Boundary**

Х	Y		
180.9	421.6		
206.306	421.6		

х	Υ		
171.018	419.644		
210	419.644		
180.9	419.6		



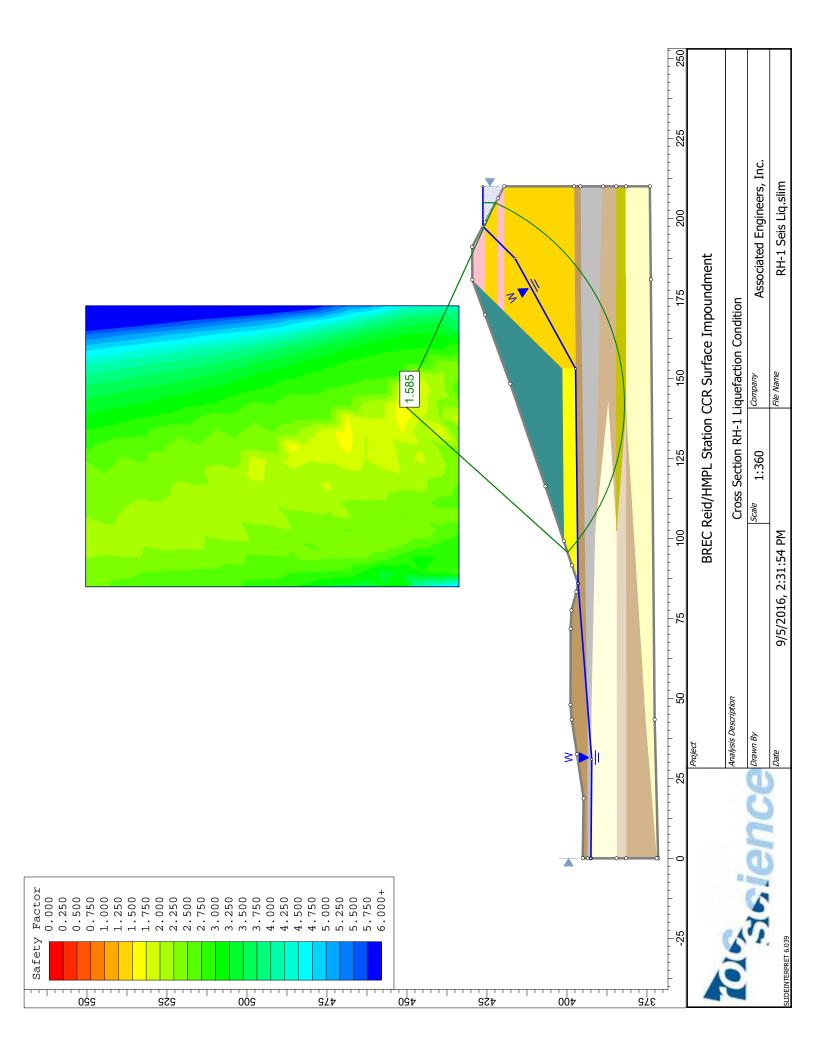
Х	Y		
102.292	384.543		
140	381.6		

## **Material Boundary**

х	Υ			
102.292	384.543			
142.735	387.164			
102.672	391.249			

х	Υ	
176.879	425.6	
180.9	425.6	

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description	Cross Section RH-1 Liquefaction Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	Date	9/5/2016, 2:31:54 PM		File Name	RH-1 Seis Liq.slim	



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-2

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-2 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:14:15 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-2 Maximum Storage Pool Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date 9/5	5/2016, 3:14:15 PN	4	File Name	RH-2.slim

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

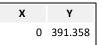
# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Granular Fill	Silty Sand (SC)	Lean Clay (CL) (Dam)	Lean clay With Sand (Dam)	Sandy Lean Clay (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	125.5	129.7	132	130	125.8	133.5	134.6
Cohesion [psf]	120	0	14.4	0	0	220	260	14.4
Friction Angle [deg]	32.3	33.7	28.7	31	33	30.4	30.6	33.3
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material	Sandy Lean Clay 2
Color		
Strength Type	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	125.8
Cohesion [psf]	200	80
Friction Angle [deg]	30	29.6
Water Surface	Water Table	Water Table
Hu Value	1	1

# **List Of Coordinates**

#### **Water Table**



	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-2 Maximum Storage Pool Loading Condition				l Loading Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:14:15 P	М	File Name	RH-2.slim

87.066 391.358 166.755 395.192 170.864 397.272 208.5 416.24 219.309 426.28 230 426.28

#### **External Boundary**

X Y 6.548e-013 364 45.37 364 208.52 387 230 390 230 395.8 230 397.8	1.6 7.9
45.37 364 208.52 387 230 390 230 395.8	1.6 7.9
208.52 387 230 390 230 395.8	7.9
230 390 230 395.8	_
230 395.8	).5
230 397.8	94
	52
230 398.7	26
230 401	1.3
230 409	9.9
230 412	2.9
230 418.	69
230 420.	43
224.631 423.3	68
223.845 423.7	98
220.564 425.5	93
214.627 428.8	42
208.516 429.1	75
202.208 429.1	34
186.355 423.8	78
183.639 422.9	77
179.243 421.	45
160.601 414.9	73
154.822 412.9	65
142.317 408.6	32
132.449 405.2	13
123.342 402.0	58
100.934 395.4	48
	15
92.3437 392.9	00
92.3437 392.9 90.2895 392.3	09
90.2895 392.3	58
90.2895 392.3 87.066 391.3	58 67
90.2895 392.3 87.066 391.3 79.843 393.6	58 67 03
90.2895 392.3 87.066 391.3 79.843 393.6 75.206 394.0	58 67 03 78

	BREC Reid/HMPL Station CCR Surface Impoundment			
(0)6	Analysis Description Cross Section RH-2 Maximum Storage Pool Loading Condition			ol Loading Condition
sience	Drawn By	Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date 9/5/2016	, 3:14:15 PM	File Name	RH-2.slim

0 393.4930 377.40 370.4

#### **Material Boundary**

X	Υ
0	370.4
45.4	370.4
100.5	377.5
45.4	377.4
0	377.4

#### **Material Boundary**

Х	Y
87.066	391.358
68	389.6
87.5	389
96.6043	389.467
205.063	395.024
208.5	395.2
230	395.894

#### **Material Boundary**

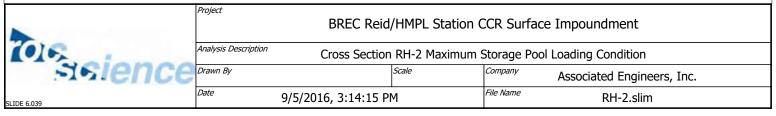
Х	Υ
87.066	391.358
208.5	397.2
230	397.852

#### **Material Boundary**

Х	Υ
90.2895	392.309
208.5	398
230	398.726

#### **Material Boundary**

х	Υ
208.5	425.2
220.564	425.593



Х	Υ
208.5	423.2
223.845	423.798

Х	Υ
208.5	400.7
230	401.3

#### **Material Boundary**

х	Υ		
92.3437	392.915		
155.8	395.9		
170.864	397.272		
208.5	400.7		

#### **Material Boundary**

х	Υ			
170.864	397.272			
181.19	407.768			
184.264	410.894			
189.983	416.707			
195.612	422.429			
197.704	424.555			
202.208	429.134			

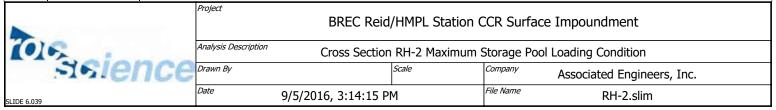
## **Material Boundary**

х	Y
197.704	424.555
208.5	425.2

## **Material Boundary**

х	Y
195.612	422.429
208.5	423.2

X	Υ				
181.19	407.768				



208.5	409.2
230	409.9

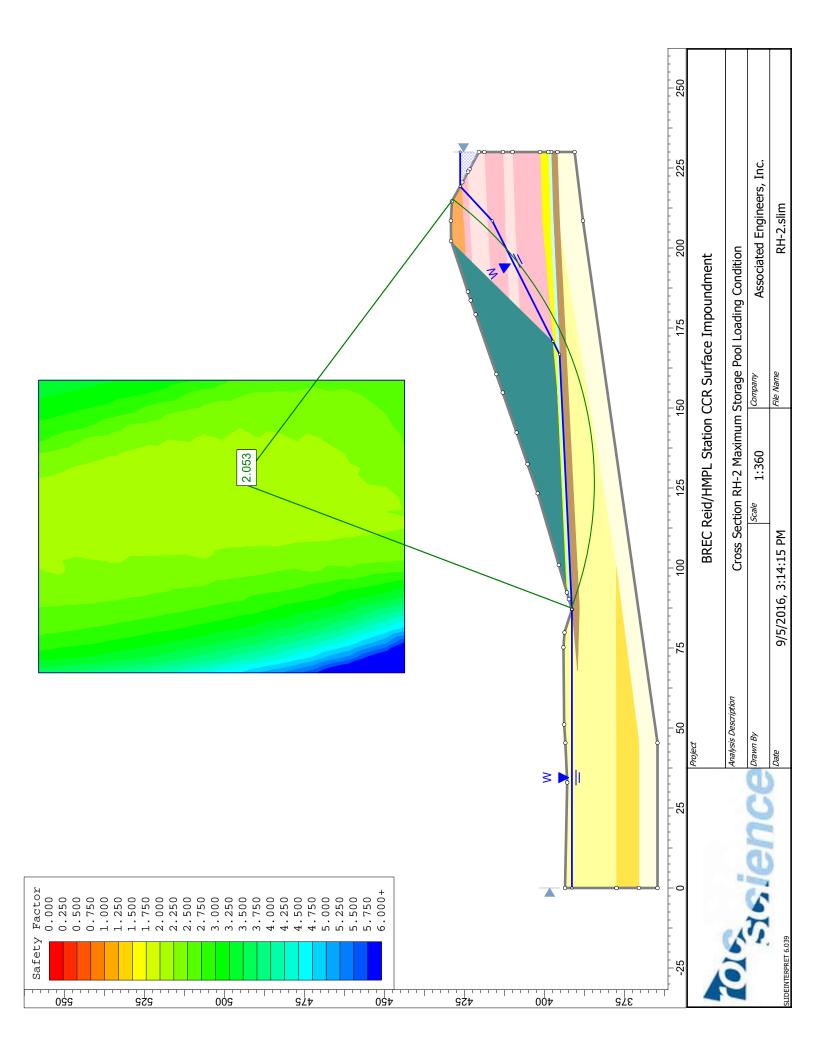
х	Υ			
184.264	410.894			
208.5	412.2			
230	412.9			

## **Material Boundary**

х	Υ		
189.983	416.707		
208.5	417.8		
230	418.69		

Х	Y
100.5	377.5
205.063	395.024

	BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description	Cross Section RH-2 Maximum Storage Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	9/5/2016, 3:14:15 PM	М	File Name	RH-2.slim		



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-2 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-2 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:14:15 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description	Cross Section RH-2 Maximum Surcharge Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	9/5/2016, 3:14:15 PM			RH-2 Surcharge.slim		

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

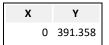
# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Granular Fill	Silty Sand (SC)	Lean Clay (CL) (Dam)	Lean clay With Sand (Dam)	Sandy Lean Clay (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	125.5	129.7	132	130	125.8	133.5	134.6
Cohesion [psf]	120	0	14.4	0	0	220	260	14.4
Friction Angle [deg]	32.3	33.7	28.7	31	33	30.4	30.6	33.3
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material	Sandy Lean Clay 2
Color		
Strength Type	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	125.8
Cohesion [psf]	200	80
Friction Angle [deg]	30	29.6
Water Surface	Water Table	Water Table
Hu Value	1	1

# **List Of Coordinates**

#### **Water Table**



	Project BREC Reid/HMPL Station CCR Surface Impoundment			e Impoundment	
(0)6	Analysis Description Cross Section RH-2 Maximum Surcharge Pool Loading Condition			ol Loading Condition	
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:14:15 P	М	File Name	RH-2 Surcharge.slim

87.066 391.358 166.755 395.192 170.864 397.272 208.5 416.24 216.878 427.61 230 427.61

#### **External Boundary**

v	V
X	264.6
6.548e-013	364.6
45.37	364.6
208.52	387.9
230	390.5
230	395.894
230	397.852
230	398.726
230	401.3
230	409.9
230	412.9
230	418.69
230	420.43
224.631	423.368
223.845	423.798
220.564	425.593
214.627	428.842
208.516	429.175
202.208	429.134
186.355	423.878
183.639	422.977
179.243	421.45
160.601	414.973
154.822	412.965
142.317	408.632
132.449	405.213
123.342	402.058
100.934	395.448
92.3437	392.915
90.2895	392.309
87.066	391.358
79.843	393.667
75.206	394.003
	393.78
45.366	393.402
32.987	392.813
79.843 75.206 51.068 45.366	393.667 394.003 393.78 393.402

	BREC Reid/HMPL Station CCR Surface Impoundment			e Impoundment	
(0)6	Analysis Description Cross Section RH-2 Maximum Surcharge Pool Loading Condition			ol Loading Condition	
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:14:15 PM	4	File Name	RH-2 Surcharge.slim

0 393.4930 377.40 370.4

#### **Material Boundary**

Х	Υ
0	370.4
45.4	370.4
100.5	377.5
45.4	377.4
0	377.4

#### **Material Boundary**

х	Υ
87.066	391.358
68	389.6
87.5	389
96.6043	389.467
205.063	395.024
208.5	395.2
230	395.894

#### **Material Boundary**

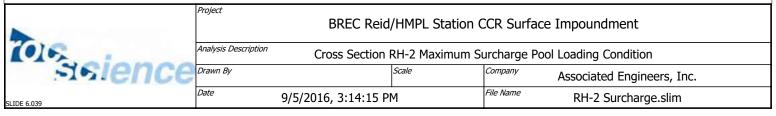
Х	Y
87.066	391.358
208.5	397.2
230	397.852

#### **Material Boundary**

Х	Υ
90.2895	392.309
208.5	398
230	398.726

#### **Material Boundary**

х	Υ
208.5	425.2
220.564	425.593



х	Υ
208.5	423.2
223.845	423.798

Х	Υ
208.5	400.7
230	401.3

#### **Material Boundary**

х	Υ
92.3437	392.915
155.8	395.9
170.864	397.272
208.5	400.7

#### **Material Boundary**

х	Υ
170.864	397.272
181.19	407.768
184.264	410.894
189.983	416.707
195.612	422.429
197.704	424.555
202.208	429.134

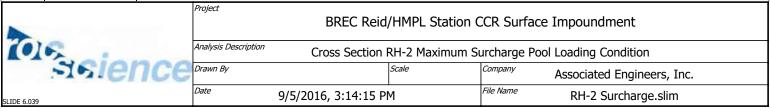
## **Material Boundary**

х	Y
197.704	424.555
208.5	425.2

## **Material Boundary**

X	Υ
195.612	422.429
208.5	423.2

Х	Υ		
181.19	407.768		



208.5	409.2
230	409.9

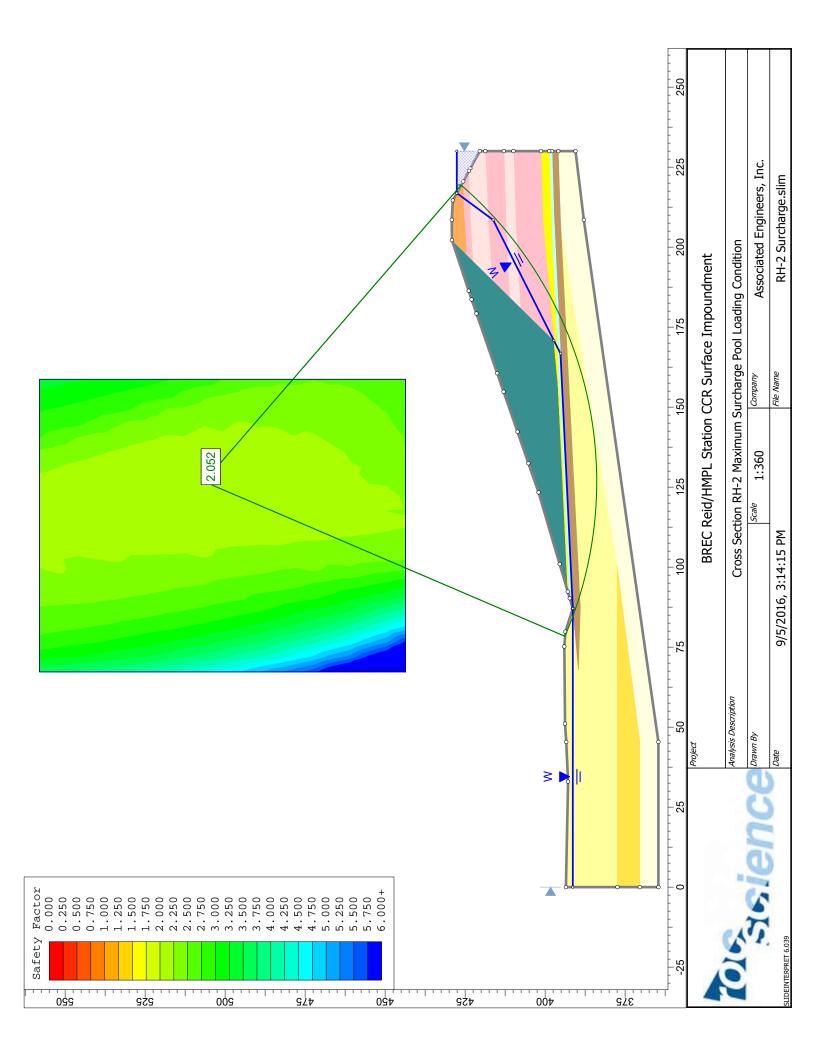
х	Υ
184.264	410.894
208.5	412.2
230	412.9

## **Material Boundary**

х	Υ
189.983	416.707
208.5	417.8
230	418.69

х	Y
100.5	377.5
205.063	395.024

	BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description Cross Section RH-2 Maximum Surcharge Pool Loading Condition						
sience	Drawn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	9/5/2016, 3:14:15 Pi	М	File Name	RH-2 Surcharge.slim		



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-2 Seis

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-2 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:14:15 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	Project	BREC Reid,	/HMPL Station (	CCR Surface	e Impoundment		
(0)6	Analysis Description Cross Section RH-2 Seismic Loading Condition						
sience	Drawn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	9/5/2016, 3:14:15 PI	М	File Name	RH-2 Seis.slim		

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

#### **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

## Loading

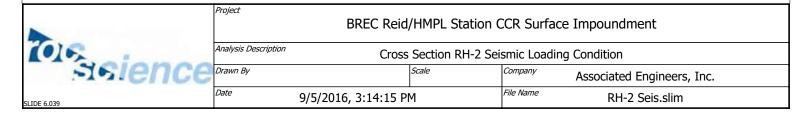
Seismic Load Coefficient (Horizontal): 0.2377

## **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Granular Fill	Silty Sand (SC)	Lean Clay (CL) (Dam)	Lean clay With Sand (Dam)	Sandy Lean Clay (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	125.5	129.7	132	130	125.8	133.5	134.6
Cohesion [psf]	120	0	14.4	0	0	220	260	14.4
Friction Angle [deg]	32.3	33.7	28.7	31	33	30.4	30.6	33.3
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	<b>Outslope Material</b>	Sandy Lean Clay 2
Color		
Strength Type	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	125.8
Cohesion [psf]	200	80
Friction Angle [deg]	30	29.6
Water Surface	Water Table	Water Table
Hu Value	1	1

## **List Of Coordinates**



#### **Water Table**

х	Υ
0	391.358
87.066	391.358
166.755	395.192
170.864	397.272
208.5	416.24
219.309	426.28
230	426.28

#### **External Boundary**

Х	Υ
6.548e-013	364.6
45.37	364.6
208.52	387.9
230	390.5
230	395.894
230	397.852
230	398.726
230	401.3
230	409.9
230	412.9
230	418.69
230	420.43
224.631	423.368
223.845	423.798
220.564	425.593
214.627	428.842
208.516	429.175
202.208	429.134
186.355	423.878
183.639	422.977
179.243	421.45
160.601	414.973
154.822	412.965
142.317	408.632
132.449	405.213
123.342	402.058
100.934	395.448
92.3437	392.915
90.2895	392.309
87.066	391.358
79.843	393.667

	Project
(0)62	Analysis
- selenc	Drawn B
SI 705 6 000	Date

	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description	Cross	Section RH-2 Sei	smic Load	ling Condition
3	Drawn By		Scale	Company	Associated Engineers, Inc.
	Date	9/5/2016 3·14·15 PI	M	File Name	RH-2 Seis slim

75.206 394.003 51.068 393.78 45.366 393.402 32.987 392.813 0 393.493 0 377.4 0 370.4

#### **Material Boundary**

Х	Υ
0	370.4
45.4	370.4
100.5	377.5
45.4	377.4
0	377.4

#### **Material Boundary**

Х	Υ
87.066	391.358
68	389.6
87.5	389
96.6043	389.467
205.063	395.024
208.5	395.2
230	395.894

## **Material Boundary**

Х	Υ
87.066	391.358
208.5	397.2
230	397.852

#### **Material Boundary**

х	Y
90.2895	392.309
208.5	398
230	398.726

#### **Material Boundary**

X Y



	BREC Reid/HMPL Station CCR Surface Impoundment					
	Analysis Description Cross Section RH-2 Seismic Loading Condition			ng Condition	l	
e:e	Drawn By		Scale	Company	Associated Engineers, Inc.	l
	Date	0/E/2016 2:14:1E DI	M	File Name	DH 2 Coic clim	ı

208.5 425.2 220.564 425.593

#### **Material Boundary**

х	Υ
208.5	423.2
223.845	423.798

#### **Material Boundary**

Х	Υ
208.5	400.7
230	401.3

## **Material Boundary**

х	Υ
92.3437	392.915
155.8	395.9
170.864	397.272
208.5	400.7

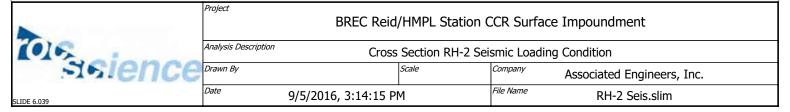
## **Material Boundary**

х	Υ
170.864	397.272
181.19	407.768
184.264	410.894
189.983	416.707
195.612	422.429
197.704	424.555
202.208	429.134

## **Material Boundary**

Х	Y
197.704	424.555
208.5	425.2

х	Υ
195.612	422.429
208.5	423.2



Х	Υ
181.19	407.768
208.5	409.2
230	409.9

# **Material Boundary**

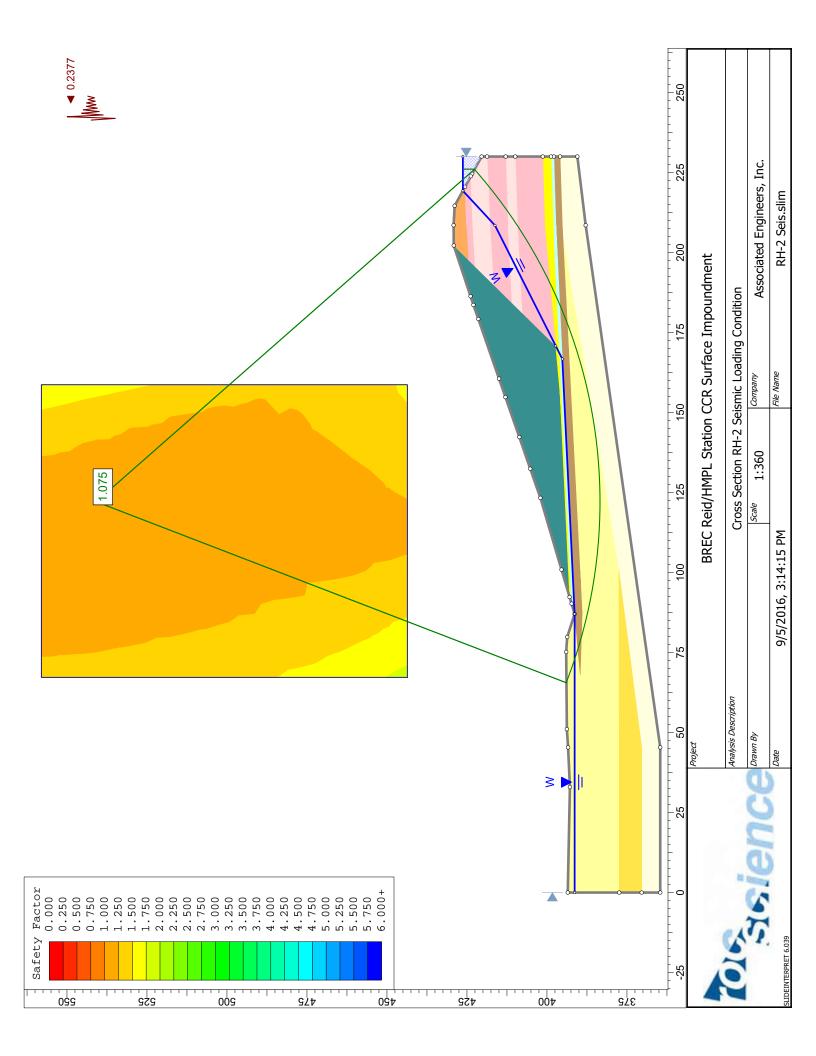
х	Υ
184.264	410.894
208.5	412.2
230	412.9

# **Material Boundary**

х	Υ
189.983	416.707
208.5	417.8
230	418.69

х	Y
100.5	377.5
205.063	395.024

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-2 Seismic Loading Condition			Condition	
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date	9/5/2016, 3:14:15 P	М	File Name	RH-2 Seis.slim



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-3

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-3 Maximum Storage Pool Loading Conditon

Company: Associated Engineers, Inc. Date Created: 8/29/2016, 2:56:23 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

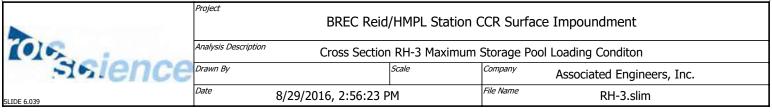
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

# **Surface Options**

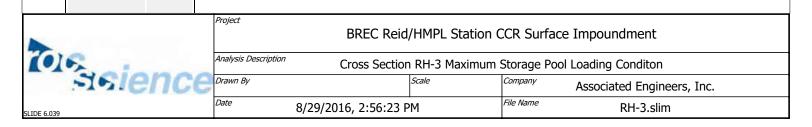
Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay	Lean Clay	Silty Clay (CL-ML)	Granular Fill and Lean Clay	Outslope Material (Dam)	Lean Clay with Sand (CL) (Dam)	Sandy Lean Clay (CL)(Dike)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	125.8	132	128	133.5	134.6	125.8
Cohesion [psf]	120	72	200	0	200	260	14.4	220
Friction Angle [deg]	32.3	30.4	33.7	31	30	30.6	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Lean Clay (CL) 2
Color	
Strength Type	Mohr-Coulomb
Unit Weight [lbs/ft3]	129.7
Cohesion [psf]	14.4
Friction Angle [deg]	28.7
Water Surface	Water Table
Hu Value	1

# **List Of Coordinates**



Х	Y
-1.11022e-016	380.175
75.344	380.175
77.002	383.846
82.5127	384.456
217.136	388.922
221.8	391.3
267.7	412.21
276.537	426.277
285	426.28

# **External Boundary**

Х	Υ
276.537	426.277
270.621	429.244
264.923	429.374
259.45	429.143
250.06	425.382
237.754	421.038
216.209	413.372
187.674	403.549
156.481	394.047
133.994	390.347
132.225	390.139
106.149	387.073
82.5127	384.456
77.002	383.846
75.344	380.175
68.53	380.645
67.51	383.799
59.536	386.037
0	385.411
0	383.8
1.38778e-017	375.3
0	371.5
0	366.9
0	359.5
40	359.5
132.2	367
172.029	370.145
265	377.7
285	378.7
285	383.6
285	390.6

	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description	Cross Section RH-3 Maximum Storage Pool Loading Conditon			
e	Drawn By	Scale Company Associated Engineers, Inc.			
	Date	8/29/2016, 2:56:23 P	M	File Name	RH-3.slim

2	85	393	3.6
2	85	396	5.6
2	85	399	9.6
2	85	422.0	33

х	Y
0	366.9
40	366.9
132.2	371.3
201	376.8
172.029	370.145

# **Material Boundary**

x	Υ
1.38778e-017	375.3
40	375.3
132.2	378.3
265	383.4
285	383.6

# **Material Boundary**

х	Υ
82.5127	384.456
132.2	386.3
265	390.4
285	390.6

# **Material Boundary**

X	Υ
0	383.8
40	383.8
67.51	383.799

х	Υ
106.149	387.073
132.2	388.3
221.8	391.3
265	393.4

	Project	BREC Reid,	/HMPL Station (	CCR Surface	e Impoundment		
(0)6	Analysis Description Cross Section RH-3 Maximum Storage Pool Loading Conditon						
sience	Drawn By  Scale  Company  Associated Engineers, Inc.						
SLIDE 6.039	<sup>Date</sup> 8/29/2016, 2:56:23 PM			File Name	RH-3.slim		

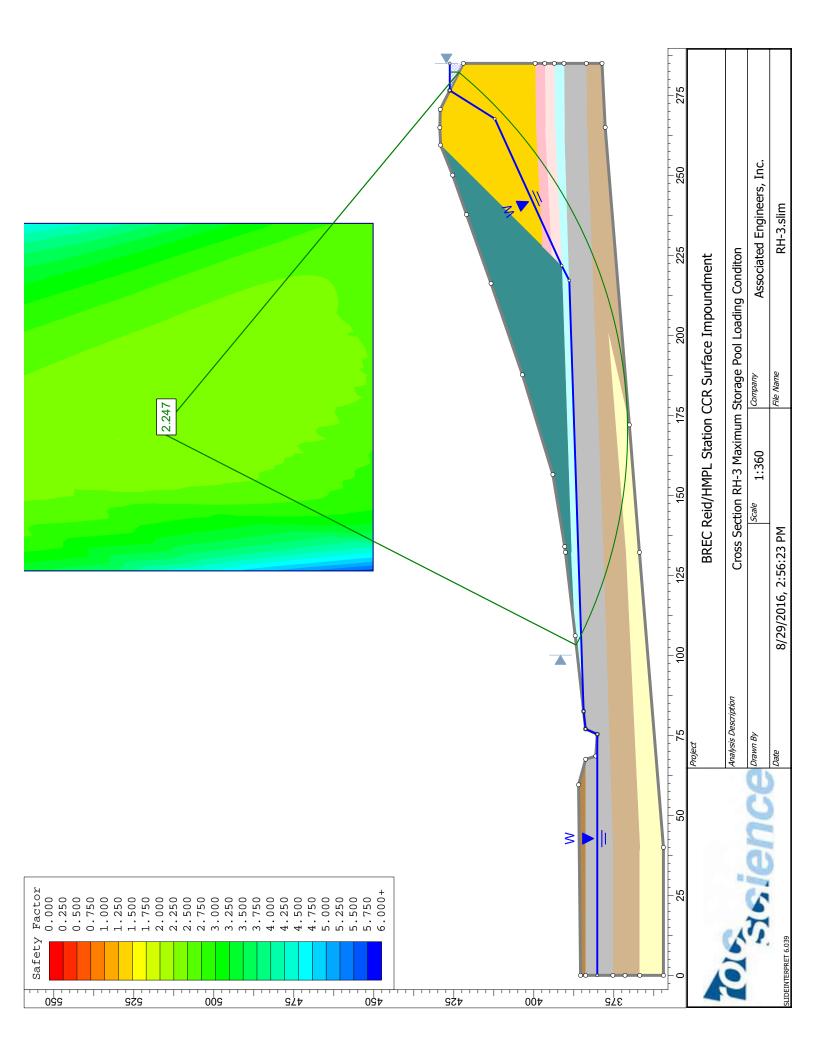
х	Υ
221.8	391.3
224.786	394.301
227.81	397.341
259.45	429.143

# **Material Boundary**

х	Υ
224.786	394.301
265	396.4
285	396.6

Х	Υ
227.81	397.341
265	399.4
285	399.6

	Project	BREC Reid,	/HMPL Station	CCR Surfac	ce Impoundment		
Analysis Description Cross Section RH-3 Maximum Storage Pool Loading Conditon					ol Loading Conditon		
sience	Drawn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	8/29/2016, 2:56:23 P	М	File Name	RH-3.slim		



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impooundment

## **Project Summary**

File Name: RH-3 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impooundment Analysis: Cross Section RH-3 Maximum Surcharge Pool Loading Conditon

Company: Associated Engineers, Inc. Date Created: 8/29/2016, 2:56:23 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

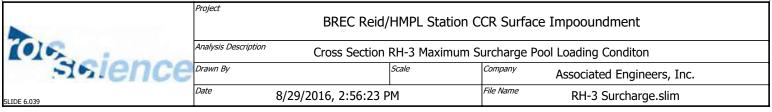
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

## **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

# **Surface Options**

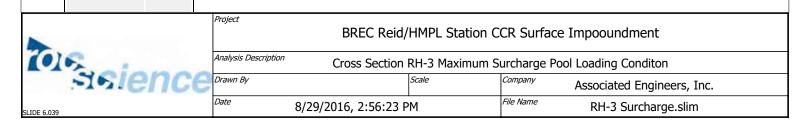
Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay	Lean Clay	Silty Clay (CL-ML)	Granular Fill and Lean Clay	Outslope Material (Dam)	Lean Clay with Sand (CL) (Dam)	Sandy Lean Clay (CL)(Dike)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	125.8	132	128	133.5	134.6	125.8
Cohesion [psf]	120	72	200	0	200	260	14.4	220
Friction Angle [deg]	32.3	30.4	33.7	31	30	30.6	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Lean Clay (CL) 2
Color	
Strength Type	Mohr-Coulomb
Unit Weight [lbs/ft3]	129.7
Cohesion [psf]	14.4
Friction Angle [deg]	28.7
Water Surface	Water Table
Hu Value	1

# **List Of Coordinates**



Х	Y
-1.11022e-016	380.175
75.344	380.175
77.002	383.846
82.5127	384.456
217.136	388.922
221.8	391.3
267.7	412.21
273.872	427.613
285	427.61

# **External Boundary**

Х	Υ
276.537	426.277
270.621	429.244
264.923	429.374
259.45	429.143
250.06	425.382
237.754	421.038
216.209	413.372
187.674	403.549
156.481	394.047
133.994	390.347
132.225	390.139
106.149	387.073
82.5127	384.456
77.002	383.846
75.344	380.175
68.53	380.645
67.51	383.799
59.536	386.037
0	385.411
0	383.8
1.38778e-017	375.3
0	371.5
0	366.9
0	359.5
40	359.5
132.2	367
172.029	370.145
265	377.7
285	378.7
285	383.6
285	390.6

	BREC Reid/HMPL Station CCR Surface Impooundment				
(0)6	Analysis Description Cross Section RH-3 Maximum Surcharge Pool Loading Conditon				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	8/29/2016, 2:56:23 P	М	File Name	RH-3 Surcharge.slim

285	393.6
285	396.6
285	399.6
285	422.033

х	Υ		
0	366.9		
40	366.9		
132.2	371.3		
201	376.8		
172.029	370.145		

# **Material Boundary**

Х	Υ
1.38778e-017	375.3
40	375.3
132.2	378.3
265	383.4
285	383.6

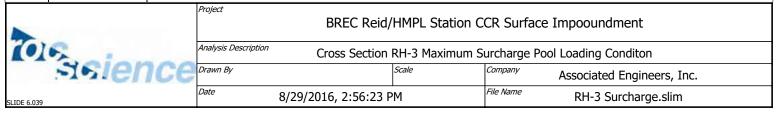
#### **Material Boundary**

х	Υ
82.5127	384.456
132.2	386.3
265	390.4
285	390.6

## **Material Boundary**

X	Υ		
0	383.8		
40	383.8		
67.51	383.799		

х	Υ
106.149	387.073
132.2	388.3
221.8	391.3
265	393.4



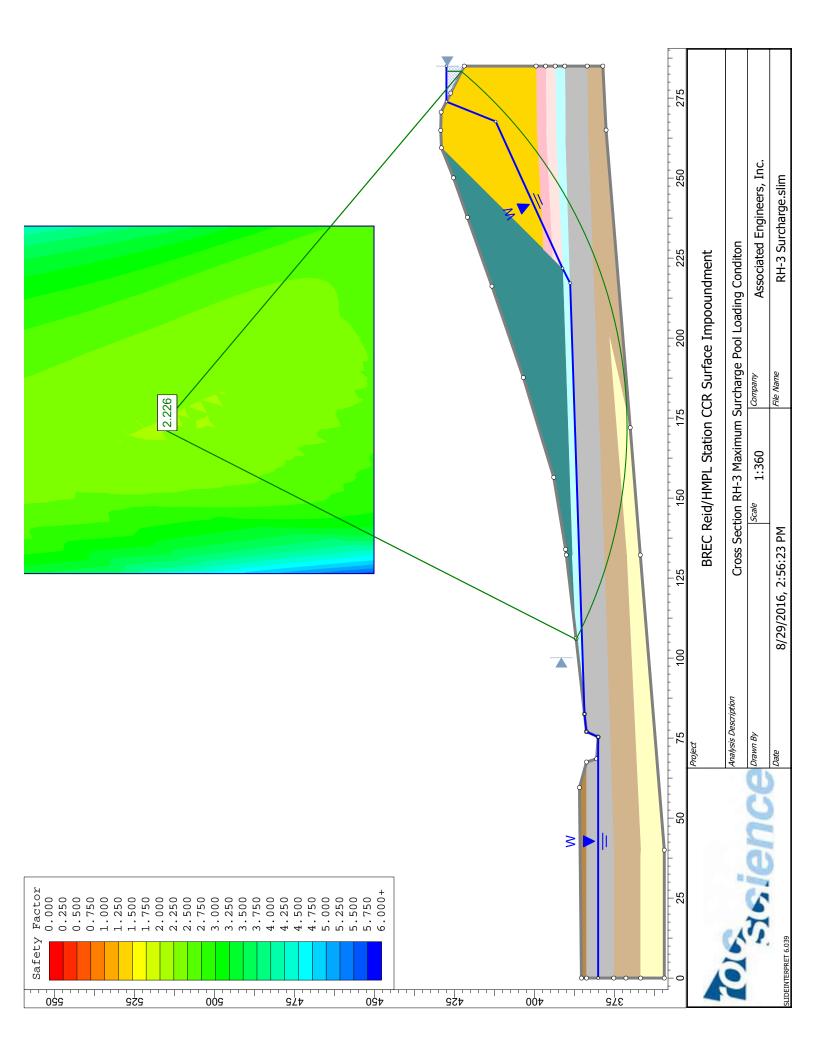
х	Υ
221.8	391.3
224.786	394.301
227.81	397.341
259.45	429.143

# **Material Boundary**

х	Y	
224.786	394.301	
265	396.4	
285	396.6	

Х	Υ
227.81	397.341
265	399.4
285	399.6

	Project	BREC Reid/	HMPL Statio	n CCR Surfac	e Impooundment
(0)6	Analysis Description Cross Section RH-3 Maximum Surcharge Pool Loading Conditon				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	8/29/2016, 2:56:23 P	М	File Name	RH-3 Surcharge.slim



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-3 Seis Slide Modeler Version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-3 Seismic Loading Conditon

Company: Associated Engineers, Inc. Date Created: 8/29/2016, 2:56:23 PM

### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

#### **Surface Options**

Surface Type: Circular

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Conditon				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim	

Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.2377

## **Material Properties**

Property	Sandy Lean Clay	Lean Clay	Silty Clay (CL-ML)	Granular Fill and Lean Clay	Outslope Material (Dam)	Lean Clay with Sand (CL) (Dam)	Sandy Lean Clay (CL)(Dike)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	125.8	132	128	133.5	134.6	125.8
Cohesion [psf]	120	72	200	0	200	260	14.4	220
Friction Angle [deg]	32.3	30.4	33.7	31	30	30.6	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Lean Clay (CL) 2
Color	
Strength Type	Mohr-Coulomb
Unit Weight [lbs/ft3]	129.7
Cohesion [psf]	14.4
Friction Angle [deg]	28.7
Water Surface	Water Table
Hu Value	1

#### **Global Minimums**

#### Method: bishop simplified

FS: 1.146920

Center: 169.832, 521.855 Radius: 150.630

Left Slip Surface Endpoint: 103.229, 386.750 Right Slip Surface Endpoint: 283.359, 422.856 Left Slope Intercept: 103.229 386.750 Right Slope Intercept: 283.359 426.279 Resisting Moment=4.1388e+007 lb-ft Driving Moment=3.60862e+007 lb-ft

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Conditon				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23 F	PM	File Name	RH-3 Seis.slim	

## Valid / Invalid Surfaces

#### Method: bishop simplified

Number of Valid Surfaces: 27798 Number of Invalid Surfaces: 813

#### **Error Codes:**

Error Code -103 reported for 812 surfaces Error Code -108 reported for 1 surface

#### **Error Codes**

The following errors were encountered during the computation:

- -103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- -108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

#### Slice Data

#### Global Minimum Query (bishop simplified) - Safety Factor: 1.14692

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	2.9517	339.413	Granular Fill and Lean Clay	0	31	80.4502	92.27	153.563	0	153.563
2	9.38923	5181.12	Silty Clay (CL-ML)	200	33.7	558.746	640.837	789.603	128.596	661.007
3	9.38923	10797.8	Silty Clay (CL-ML)	200	33.7	784.082	899.279	1423.39	374.861	1048.53
4	8.05892	13218.6	Lean Clay	72	30.4	715.465	820.581	1841.85	565.927	1275.92
5	8.05892	16561.5	Lean Clay	72	30.4	847.784	972.34	2244.15	709.561	1534.59
6	8.05892	19639.6	Lean Clay	72	30.4	970.425	1113	2598.53	824.191	1774.34
7	7.11739	19518.9	Sandy Lean Clay	120	32.3	1191.41	1366.45	2878.89	907.191	1971.7
8	7.11739	21628	Sandy Lean Clay	120	32.3	1296.89	1487.43	3125.23	962.17	2163.06
9	7.11739	23899.4	Sandy Lean Clay	120	32.3	1421.59	1630.45	3385.29	995.992	2389.3
10	7.11739	25852.9	Sandy Lean Clay	120	32.3	1527.15	1751.52	3589.6	1008.79	2580.81
11	7.97951	30912.3	Lean Clay	72	30.4	1474.5	1691.13	3758.42	998.688	2759.74
12	7.97951	32613.8	Lean Clay	72	30.4	1555.8	1784.38	3881.46	962.77	2918.69
13	7.97951	34074	Lean Clay	72	30.4	1630.67	1870.25	3964.78	899.728	3065.05
14	7.97951	35060.3	Lean Clay	72	30.4	1686.13	1933.86	3982.32	808.85	3173.47
15	7.97951	35547.6	Lean Clay	72	30.4	1722.09	1975.1	3932.9	689.137	3243.76
16	7.97951	35611.6	Lean Clay	72	30.4	1688.19	1936.22	3846.3	668.812	3177.49
17	8.30823	36828.4	Silty Clay (CL-ML)	200	33.7	1874.3	2149.67	3621.62	698.224	2923.4
18	8.30823	36298.1	Silty Clay (CL-ML)	200	33.7	1786.34	2048.79	3462.85	690.709	2772.14
19	4.95864	21118.6	Granular Fill and Lean Clay	0	31	1451.21	1664.42	3429.39	659.333	2770.06
20	5.12784	21134.1	Lean Clay with Sand (CL) (Dam)	260	30.6	1539.33	1765.49	3159.94	614.305	2545.64
21	4.73463	18862	Lean Clay (CL) (Dam)	220	30.4	1443.82	1655.95	3001.55	554.028	2447.52
22	6.60985	24515	Sandy Lean Clay (CL)(Dike)	14.4	33.3	1310.13	1502.61	2725.55	459.958	2265.59
23	6.60985	20127.3	Sandy Lean Clay (CL)(Dike)	14.4	33.3	1061.56	1217.53	2150.92	319.337	1831.59

BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description Cross S	Section RH-3 Sei	smic Loadir	ng Conditon		
sience	Drawn By	Scale	Company	Associated Engineers, Inc.		
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23 F	PM	File Name	RH-3 Seis.slim		

24 6.60985 13526.4	Sandy Lean Clay (CL)(Dike)	14.4	33.3 565.291	648.343	1510.28	545.202	965.081
25 6.60985 5362.88	Sandy Lean Clay (CL)(Dike)	14.4	33.3 141.118	161.851	659.69	435.217	224.473

## **Interslice Data**

Global Minimum Query (bishop simplified) - Safety Factor: 1.14692

Slice	X	Υ	Interslice	Interslice	Interslice
Number	coordinate	coordinate - Bottom	Normal Force	Shear Force	Force Angle
Humber	[ft]	[ft]	[lbs]	[lbs]	[degrees]
1	103.229	386.75	0	0	0
2	106.18	385.335	374.121	0	0
3	115.57	381.338	7543.64	0	0
4	124.959	378.064	16998.2	0	0
5	133.018	375.793	23804.5	0	0
6	141.077	373.995	30733.8	0	0
7	149.136	372.654	37371.2	0	0
8	156.253	371.839	43557.5	0	0
9	163.37	371.364	49129.3	0	0
10	170.488	371.227	54029.8	0	0
11	177.605	371.426	58037.4	0	0
12	185.585	372.051	60103.9	0	0
13	193.564	373.107	60668.5	0	0
14	201.544	374.601	59653.7	0	0
15	209.523	376.549	57017.3	0	0
16	217.503	378.968	52794.3	0	0
17	225.482	381.882	46588	0	0
18	233.79	385.478	40381	0	0
19	242.099	389.693	31998	0	0
20	247.057	392.528	24451.6	0	0
21	252.185	395.731	17198.2	0	0
22	256.92	398.953	9880.41	0	0
23	263.53	403.914	-811.059	0	0
24	270.139	409.482	-10555.3	0	0
25	276.749	415.751	-19503.3	0	0
26	283.359	422.856	365.715	0	0

# **List Of Coordinates**

Х	Υ
-1.11022e-016	380.175
75.344	380.175
77.002	383.846
82.5127	384.456
217.136	388.922
221.8	391.3
267.7	412.21
276.537	426.277

BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description Cross S	Section RH-3 Sei	smic Loadir	ng Conditon		
sience	Drawn By	Scale	Company	Associated Engineers, Inc.		
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23 F	PM	File Name	RH-3 Seis.slim		

## **External Boundary**

Υ
426.277
429.244
429.374
429.143
425.382
421.038
413.372
403.549
394.047
390.347
390.139
387.073
384.456
383.846
380.175
380.645
383.799
386.037
385.411
383.8
375.3
371.5
366.9
359.5
359.5
367
370.145
377.7
378.7
383.6
390.6
393.6
396.6
399.6
422.033

х	Y
0	366.9
40	366.9
132.2	371.3
201	376.8
172.029	370.145

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)	Analysis Description Cross	Analysis Description Cross Section RH-3 Seismic Loading Conditon			
sience	Drawn By	Scale	Company Associated Engineers, Inc.		
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim	

Y
375.3
375.3
378.3
383.4
383.6

## **Material Boundary**

х	Υ		
82.5127	384.456		
132.2	386.3		
265	390.4		
285	390.6		

# **Material Boundary**

Х	Υ
0	383.8
40	383.8
67.51	383.799

# **Material Boundary**

х	Y			
106.149	387.073			
132.2	388.3			
221.8	391.3			
265	393.4			
285	393.6			

# **Material Boundary**

х	Υ
221.8	391.3
224.786	394.301
227.81	397.341
259.45	429.143

# **Material Boundary**

х	Υ		
224.786	394.301		
265	396.4		
285	396.6		

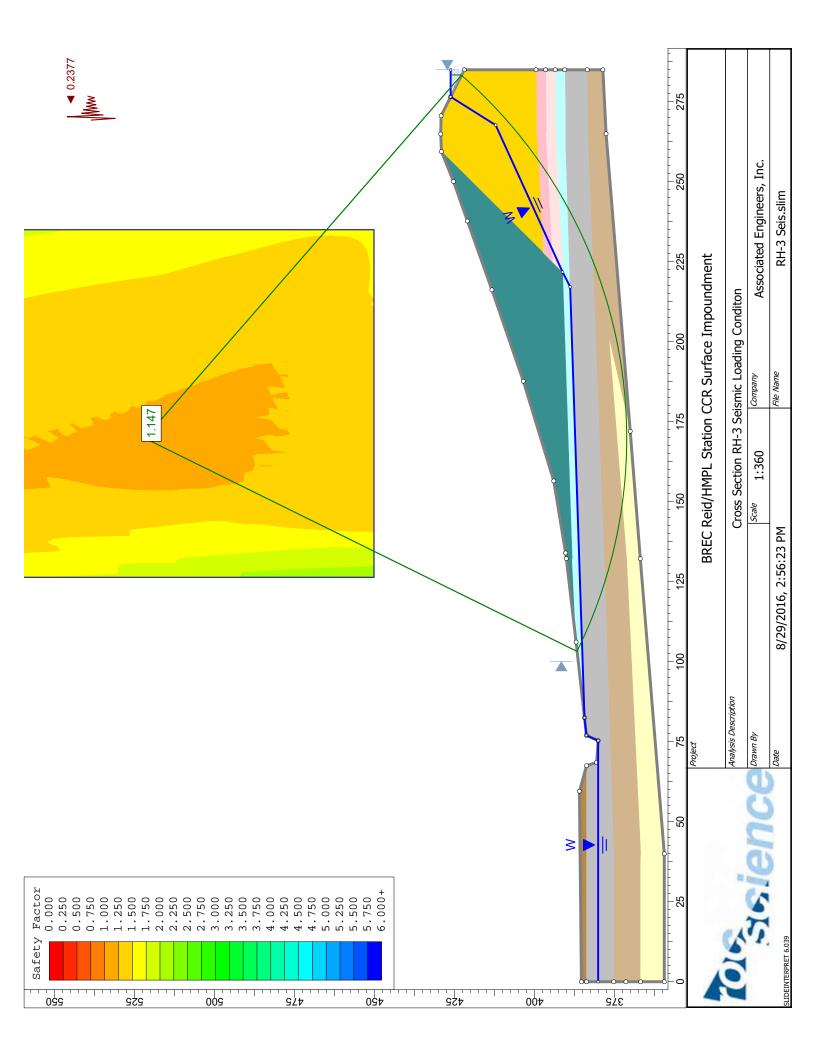
## **Material Boundary**

X Y

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Conditon					
sience	Drawn By Scale Company Associated Engineers, Inc					
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim		

227.81 397.341 265 399.4 285 399.6

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDEINTERPRET 6.039	B/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim		



# Slide Analysis Information BREC Reid HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-4

Last saved with Slide version: 6.039

Project Title: BREC Reid HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-4 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/4/2016, 12:09:41 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

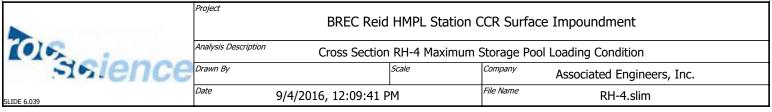
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Lean Clay (CL)	Silty Clay (CL-ML)	Lean Clay With Sand (CL) (Dam)	Lean Clay (CL) (Dam)	Silty Sand (SM)	Outslope Material (Dam)	Lean Clay (CL) (1)	Lean Clay (CL)/Clayey Sand (SC)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	134.1	125.8	133.5	125.8	130	128	129.7	125.8
Cohesion [psf]	72	200	260	220	0	200	14.4	80
Friction Angle [deg]	30.4	33.7	30.6	30.4	33	30	28.7	29.6
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

# **List Of Coordinates**

Х	Υ
2.77556e-017	393.033
55.19	393.033
71.2842	395.917
141	397
151.1	397
185.3	405.78
200.877	426.28
215	426.28

	BREC Reid	HMPL Station C	CCR Surface	Impoundment	
(0)6	Analysis Description Cross Section RH-4 Maximum Storage Pool Loading Condition				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	<sup>Date</sup> 9/4/2016, 12:09:41 P	М	File Name	RH-4.slim	

## **External Boundary**

X	٧
215	418.777
213.982	419.318
205.956	423.582
196.055	
	428.841
189.723	429.373
183.027	428.888
164.711	421.366
139.268	413.482
115.733	405.895
88.257	398.958
75.6909	396.706
71.2842	395.917
55.19	393.033
31.267	393.626
21.125	395.865
0	405.212
2.77556e-017	393.033
0	375.2
70.69	375.4
189.72	377.7
215	377.9
215	385.1
215	398.2
215	409.5

# **Material Boundary**

X	Υ
141	400
141	397
145.463	397
151.1	397
163.168	409.053
173.086	418.959
177.455	423.323
183.027	428.888

х	Υ
163.168	409.053
189.8	409.4



	BREC Reid HMPL Station CCR Surface Impoundment				
	Analysis Description	Cross Section	RH-4 Maximum S	Storage Po	ool Loading Condition
e	Drawn By		Scale	Company	Associated Engineers, Inc.
	Date	9/4/2016, 12:09:41 P	······································	File Name	RH-4.slim

Х	Y
151.1	397
189.8	398
215	398.2

# **Material Boundary**

х	Υ
141	397
70.7	392.8
36.8	391
70.7	389.5
79.6711	389.266
189.8	386.4
215	385.1

# **Material Boundary**

Х	Y
71.2842	395.917
141	397

# **Material Boundary**

Х	Υ
88.257	398.958
141	400

# **Material Boundary**

Х	Y
173.086	418.959
189.8	419.1

# **Material Boundary**

Х	Υ
189.8	419.1
213.982	419.318

	BREC Reid HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-4 Maximum Storage Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date 9/4/2016, 12:09:41	PM	File Name	RH-4.slim	

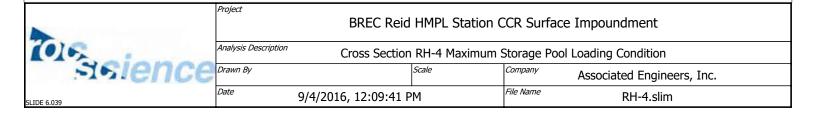
Х	Υ
177.455	423.323
189.8	423.4

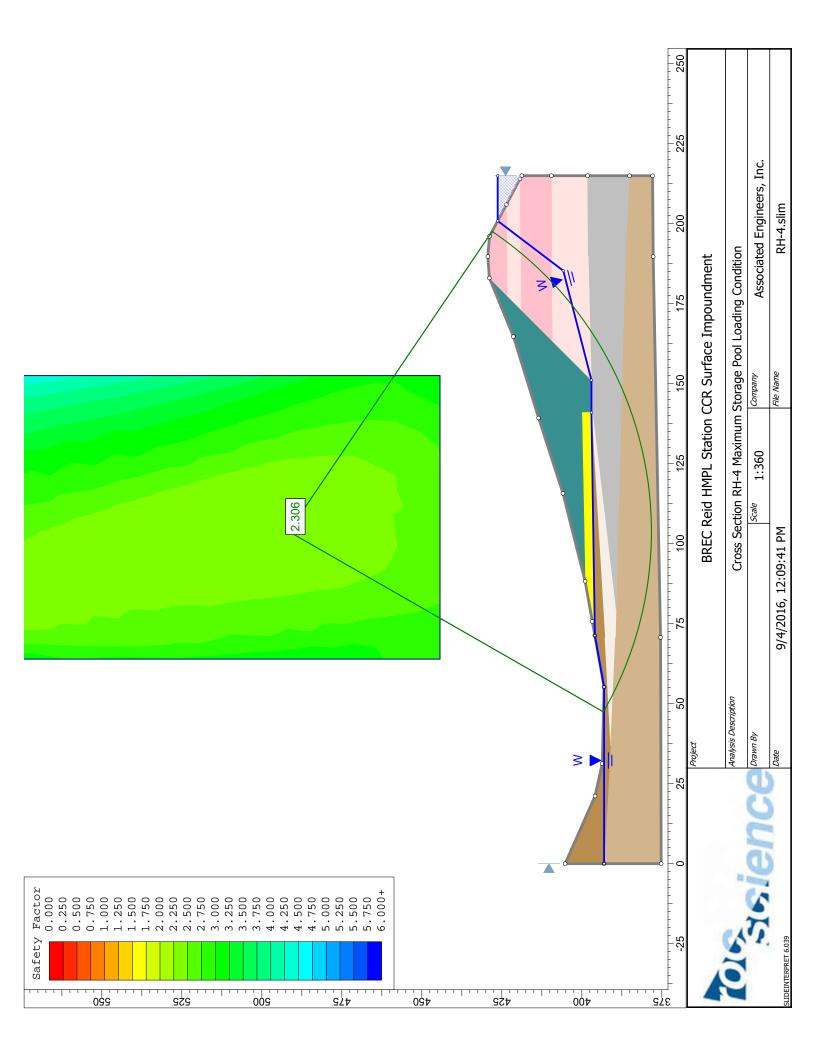
х	Υ
189.8	423.4
205.956	423.582

# **Material Boundary**

Х	Υ
2.77556e-017	393.033
36.8	391

х	Y
79.6711	389.266
145.463	397





# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-4 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-4 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/4/2016, 12:09:41 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

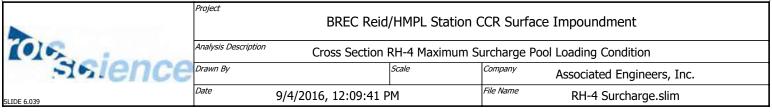
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Lean Clay (CL)	Silty Clay (CL-ML)	Lean Clay With Sand (CL) (Dam)	Lean Clay (CL) (Dam)	Silty Sand (SM)	Outslope Material (Dam)	Lean Clay (CL) (1)	Lean Clay (CL)/Clayey Sand (SC)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	134.1	125.8	133.5	125.8	130	128	129.7	125.8
Cohesion [psf]	72	200	260	220	0	200	14.4	80
Friction Angle [deg]	30.4	33.7	30.6	30.4	33	30	28.7	29.6
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

# **List Of Coordinates**

Х	Υ
2.77556e-017	393.033
55.19	393.033
71.2842	395.917
141	397
151.1	397
185.3	405.78
198.372	427.61
215	427.61

	BREC Reid,	/HMPL Station (	CCR Surface	Impoundment	
(0)6	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	9/4/2016, 12:09:41 P	М	File Name	RH-4 Surcharge.slim	

# **External Boundary**

Х	Υ
215	418.777
213.982	419.318
205.956	423.582
196.055	428.841
189.723	429.373
183.027	428.888
164.711	421.366
139.268	413.482
115.733	405.895
88.257	398.958
75.6909	396.706
71.2842	395.917
55.19	393.033
31.267	393.626
21.125	395.865
0	405.212
2.77556e-017	393.033
0	375.2
70.69	375.4
189.72	377.7
215	377.9
215	385.1
215	398.2
215	409.5

# **Material Boundary**

X	Υ
141	400
141	397
145.463	397
151.1	397
163.168	409.053
173.086	418.959
177.455	423.323
183.027	428.888

х	Υ
163.168	409.053
189.8	409.4



	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				
e	Drawn By Scale Company Associated Engineers, Inc.				
	Date	9/4/2016, 12:09:41 P	·M	File Name	RH-4 Surcharge.slim

X	Υ
151.1	397
189.8	398
215	398.2

# **Material Boundary**

Х	Υ
141	397
70.7	392.8
36.8	391
70.7	389.5
79.6711	389.266
189.8	386.4
215	385.1

# **Material Boundary**

Х	Y
71.2842	395.917
141	397

# **Material Boundary**

х	Y
88.257	398.958
141	400

# **Material Boundary**

Х	Y
173.086	418.959
189.8	419.1

# **Material Boundary**

х	Υ
189.8	419.1
213.982	419.318

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	9/4/2016, 12:09:41 F	PM	File Name	RH-4 Surcharge.slim	

Х	Υ
177.455	423.323
189.8	423.4

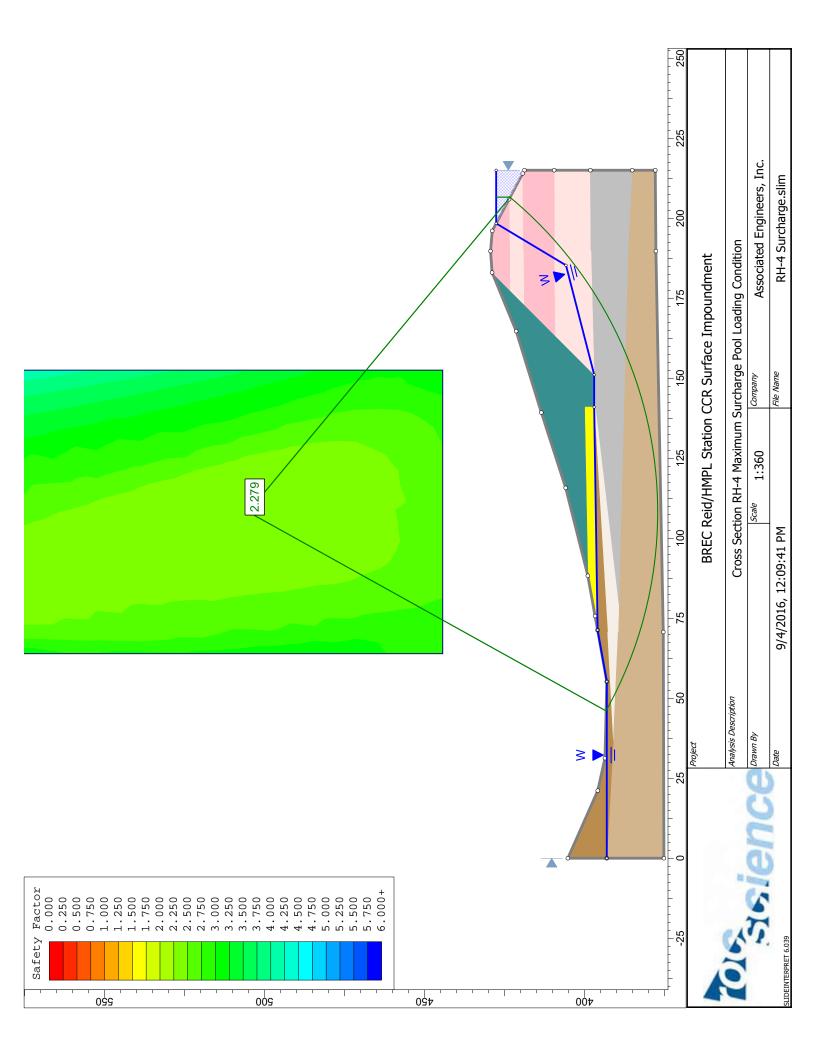
х	Y
189.8	423.4
205.956	423.582

# **Material Boundary**

Х	Υ
2.77556e-017	393.033
36.8	391

х	Y
79.6711	389.266
145.463	397

	Project	BREC Reid/	HMPL Station C	CCR Surface	Impoundment
(0)6	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				Loading Condition
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date 9/4	4/2016, 12:09:41 P	М	File Name	RH-4 Surcharge.slim



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-4 Seis 2

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-4 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/4/2016, 12:09:41 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

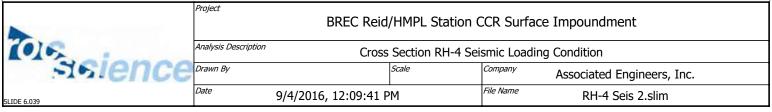
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# Loading

Seismic Load Coefficient (Horizontal): 0.2377

# **Material Properties**

Property	Lean clay	Silty clay	Lean Clay with sand (dike)	Lean clay (dike)	Silty sand (SM)	Sandy wedge	Lean clay OG	Lean clay with sand
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	134.1	125.8	133.5	125.8	130	130	129.7	125.8
Cohesion [psf]	72	200	260	220	0	200	14.4	80
Friction Angle [deg]	30.4	33.7	30.6	30.4	33	30	28.7	29.6
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

# **List Of Coordinates**

Х	Υ
2.77556e-017	393.033
55.19	393.033
71.2842	395.917
141	397
151.1	397
185.3	405.78
200.877	426.28
215	426.28

-	BREC Reid/HMPL Station CCR Surface Impoundment			
	Analysis Description Cross Section RH-4 Seismic Loading Condition			
sience	Drawn By	Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	9/4/2016, 12:09:41 PM		File Name	RH-4 Seis 2.slim

## **External Boundary**

Х	Υ
215	418.777
213.982	419.318
205.956	423.582
196.055	428.841
189.723	429.373
183.027	428.888
164.711	421.366
139.268	413.482
115.733	405.895
88.257	398.958
75.6909	396.706
71.2842	395.917
55.19	393.033
31.267	393.626
21.125	395.865
0	405.212
2.77556e-017	393.033
0	375.2
70.69	375.4
189.72	377.7
215	377.9
215	385.1
215	398.2
215	409.5

# **Material Boundary**

X	Υ
141	400
141	397
145.463	397
151.1	397
163.168	409.053
173.086	418.959
177.455	423.323
183.027	428.888

Х	Υ
163.168	409.053



	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description	Cross	Section RH-4 Sei	smic Loadi	ng Condition
e	Drawn By		Scale	Company	Associated Engineers, Inc.
	Date	9/4/2016, 12:09:41 P	M	File Name	RH-4 Seis 2.slim

189.8	409.4
215	409.5

#### **Material Boundary**

Х	Υ
151.1	397
189.8	398
215	398.2

#### **Material Boundary**

Х	Υ
141	397
70.7	392.8
36.8	391
70.7	389.5
79.6711	389.266
189.8	386.4
215	385.1

#### **Material Boundary**

X	Y
71.2842	395.917
141	397

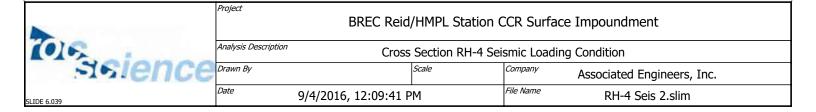
#### **Material Boundary**

Х	Y
88.257	398.958
141	400

# **Material Boundary**

х	Y
173.086	418.959
189.8	419.1

X	Y
189.8	419.1
213.982	419.318



# **Material Boundary**

х	Υ
177.455	423.323
189.8	423.4

# **Material Boundary**

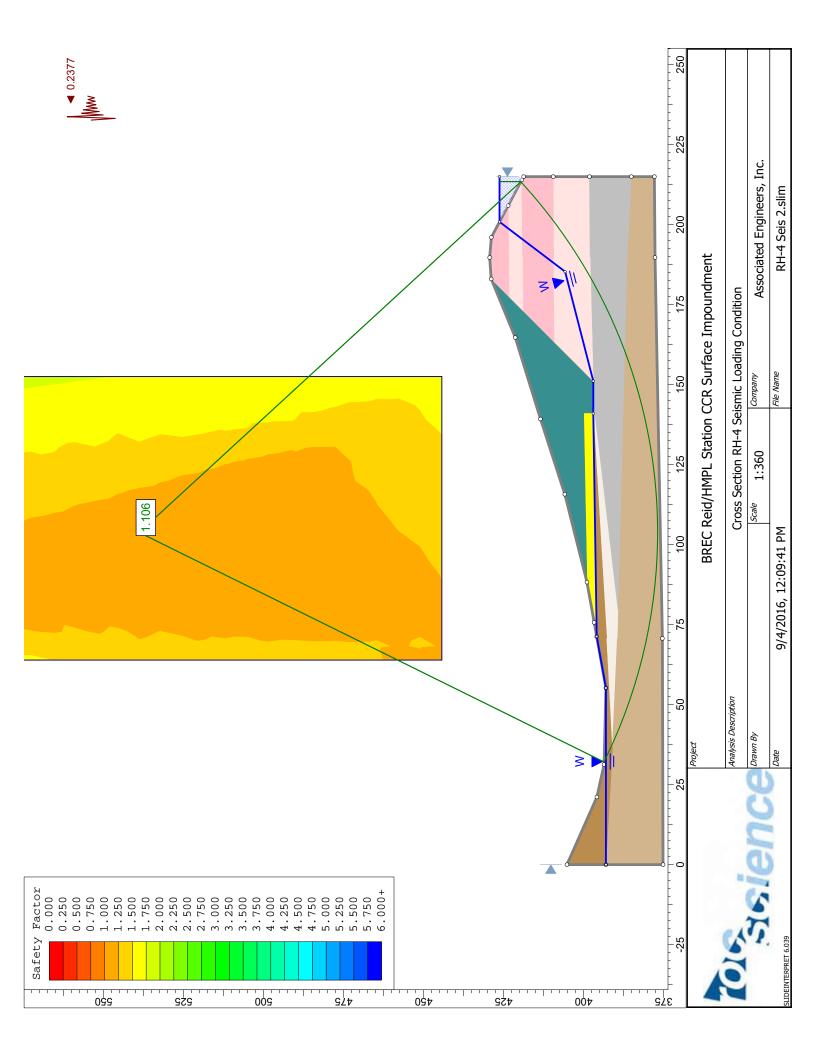
х	Y
189.8	423.4
205.956	423.582

# **Material Boundary**

Х	Υ
2.77556e-017	393.033
36.8	391

х	Y
79.6711	389.266
145.463	397

	Project	BREC Reid	/HMPL Station (	CCR Surface	e Impoundment
(0)6	Analysis Description Cross Section RH-4 Seismic Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/4/2016, 12:09:41 P	M	File Name	RH-4 Seis 2.slim







# **Green Station CCR Surface Impoundment**

Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

October 11, 2016

#### **Prepared By:**



Project ID: 160028A

# Big Rivers Electric Corporation Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

#### **CCR Surface Impoundment Information**

Name: Green Station CCR Surface Impoundment

Operator: Sebree Generating Station

Address: 9000 Highway 2096

Robards, Kentucky 42452

CCR Unit Identification Number: Kentucky State Dam Inventory System ID No. 0980

#### **Qualified Professional Engineer**

Name: David A. Lamb

Company: Associated Engineers, Inc.

Kentucky P.E. Number: 17822

#### **Regulatory Applicability**

As part of the § 257.73 Structural integrity criteria for existing CCR surface impoundments requirements, an owner or operator of an existing CCR surface impoundment must no later than October 17, 2016:

Conduct an initial safety factor assessment for each CCR unit and document whether the calculated factors of safety for each CCR unit achieve the minimum safety factors specified below for the critical cross section of the embankment. The critical cross section is the cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments must be supported by appropriate engineering calculations, documenting whether the CCR unit achieves the following minimum factors of safety:

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.40.
- 3. The calculated seismic factor of safety must equal or exceed 1.00.

4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

## From: VI. Development of the Final Rule - Technical Requirements

#### General Safety Factor Assessment Considerations

Generally accepted engineering methodologies specify that the determination of the structural stability factors of safety specified above is to be calculated by the qualified professional engineer using conventional analysis procedures or, if necessary, special analysis procedures. Conventional analysis procedures include, but are not limited to, limit equilibrium methods of slope stability analysis, whereas, special analysis procedures include, but are not limited to, finite element methods, finite difference methods, three-dimensional methods, or probabilistic methods. Whichever methodology is used to determine the factors of safety of the CCR surface impoundment, the qualified professional engineer must document the methodology used, as well as the basis for using that methodology, and the analysis must be supported by appropriate engineering calculations.

# The Calculated Static Factor of Safety Under the Long-Term, Maximum Storage Pool Loading Condition

It is generally accepted practice to analyze the stability of the downstream slope of the dam embankment for steady-state seepage (or steady seepage) conditions with the reservoir at its normal operating pool elevation (usually the spillway crest elevation) since this is the loading condition the embankment will experience most. This condition is called steady seepage with maximum storage pool. The maximum storage pool loading is the maximum water level that can be maintained that will result in the full development of a steady-state seepage condition. Maximum storage pool loading conditions need to be calculated to ensure that the CCR surface impoundment can withstand a maximum expected pool elevation with full development of saturation in the embankment under long-term loading. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum storage pool loading condition meet or exceed 1.5.

## The Calculated Static Factor of Safety Under the Maximum Surcharge Pool Loading Condition

The maximum surcharge pool loading condition is calculated to evaluate the effect of a raised level (e.g., flood surcharge) on the stability of the downstream slope. This ensures that the CCR surface impoundment can withstand a temporary rise in pool elevation above the maximum storage pool elevation for which the CCR surface impoundment may normally be subject under inflow design flood stage, for a short-term until the inflow design flood is passed through the CCR surface impoundment. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum surcharge pool loading condition meet or exceed 1.4.

#### The Calculated Seismic Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold must meet a seismic factor of safety equal to or greater than 1.0. All CCR surface

impoundments must also be capable of withstanding a design earthquake without damage to the foundation or embankment that would cause a discharge of its contents. To further support the location criteria established in this rule, CCR surface impoundments and any lateral expansion exceeding a specific height and/or volume threshold must be assessed under seismic loading conditions for a seismic loading event with a 2% probability of exceedance in 50 years, equivalent to a return period of approximately 2,500 years, based on the USGS seismic hazard maps for seismic events with this return period for the region where the CCR unit is located. EPA chose the 2% exceedance probability in 50 years event based on its common use in seismic design criteria throughout engineering.

### The Calculated Liquefaction Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold and have been determined to contain soils susceptible to liquefaction must meet a liquefaction factor of safety equal to or greater than 1.20. A prudent engineering analysis of structural stability also includes a liquefaction potential analysis and analysis of post-liquefaction static factors of safety. As discussed previously, liquefaction is a phenomenon which typically occurs in loose, saturated or partially-saturated soils in which the effective stress of the soils reduces to zero, corresponding to a total loss of shear strength of the soil. The most common occurrence of liquefaction is in loose soils, typically sands. The liquefaction FOS determination in the final rule is used to determine if a CCR unit would remain stable if the soils of the embankment of the CCR unit were to experience liquefaction. Liquefaction analysis is only necessary in instances where CCR surface impoundments show, through representative soil sampling, construction documentation, or anecdotal evidence from personnel with knowledge of the CCR unit's construction, that soils of the embankment are susceptible to liquefaction.

# <u>Failure To Demonstrate Minimum Safety Factors or Failure To Complete a Timely Safety Factor Assessment</u>

As previously discussed, the rule requires an owner or operator to document that the calculated factors of safety for each CCR surface impoundment achieve the minimum safety factors specified in the rule. For any CCR surface impoundment that does not meet these requirements, the owner or operator must either take any engineering measure necessary to ensure that the unit meets the requirements by the rule's deadlines, or cease placement of CCR and non-CCR waste into the unit and initiate closure of such CCR unit as provided in section 257.102 within six months. Similarly, if an owner or operator fails to complete the initial safety factor assessment or any subsequent periodic factor safety assessment by the deadlines established in the rule, the owner or operator must cease placing CCR and non-CCR waste into the unit and initiate closure within six months.

#### **Description of Impoundment**

An aerial photo of the CCR unit is provided as Attachment A and an excerpt from U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps showing the location of the CCR unit is provided as Attachment B.

The CCR unit has been in place for 40 plus years The CCR unit operator has general maintenance and repair procedures in place as they determine necessary. There are no known occurrences of structural instability of the CCR unit.

The CCR unit has been in place for 40 plus years. The CCR unit is used for the placement of coal combustion residual material; currently slurried bottom ash. The immediate watershed that drains to the CCR unit, and in which the CCR unit is considered to be located, is unnamed and 54.13 acres in size. The unnamed watershed discharges from the CCR impoundment outflow structure and is routed to the Green River.

The CCR unit is a combined incised/earthen embankment structure. Embankments form the west, south and east sides of the impoundment and the north side is incised. The Green River is located approximately 400 feet east of the structure. Due to surface relief, only the toe area of the south dike is potentially subject to flooding. The predominant features were small stream valleys draining eastward to the Green River. Most of the central portion of the south dike was constructed on a subdued ridge. The toe of the outboard slope intersected a lower drainage area. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands.

The west dike is generally less than five feet in height and the south dike reaches a maximum height of 19.5 feet. The east dike reaches a maximum height of approximately eight feet and is buttressed with a secondary parallel embankment that serves as a 40-foot wide roadway. The Burns and Roe, Inc. Engineering and Consultants June 30, 1978 site grading plans show the original construction layout and ground contours for the impoundment site. Bottom ash has been placed above the normal pool along the inboard side, essentially creating reclaimed land

Depth of impounded water and CCR is 16 feet and 46 feet (at respective locations of maximum impounded water and CCR depths). Elevation of impounded water and CCR is 394 feet and 408 feet, respectively, above mean sea level. These approximate depths and respective elevations are based on the most recent (December 2015) flight derived topographic contours and bathymetric survey data.

The remaining storage capacity is approximately 172,000 cubic yards (if CCR can be placed to the elevation of the current water surface). This volume was calculated based on the maximum allowable storage volume and the current volume of CCR stored in the facility based on the most recent bathymetric survey.

The approximate volume of impounded water and CCR is 981,000 cubic yards (approximate water volume is 172,000 cubic yards and approximate CCR volume is 809,000 cubic yards). This volume was calculated based on the maximum storage capacity, the current amount of CCR stored in the facility based on the most recent bathymetric survey, and the best available as-built data for the structure construction prior to placement of CCR.

The impoundment discharge consists of two corrugated steel pipes, each 30 inches in diameter. The pipe intakes are through a concrete common headwall collection structure with a variable height steel debris deflector on each pipe intake.

#### **Calculated Safety Factors**

Results of the initial safety factor assessment for the critical cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments are supported by appropriate engineering calculations.

The safety factor analysis was conducted using the Rocscience Inc. Slide geotechnical software by evaluating four cross sections along the embankment. Slide provides numerical tools to analyze the stability of embankments using limit equilibrium methods. At each cross section, drilling, surveying, laboratory testing, and a slope stability analysis were performed. Based on the four analysis scenarios, the lowest factor of safety for all scenarios was not found at a single cross section; therefore, the results listed below are the lowest factor of safety realized from all analyzed cross sections for each scenario. The safety factor assessments are supported by appropriate engineering calculations and the Slide modeling results for the Green CCR impoundment are attached to this report.

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition equals: 1.800
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition equals: 1.800
- 3. The calculated seismic factor of safety equals: 1.002
- 4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety equals: 1.800

#### **Sources of Information**

Geotechnical and other information provided by Associated Engineers, Inc.

Engineering design drawings and other information provided by Big Rivers Electric Corporation

United States Geological Survey U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps

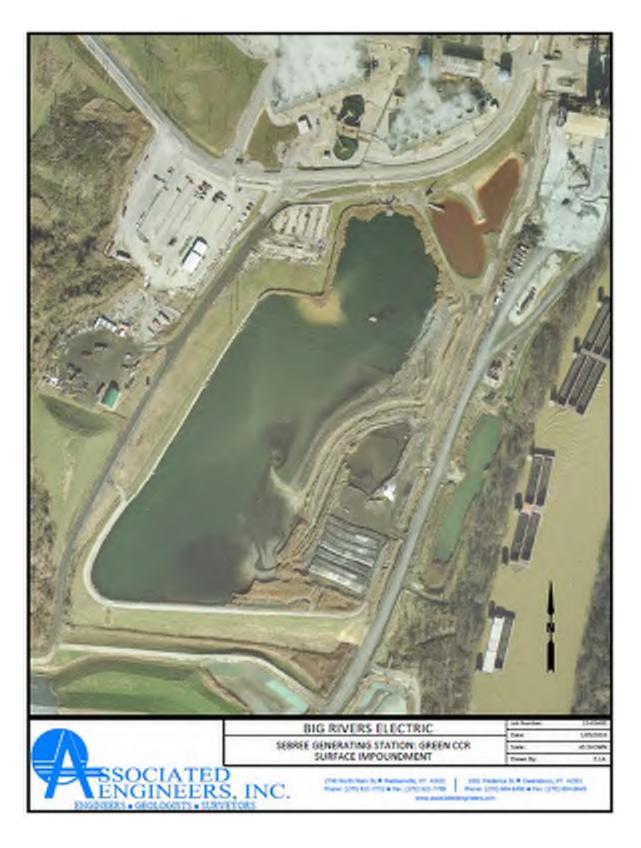
# Professional Engineer Certification [Per 40 CFR § 257.73] Green CCR Impoundment Initial Safety Factor Assessment

I hereby certify that myself or an agent under my review has prepared this Initial Safety Factor Assessment (Assessment), and being familiar with the provisions of the final rule to regulate the disposal of coal combustion residuals (CCR) as solid waste under subtitle D of the Resource Conservation and Recovery Act (RCRA), attest that this Assessment has been prepared in accordance with good engineering practices and meets the intent of 40 CFR Part 257.73. To the best of my knowledge and belief, the information contained in this Assessment is true, complete, and accurate.

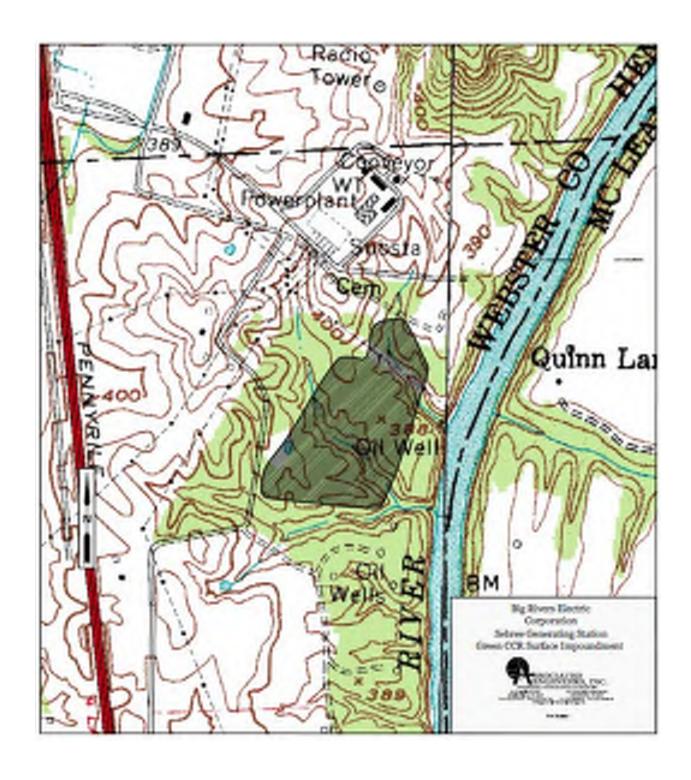
David A. Lamb

State of Kamercky Liconse 16. 17822

Date: 10/11/16



Attachment A. Aerial Photo of the Green CCR Surface Impoundment



Attachment B. Topographic Map showing the Green CCR Surface Impoundment

# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-1

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

# **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



	BREC Green Station CCR Surface Impoundment				
	Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition				
e	Drawn By		Scale	Company	Associated Engineers, Inc
	Date	9/5/2016, 3:43:18 Pi	М	File Name	GR-1.slim

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Sandy Lean Clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Y
379.185
379.172
377.695
376.221
375.6
376.406
377.906
378.317
378.983

	Projec	t
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	BREC Green Station CCR Surface Impoundment				
	Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition				
9	Drawn By Scale			Company	Associated Engineers, Inc
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Date 9/5/2016, 3:43:18 PM GR-1.slim

85.7592	379.391
140.9	382.48
170.48	393.92

# **External Boundary**

Х	Υ
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
170.48	376.396
170.48	393.967
161.556	396.809
153.137	396.9
143.057	396.719
119.757	389.859
96.35	382.029
84.12	378.983
76.996	378.317
70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

Х	Y
0	353.7
55	353.7
77	356.8
153	364.9

	Project	BREC Gr	een Station CCF	R Surface Ir	mpoundment
(0)6	Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition				Pool Loading Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date	9/5/2016, 3:43:18 Pf	М	File Name	GR-1.slim

170.48 366.698

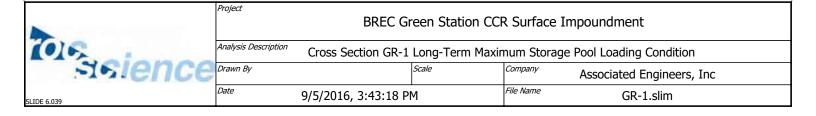
## **Material Boundary**

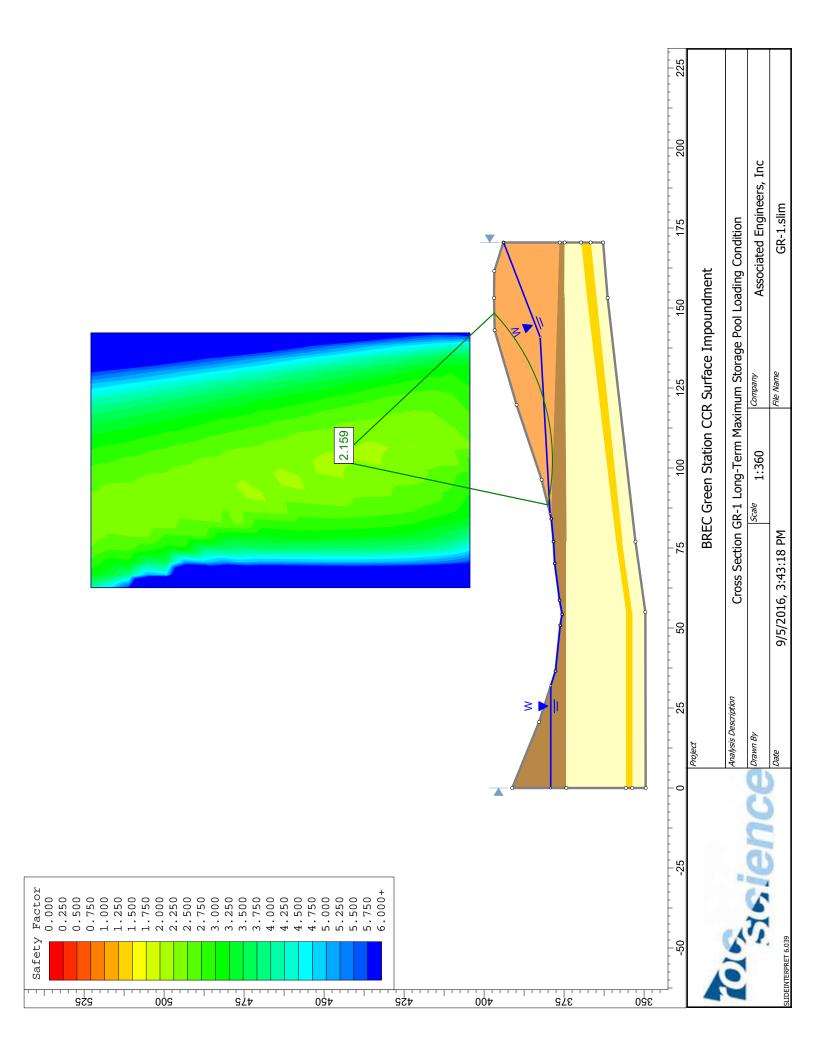
Х	Υ
0	355.7
55	355.7
77	358.8
153	367.9
170.48	369.698

#### **Material Boundary**

Х	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

X	Υ
84.12	378.983
153	376.9
170.48	376.396





# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-1 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-1 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

# **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



	BREC Green Station CCR Surface Impoundment				
	Analysis Description Cross Section GR-1 Maximum Surcharge Pool Loading Condition				
9	Drawn By	Scale Company Associated Engineers, Inc			Associated Engineers, Inc
	Date	9/5/2016, 3:43:18 PI	М	File Name	GR-1 Surcharge.slim

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean clay (CL)	Sandy lean clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Y
0	379.185
31.955	379.172
36.551	377.695
50.832	376.221
54.305	375.6
58.684	376.406
70.171	377.906
76.996	378.317
84.12	378.983

	BREC Green Station CCR Surface Impoundment			
(0)6	Analysis Description Cross Section (	ction GR-1 Maximum Surcharge Pool Loading Condition		
sience	Drawn By	Scale	Company	Associated Engineers, Inc
SLIDE 6.039	<sup>Date</sup> 9/5/2016, 3:43:18 PI	М	File Name	GR-1 Surcharge.slim

85.7592 379.391 140.9 382.48 165.719 395.483 170.48 395.48

# **External Boundary**

х	Y
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
170.48	376.396
170.48	393.967
161.556	396.809
153.137	396.9
143.057	396.719
119.757	389.859
96.35	382.029
84.12	378.983
76.996	378.317
70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

X	Υ
0	353.7
55	353.7
77	356.8



	BREC Green Station CCR Surface Impoundment				
	Analysis Description	Cross Section	GR-1 Maximum S	urcharge	Pool Loading Condition
e	Drawn By Scale Company Associated Engineers, Inc				
	Date	9/5/2016, 3:43:18 P	 М	File Name	GR-1 Surcharge.slim

153 364.9 170.48 366.698

# **Material Boundary**

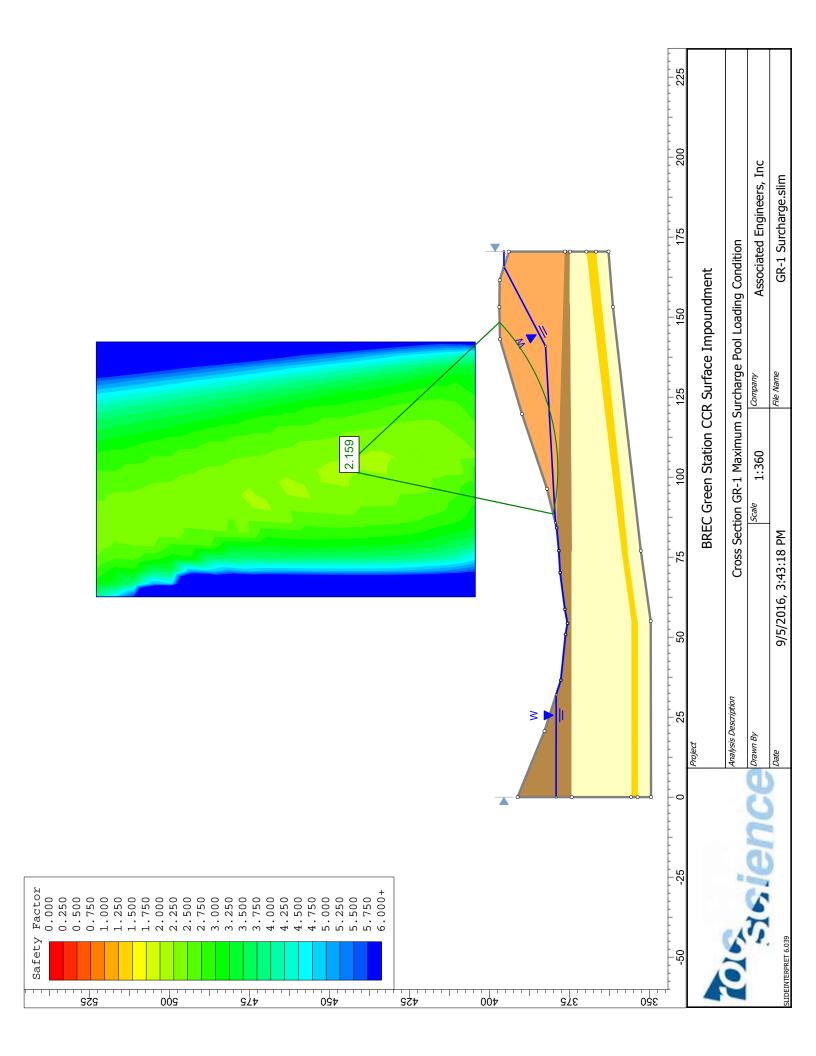
X	Υ
0	355.7
55	355.7
77	358.8
153	367.9
170.48	369.698

# **Material Boundary**

Х	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

Х	Υ
84.12	378.983
153	376.9
170.48	376.396

	Project	BREC Gr	een Station CCI	R Surface Ir	mpoundment
(0)6	Analysis Description Cross Section GR-1 Maximum Surcharge Pool Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date	9/5/2016, 3:43:18 P	М	File Name	GR-1 Surcharge.slim



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-1 Seis

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment Analysis: Cross Section GR-1 Seismic Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

# **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

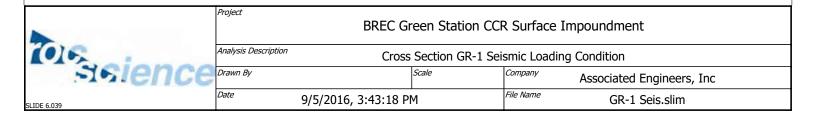
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

# **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# Loading

Seismic Load Coefficient (Horizontal): 0.2364

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean clay (CL)	Sandy Lean Clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Y
0	379.185
31.955	379.172
36.551	377.695
50.832	376.221
54.305	375.6

	BREC Green Station CCR Surface Impoundment				mpoundment
(0)6	Analysis Description Cross Section GR-1 Seismic Loading Condition			g Condition	
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date (	9/5/2016, 3:43:18 PM		File Name	GR-1 Seis.slim

58.684 376.406 70.171 377.906 76.996 378.317 84.12 378.983 85.7592 379.391 140.9 382.48 170.48 393.92

#### **External Boundary**

Х	Y
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
170.48	376.396
170.48	393.967
161.556	396.809
153.137	396.9
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84.12	378.983
76.996	378.317
70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

# **Material Boundary**

X Y



	BREC Green Station CCR Surface Impoundment				
	Analysis Description	Cross Section GR-1 Seismic Loading Condition			
e	Drawn By	Scale		Company	Associated Engineers, Inc
	Date	9/5/2016, 3:43:18 Pf	М	File Name	GR-1 Seis.slim

170.48	366.698
153	364.9
77	356.8
55	353.7
0	353.7

# **Material Boundary**

Х	Υ
0	355.7
55	355.7
77	358.8
153	367.9
170.48	369.698

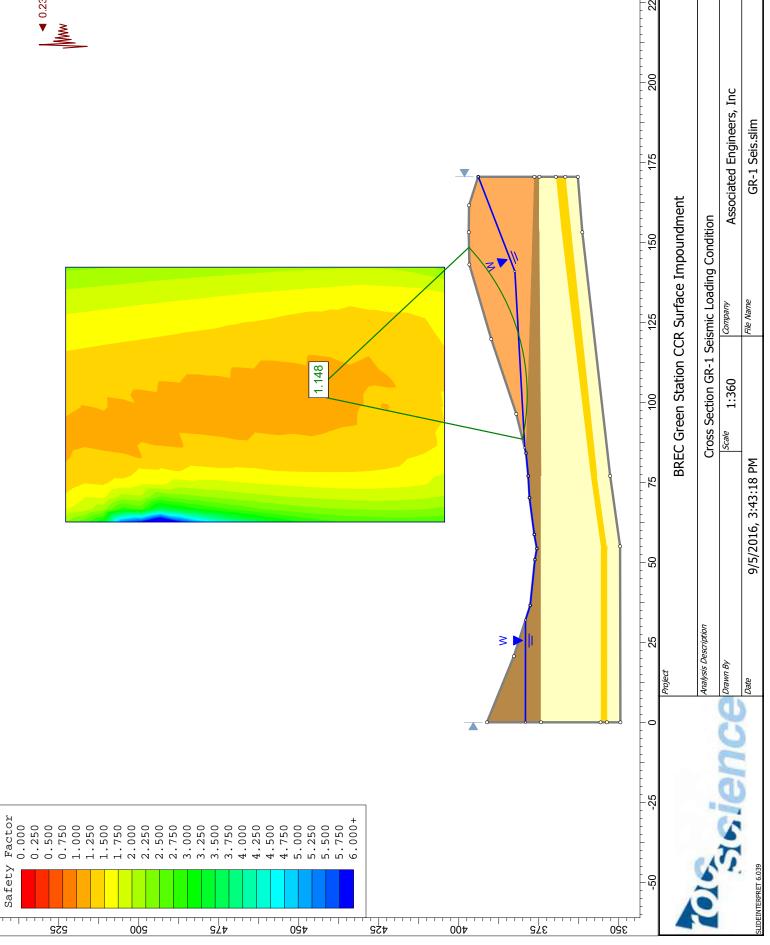
# **Material Boundary**

X	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

Х	Υ
84.12	378.983
153	376.9
170.48	376.396

	BREC Green Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section GR-1 Seismic Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc					
SLIDE 6.039	Date	9/5/2016, 3:43:18 PM		File Name	GR-1 Seis.slim	

Safety



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-2

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

# **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

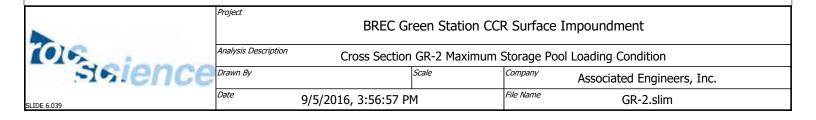
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

х	Y		
0	373.571		
10.818	375.37		
27.54	376.571		
42.515	377.335		
44.31	377.146		
48.515	378.327		
102.8	382.2		

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Maximum Storage Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	<i>Date</i> 9	9/5/2016, 3:56:57 PM		File Name	GR-2.slim

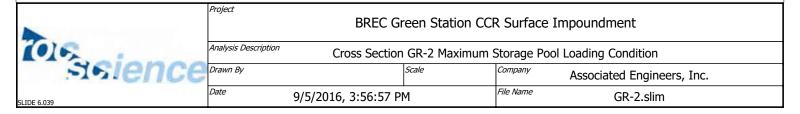
128.457 393.92 130 393.92

# **External Boundary**

V	V
X	Y
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

# **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

# **Material Boundary**

Х	Y		
0	359		
42.5	353.3		
79	345.5		

# **Material Boundary**

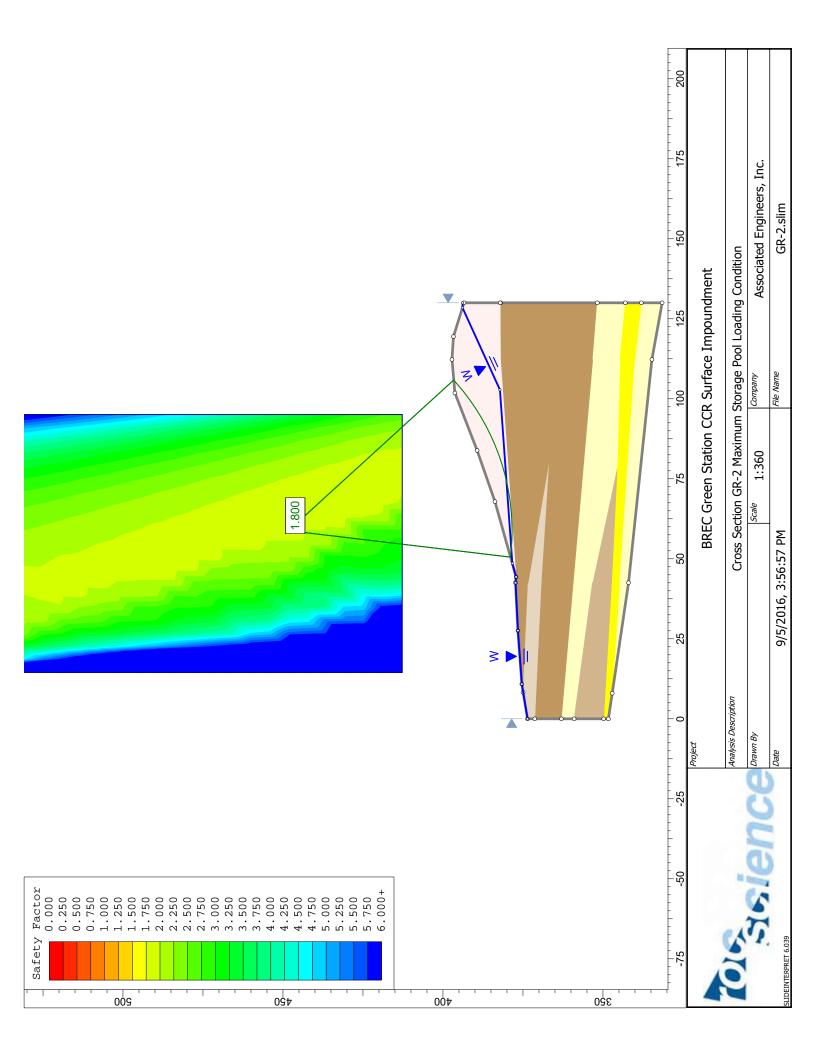
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

# **Material Boundary**

х	Υ		
0	371.3		
42.5	369.3		
80	366.9		
42.5	373.4		
8.27071	374.946		

Х	Υ		
44.31	377.146		
112.2	382		
130	382.1		

	BREC Green Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section GR-2 Maximum Storage Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	М	File Name	GR-2.slim	



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

### **Project Summary**

File Name: GR-2 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

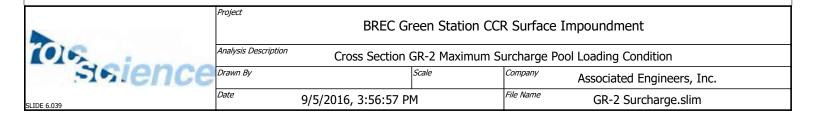
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

X	Υ			
0	373.571			
10.818	375.37			
27.54	376.571			
42.515	377.335			
44.31	377.146			
48.515	378.327			
102.8	382.2			

	BREC Green Station CCR Surface Impoundment						
(0)6	Analysis Description Cross Section GR-2 Maximum Surcharge Pool Loading Condition						
sience	Drawn By	Scale	Company	Associated Engineers, Inc.			
SLIDE 6.039	9/5/2016, 3:56:57 PM		File Name	GR-2 Surcharge.slim			

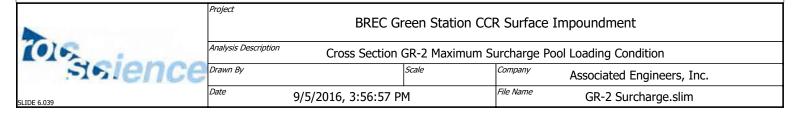
123.45 395.483 130 395.48

#### **External Boundary**

Х	Υ
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

#### **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

# **Material Boundary**

Х	Υ
0	359
42.5	353.3
79	345.5

# **Material Boundary**

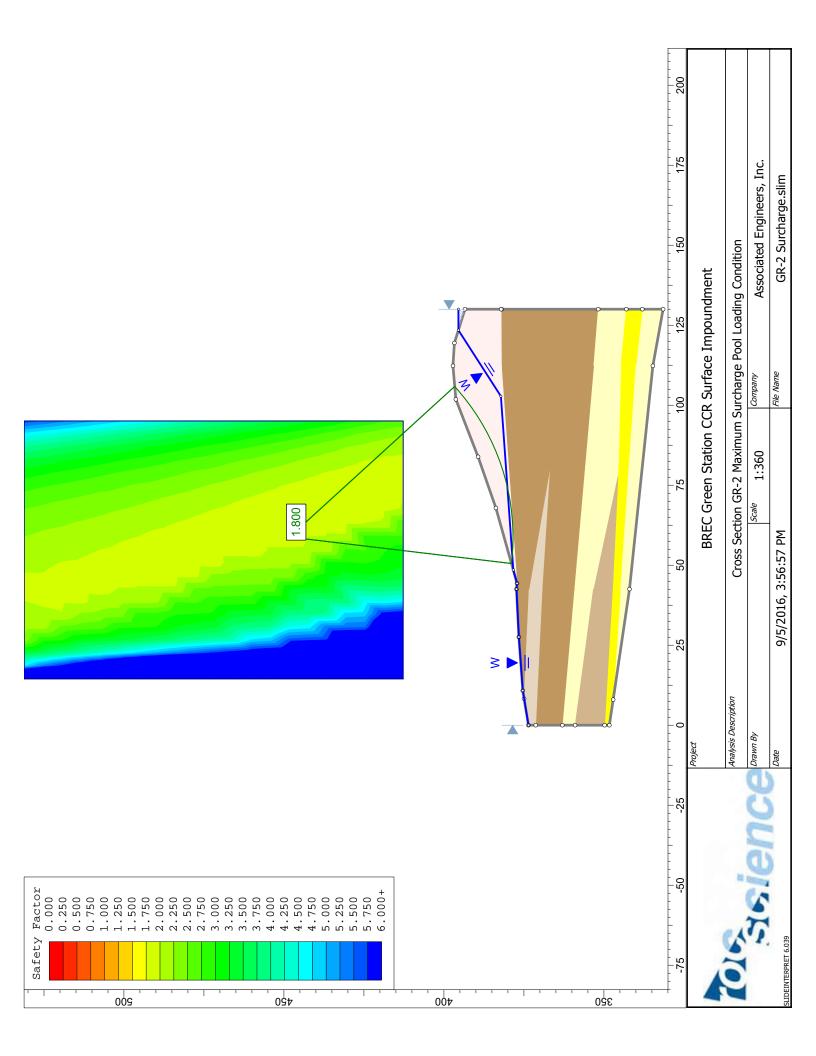
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

# **Material Boundary**

х	Υ	
0	371.3	
42.5	369.3	
80	366.9	
42.5	373.4	
8.27071	374.946	

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR Surface Impoundment				mpoundment
(0)6	Analysis Description Cross Section GR-2 Maximum Surcharge Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	М	File Name	GR-2 Surcharge.slim



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Seis 2

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment Analysis: Cross Section GR-2 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

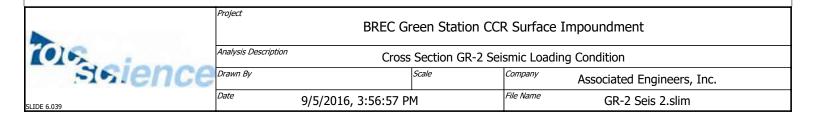
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

# **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# Loading

Seismic Load Coefficient (Horizontal): 0.2364

# **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

х	Υ
0	373.571
10.818	375.37

	BREC Green Station CCR Surface Impoundment			
(0)6	Analysis Description Cross Section GR-2 Seismic Loading Condition			
sience	Drawn By	Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	<sup>Date</sup> 9/5/2016, 3	3:56:57 PM	File Name	GR-2 Seis 2.slim

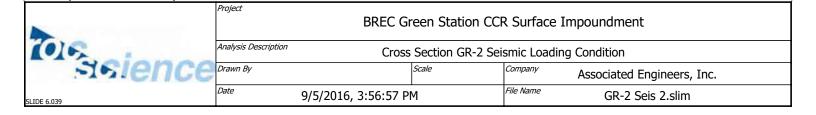
27.54 376.571 42.515 377.335 44.31 377.146 48.515 378.327 102.8 382.2 128.457 393.92 130 393.92

# **External Boundary**

Х	Υ
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

# **Material Boundary**

X Y 7.98633 347.116



42.5	344.8
112.2	340
130	338

# **Material Boundary**

Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

# **Material Boundary**

Х	Υ
0	359
42.5	353.3
79	345.5

# **Material Boundary**

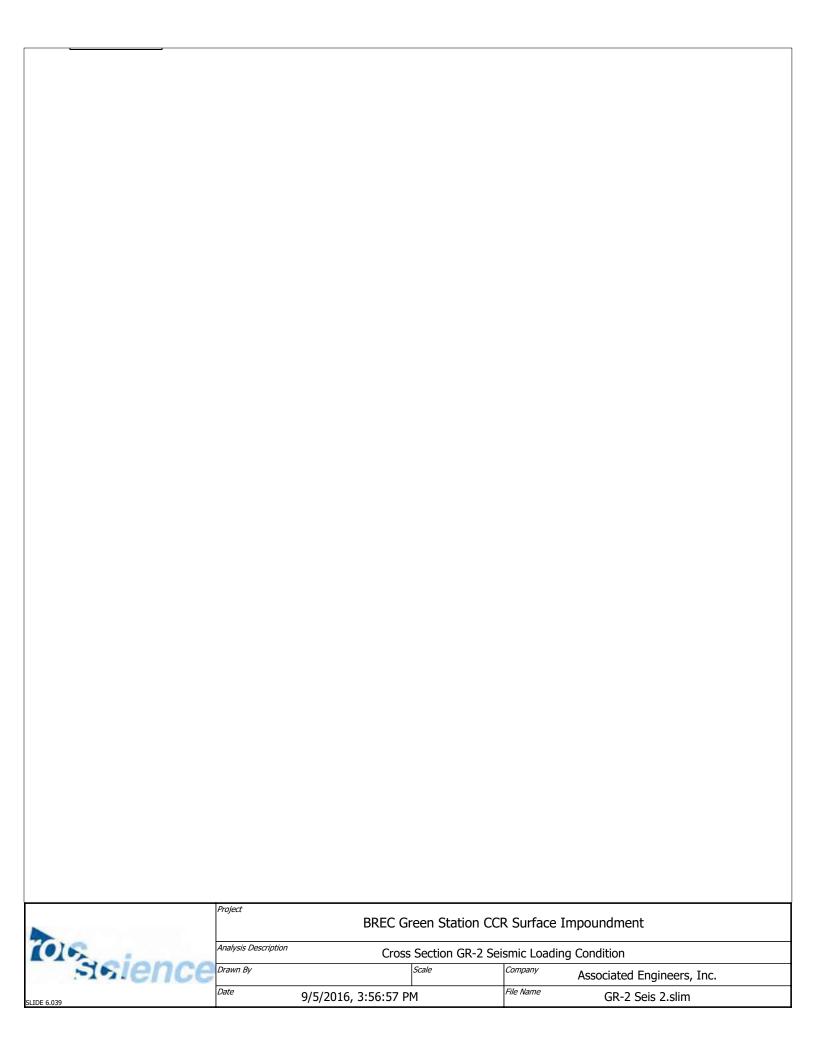
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

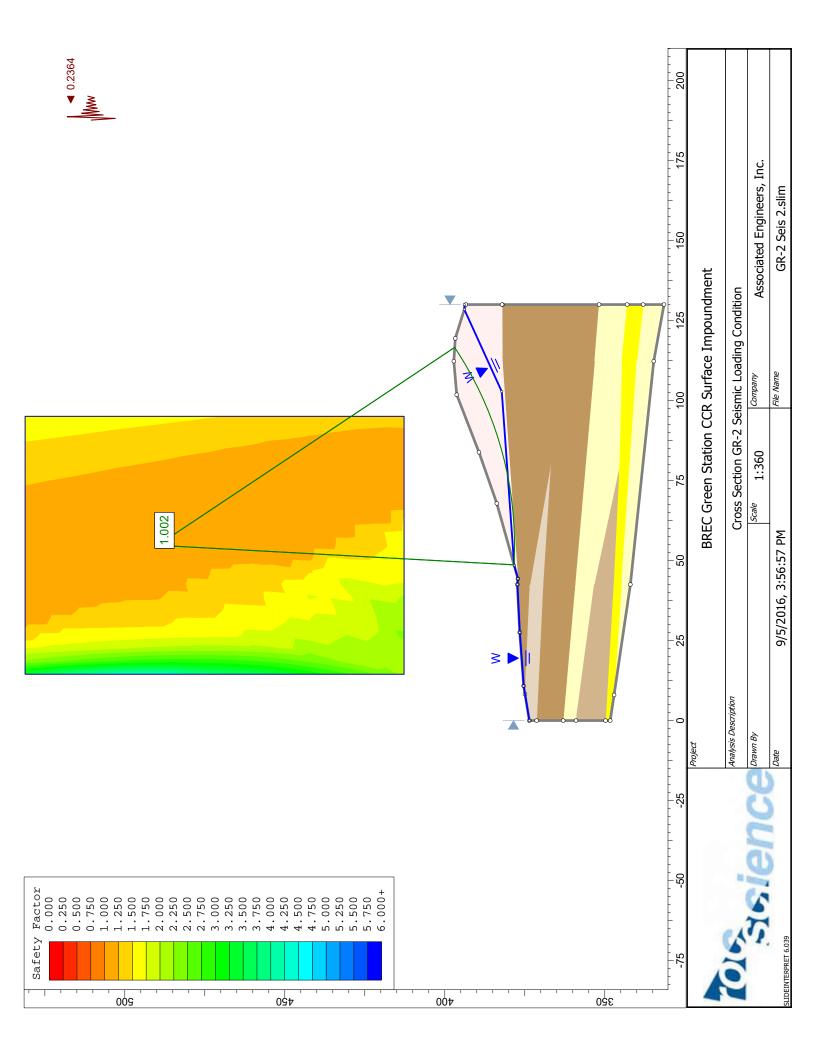
# **Material Boundary**

х	Υ
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR S				mpoundment
(0)6	Analysis Description Cross Section GR-2 Seismic Loading Condition			g Condition	
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	4	File Name	GR-2 Seis 2.slim





# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Liq

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Liquefaction Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

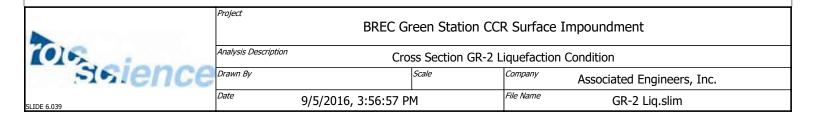
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	0	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

х	Υ
0	373.571
10.818	375.37
27.54	376.571
42.515	377.335
44.31	377.146
48.515	378.327
102.8	382.2

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Liquefaction Condition			Condition	
sience	Drawn By Scale Company Associated Engineers, In				
SLIDE 6.039	Date 9/	5/2016, 3:56:57 PN	4	File Name	GR-2 Liq.slim

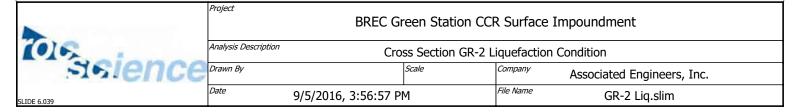
128.457 393.92 130 393.92

# **External Boundary**

х	Y
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

# **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

# **Material Boundary**

Х	Υ
0	359
42.5	353.3
79	345.5

# **Material Boundary**

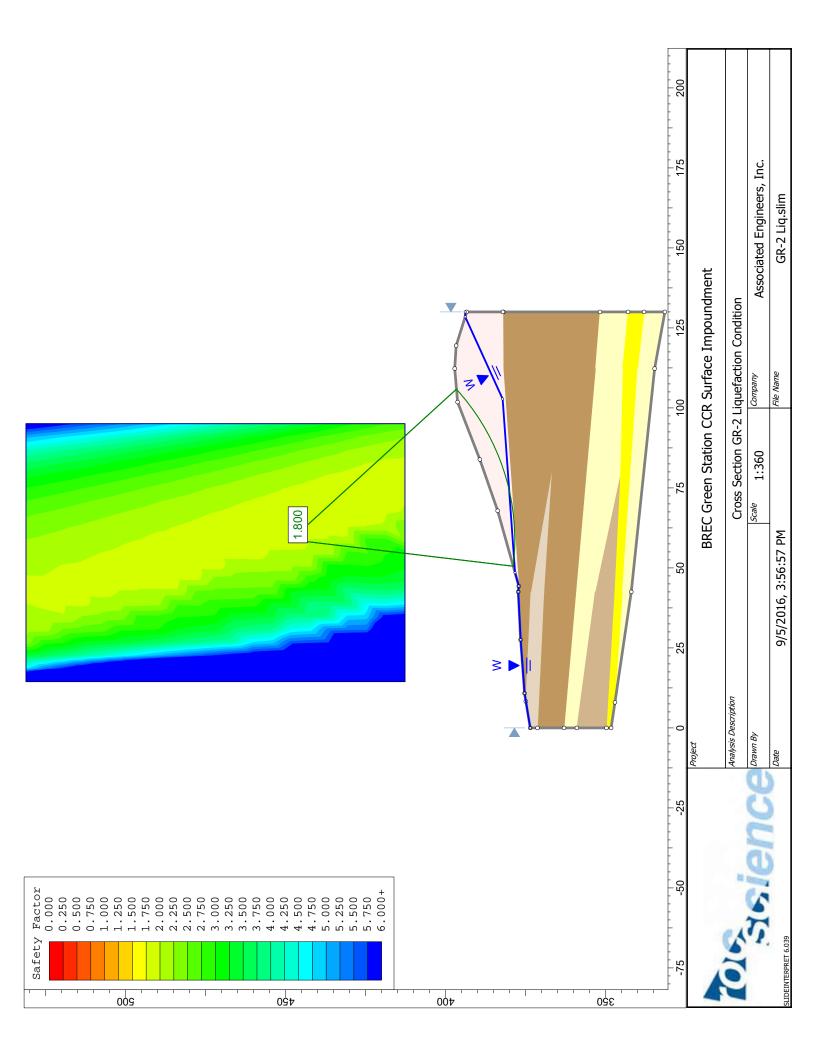
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

# **Material Boundary**

х	Y
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	Project	BREC Gr	een Station CCI	R Surface I	mpoundment
(0)6	Analysis Description Cross Section GR-2 Liquefaction Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	М	File Name	GR-2 Liq.slim





# **Green Station CCR Surface Impoundment**

Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

October 11, 2016

#### **Prepared By:**



Project ID: 160028A

# Big Rivers Electric Corporation Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

#### **CCR Surface Impoundment Information**

Name: Green Station CCR Surface Impoundment

Operator: Sebree Generating Station

Address: 9000 Highway 2096

Robards, Kentucky 42452

CCR Unit Identification Number: Kentucky State Dam Inventory System ID No. 0980

#### **Qualified Professional Engineer**

Name: David A. Lamb

Company: Associated Engineers, Inc.

Kentucky P.E. Number: 17822

#### **Regulatory Applicability**

As part of the § 257.73 Structural integrity criteria for existing CCR surface impoundments requirements, an owner or operator of an existing CCR surface impoundment must no later than October 17, 2016:

Conduct an initial safety factor assessment for each CCR unit and document whether the calculated factors of safety for each CCR unit achieve the minimum safety factors specified below for the critical cross section of the embankment. The critical cross section is the cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments must be supported by appropriate engineering calculations, documenting whether the CCR unit achieves the following minimum factors of safety:

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.40.
- 3. The calculated seismic factor of safety must equal or exceed 1.00.

4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

#### From: VI. Development of the Final Rule - Technical Requirements

#### General Safety Factor Assessment Considerations

Generally accepted engineering methodologies specify that the determination of the structural stability factors of safety specified above is to be calculated by the qualified professional engineer using conventional analysis procedures or, if necessary, special analysis procedures. Conventional analysis procedures include, but are not limited to, limit equilibrium methods of slope stability analysis, whereas, special analysis procedures include, but are not limited to, finite element methods, finite difference methods, three-dimensional methods, or probabilistic methods. Whichever methodology is used to determine the factors of safety of the CCR surface impoundment, the qualified professional engineer must document the methodology used, as well as the basis for using that methodology, and the analysis must be supported by appropriate engineering calculations.

# The Calculated Static Factor of Safety Under the Long-Term, Maximum Storage Pool Loading Condition

It is generally accepted practice to analyze the stability of the downstream slope of the dam embankment for steady-state seepage (or steady seepage) conditions with the reservoir at its normal operating pool elevation (usually the spillway crest elevation) since this is the loading condition the embankment will experience most. This condition is called steady seepage with maximum storage pool. The maximum storage pool loading is the maximum water level that can be maintained that will result in the full development of a steady-state seepage condition. Maximum storage pool loading conditions need to be calculated to ensure that the CCR surface impoundment can withstand a maximum expected pool elevation with full development of saturation in the embankment under long-term loading. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum storage pool loading condition meet or exceed 1.5.

#### The Calculated Static Factor of Safety Under the Maximum Surcharge Pool Loading Condition

The maximum surcharge pool loading condition is calculated to evaluate the effect of a raised level (e.g., flood surcharge) on the stability of the downstream slope. This ensures that the CCR surface impoundment can withstand a temporary rise in pool elevation above the maximum storage pool elevation for which the CCR surface impoundment may normally be subject under inflow design flood stage, for a short-term until the inflow design flood is passed through the CCR surface impoundment. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum surcharge pool loading condition meet or exceed 1.4.

#### The Calculated Seismic Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold must meet a seismic factor of safety equal to or greater than 1.0. All CCR surface

impoundments must also be capable of withstanding a design earthquake without damage to the foundation or embankment that would cause a discharge of its contents. To further support the location criteria established in this rule, CCR surface impoundments and any lateral expansion exceeding a specific height and/or volume threshold must be assessed under seismic loading conditions for a seismic loading event with a 2% probability of exceedance in 50 years, equivalent to a return period of approximately 2,500 years, based on the USGS seismic hazard maps for seismic events with this return period for the region where the CCR unit is located. EPA chose the 2% exceedance probability in 50 years event based on its common use in seismic design criteria throughout engineering.

#### The Calculated Liquefaction Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold and have been determined to contain soils susceptible to liquefaction must meet a liquefaction factor of safety equal to or greater than 1.20. A prudent engineering analysis of structural stability also includes a liquefaction potential analysis and analysis of post-liquefaction static factors of safety. As discussed previously, liquefaction is a phenomenon which typically occurs in loose, saturated or partially-saturated soils in which the effective stress of the soils reduces to zero, corresponding to a total loss of shear strength of the soil. The most common occurrence of liquefaction is in loose soils, typically sands. The liquefaction FOS determination in the final rule is used to determine if a CCR unit would remain stable if the soils of the embankment of the CCR unit were to experience liquefaction. Liquefaction analysis is only necessary in instances where CCR surface impoundments show, through representative soil sampling, construction documentation, or anecdotal evidence from personnel with knowledge of the CCR unit's construction, that soils of the embankment are susceptible to liquefaction.

# <u>Failure To Demonstrate Minimum Safety Factors or Failure To Complete a Timely Safety Factor Assessment</u>

As previously discussed, the rule requires an owner or operator to document that the calculated factors of safety for each CCR surface impoundment achieve the minimum safety factors specified in the rule. For any CCR surface impoundment that does not meet these requirements, the owner or operator must either take any engineering measure necessary to ensure that the unit meets the requirements by the rule's deadlines, or cease placement of CCR and non-CCR waste into the unit and initiate closure of such CCR unit as provided in section 257.102 within six months. Similarly, if an owner or operator fails to complete the initial safety factor assessment or any subsequent periodic factor safety assessment by the deadlines established in the rule, the owner or operator must cease placing CCR and non-CCR waste into the unit and initiate closure within six months.

#### **Description of Impoundment**

An aerial photo of the CCR unit is provided as Attachment A and an excerpt from U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps showing the location of the CCR unit is provided as Attachment B.

The CCR unit has been in place for 40 plus years The CCR unit operator has general maintenance and repair procedures in place as they determine necessary. There are no known occurrences of structural instability of the CCR unit.

The CCR unit has been in place for 40 plus years. The CCR unit is used for the placement of coal combustion residual material; currently slurried bottom ash. The immediate watershed that drains to the CCR unit, and in which the CCR unit is considered to be located, is unnamed and 54.13 acres in size. The unnamed watershed discharges from the CCR impoundment outflow structure and is routed to the Green River.

The CCR unit is a combined incised/earthen embankment structure. Embankments form the west, south and east sides of the impoundment and the north side is incised. The Green River is located approximately 400 feet east of the structure. Due to surface relief, only the toe area of the south dike is potentially subject to flooding. The predominant features were small stream valleys draining eastward to the Green River. Most of the central portion of the south dike was constructed on a subdued ridge. The toe of the outboard slope intersected a lower drainage area. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands.

The west dike is generally less than five feet in height and the south dike reaches a maximum height of 19.5 feet. The east dike reaches a maximum height of approximately eight feet and is buttressed with a secondary parallel embankment that serves as a 40-foot wide roadway. The Burns and Roe, Inc. Engineering and Consultants June 30, 1978 site grading plans show the original construction layout and ground contours for the impoundment site. Bottom ash has been placed above the normal pool along the inboard side, essentially creating reclaimed land

Depth of impounded water and CCR is 16 feet and 46 feet (at respective locations of maximum impounded water and CCR depths). Elevation of impounded water and CCR is 394 feet and 408 feet, respectively, above mean sea level. These approximate depths and respective elevations are based on the most recent (December 2015) flight derived topographic contours and bathymetric survey data.

The remaining storage capacity is approximately 172,000 cubic yards (if CCR can be placed to the elevation of the current water surface). This volume was calculated based on the maximum allowable storage volume and the current volume of CCR stored in the facility based on the most recent bathymetric survey.

The approximate volume of impounded water and CCR is 981,000 cubic yards (approximate water volume is 172,000 cubic yards and approximate CCR volume is 809,000 cubic yards). This volume was calculated based on the maximum storage capacity, the current amount of CCR stored in the facility based on the most recent bathymetric survey, and the best available as-built data for the structure construction prior to placement of CCR.

The impoundment discharge consists of two corrugated steel pipes, each 30 inches in diameter. The pipe intakes are through a concrete common headwall collection structure with a variable height steel debris deflector on each pipe intake.

#### **Calculated Safety Factors**

Results of the initial safety factor assessment for the critical cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments are supported by appropriate engineering calculations.

The safety factor analysis was conducted using the Rocscience Inc. Slide geotechnical software by evaluating four cross sections along the embankment. Slide provides numerical tools to analyze the stability of embankments using limit equilibrium methods. At each cross section, drilling, surveying, laboratory testing, and a slope stability analysis were performed. Based on the four analysis scenarios, the lowest factor of safety for all scenarios was not found at a single cross section; therefore, the results listed below are the lowest factor of safety realized from all analyzed cross sections for each scenario. The safety factor assessments are supported by appropriate engineering calculations and the Slide modeling results for the Green CCR impoundment are attached to this report.

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition equals: 1.800
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition equals: 1.800
- 3. The calculated seismic factor of safety equals: 1.002
- 4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety equals: 1.800

#### **Sources of Information**

Geotechnical and other information provided by Associated Engineers, Inc.

Engineering design drawings and other information provided by Big Rivers Electric Corporation

United States Geological Survey U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps

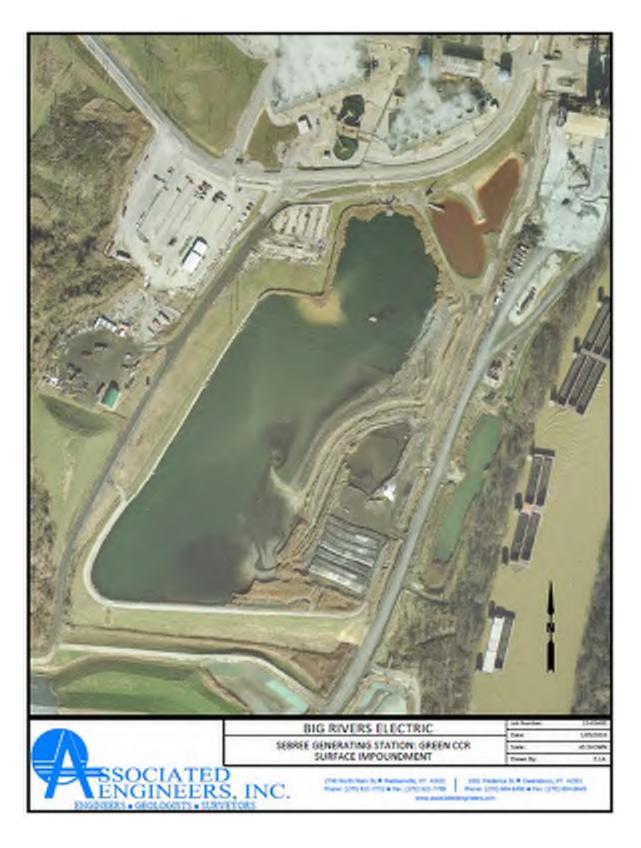
# Professional Engineer Certification [Per 40 CFR § 257.73] Green CCR Impoundment Initial Safety Factor Assessment

I hereby certify that myself or an agent under my review has prepared this Initial Safety Factor Assessment (Assessment), and being familiar with the provisions of the final rule to regulate the disposal of coal combustion residuals (CCR) as solid waste under subtitle D of the Resource Conservation and Recovery Act (RCRA), attest that this Assessment has been prepared in accordance with good engineering practices and meets the intent of 40 CFR Part 257.73. To the best of my knowledge and belief, the information contained in this Assessment is true, complete, and accurate.

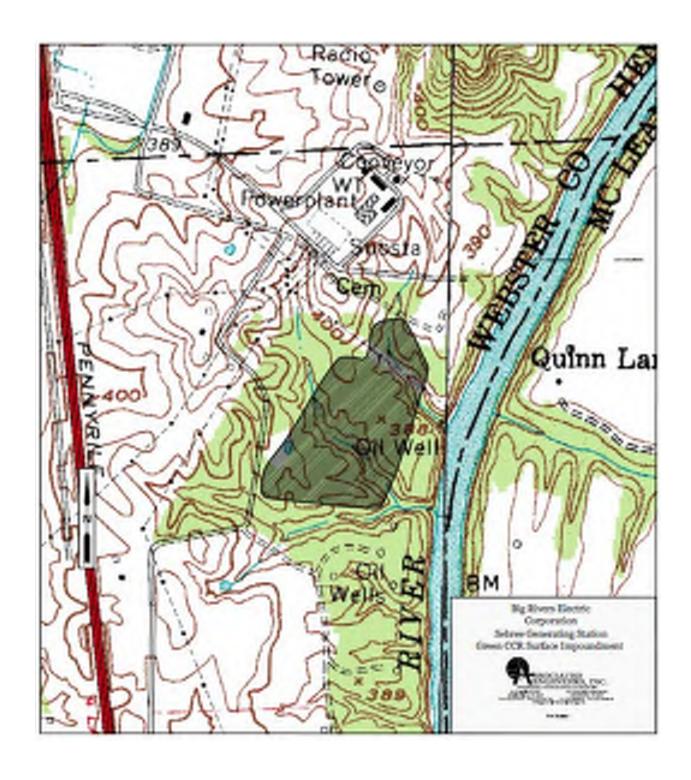
David A. Lamb

State of Kamercky Liconse 16. 17822

Date: 10/11/16



Attachment A. Aerial Photo of the Green CCR Surface Impoundment



Attachment B. Topographic Map showing the Green CCR Surface Impoundment

# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-1

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



	BREC Green Station CCR Surface Impoundment				
-	Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition			e Pool Loading Condition	
e	Drawn By		Scale	Company	Associated Engineers, Inc
	Date	9/5/2016, 3:43:18 Pi	М	File Name	GR-1.slim

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Sandy Lean Clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Y
379.185
379.172
377.695
376.221
375.6
376.406
377.906
378.317
378.983

	Projec	t
-		
(0)	Analys	sis
sisien	Drawn	7 E
SLIDE 6 030	Date	

	BREC Green Station CCR Surface Impoundment				
	Analysis Description Cross Section GR-1 Long-Term Maximum Storage Pool Loading Condition			ge Pool Loading Condition	
9	Drawn By		Scale	Company	Associated Engineers, Inc
	Date	0/5/2016 3:43:18 D	М	File Name	GP_1 clim

Date 9/5/2016, 3:43:18 PM GR-1.slim

85.7592	379.391
140.9	382.48
170.48	393.92

# **External Boundary**

Х	Υ
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
170.48	376.396
170.48	393.967
161.556	396.809
153.137	396.9
143.057	396.719
119.757	389.859
96.35	382.029
84.12	378.983
76.996	378.317
70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

X	Y
0	353.7
55	353.7
77	356.8
153	364.9

	Project	BREC Gr	BREC Green Station CCR Surface Impoundment		
(0)6	Analysis Description	Cross Section GR-1	Long-Term Maxir	num Storage	Pool Loading Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date	9/5/2016, 3:43:18 Pf	М	File Name	GR-1.slim

170.48 366.698

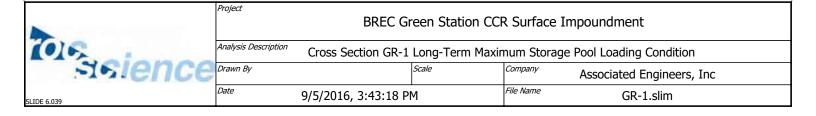
#### **Material Boundary**

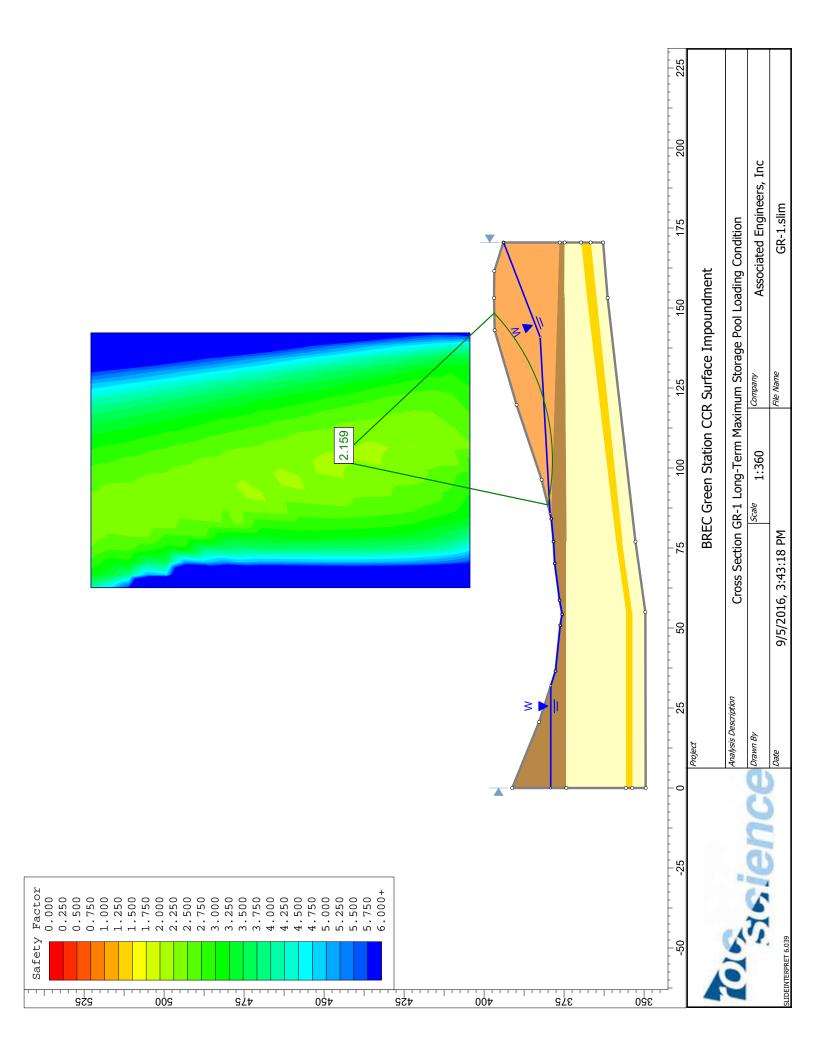
Х	Υ
0	355.7
55	355.7
77	358.8
153	367.9
170.48	369.698

#### **Material Boundary**

Х	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

X	Υ
84.12	378.983
153	376.9
170.48	376.396





# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-1 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-1 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



		BREC Gr	een Station CCF	R Surface	Impoundment
	Analysis Description	Cross Section (	GR-1 Maximum Sı	ırcharge F	Pool Loading Condition
9	Drawn By	Scale Company Associated Engineers, Inc			Associated Engineers, Inc
	<sup>Date</sup> 9/5/2016, 3:43:18 PM		File Name	GR-1 Surcharge.slim	

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean clay (CL)	Sandy lean clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Y
0	379.185
31.955	379.172
36.551	377.695
50.832	376.221
54.305	375.6
58.684	376.406
70.171	377.906
76.996	378.317
84.12	378.983

	BREC Green Station CCR Surface Impoundment			
(0)6	Analysis Description Cross Section (	GR-1 Maximum Su	ırcharge Pool	Loading Condition
sience	Drawn By	Scale	Company	Associated Engineers, Inc
SLIDE 6.039	<sup>Date</sup> 9/5/2016, 3:43:18 PI	М	File Name	GR-1 Surcharge.slim

85.7592 379.391 140.9 382.48 165.719 395.483 170.48 395.48

# **External Boundary**

х	Y
0	349.5
55	349.6
77	352.7
153.14	361.4
170.48	362.8
170.48	366.698
170.48	369.698
170.48	374.816
170.48	376.396
170.48	393.967
161.556	396.809
153.137	396.9
143.057	396.719
119.757	389.859
96.35	382.029
84.12	378.983
76.996	378.317
70.171	377.906
58.684	376.406
54.305	375.6
50.832	376.221
36.551	377.695
20.651	382.805
0	391.212
0	374.3
0	355.7
0	353.7

X	Υ
0	353.7
55	353.7
77	356.8



	BREC Green Station CCR Surface Impoundment						
	Analysis Description	Cross Section GR-1 Maximum Surcharge Pool Loading Condition					
e	Drawn By		Scale	Company	Associated Engineers, Inc		
	Date 9/5/2016, 3:43:18 PM		File Name	GR-1 Surcharge.slim			

153 364.9 170.48 366.698

# **Material Boundary**

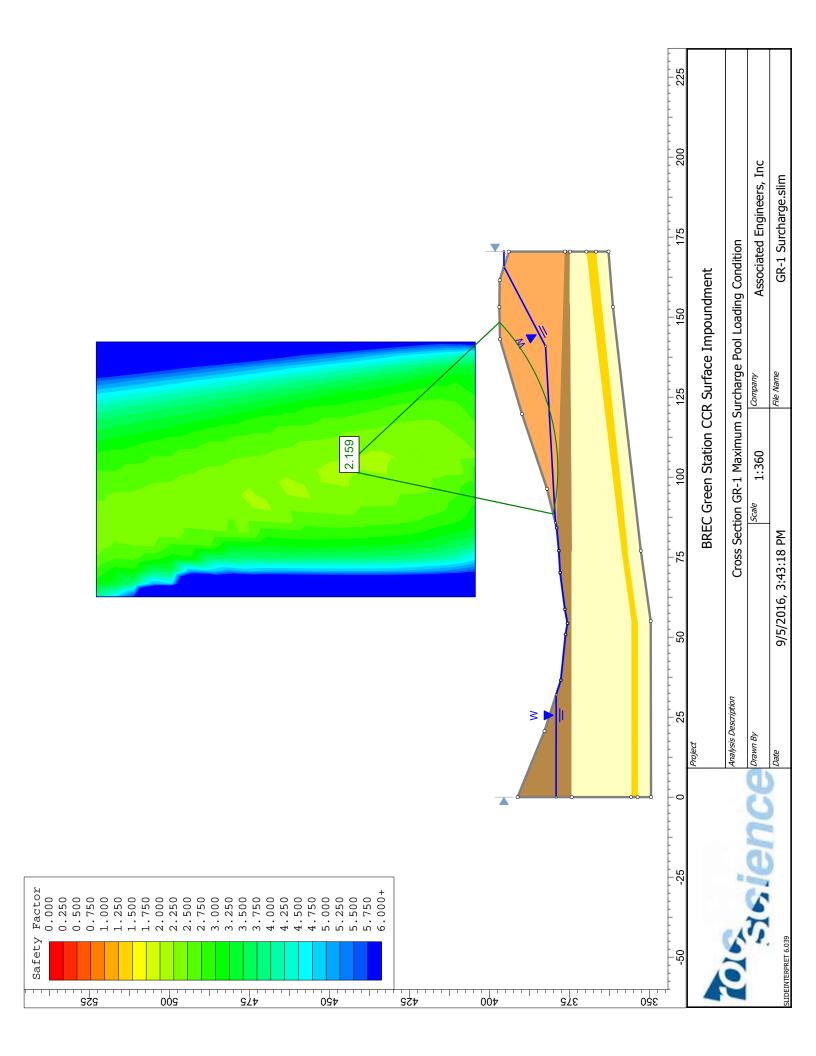
X	Υ		
0	355.7		
55	355.7		
77	358.8		
153	367.9		
170.48	369.698		

# **Material Boundary**

Х	Υ		
0	374.3		
77	374.3		
153	374.9		
170.48	374.816		

Х	Y		
84.12	378.983		
153	376.9		
170.48	376.396		

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-1 Maximum Surcharge Pool Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date	9/5/2016, 3:43:18 P	М	File Name	GR-1 Surcharge.slim



# Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-1 Seis

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment Analysis: Cross Section GR-1 Seismic Loading Condition

Company: Associated Engineers, Inc Date Created: 9/5/2016, 3:43:18 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

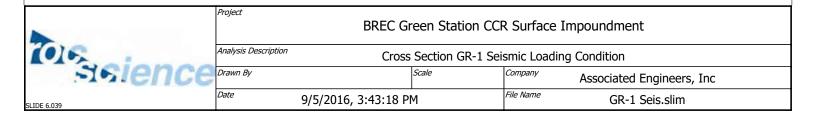
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

# **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

# Loading

Seismic Load Coefficient (Horizontal): 0.2364

# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean clay (CL)	Sandy Lean Clay With Gravel (CL) (Dam)
Color				
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	134.3	135.4	137.69
Cohesion [psf]	316.8	403.2	820.8	72
Friction Angle [deg]	24.3	30.2	24.6	27.4
Water Surface	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Υ		
0	379.185		
31.955	379.172		
36.551	377.695		
50.832	376.221		
54.305	375.6		

	BREC Green Station CCR Surface Impoundment				
(0)6	Inalysis Description Cross Section GR-1 Seismic Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc
SLIDE 6.039	Date (	9/5/2016, 3:43:18 PM		File Name	GR-1 Seis.slim

58.684 376.406 70.171 377.906 76.996 378.317 84.12 378.983 85.7592 379.391 140.9 382.48 170.48 393.92

#### **External Boundary**

Х	Y		
0	349.5		
55	349.6		
77	352.7		
153.14	361.4		
170.48	362.8		
170.48	366.698		
170.48	369.698		
170.48	374.816		
170.48	376.396		
170.48	393.967		
161.556	396.809		
153.137	396.9		
143.057	396.719		
119.757	389.859		
96.35	382.029		
84.12	378.983		
76.996	378.317		
70.171	377.906		
58.684	376.406		
54.305	375.6		
50.832	376.221		
36.551	377.695		
20.651	382.805		
0	391.212		
0	374.3		
0	355.7		
0	353.7		

# **Material Boundary**

X Y



	BREC Green Station CCR Surface Impoundment					
	Analysis Description	Cross Section GR-1 Seismic Loading Condition				
e	Drawn By		Scale	Company	Associated Engineers, Inc	
	<sup>Date</sup> 9/5/2016, 3:43:18 PM		File Name	GR-1 Seis.slim		

0	353.7
55	353.7
77	356.8
153	364.9
170.48	366.698

Х	Υ
0	355.7
55	355.7
77	358.8
153	367.9
170.48	369.698

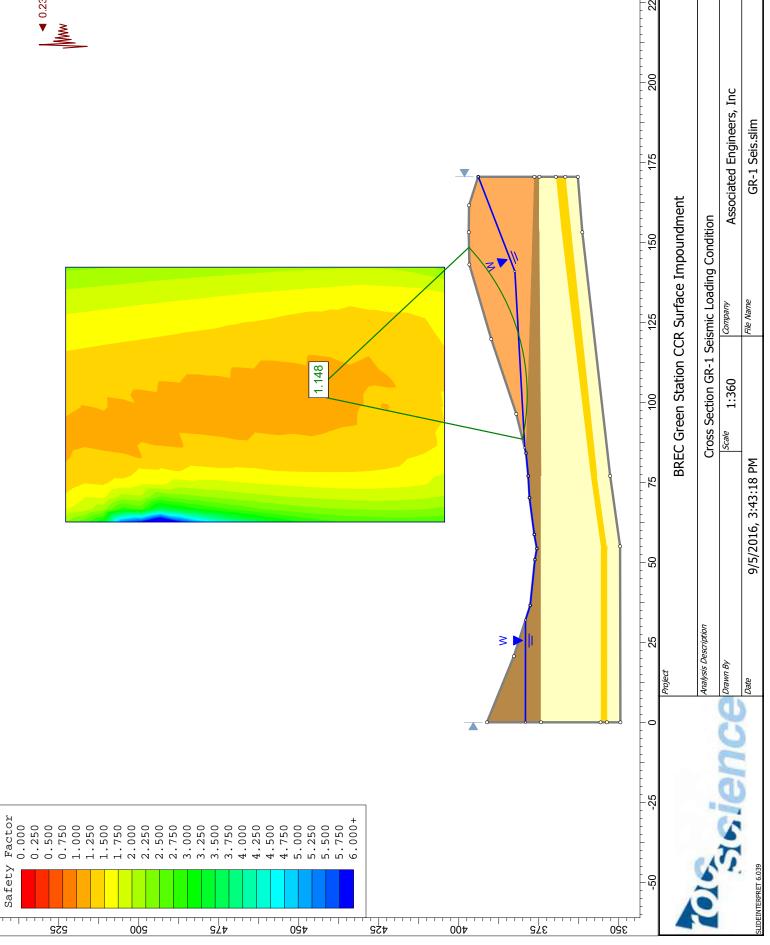
## **Material Boundary**

X	Υ
0	374.3
77	374.3
153	374.9
170.48	374.816

Х	Υ
84.12	378.983
153	376.9
170.48	376.396

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-1 Seismic Loading Condition				
sience	Drawn By	Scale Company Associated Engineers, Inc			
SLIDE 6.039	Date	9/5/2016, 3:43:18 PM		File Name	GR-1 Seis.slim

Safety



## Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

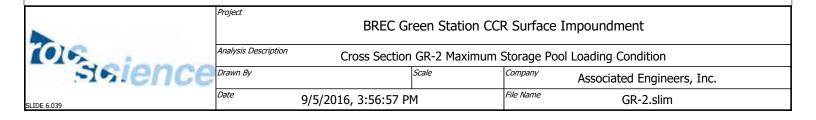
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

х	Υ
0	373.571
10.818	375.37
27.54	376.571
42.515	377.335
44.31	377.146
48.515	378.327
102.8	382.2

	Project BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Maximum Storage Pool Loading Condition			Loading Condition	
sience	Drawn By	Scale Company Associated Engineers, Inc.			
SLIDE 6.039	<i>Date</i> 9	9/5/2016, 3:56:57 PM		File Name	GR-2.slim

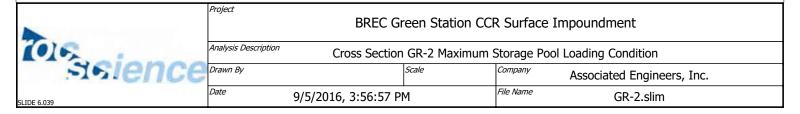
128.457 393.92 130 393.92

#### **External Boundary**

V	V
X	Y
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

#### **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

Х	Υ
0	359
42.5	353.3
79	345.5

## **Material Boundary**

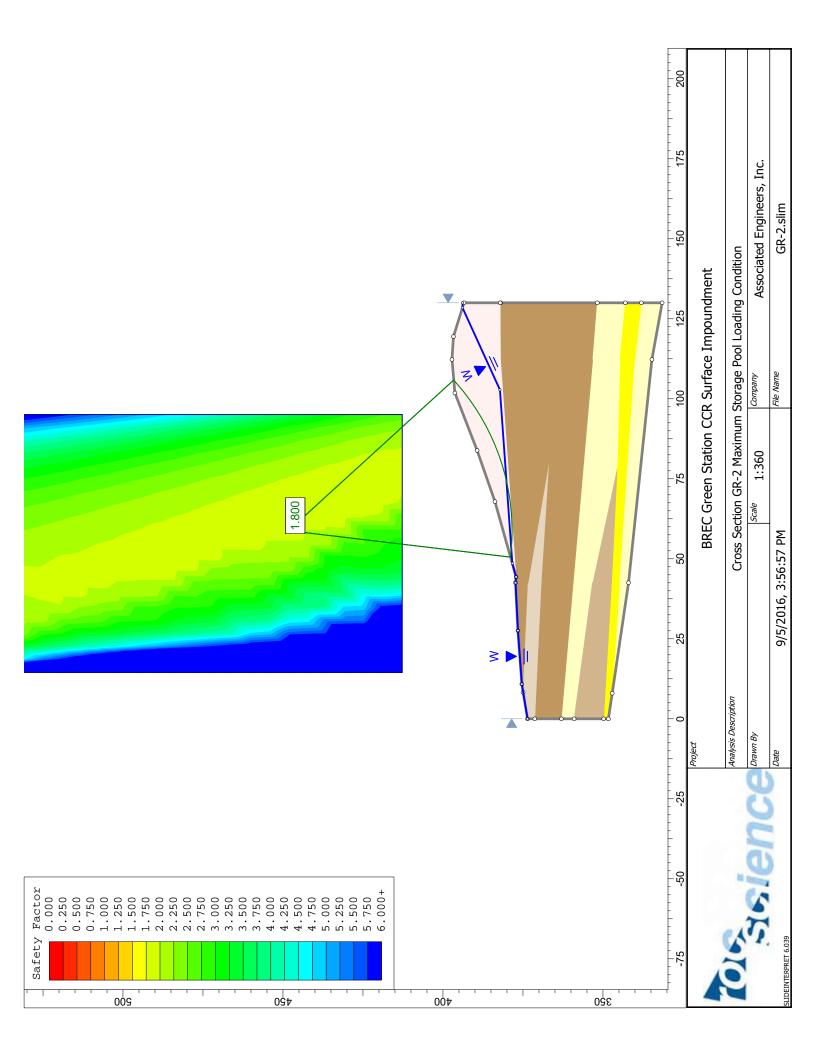
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

## **Material Boundary**

х	Υ
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR Surface Impoundment						
(0)6	Analysis Description Cross Section GR-2 Maximum Storage Pool Loading Condition						
sience	Drawn By	Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM		File Name	GR-2.slim		



## Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

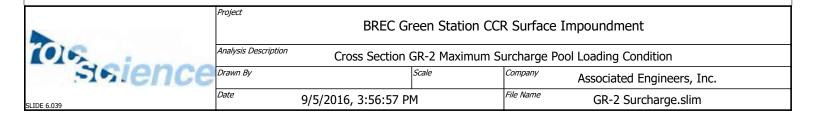
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

X	Y
0	373.571
10.818	375.37
27.54	376.571
42.515	377.335
44.31	377.146
48.515	378.327
102.8	382.2

	BREC Green Station CCR Surface Impoundment				npoundment
7016	Analysis Description Cross Section GR-2 Maximum Surcharge Pool Loading Condition			l Loading Condition	
sience	Prawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date 9/	5/2016, 3:56:57 Pl	M	File Name	GR-2 Surcharge.slim

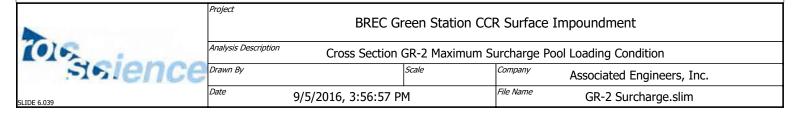
123.45 395.483 130 395.48

#### **External Boundary**

Х	Υ
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

#### **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

Х	Υ
0	359
42.5	353.3
79	345.5

## **Material Boundary**

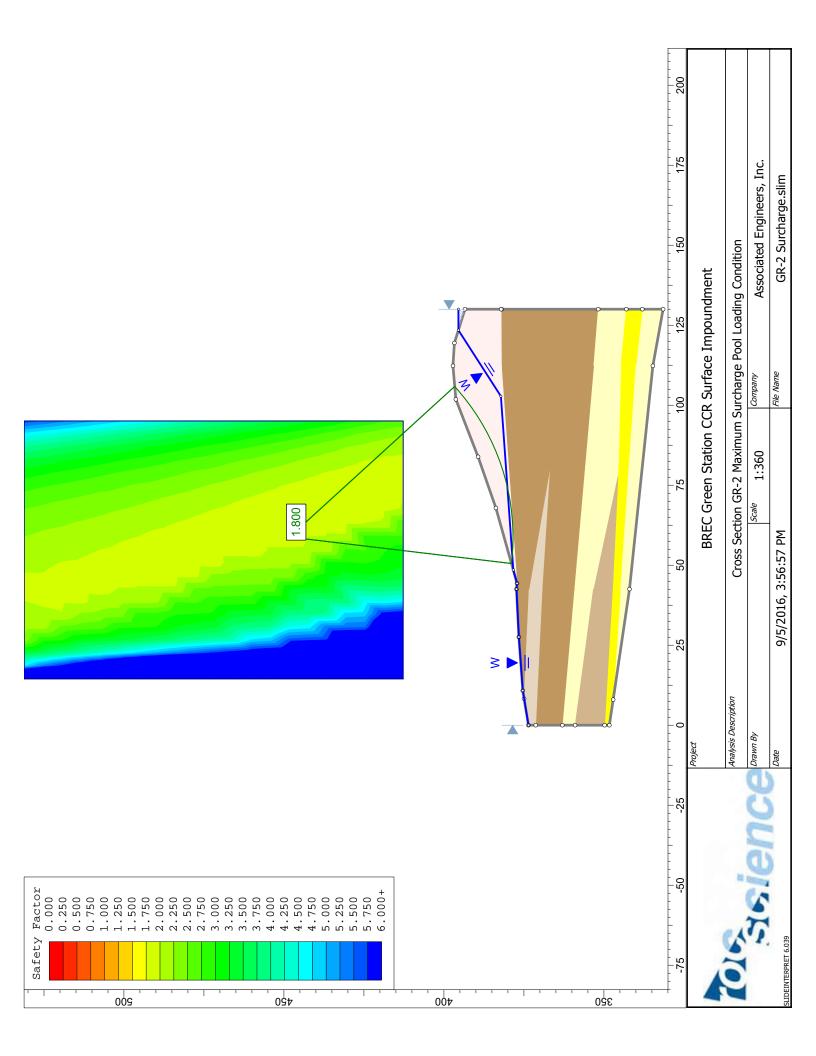
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

## **Material Boundary**

х	Υ
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR Surface Impoundment  Analysis Description Cross Section GR-2 Maximum Surcharge Pool Loading Condition				mpoundment
(0)6					ol Loading Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	9/5/2016, 3:56:57 PM		File Name	GR-2 Surcharge.slim	



## Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Seis 2

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment Analysis: Cross Section GR-2 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

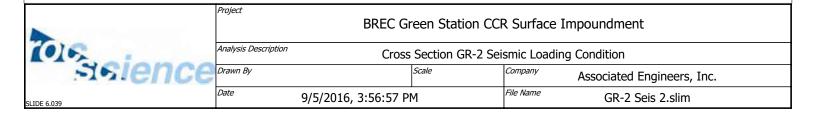
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

## **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## Loading

Seismic Load Coefficient (Horizontal): 0.2364

## **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	33	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

х	Υ
0	373.571
10.818	375.37

	Analysis Description  Cross Section GR-2 Seismic Loading Condition  Drawn By  Scale  Company  Associated Engineers, Inc.			
(0)6				
sience				
SLIDE 6.039	<sup>Date</sup> 9/5/2016, 3	9/5/2016, 3:56:57 PM		GR-2 Seis 2.slim

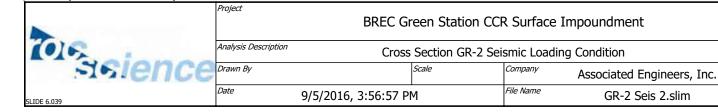
27.54 376.571 42.515 377.335 44.31 377.146 48.515 378.327 102.8 382.2 128.457 393.92 130 393.92

#### **External Boundary**

Х	Υ
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

#### **Material Boundary**

X Y 7.98633 347.116



42.5	344.8
112.2	340
130	338

Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

## **Material Boundary**

Х	Υ
0	359
42.5	353.3
79	345.5

#### **Material Boundary**

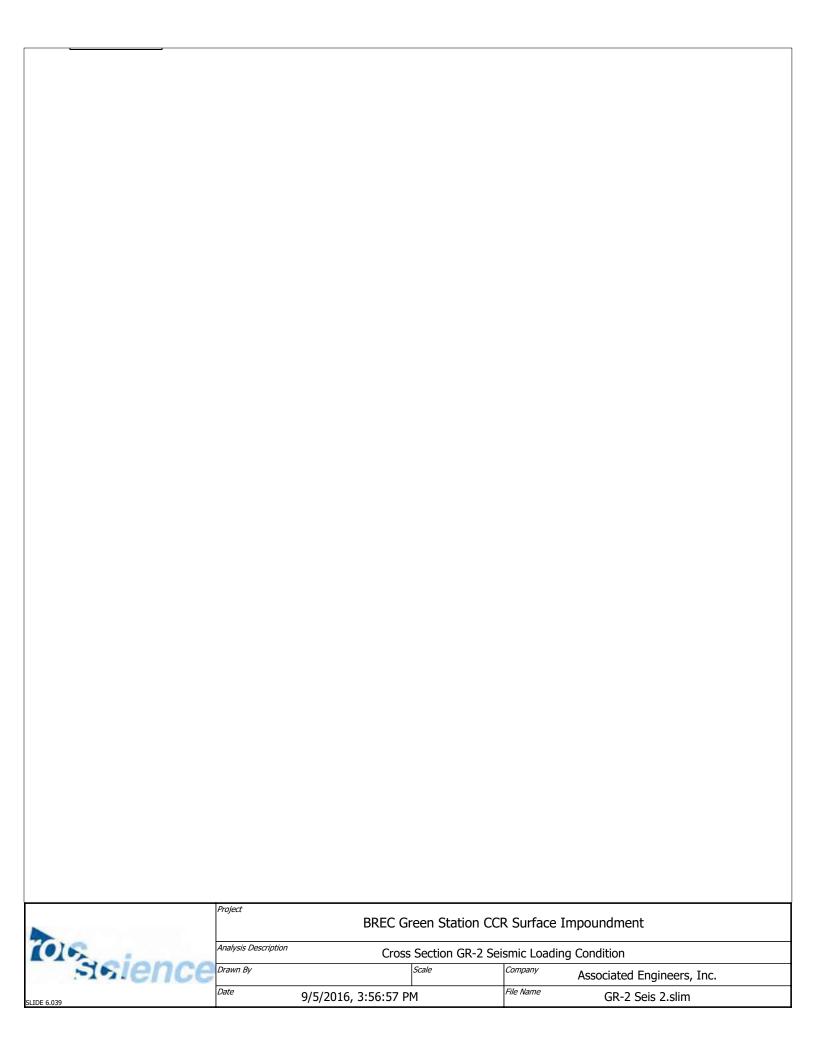
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

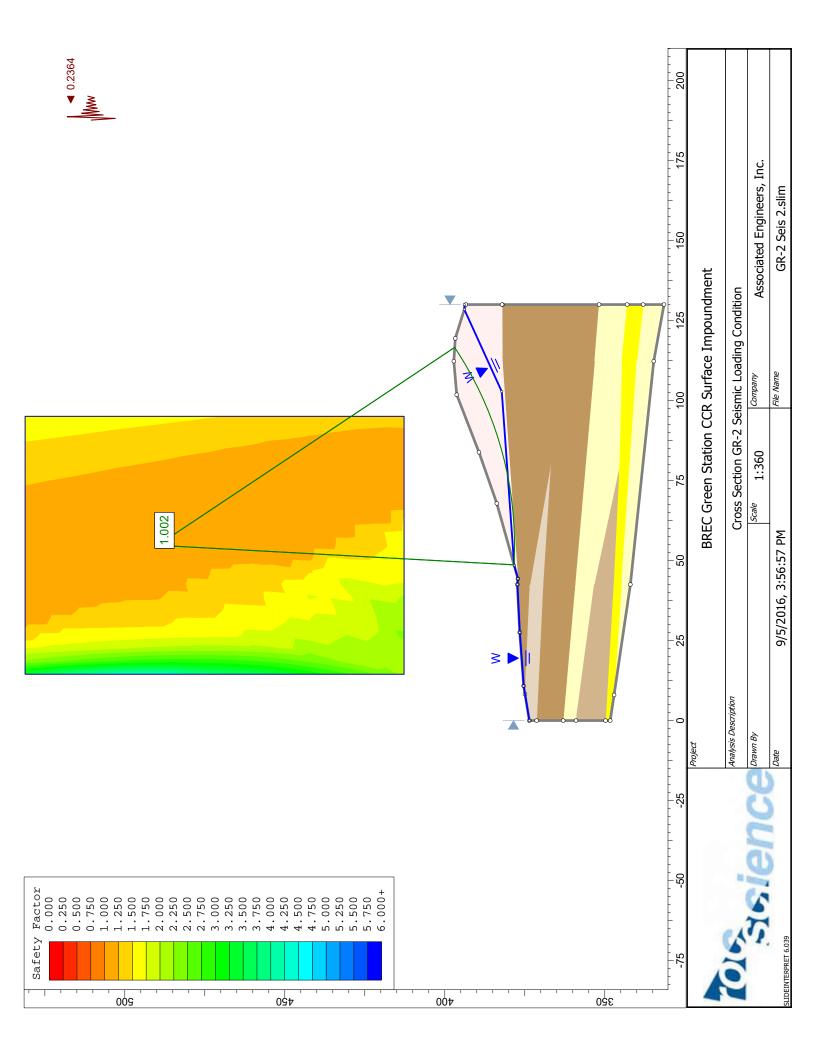
## **Material Boundary**

х	Υ
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Seismic Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	4	File Name	GR-2 Seis 2.slim





## Slide Analysis Information BREC Green Station CCR Surface Impoundment

#### **Project Summary**

File Name: GR-2 Liq

Last saved with Slide version: 6.039

Project Title: BREC Green Station CCR Surface Impoundment

Analysis: Cross Section GR-2 Liquefaction Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:56:57 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

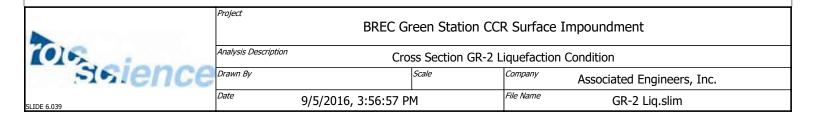
Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None



#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Poorly Graded Sand With Silt (SP-SM)	Lean Clay with Sand (CL)	Lean clay (CL)	Lean Clay With Sand 2	Lean clay With Sand (Dam)
Color						
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128.1	126	131.9	135.4	126.6	140.6
Cohesion [psf]	316.8	0	374	820.8	0	72
Friction Angle [deg]	24.3	0	27.7	24.6	30.8	24.1
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

х	Υ
0	373.571
10.818	375.37
27.54	376.571
42.515	377.335
44.31	377.146
48.515	378.327
102.8	382.2

	Project	BREC Gr	een Station CCI	R Surface I	mpoundment
(0)6	Analysis Description Cross Section GR-2 Liquefaction Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:56:57 PI	М	File Name	GR-2 Liq.slim

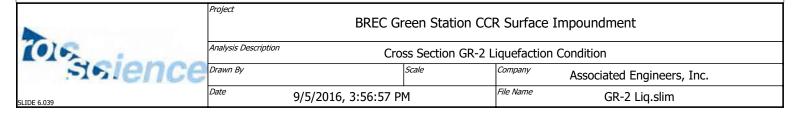
128.457 393.92 130 393.92

#### **External Boundary**

х	Y
3.298e-013	348.3
7.98633	347.116
42.51	342
112.29	334.7
130	331.5
130	338
130	343
130	351.773
130	382.006
130	382.1
130	393.439
119.451	396.731
112.288	397.189
101.806	396.285
83.858	389.332
67.857	383.758
44.31	377.146
42.515	377.335
27.54	376.571
10.818	375.37
8.27071	374.946
0	373.571
0	371.3
0	363
0	359
3.10558e-013	349.774

#### **Material Boundary**

х	Υ
7.98633	347.116
42.5	344.8
112.2	340
130	338



Х	Υ
3.10558e-013	349.774
42.5	347.3
79	345.5
112.2	344.7
130	343

Х	Υ
0	359
42.5	353.3
79	345.5

## **Material Boundary**

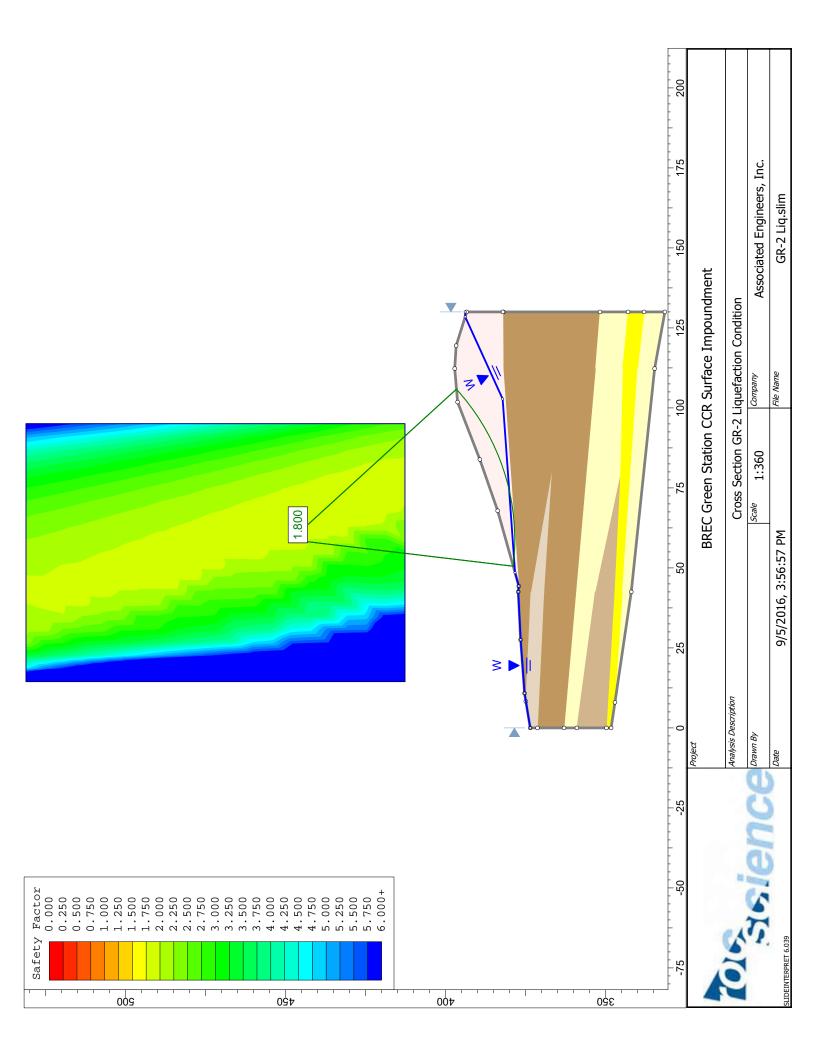
Х	Υ
0	363
42.5	359.3
112.2	353.2
130	351.773

## **Material Boundary**

х	Y
0	371.3
42.5	369.3
80	366.9
42.5	373.4
8.27071	374.946

Х	Υ
44.31	377.146
112.2	382
130	382.1

	BREC Green Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section GR-2 Liquefaction Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:56:57 PM	М	File Name	GR-2 Liq.slim





## **Reid/HMPL Station CCR Surface Impoundment**

Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

October 11, 2016

#### **Prepared By:**



Project ID: 160027A

# Big Rivers Electric Corporation Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule Structural Integrity Criteria for Existing CCR Surface Impoundments Initial Safety Factor Assessment

#### **CCR Surface Impoundment Information**

Name: Reid/HMPL Station CCR Surface Impoundment

Operator: Sebree Generating Station

Address: 9000 Highway 2096

Robards, Kentucky 42452

CCR Unit Identification Number: Kentucky State Dam Inventory System ID No. 0855

#### **Qualified Professional Engineer**

Name: David A. Lamb

Company: Associated Engineers, Inc.

Kentucky P.E. Number: 17822

#### **Regulatory Applicability**

As part of the § 257.73 Structural integrity criteria for existing CCR surface impoundments requirements, an owner or operator of an existing CCR surface impoundment must no later than October 17, 2016:

Conduct an initial safety factor assessment for each CCR unit and document whether the calculated factors of safety for each CCR unit achieve the minimum safety factors specified below for the critical cross section of the embankment. The critical cross section is the cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments must be supported by appropriate engineering calculations, documenting whether the CCR unit achieves the following minimum factors of safety:

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.40.
- 3. The calculated seismic factor of safety must equal or exceed 1.00.

4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

#### From: VI. Development of the Final Rule - Technical Requirements

#### **General Safety Factor Assessment Considerations**

Generally accepted engineering methodologies specify that the determination of the structural stability factors of safety specified above is to be calculated by the qualified professional engineer using conventional analysis procedures or, if necessary, special analysis procedures. Conventional analysis procedures include, but are not limited to, limit equilibrium methods of slope stability analysis, whereas, special analysis procedures include, but are not limited to, finite element methods, finite difference methods, three-dimensional methods, or probabilistic methods. Whichever methodology is used to determine the factors of safety of the CCR surface impoundment, the qualified professional engineer must document the methodology used, as well as the basis for using that methodology, and the analysis must be supported by appropriate engineering calculations.

## The Calculated Static Factor of Safety Under the Long-Term, Maximum Storage Pool Loading Condition

It is generally accepted practice to analyze the stability of the downstream slope of the dam embankment for steady-state seepage (or steady seepage) conditions with the reservoir at its normal operating pool elevation (usually the spillway crest elevation) since this is the loading condition the embankment will experience most. This condition is called steady seepage with maximum storage pool. The maximum storage pool loading is the maximum water level that can be maintained that will result in the full development of a steady-state seepage condition. Maximum storage pool loading conditions need to be calculated to ensure that the CCR surface impoundment can withstand a maximum expected pool elevation with full development of saturation in the embankment under long-term loading. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum storage pool loading condition meet or exceed 1.5.

#### The Calculated Static Factor of Safety Under the Maximum Surcharge Pool Loading Condition

The maximum surcharge pool loading condition is calculated to evaluate the effect of a raised level (e.g., flood surcharge) on the stability of the downstream slope. This ensures that the CCR surface impoundment can withstand a temporary rise in pool elevation above the maximum storage pool elevation for which the CCR surface impoundment may normally be subject under inflow design flood stage, for a short-term until the inflow design flood is passed through the CCR surface impoundment. The final rule requires that the calculated static factor of safety for the critical cross section of the CCR surface impoundment under the long-term maximum surcharge pool loading condition meet or exceed 1.4.

#### The Calculated Seismic Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold must meet a seismic factor of safety equal to or greater than 1.0. All CCR surface impoundments must also be capable of withstanding a design earthquake without damage to

the foundation or embankment that would cause a discharge of its contents. To further support the location criteria established in this rule, CCR surface impoundments and any lateral expansion exceeding a specific height and/or volume threshold must be assessed under seismic loading conditions for a seismic loading event with a 2% probability of exceedance in 50 years, equivalent to a return period of approximately 2,500 years, based on the USGS seismic hazard maps for seismic events with this return period for the region where the CCR unit is located. EPA chose the 2% exceedance probability in 50 years event based on its common use in seismic design criteria throughout engineering.

#### The Calculated Liquefaction Factor of Safety

All CCR surface impoundments, including any lateral expansions that exceed the size threshold and have been determined to contain soils susceptible to liquefaction must meet a liquefaction factor of safety equal to or greater than 1.20. A prudent engineering analysis of structural stability also includes a liquefaction potential analysis and analysis of post-liquefaction static factors of safety. As discussed previously, liquefaction is a phenomenon which typically occurs in loose, saturated or partially-saturated soils in which the effective stress of the soils reduces to zero, corresponding to a total loss of shear strength of the soil. The most common occurrence of liquefaction is in loose soils, typically sands. The liquefaction FOS determination in the final rule is used to determine if a CCR unit would remain stable if the soils of the embankment of the CCR unit were to experience liquefaction. Liquefaction analysis is only necessary in instances where CCR surface impoundments show, through representative soil sampling, construction documentation, or anecdotal evidence from personnel with knowledge of the CCR unit's construction, that soils of the embankment are susceptible to liquefaction.

#### <u>Failure To Demonstrate Minimum Safety Factors or Failure To Complete a Timely</u> Safety Factor Assessment

As previously discussed, the rule requires an owner or operator to document that the calculated factors of safety for each CCR surface impoundment achieve the minimum safety factors specified in the rule. For any CCR surface impoundment that does not meet these requirements, the owner or operator must either take any engineering measure necessary to ensure that the unit meets the requirements by the rule's deadlines, or cease placement of CCR and non-CCR waste into the unit and initiate closure of such CCR unit as provided in section 257.102 within six months. Similarly, if an owner or operator fails to complete the initial safety factor assessment or any subsequent periodic factor safety assessment by the deadlines established in the rule, the owner or operator must cease placing CCR and non-CCR waste into the unit and initiate closure within six months.

#### **Description of Impoundment**

An aerial photo of the CCR unit is provided as Attachment A and an excerpt from U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps showing the location of the CCR unit is provided as Attachment B.

The CCR unit has been in place for 40 plus years The CCR unit operator has general maintenance and repair procedures in place as they determine necessary. There are no known occurrences of structural instability of the CCR unit.

The CCR unit is used for the placement of coal combustion residual material; currently slurried bottom ash. The immediate watershed that drains to the CCR unit, and in which the CCR unit is considered to be located, is unnamed and 25.45 acres in size. The unnamed watershed discharges from the CCR impoundment outflow structure and is routed to the Green River.

The CCR unit is a combined incised/earthen embankment structure. Embankments form the west, south and east sides of the impoundment and the north side is incised. The original terrain on which the pond was constructed generally sloped toward the west. Although the Green River is located less than 0.5 miles from the site, the structure does not extend significantly into the floodplain. Underlying preconstruction soils consisted of Loring-Grenada, Loring-Zanesville-Wellston (Henderson County) and Loring-Wellston-Zanesville (Webster County) soil associations which are generally characterized as well drained to moderately well drained soils on nearly level to sloping uplands.

The embankment reaches its greatest relief of approximately 42 feet on the west side. The Burns & McDonnell Engineering Co. October 8, 1971 design drawings show the inboard slope and central core portion of the dike to be constructed of compacted soil fill and the outboard slope to be consisted of sand fill. A sand blanket drain was designed for the outboard third of the base of the dike for the majority of the length and the plans show a crushed limestone drainage layer with a minimum thickness of 18 inches topped with a minimum six inches thick sand layer which extends across the entire width of the dike cross section in the southwest corner. The plans also show a cut-off trench in the original ground below dike crest and extending for the entire length of the dike.

Depth of impounded water and CCR is 16 feet and 39 feet (at respective locations of maximum impounded water and CCR depths). Elevation of impounded water and CCR is 426 feet and 440 feet, respectively, above mean sea level. These approximate depths and respective elevations are based on the most recent (December 2015) flight derived topographic contours and bathymetric survey data.

The remaining storage capacity is approximately 85,000 cubic yards (if CCR can be placed to the elevation of the current water surface). This volume was calculated based on the maximum allowable storage volume and the current volume of CCR stored in the facility based on the most recent bathymetric survey.

The approximate volume of impounded water and CCR is 767,000 cubic yards (approximate water volume is 85,000 cubic yards and approximate CCR volume is 682,000 cubic yards). This volume was calculated based on the maximum storage capacity, the current amount of CCR stored in the facility based on the most recent bathymetric survey, and the best available as-built data for the structure construction prior to placement of CCR.

The impoundment discharge consists of a rectangular concrete drop structure with a variable

height steel debris skimmer. The pool elevation can be controlled by adding or removing stop logs. The discharge structure connects to a 24-inch diameter smooth walled metal pipe underground conveyance.

#### **Calculated Safety Factors**

Results of the initial safety factor assessment for the critical cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments are supported by appropriate engineering calculations.

The safety factor analysis was conducted using the Rocscience Inc. Slide geotechnical software by evaluating four cross sections along the embankment. Slide provides numerical tools to analyze the stability of embankments using limit equilibrium methods. At each cross section, drilling, surveying, laboratory testing, and a slope stability analysis were performed. Based on the four analysis scenarios, the lowest factor of safety for all scenarios was not found at a single cross section; therefore, the results listed below are the lowest factor of safety realized from all analyzed cross sections for each scenario. The safety factor assessments are supported by appropriate engineering calculations and the Slide modeling results for the Reid/HMPL CCR impoundment are attached to this report.

- 1. The calculated static factor of safety under the long-term, maximum storage pool loading condition equals: 2.053
- 2. The calculated static factor of safety under the maximum surcharge pool loading condition equals: 2.052
- 3. The calculated seismic factor of safety equals: 1.075
- 4. For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety equals: 1.585

#### **Sources of Information**

Geotechnical and other information provided by Associated Engineers, Inc.

Engineering design drawings and other information provided by Big Rivers Electric Corporation

United States Geological Survey U.S. Geological Survey (USGS) 7.5 minute Robards and Delaware topographic quadrangle maps

### Professional Engineer Certification [Per 40 CFR § 257.73] Reid/HMPL CCR Impoundment Initial Safety Factor Assessment

I hereby certify that myself or an agent under my review has prepared this Initial Safety Factor Assessment (Assessment), and being familiar with the provisions of the final rule to regulate the disposal of coal combustion residuals (CCR) as solid waste under subtitle D of the Resource Conservation and Recovery Act (RCRA), attest that this Assessment has been prepared in accordance with good engineering practices and meets the intent of 40 CFR Part 257.73. To the best of my knowledge and belief, the information contained in this Assessment is true, complete, and accurate.

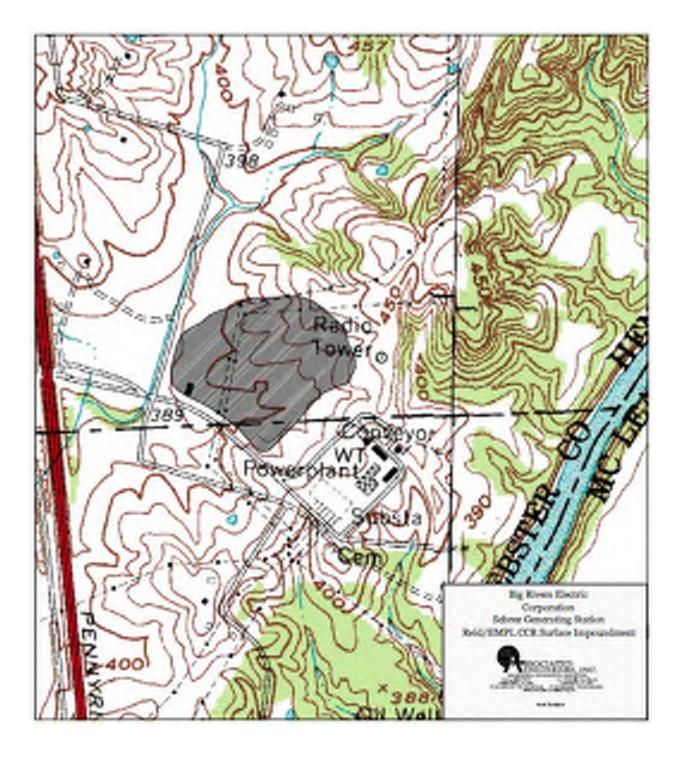
David A Lapibolino A.

State of Reglucky/bacense ₩6. 17822

Date: 10/11/16



Attachment A. Aerial Photo of the Reid/HMPL CCR Surface Impoundment



Attachment B. Topographic Map showing the Reid/HMPL CCR Surface Impoundment

## Slide Analysis Information Big Rivers Electric Corporation

#### **Project Summary**

File Name: RH-1

Last saved with Slide version: 6.039
Project Title: Big Rivers Electric Corporation

Analysis: Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	Big Rivers Electric Corporation				
(0)6	Analysis Description Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 2:31:54 PM	М	File Name	RH-1.slim

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

## **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	72	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	31	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

## **List Of Coordinates**

#### **Water Table**

х	Υ
0	392.5
30.9	392.24
85.846	396.532

	Project Big Rivers Electric Corporation				
(0)6	Analysis Description Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	Date 9/5/2016	, 2:31:54 PM	File Name	RH-1.slim	

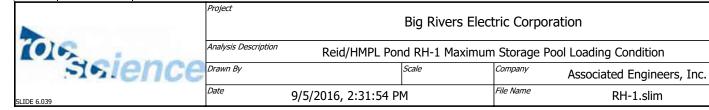
153	397.3
187.5	416.24
197.467	426.28
210	426.28

#### **External Boundary**

Х	Υ
0	371.5
43.37	372.6
180.89	373.8
210	374.1
210	381.6
210	384.6
210	388.7
210	395.8
210	397.8
210	419.644
206.306	421.6
198.751	425.6
191.136	429.632
180.794	429.579
169.909	425.6
148.327	417.711
116.257	406.72
99.1875	401.002
91.5524	398.444
85.846	396.532
83.24	397.044
77.457	398.595
71.748	398.856
47.962	398.89
43.367	398.519
32.528	396.852
18.716	394.794
0	395.066
0	393.5
0	392.5
0	384.5
0	381.5
0	372

#### **Material Boundary**

X Y



0	372
40.0	275.5
43.3	375.5
140	381.6
180.9	381.6
210	381.6

Х	Υ
0	381.5
43.3	381.5
140	381.6

#### **Material Boundary**

х	Υ
0	384.5
43.3	384.5
102.292	384.543
180	384.6
210	384.6

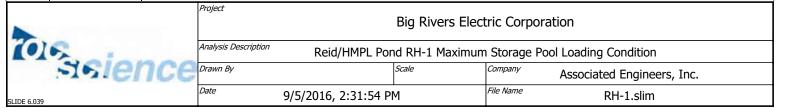
#### **Material Boundary**

х	Υ
0	392.5
43.3	392.5
102.672	391.249
180.9	389.6
210	388.7

#### **Material Boundary**

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

Х	Υ
85.846	396.532
153	397.3
180.9	397.6



Х	Y			
99.1875	401.002			
153	401.5			
153	401.333			
153	401.333			
153	397.3			

# **Material Boundary**

х	Υ			
153	401.333			
171.018	419.644			
172.943	421.6			
176.879	425.6			
180.794	429.579			

# **Material Boundary**

х	Y		
180.9	425.6		
198.751	425.6		

## **Material Boundary**

х	Y		
172.943	421.6		
180.9	421.6		

## **Material Boundary**

Х	Y	
180.9	421.6	
206.306	421.6	

х	Y		
171.018	419.644		
210	419.644		
180.9	419.6		



	Big Rivers Electric Corporation					
_	Analysis Description Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition					
e	Drawn By Scale		Company	Associated Engineers, Inc.		
	<sup>Date</sup> 9/5/2016, 2:31:54 PM		File Name	RH-1.slim		

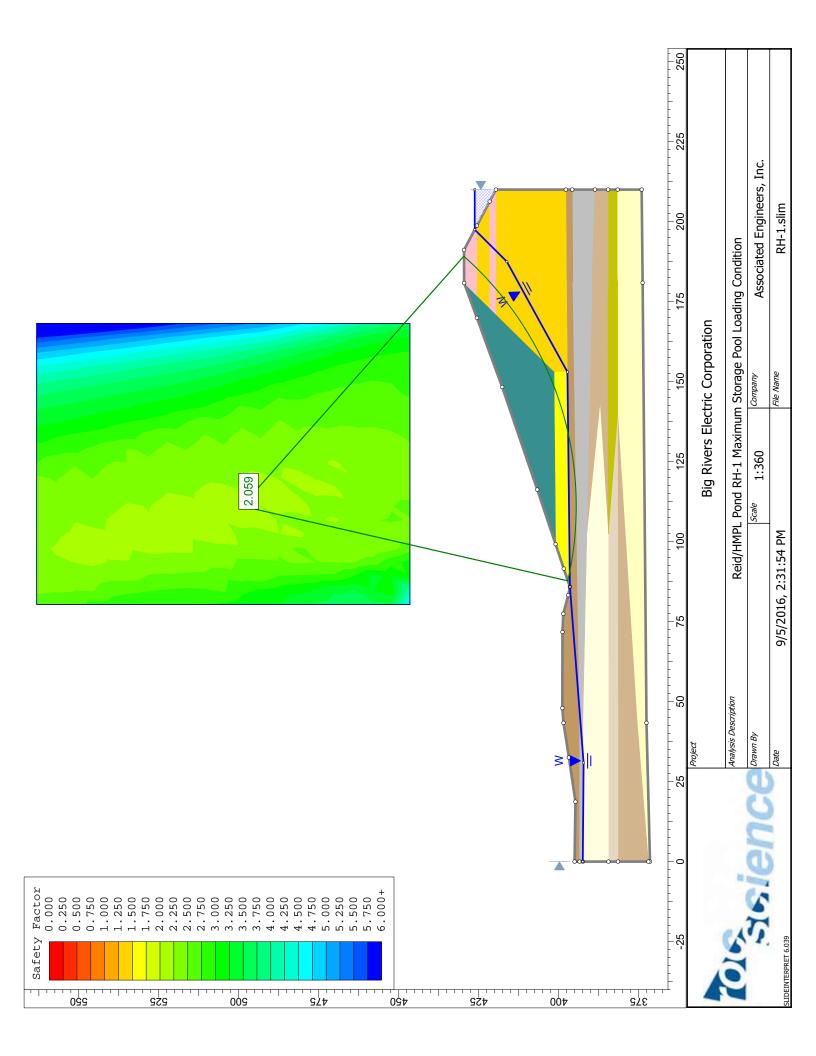
Х	Υ		
102.292	384.543		
140	381.6		

# **Material Boundary**

х	Y			
102.292	384.543			
142.735	387.164			
102.672	391.249			

х	Y	
176.879	425.6	
180.9	425.6	

	Big Rivers Electric Corporation					
(0)6	Analysis Description Reid/HMPL Pond RH-1 Maximum Storage Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date	9/5/2016, 2:31:54 PM	М	File Name	RH-1.slim	



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-1 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-1 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

## **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-1 Maximum Surcharge Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	9/5/2016, 2:31:54 PM		File Name	RH-1 Surcharge.slim		

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	72	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	31	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Υ
0	392.5
30.9	392.24
85.846	396.532

	Project BREC Reid/HMPL Station CCR Surface Impoundment			
(0)6	Analysis Description Cross Section RH-1 Maximum Surcharge Pool Loading Condition			
sience	Drawn By	Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date 9/5/2016, 2:31:	54 PM	File Name	RH-1 Surcharge.slim

153	397.3
187.5	416.24
194.956	427.61
210	427.61

# **External Boundary**

Х	Υ
0	371.5
43.37	372.6
180.89	373.8
210	374.1
210	381.6
210	384.6
210	388.7
210	395.8
210	397.8
210	419.644
206.306	421.6
198.751	425.6
191.136	429.632
180.794	429.579
169.909	425.6
148.327	417.711
116.257	406.72
99.1875	401.002
91.5524	398.444
85.846	396.532
83.24	397.044
77.457	398.595
71.748	398.856
47.962	398.89
43.367	398.519
32.528	396.852
18.716	394.794
0	395.066
0	393.5
0	392.5
0	384.5
0	381.5
0	372

# **Material Boundary**

X Y



	BREC Reid/HMPL Station CCR Surface Impoundment					
	Analysis Description	Cross Section RH-1 Maximum Surcharge Pool Loading Condition				
9	Drawn By	Scale Company Associated Engineers, Inc.				
	Date	9/5/2016, 2:31:54 Pl	 М	File Name	RH-1 Surcharge.slim	

0	372
43.3	375.5
140	381.6
180.9	381.6
210	381.6

Х	Υ
0	381.5
43.3	381.5
140	381.6

## **Material Boundary**

х	Υ
0	384.5
43.3	384.5
102.292	384.543
180	384.6
210	384.6

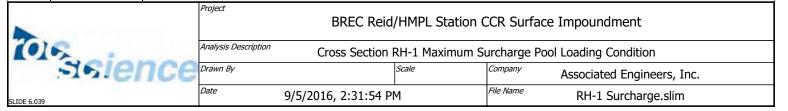
## **Material Boundary**

х	Υ
0	392.5
43.3	392.5
102.672	391.249
180.9	389.6
210	388.7

## **Material Boundary**

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

X	Υ		
85.846	396.532		
153	397.3		
180.9	397.6		



Х	Y		
99.1875	401.002		
153	401.5		
153	401.333		
153	401.333		
153	397.3		

#### **Material Boundary**

х	Y			
153	401.333			
171.018	419.644			
172.943	421.6			
176.879	425.6			
180.794	429.579			

#### **Material Boundary**

Х	Υ	
180.9	425.6	
198.751	425.6	

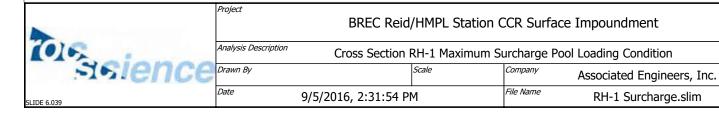
#### **Material Boundary**

х	Υ		
172.943	421.6		
180.9	421.6		

#### **Material Boundary**

Х	Υ		
180.9	421.6		
206.306	421.6		

х	Υ		
171.018	419.644		
210	419.644		
180.9	419.6		



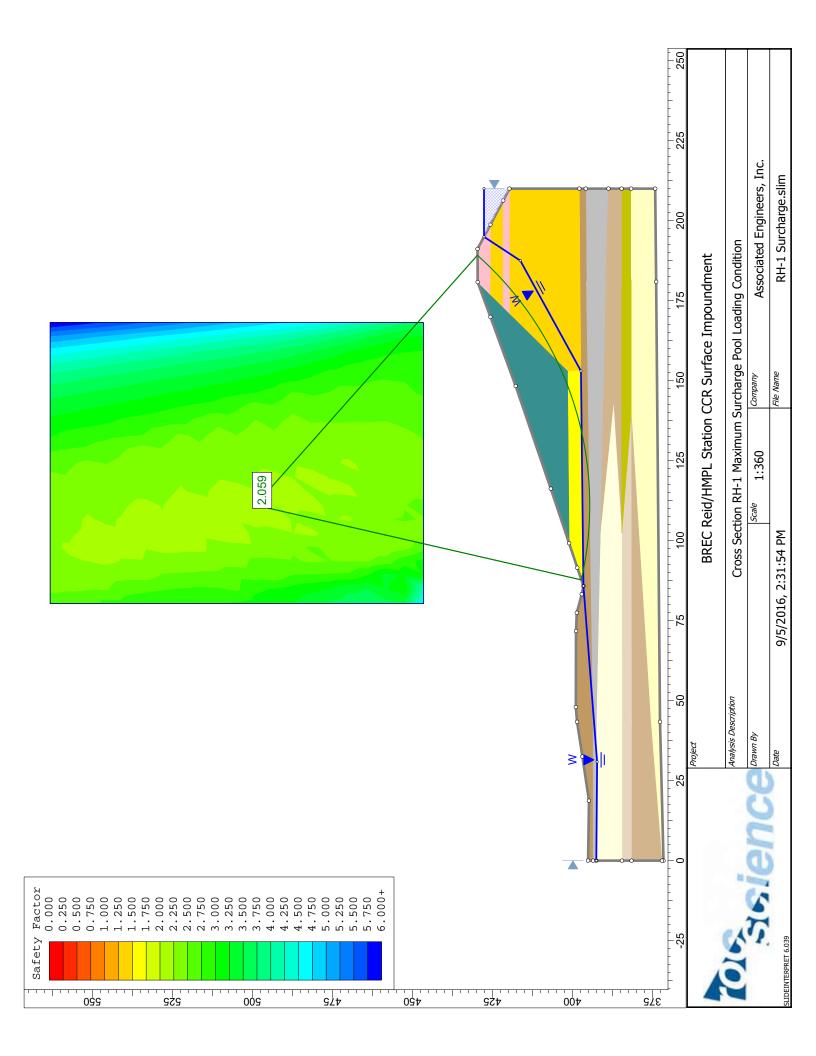
Х	Υ	
102.292	384.543	
140	381.6	

# **Material Boundary**

х	Y			
102.292	384.543			
142.735	387.164			
102.672	391.249			

х	Υ	
176.879	425.6	
180.9	425.6	

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-1 Maximum Surcharge Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date	9/5/2016, 2:31:54 PI	М	File Name	RH-1 Surcharge.slim	



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-1 Seis

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-1 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

## **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-1 Seismic Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	9/5/2016, 2:31:54 PM			File Name	RH-1 Seis.slim	

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disable

Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

## Loading

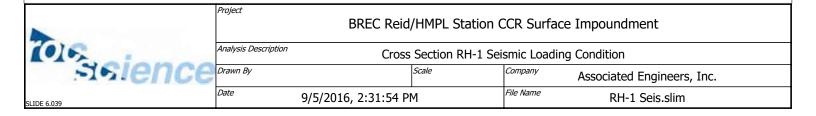
Seismic Load Coefficient (Horizontal): 0.2377

# **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	72	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	31	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

# **List Of Coordinates**



# Water Table

х	Υ
0	392.5
30.9	392.24
85.846	396.532
153	397.3
187.5	416.24
197.467	426.28
210	426.28

# **External Boundary**

Х	Υ
0	371.5
43.37	372.6
180.89	373.8
210	374.1
210	381.6
210	384.6
210	388.7
210	395.8
210	397.8
210	419.644
206.306	421.6
198.751	425.6
191.136	429.632
180.794	429.579
169.909	425.6
148.327	417.711
116.257	406.72
99.1875	401.002
91.5524	398.444
85.846	396.532
83.24	397.044
77.457	398.595
71.748	398.856
47.962	398.89
43.367	398.519
32.528	396.852
18.716	394.794
0	395.066
0	393.5
0	392.5
0	384.5

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SLIDE 6.039	7

	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-1 Seismic Loading Condition				
ì	Orawn By Scale Company Associated Engineers, Inc.		Associated Engineers, Inc.		
	Date	9/5/2016, 2:31:54 PI	М	File Name	RH-1 Seis.slim

0	381.5
0	372

Х	Υ
0	372
43.3	375.5
140	381.6
180.9	381.6
210	381.6

# **Material Boundary**

Х	Υ
0	381.5
43.3	381.5
140	381.6

# **Material Boundary**

X	Υ
0	384.5
43.3	384.5
102.292	384.543
180	384.6
210	384.6

# **Material Boundary**

х	Υ
0	392.5
43.3	392.5
102.672	391.249
180.9	389.6
210	388.7

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

	BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description Cross Section RH-1 Seismic Loading Condition						
sience	Drawn By Scale Company Associated Engineers, Inc.						
SLIDE 6.039	Date	9/5/2016, 2:31:54 PI	М	File Name	RH-1 Seis.slim		

X	Y		
85.846	396.532		
153	397.3		
180.9	397.6		
210	397.8		

## **Material Boundary**

х	Υ		
99.1875	401.002		
153	401.5		
153	401.333		
153	401.333		
153	397.3		

## **Material Boundary**

х	Y		
153	401.333		
171.018	419.644		
172.943	421.6		
176.879	425.6		
180.794	429.579		

## **Material Boundary**

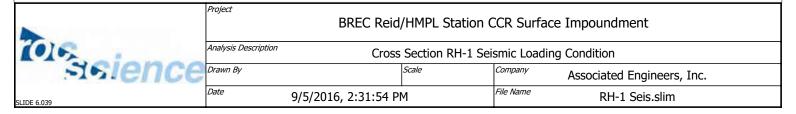
х	Υ		
180.9	425.6		
198.751	425.6		

# **Material Boundary**

х	Y		
172.943	421.6		
180.9	421.6		

# **Material Boundary**

Х	Υ		
180.9	421.6		
206.306	421.6		



X	Y		
171.018	419.644		
210	419.644		
180.9	419.6		

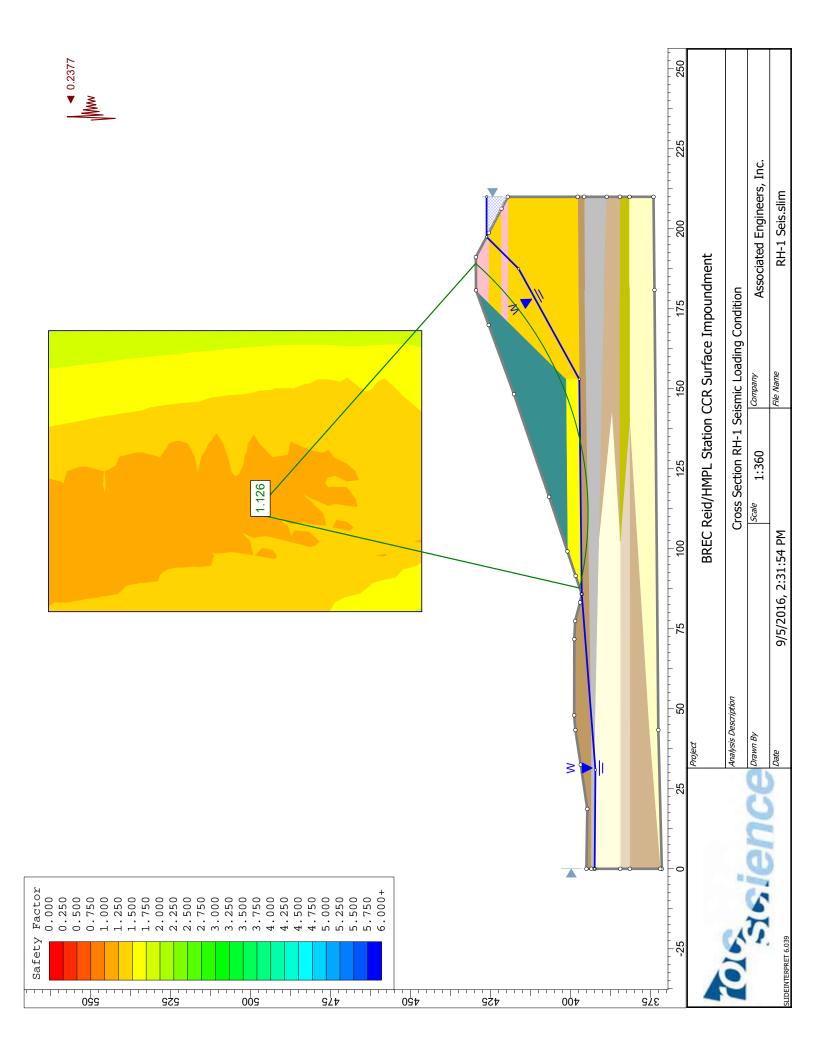
х	Υ		
102.292	384.543		
140	381.6		

# **Material Boundary**

х	Y		
102.292	384.543		
142.735	387.164		
102.672	391.249		

X	Υ	
176.879	425.6	
180.9	425.6	

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-1 Seismic Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date	9/5/2016, 2:31:54 PM	М	File Name	RH-1 Seis.slim	



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-1 Seis Liq

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-1 Liquefaction Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 2:31:54 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

## **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

# **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-1 Liquefaction Condition				Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 2:31:54 PI	М	File Name	RH-1 Seis Liq.slim

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay (CL), Silty	Silt With Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL) 2	Sandy Lean Clay (Dam)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	126	123	125.8	129.7	134.6	125.8
Cohesion [psf]	120	72	72	0	200	14.4	14.4	220
Friction Angle [deg]	32.3	30.4	30.4	0	33.7	28.7	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material (Dam)	Silty Sand (SM)	Sandy Lean Clay (CL) 2
Color			
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	130	124.6
Cohesion [psf]	200	0	740
Friction Angle [deg]	30	33	23.2
Water Surface	Water Table	Water Table	Water Table
Hu Value	1	1	1

# **List Of Coordinates**

#### **Water Table**

х	Υ
0	392.5
30.9	392.24
85.846	396.532

	Project BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-1 Liquefaction Condition				Condition
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 2:31:54 P	М	File Name	RH-1 Seis Liq.slim

153	397.3
187.5	416.24
197.467	426.28
210	426.28

# **External Boundary**

Х	Υ
0	371.5
43.37	372.6
180.89	373.8
210	374.1
210	381.6
210	384.6
210	388.7
210	395.8
210	397.8
210	419.644
206.306	421.6
198.751	425.6
191.136	429.632
180.794	429.579
169.909	425.6
148.327	417.711
116.257	406.72
99.1875	401.002
91.5524	398.444
85.846	396.532
83.24	397.044
77.457	398.595
71.748	398.856
47.962	398.89
43.367	398.519
32.528	396.852
18.716	394.794
0	395.066
0	393.5
0	392.5
0	384.5
0	381.5
0	372

# **Material Boundary**

X Y



	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-1 Liquefaction Condition				
e	Drawn By		Scale	<i>Company</i> Associ	ated Engineers, Inc.
	Date	9/5/2016, 2:31:54 PI	М	ile Name RI	H-1 Seis Liq.slim

0 372 43.3 375.5 140 381.6 180.9 381.6 210 381.6

## **Material Boundary**

Х	Y
0	381.5
43.3	381.5
140	381.6

## **Material Boundary**

х	Υ
0	384.5
43.3	384.5
102.292	384.543
180	384.6
210	384.6

## **Material Boundary**

х	Υ
0	392.5
43.3	392.5
102.672	391.249
180.9	389.6
210	388.7

# **Material Boundary**

Х	Υ
0	393.5
43.3	393.5
180.9	395.6
210	395.8

Х	Υ
85.846	396.532
153	397.3
180.9	397.6



	BREC Reid/HMPL Station CCR Surface Impoundment				
Analysis Description Cross Section RH-1 Liquefaction Condition				n Condition	
e	Drawn By		Scale	Associated Engineers, Inc.	
	<sup>Date</sup> 9/5/2016, 2:31:54 PM		File Name	RH-1 Seis Liq.slim	

х	Υ		
99.1875	401.002		
153	401.5		
153	401.333		
153	401.333		
153	397.3		

#### **Material Boundary**

х	Y		
153	401.333		
171.018	419.644		
172.943	421.6		
176.879	425.6		
180.794	429.579		

#### **Material Boundary**

Х	Y	
180.9	425.6	
198.751	425.6	

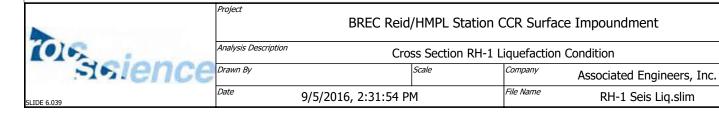
#### **Material Boundary**

Х	Υ	
172.943	421.6	
180.9	421.6	

#### **Material Boundary**

Х	Υ	
180.9	421.6	
206.306	421.6	

х	Υ
171.018	419.644
210	419.644
180.9	419.6



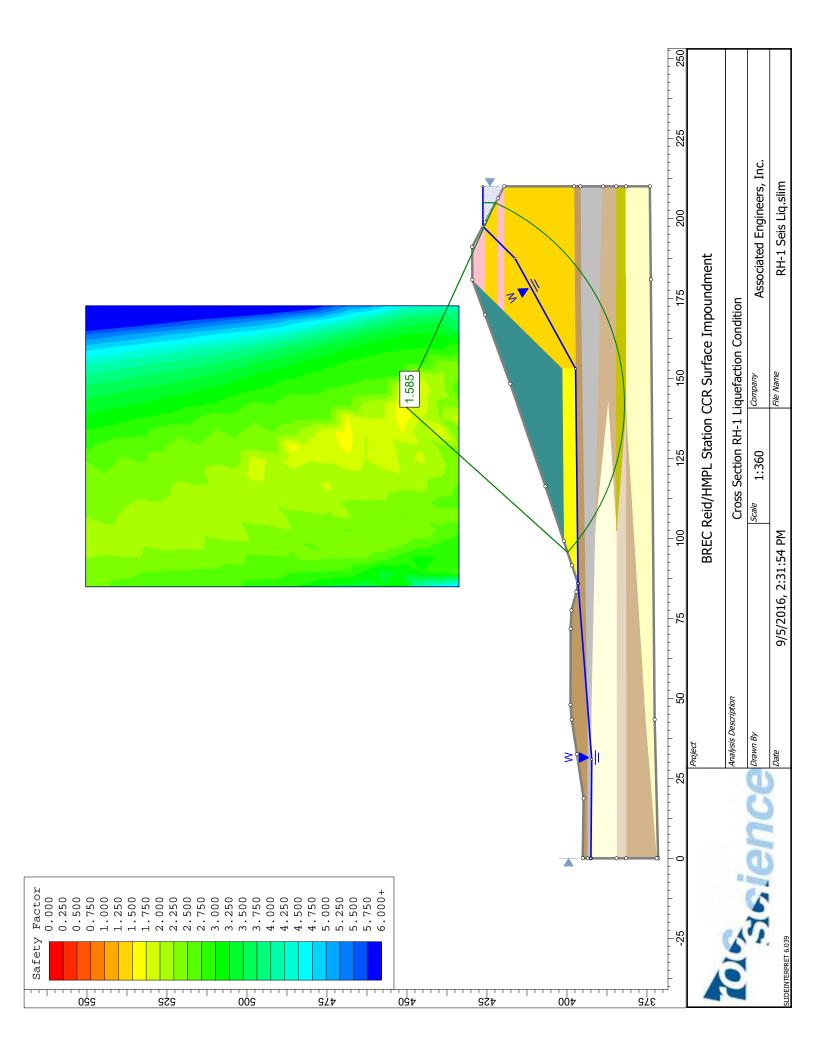
Х	Y
102.292	384.543
140	381.6

# **Material Boundary**

х	Y		
102.292	384.543		
142.735	387.164		
102.672	391.249		

х	Υ	
176.879	425.6	
180.9	425.6	

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-1 Liquefaction Condition				
Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	9/5/2016, 2:31:54 PM			File Name	RH-1 Seis Liq.slim



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-2

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-2 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:14:15 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

## **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	Project	BREC Reid/	/HMPL Station (	CCR Surface	: Impoundment
(0)6	Analysis Description Cross Section RH-2 Maximum Storage Pool Loading Condition				Loading Condition
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date 9/5	5/2016, 3:14:15 PN	4	File Name	RH-2.slim

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

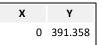
# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Granular Fill	Silty Sand (SC)	Lean Clay (CL) (Dam)	Lean clay With Sand (Dam)	Sandy Lean Clay (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	125.5	129.7	132	130	125.8	133.5	134.6
Cohesion [psf]	120	0	14.4	0	0	220	260	14.4
Friction Angle [deg]	32.3	33.7	28.7	31	33	30.4	30.6	33.3
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material	Sandy Lean Clay 2
Color		
Strength Type	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	125.8
Cohesion [psf]	200	80
Friction Angle [deg]	30	29.6
Water Surface	Water Table	Water Table
Hu Value	1	1

# **List Of Coordinates**

#### **Water Table**



	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-2 Maximum Storage Pool Loading Condition				l Loading Condition
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date	9/5/2016, 3:14:15 P	М	File Name	RH-2.slim

87.066 391.358 166.755 395.192 170.864 397.272 208.5 416.24 219.309 426.28 230 426.28

#### **External Boundary**

X Y 6.548e-013 364 45.37 364 208.52 387 230 390 230 395.8 230 397.8	1.6 7.9
45.37 364 208.52 387 230 390 230 395.8	1.6 7.9
208.52 387 230 390 230 395.8	7.9
230 390 230 395.8	_
230 395.8	).5
230 397.8	94
	52
230 398.7	26
230 401	1.3
230 409	9.9
230 412	2.9
230 418.	69
230 420.	43
224.631 423.3	68
223.845 423.7	98
220.564 425.5	93
214.627 428.8	42
208.516 429.1	75
202.208 429.1	34
186.355 423.8	78
183.639 422.9	77
179.243 421.	45
160.601 414.9	73
154.822 412.9	65
142.317 408.6	32
132.449 405.2	13
123.342 402.0	58
100.934 395.4	48
	15
92.3437 392.9	00
92.3437 392.9 90.2895 392.3	09
90.2895 392.3	58
90.2895 392.3 87.066 391.3	58 67
90.2895 392.3 87.066 391.3 79.843 393.6	58 67 03
90.2895 392.3 87.066 391.3 79.843 393.6 75.206 394.0	58 67 03 78

	Project 	BREC Reid/HMPL Stati	on CCR Surfac	e Impoundment	
(0)6	Analysis Description Cross Section RH-2 Maximum Storage Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date 9/5/2016	, 3:14:15 PM	File Name	RH-2.slim	

0 393.4930 377.40 370.4

## **Material Boundary**

Х	Υ
0	370.4
45.4	370.4
100.5	377.5
45.4	377.4
0	377.4

## **Material Boundary**

Х	Y
87.066	391.358
68	389.6
87.5	389
96.6043	389.467
205.063	395.024
208.5	395.2
230	395.894

## **Material Boundary**

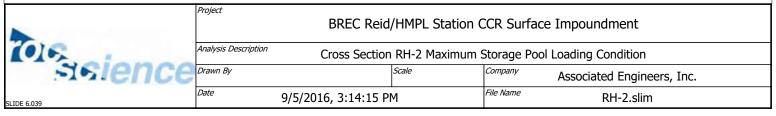
Х	Υ
87.066	391.358
208.5	397.2
230	397.852

## **Material Boundary**

Х	Υ
90.2895	392.309
208.5	398
230	398.726

#### **Material Boundary**

х	Υ
208.5	425.2
220.564	425.593



Х	Υ
208.5	423.2
223.845	423.798

Х	Υ
208.5	400.7
230	401.3

## **Material Boundary**

х	Y		
92.3437	392.915		
155.8	395.9		
170.864	397.272		
208.5	400.7		

## **Material Boundary**

х	Υ			
170.864	397.272			
181.19	407.768			
184.264	410.894			
189.983	416.707			
195.612	422.429			
197.704	424.555			
202.208	429.134			

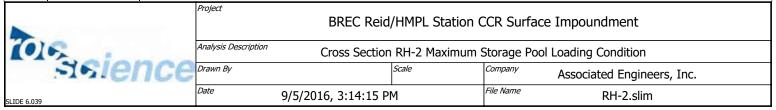
# **Material Boundary**

х	Y
197.704	424.555
208.5	425.2

# **Material Boundary**

х	Y
195.612	422.429
208.5	423.2

X	Υ		
181.19	407.768		



208.5	409.2		
230	409.9		

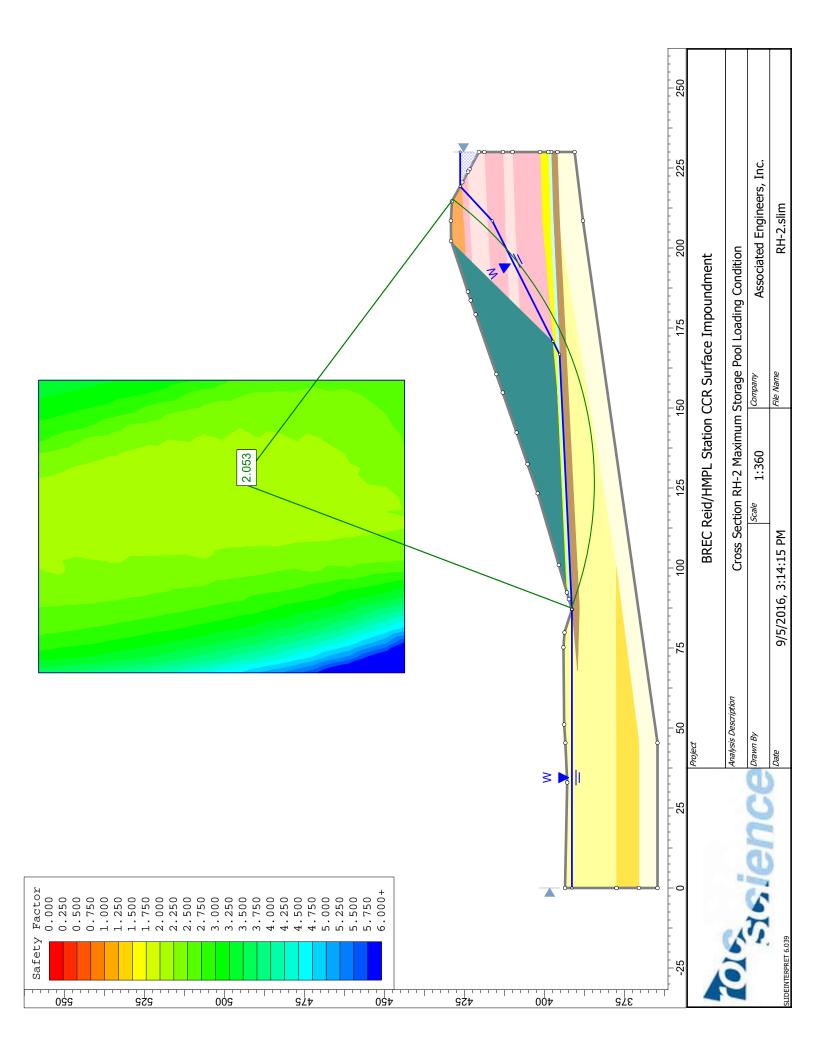
х	Υ
184.264	410.894
208.5	412.2
230	412.9

# **Material Boundary**

х	Υ
189.983	416.707
208.5	417.8
230	418.69

Х	Υ		
100.5	377.5		
205.063	395.024		

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-2 Maximum Storage Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	9/5/2016, 3:14:15 PM		File Name	RH-2.slim		



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-2 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-2 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:14:15 PM

## **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

## **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-2 Maximum Surcharge Pool Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	9/5/2016, 3:14:15 PM		File Name	RH-2 Surcharge.slim		

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

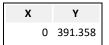
# **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Granular Fill	Silty Sand (SC)	Lean Clay (CL) (Dam)	Lean clay With Sand (Dam)	Sandy Lean Clay (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	125.5	129.7	132	130	125.8	133.5	134.6
Cohesion [psf]	120	0	14.4	0	0	220	260	14.4
Friction Angle [deg]	32.3	33.7	28.7	31	33	30.4	30.6	33.3
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Outslope Material	Sandy Lean Clay 2
Color		
Strength Type	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	125.8
Cohesion [psf]	200	80
Friction Angle [deg]	30	29.6
Water Surface	Water Table	Water Table
Hu Value	1	1

# **List Of Coordinates**

#### **Water Table**



	Project BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description	Cross Section I	Cross Section RH-2 Maximum Surcharge Pool Loading Condition		
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:14:15 P	М	File Name	RH-2 Surcharge.slim

87.066 391.358 166.755 395.192 170.864 397.272 208.5 416.24 216.878 427.61 230 427.61

#### **External Boundary**

v	V
X	264.6
6.548e-013	364.6
45.37	364.6
208.52	387.9
230	390.5
230	395.894
230	397.852
230	398.726
230	401.3
230	409.9
230	412.9
230	418.69
230	420.43
224.631	423.368
223.845	423.798
220.564	425.593
214.627	428.842
208.516	429.175
202.208	429.134
186.355	423.878
183.639	422.977
179.243	421.45
160.601	414.973
154.822	412.965
142.317	408.632
132.449	405.213
123.342	402.058
100.934	395.448
92.3437	392.915
90.2895	392.309
87.066	391.358
79.843	393.667
75.206	394.003
	393.78
45.366	393.402
32.987	392.813
79.843 75.206 51.068 45.366	393.667 394.003 393.78 393.402

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-2 Maximum Surcharge Pool Loading Condit			ol Loading Condition	
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:14:15 PM	4	File Name	RH-2 Surcharge.slim

0 393.4930 377.40 370.4

## **Material Boundary**

Х	Υ
0	370.4
45.4	370.4
100.5	377.5
45.4	377.4
0	377.4

## **Material Boundary**

х	Υ
87.066	391.358
68	389.6
87.5	389
96.6043	389.467
205.063	395.024
208.5	395.2
230	395.894

## **Material Boundary**

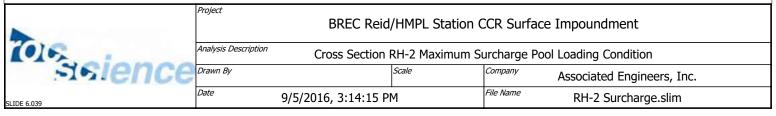
Х	Y
87.066	391.358
208.5	397.2
230	397.852

## **Material Boundary**

Х	Υ
90.2895	392.309
208.5	398
230	398.726

#### **Material Boundary**

х	Υ
208.5	425.2
220.564	425.593



Х	Υ
208.5	423.2
223.845	423.798

Х	Υ
208.5	400.7
230	401.3

## **Material Boundary**

х	Υ
92.3437	392.915
155.8	395.9
170.864	397.272
208.5	400.7

## **Material Boundary**

х	Υ
170.864	397.272
181.19	407.768
184.264	410.894
189.983	416.707
195.612	422.429
197.704	424.555
202.208	429.134

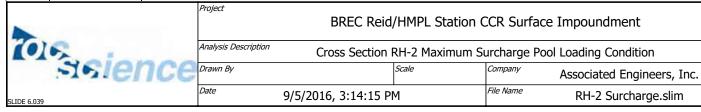
# **Material Boundary**

х	Y
197.704	424.555
208.5	425.2

# **Material Boundary**

X	Y
195.612	422.429
208.5	423.2

X	Υ
181.19	407.768



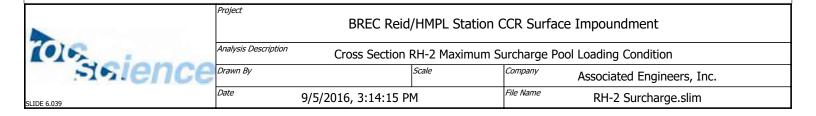
208.5	409.2
230	409.9

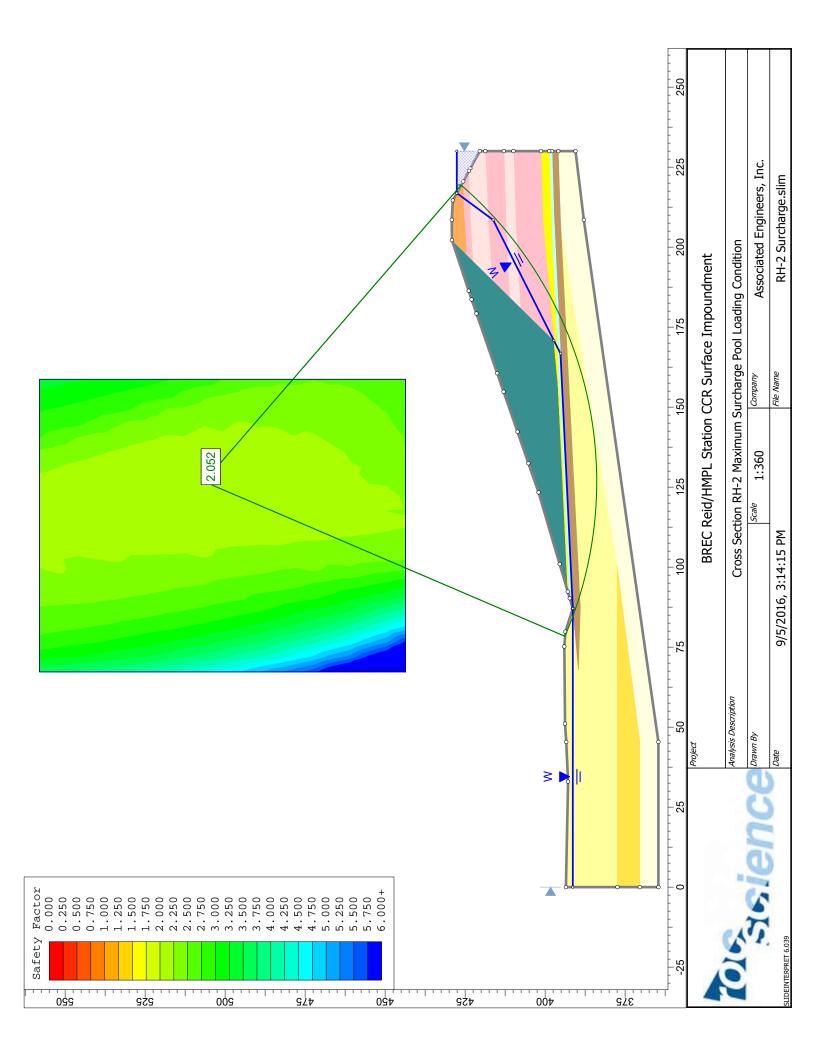
х	Υ
184.264	410.894
208.5	412.2
230	412.9

#### **Material Boundary**

х	Υ
189.983	416.707
208.5	417.8
230	418.69

Х	Υ
100.5	377.5
205.063	395.024





# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

## **Project Summary**

File Name: RH-2 Seis

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-2 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/5/2016, 3:14:15 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified Janbu simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

	Project	BREC Reid,	/HMPL Station (	CCR Surface	e Impoundment
(0)6	Analysis Description Cross Section RH-2 Seismic Loading Condition				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/5/2016, 3:14:15 PI	М	File Name	RH-2 Seis.slim

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

#### **Surface Options**

Surface Type: Circular Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled

Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

### Loading

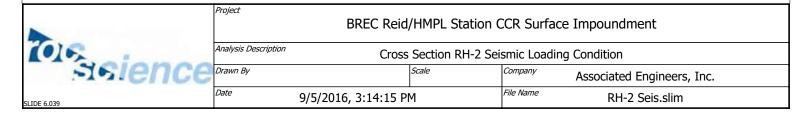
Seismic Load Coefficient (Horizontal): 0.2377

## **Material Properties**

Property	Sandy Lean Clay (CL)	Clayey Sand (SC)	Lean Clay (CL)	Granular Fill	Silty Sand (SC)	Lean Clay (CL) (Dam)	Lean clay With Sand (Dam)	Sandy Lean Clay (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	125.5	129.7	132	130	125.8	133.5	134.6
Cohesion [psf]	120	0	14.4	0	0	220	260	14.4
Friction Angle [deg]	32.3	33.7	28.7	31	33	30.4	30.6	33.3
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	<b>Outslope Material</b>	Sandy Lean Clay 2
Color		
Strength Type	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	128	125.8
Cohesion [psf]	200	80
Friction Angle [deg]	30	29.6
Water Surface	Water Table	Water Table
Hu Value	1	1

## **List Of Coordinates**



#### **Water Table**

х	Υ
0	391.358
87.066	391.358
166.755	395.192
170.864	397.272
208.5	416.24
219.309	426.28
230	426.28

## **External Boundary**

Х	Υ
6.548e-013	364.6
45.37	364.6
208.52	387.9
230	390.5
230	395.894
230	397.852
230	398.726
230	401.3
230	409.9
230	412.9
230	418.69
230	420.43
224.631	423.368
223.845	423.798
220.564	425.593
214.627	428.842
208.516	429.175
202.208	429.134
186.355	423.878
183.639	422.977
179.243	421.45
160.601	414.973
154.822	412.965
142.317	408.632
132.449	405.213
123.342	402.058
100.934	395.448
92.3437	392.915
90.2895	392.309
87.066	391.358
79.843	393.667

	Project
(0)6	Analysis
Asien	Drawn B
CLIDE 6 030	Date

	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-2 Seismic Loading Condition				
ì	Drawn By Scale Company Associated Engineers, Inc.				
	Date	9/5/2016 3·14·15 PI	М	File Name	RH-2 Seis slim

75.206 394.003 51.068 393.78 45.366 393.402 32.987 392.813 0 393.493 0 377.4 0 370.4

#### **Material Boundary**

Х	Υ
0	370.4
45.4	370.4
100.5	377.5
45.4	377.4
0	377.4

#### **Material Boundary**

Х	Υ
87.066	391.358
68	389.6
87.5	389
96.6043	389.467
205.063	395.024
208.5	395.2
230	395.894

## **Material Boundary**

Х	Υ
87.066	391.358
208.5	397.2
230	397.852

#### **Material Boundary**

х	Y
90.2895	392.309
208.5	398
230	398.726

#### **Material Boundary**

X Y



	BREC Reid/HMPL Station CCR Surface Impoundment					
	Analysis Description Cross Section RH-2 Seismic Loading Condition			l		
e:e	Drawn By		Scale	Company	Associated Engineers, Inc.	l
	Date	0/E/2016 2:14:1E DI	M	File Name	DH 2 Coic clim	ı

208.5 425.2 220.564 425.593

## **Material Boundary**

х	Υ
208.5	423.2
223.845	423.798

#### **Material Boundary**

Х	Υ
208.5	400.7
230	401.3

## **Material Boundary**

х	Υ
92.3437	392.915
155.8	395.9
170.864	397.272
208.5	400.7

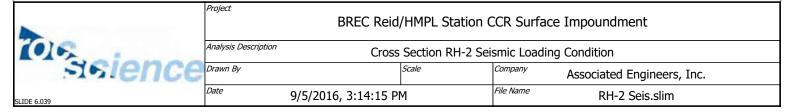
## **Material Boundary**

х	Y
170.864	397.272
181.19	407.768
184.264	410.894
189.983	416.707
195.612	422.429
197.704	424.555
202.208	429.134

## **Material Boundary**

Х	Y
197.704	424.555
208.5	425.2

х	Υ
195.612	422.429
208.5	423.2



Х	Υ
181.19	407.768
208.5	409.2
230	409.9

## **Material Boundary**

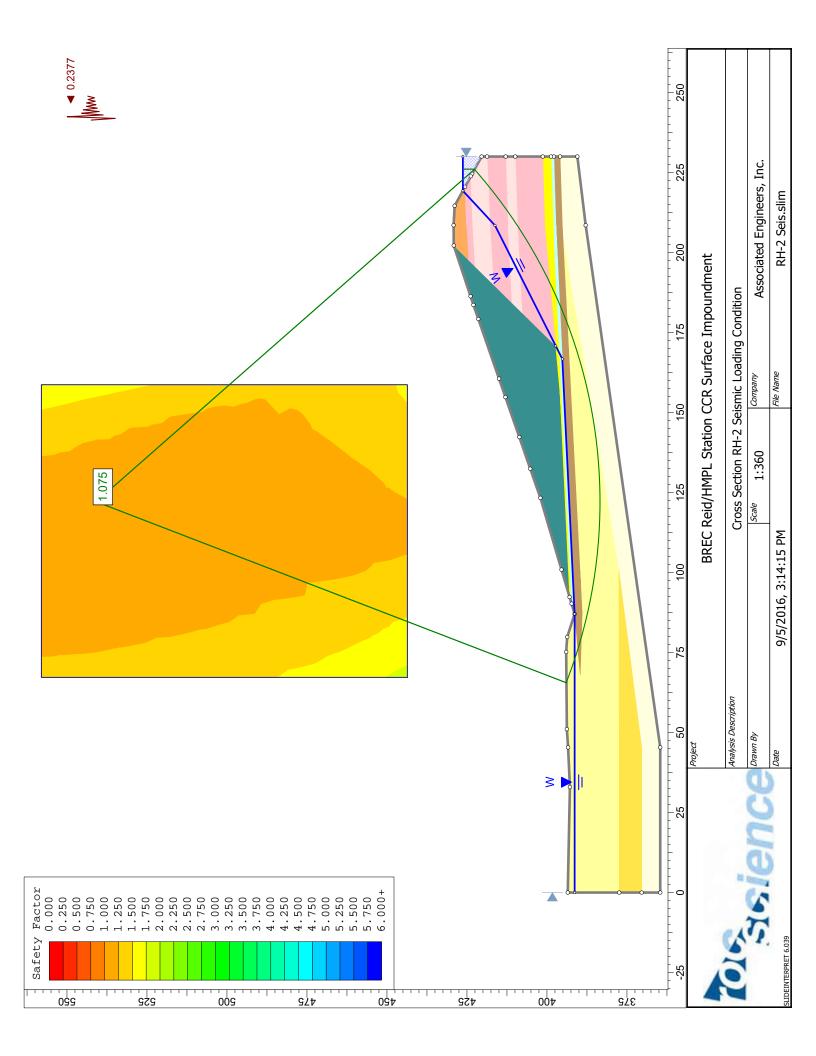
х	Υ
184.264	410.894
208.5	412.2
230	412.9

## **Material Boundary**

х	Υ
189.983	416.707
208.5	417.8
230	418.69

х	Y
100.5	377.5
205.063	395.024

	Project	BREC Reid,	/HMPL Station (	CCR Surface	Impoundment
(0)6	Analysis Description	S Description Cross Section RH-2 Seismic Loading Condition			
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date	9/5/2016, 3:14:15 P	М	File Name	RH-2 Seis.slim



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

#### **Project Summary**

File Name: RH-3

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-3 Maximum Storage Pool Loading Conditon

Company: Associated Engineers, Inc. Date Created: 8/29/2016, 2:56:23 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

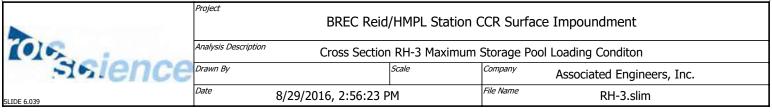
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

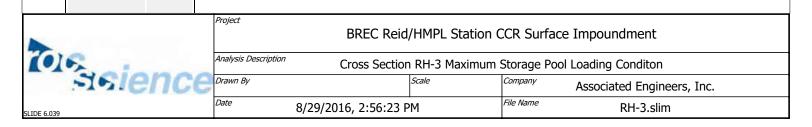
## **Material Properties**

Property	Sandy Lean Clay	Lean Clay	Silty Clay (CL-ML)	Granular Fill and Lean Clay	Outslope Material (Dam)	Lean Clay with Sand (CL) (Dam)	Sandy Lean Clay (CL)(Dike)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	125.8	132	128	133.5	134.6	125.8
Cohesion [psf]	120	72	200	0	200	260	14.4	220
Friction Angle [deg]	32.3	30.4	33.7	31	30	30.6	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Lean Clay (CL) 2
Color	
Strength Type	Mohr-Coulomb
Unit Weight [lbs/ft3]	129.7
Cohesion [psf]	14.4
Friction Angle [deg]	28.7
Water Surface	Water Table
Hu Value	1

## **List Of Coordinates**

#### **Water Table**



Х	Y
-1.11022e-016	380.175
75.344	380.175
77.002	383.846
82.5127	384.456
217.136	388.922
221.8	391.3
267.7	412.21
276.537	426.277
285	426.28

## **External Boundary**

Х	Υ
276.537	426.277
270.621	429.244
264.923	429.374
259.45	429.143
250.06	425.382
237.754	421.038
216.209	413.372
187.674	403.549
156.481	394.047
133.994	390.347
132.225	390.139
106.149	387.073
82.5127	384.456
77.002	383.846
75.344	380.175
68.53	380.645
67.51	383.799
59.536	386.037
0	385.411
0	383.8
1.38778e-017	375.3
0	371.5
0	366.9
0	359.5
40	359.5
132.2	367
172.029	370.145
265	377.7
285	378.7
285	383.6
285	390.6

	Project	BREC Reid,	/HMPL Station (	CCR Surfa	ace Impoundment
	Analysis Description	Cross Section	RH-3 Maximum	Storage P	ool Loading Conditon
e	Drawn By		Scale	Company	Associated Engineers, Inc.
	Date	8/29/2016, 2:56:23 P	M	File Name	RH-3.slim

2	85	393	3.6
2	85	396	5.6
2	85	399	9.6
2	85	422.0	33

х	Y
0	366.9
40	366.9
132.2	371.3
201	376.8
172.029	370.145

## **Material Boundary**

x	Υ
1.38778e-017	375.3
40	375.3
132.2	378.3
265	383.4
285	383.6

## **Material Boundary**

х	Υ
82.5127	384.456
132.2	386.3
265	390.4
285	390.6

## **Material Boundary**

X	Υ
0	383.8
40	383.8
67.51	383.799

х	Υ
106.149	387.073
132.2	388.3
221.8	391.3
265	393.4

	Project	BREC Reid,	/HMPL Station (	CCR Surface	e Impoundment	
(0)6	Analysis Description Cross Section RH-3 Maximum Storage Pool Loading Conditon				Loading Conditon	
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date 8/29,	/2016, 2:56:23 P	М	File Name	RH-3.slim	

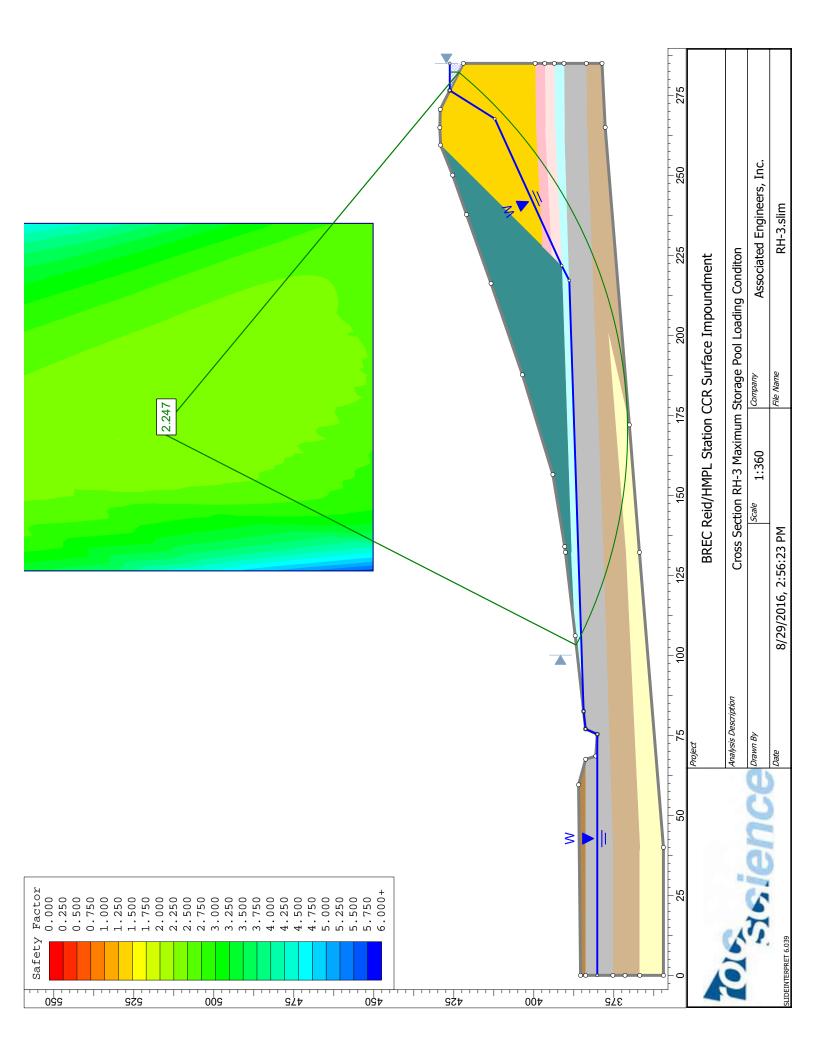
х	Υ
221.8	391.3
224.786	394.301
227.81	397.341
259.45	429.143

## **Material Boundary**

х	Υ
224.786	394.301
265	396.4
285	396.6

Х	Υ
227.81	397.341
265	399.4
285	399.6

	Project	BREC Reid,	/HMPL Station	CCR Surfac	ce Impoundment
(0)6	Analysis Description	Cross Section	RH-3 Maximum	Storage Poo	ol Loading Conditon
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	8/29/2016, 2:56:23 P	М	File Name	RH-3.slim



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impooundment

#### **Project Summary**

File Name: RH-3 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impooundment Analysis: Cross Section RH-3 Maximum Surcharge Pool Loading Conditon

Company: Associated Engineers, Inc. Date Created: 8/29/2016, 2:56:23 PM

### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116

	BREC Reid/HMPL Station CCR Surface Impooundment	e Impooundment				
(0)6	Analysis Description Cross Section	on RH-3 Maximum S	Surcharge Po	ool Loading Conditon		
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDE 6.039	Date 8/29/2016, 2:56:2	3 PM	File Name	RH-3 Surcharge.slim		

Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

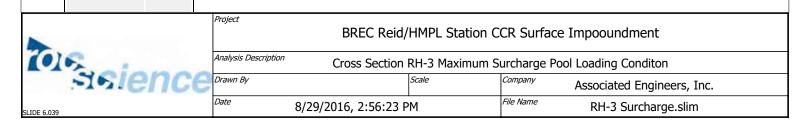
## **Material Properties**

Property	Sandy Lean Clay	Lean Clay	Silty Clay (CL-ML)	Granular Fill and Lean Clay	Outslope Material (Dam)	Lean Clay with Sand (CL) (Dam)	Sandy Lean Clay (CL)(Dike)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	125.8	132	128	133.5	134.6	125.8
Cohesion [psf]	120	72	200	0	200	260	14.4	220
Friction Angle [deg]	32.3	30.4	33.7	31	30	30.6	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Lean Clay (CL) 2
Color	
Strength Type	Mohr-Coulomb
Unit Weight [lbs/ft3]	129.7
Cohesion [psf]	14.4
Friction Angle [deg]	28.7
Water Surface	Water Table
Hu Value	1

## **List Of Coordinates**

#### **Water Table**



Х	Y
-1.11022e-016	380.175
75.344	380.175
77.002	383.846
82.5127	384.456
217.136	388.922
221.8	391.3
267.7	412.21
273.872	427.613
285	427.61

## **External Boundary**

Х	Υ
276.537	426.277
270.621	429.244
264.923	429.374
259.45	429.143
250.06	425.382
237.754	421.038
216.209	413.372
187.674	403.549
156.481	394.047
133.994	390.347
132.225	390.139
106.149	387.073
82.5127	384.456
77.002	383.846
75.344	380.175
68.53	380.645
67.51	383.799
59.536	386.037
0	385.411
0	383.8
1.38778e-017	375.3
0	371.5
0	366.9
0	359.5
40	359.5
132.2	367
172.029	370.145
265	377.7
285	378.7
285	383.6
285	390.6

0)6	
siglence	4
5.6.000	4

	BREC Reid/HMPL Station CCR Surface Impooundment					
	Analysis Description	Cross Section RH-3 Maximum Surcharge Pool Loading Conditon				
9	Drawn By		Scale	Company	Associated Engineers, Inc.	
	Date	ote 9/20/2016 2:56:22 DM		File Name	DH 2 Curchargo clim	

285	393.6
285	396.6
285	399.6
285	422.033

х	Y		
0	366.9		
40	366.9		
132.2	371.3		
201	376.8		
172.029	370.145		

## **Material Boundary**

Х	Υ
1.38778e-017	375.3
40	375.3
132.2	378.3
265	383.4
285	383.6

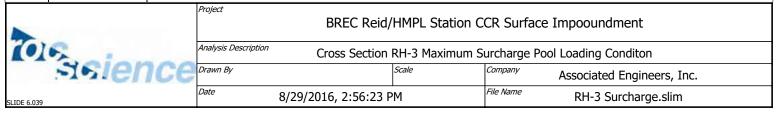
#### **Material Boundary**

х	Υ
82.5127	384.456
132.2	386.3
265	390.4
285	390.6

## **Material Boundary**

X	Υ
0	383.8
40	383.8
67.51	383.799

х	Υ
106.149	387.073
132.2	388.3
221.8	391.3
265	393.4



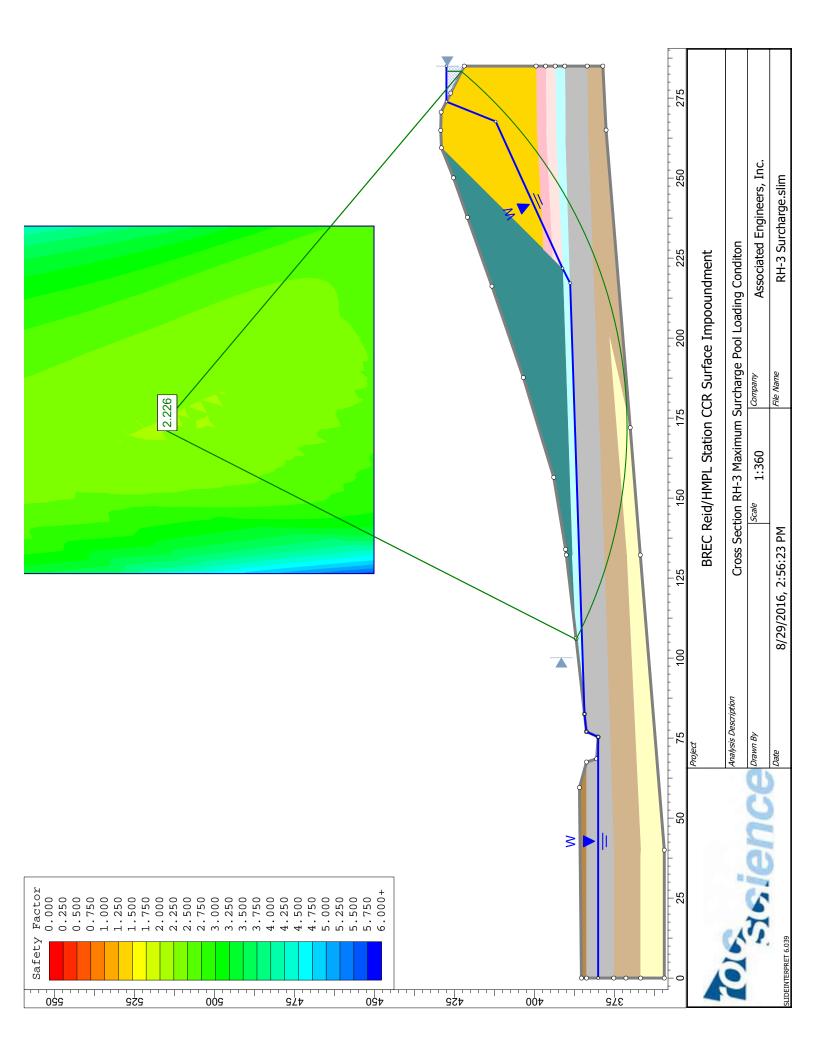
х	Υ
221.8	391.3
224.786	394.301
227.81	397.341
259.45	429.143

## **Material Boundary**

х	Y	
224.786	394.301	
265	396.4	
285	396.6	

Х	Υ
227.81	397.341
265	399.4
285	399.6

	Project	BREC Reid/	HMPL Statio	n CCR Surfac	e Impooundment
(0)6	Analysis Description Cross Section RH-3 Maximum Surcharge Pool Loading Conditon				
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	8/29/2016, 2:56:23 P	М	File Name	RH-3 Surcharge.slim



## Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

#### **Project Summary**

File Name: RH-3 Seis Slide Modeler Version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-3 Seismic Loading Conditon

Company: Associated Engineers, Inc. Date Created: 8/29/2016, 2:56:23 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116

Random Number Generation Method: Park and Miller v.3

#### **Surface Options**

Surface Type: Circular

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Conditon				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim	

Search Method: Grid Search Radius Increment: 10 Composite Surfaces: Disabled Reverse Curvature: Invalid Surfaces Minimum Elevation: Not Defined Minimum Depth: Not Defined

#### Loading

Seismic Load Coefficient (Horizontal): 0.2377

## **Material Properties**

Property	Sandy Lean Clay	Lean Clay	Silty Clay (CL-ML)	Granular Fill and Lean Clay	Outslope Material (Dam)	Lean Clay with Sand (CL) (Dam)	Sandy Lean Clay (CL)(Dike)	Lean Clay (CL) (Dam)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	132.7	134.1	125.8	132	128	133.5	134.6	125.8
Cohesion [psf]	120	72	200	0	200	260	14.4	220
Friction Angle [deg]	32.3	30.4	33.7	31	30	30.6	33.3	30.4
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

Property	Lean Clay (CL) 2
Color	
Strength Type	Mohr-Coulomb
Unit Weight [lbs/ft3]	129.7
Cohesion [psf]	14.4
Friction Angle [deg]	28.7
Water Surface	Water Table
Hu Value	1

#### **Global Minimums**

#### Method: bishop simplified

FS: 1.146920

Center: 169.832, 521.855 Radius: 150.630

Left Slip Surface Endpoint: 103.229, 386.750 Right Slip Surface Endpoint: 283.359, 422.856 Left Slope Intercept: 103.229 386.750 Right Slope Intercept: 283.359 426.279 Resisting Moment=4.1388e+007 lb-ft Driving Moment=3.60862e+007 lb-ft

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Conditon				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23 F	PM	File Name	RH-3 Seis.slim	

#### Valid / Invalid Surfaces

#### Method: bishop simplified

Number of Valid Surfaces: 27798 Number of Invalid Surfaces: 813

#### **Error Codes:**

Error Code -103 reported for 812 surfaces Error Code -108 reported for 1 surface

#### **Error Codes**

The following errors were encountered during the computation:

- -103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.
- -108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

#### Slice Data

#### Global Minimum Query (bishop simplified) - Safety Factor: 1.14692

Slice Number	Width [ft]	Weight [lbs]	Base Material	Base Cohesion [psf]	Base Friction Angle [degrees]	Shear Stress [psf]	Shear Strength [psf]	Base Normal Stress [psf]	Pore Pressure [psf]	Effective Normal Stress [psf]
1	2.9517	339.413	Granular Fill and Lean Clay	0	31	80.4502	92.27	153.563	0	153.563
2	9.38923	5181.12	Silty Clay (CL-ML)	200	33.7	558.746	640.837	789.603	128.596	661.007
3	9.38923	10797.8	Silty Clay (CL-ML)	200	33.7	784.082	899.279	1423.39	374.861	1048.53
4	8.05892	13218.6	Lean Clay	72	30.4	715.465	820.581	1841.85	565.927	1275.92
5	8.05892	16561.5	Lean Clay	72	30.4	847.784	972.34	2244.15	709.561	1534.59
6	8.05892	19639.6	Lean Clay	72	30.4	970.425	1113	2598.53	824.191	1774.34
7	7.11739	19518.9	Sandy Lean Clay	120	32.3	1191.41	1366.45	2878.89	907.191	1971.7
8	7.11739	21628	Sandy Lean Clay	120	32.3	1296.89	1487.43	3125.23	962.17	2163.06
9	7.11739	23899.4	Sandy Lean Clay	120	32.3	1421.59	1630.45	3385.29	995.992	2389.3
10	7.11739	25852.9	Sandy Lean Clay	120	32.3	1527.15	1751.52	3589.6	1008.79	2580.81
11	7.97951	30912.3	Lean Clay	72	30.4	1474.5	1691.13	3758.42	998.688	2759.74
12	7.97951	32613.8	Lean Clay	72	30.4	1555.8	1784.38	3881.46	962.77	2918.69
13	7.97951	34074	Lean Clay	72	30.4	1630.67	1870.25	3964.78	899.728	3065.05
14	7.97951	35060.3	Lean Clay	72	30.4	1686.13	1933.86	3982.32	808.85	3173.47
15	7.97951	35547.6	Lean Clay	72	30.4	1722.09	1975.1	3932.9	689.137	3243.76
16	7.97951	35611.6	Lean Clay	72	30.4	1688.19	1936.22	3846.3	668.812	3177.49
17	8.30823	36828.4	Silty Clay (CL-ML)	200	33.7	1874.3	2149.67	3621.62	698.224	2923.4
18	8.30823	36298.1	Silty Clay (CL-ML)	200	33.7	1786.34	2048.79	3462.85	690.709	2772.14
19	4.95864	21118.6	Granular Fill and Lean Clay	0	31	1451.21	1664.42	3429.39	659.333	2770.06
20	5.12784	21134.1	Lean Clay with Sand (CL) (Dam)	260	30.6	1539.33	1765.49	3159.94	614.305	2545.64
21	4.73463	18862	Lean Clay (CL) (Dam)	220	30.4	1443.82	1655.95	3001.55	554.028	2447.52
22	6.60985	24515	Sandy Lean Clay (CL)(Dike)	14.4	33.3	1310.13	1502.61	2725.55	459.958	2265.59
23	6.60985	20127.3	Sandy Lean Clay (CL)(Dike)	14.4	33.3	1061.56	1217.53	2150.92	319.337	1831.59

BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description Cross S	Section RH-3 Sei	smic Loadir	ng Conditon		
sience	Drawn By	Scale	Company	Associated Engineers, Inc.		
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23 F	PM	File Name	RH-3 Seis.slim		

24 6.60985 13526.4	Sandy Lean Clay (CL)(Dike)	14.4	33.3 565.291	648.343	1510.28	545.202	965.081
25 6.60985 5362.88	Sandy Lean Clay (CL)(Dike)	14.4	33.3 141.118	161.851	659.69	435.217	224.473

#### **Interslice Data**

Global Minimum Query (bishop simplified) - Safety Factor: 1.14692

Slice	X	Υ	Interslice	Interslice	Interslice
Number	coordinate	coordinate - Bottom	Normal Force	<b>Shear Force</b>	Force Angle
Humber	[ft]	[ft]	[lbs]	[lbs]	[degrees]
1	103.229	386.75	0	0	0
2	106.18	385.335	374.121	0	0
3	115.57	381.338	7543.64	0	0
4	124.959	378.064	16998.2	0	0
5	133.018	375.793	23804.5	0	0
6	141.077	373.995	30733.8	0	0
7	149.136	372.654	37371.2	0	0
8	156.253	371.839	43557.5	0	0
9	163.37	371.364	49129.3	0	0
10	170.488	371.227	54029.8	0	0
11	177.605	371.426	58037.4	0	0
12	185.585	372.051	60103.9	0	0
13	193.564	373.107	60668.5	0	0
14	201.544	374.601	59653.7	0	0
15	209.523	376.549	57017.3	0	0
16	217.503	378.968	52794.3	0	0
17	225.482	381.882	46588	0	0
18	233.79	385.478	40381	0	0
19	242.099	389.693	31998	0	0
20	247.057	392.528	24451.6	0	0
21	252.185	395.731	17198.2	0	0
22	256.92	398.953	9880.41	0	0
23	263.53	403.914	-811.059	0	0
24	270.139	409.482	-10555.3	0	0
25	276.749	415.751	-19503.3	0	0
26	283.359	422.856	365.715	0	0

## **List Of Coordinates**

#### **Water Table**

Х	Υ
-1.11022e-016	380.175
75.344	380.175
77.002	383.846
82.5127	384.456
217.136	388.922
221.8	391.3
267.7	412.21
276.537	426.277

BREC Reid/HMPL Station CCR Surface Impoundment						
(0)6	Analysis Description Cross S	Section RH-3 Sei	smic Loadir	ng Conditon		
sience	Drawn By	Scale	Company	Associated Engineers, Inc.		
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23 F	PM	File Name	RH-3 Seis.slim		

#### **External Boundary**

Υ
426.277
429.244
429.374
429.143
425.382
421.038
413.372
403.549
394.047
390.347
390.139
387.073
384.456
383.846
380.175
380.645
383.799
386.037
385.411
383.8
375.3
371.5
366.9
359.5
359.5
367
370.145
377.7
378.7
383.6
390.6
393.6
396.6
399.6
422.033

х	Y
0	366.9
40	366.9
132.2	371.3
201	376.8
172.029	370.145

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)	Analysis Description Cross	Analysis Description Cross Section RH-3 Seismic Loading Conditon			
sience	Drawn By	Scale	Company Associated Engineers, Inc.		
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim	

Y
375.3
375.3
378.3
383.4
383.6

## **Material Boundary**

х	Υ		
82.5127	384.456		
132.2	386.3		
265	390.4		
285	390.6		

## **Material Boundary**

Х	Υ
0	383.8
40	383.8
67.51	383.799

## **Material Boundary**

х	Y			
106.149	387.073			
132.2	388.3			
221.8	391.3			
265	393.4			
285	393.6			

## **Material Boundary**

х	Υ
221.8	391.3
224.786	394.301
227.81	397.341
259.45	429.143

## **Material Boundary**

х	Υ		
224.786	394.301		
265	396.4		
285	396.6		

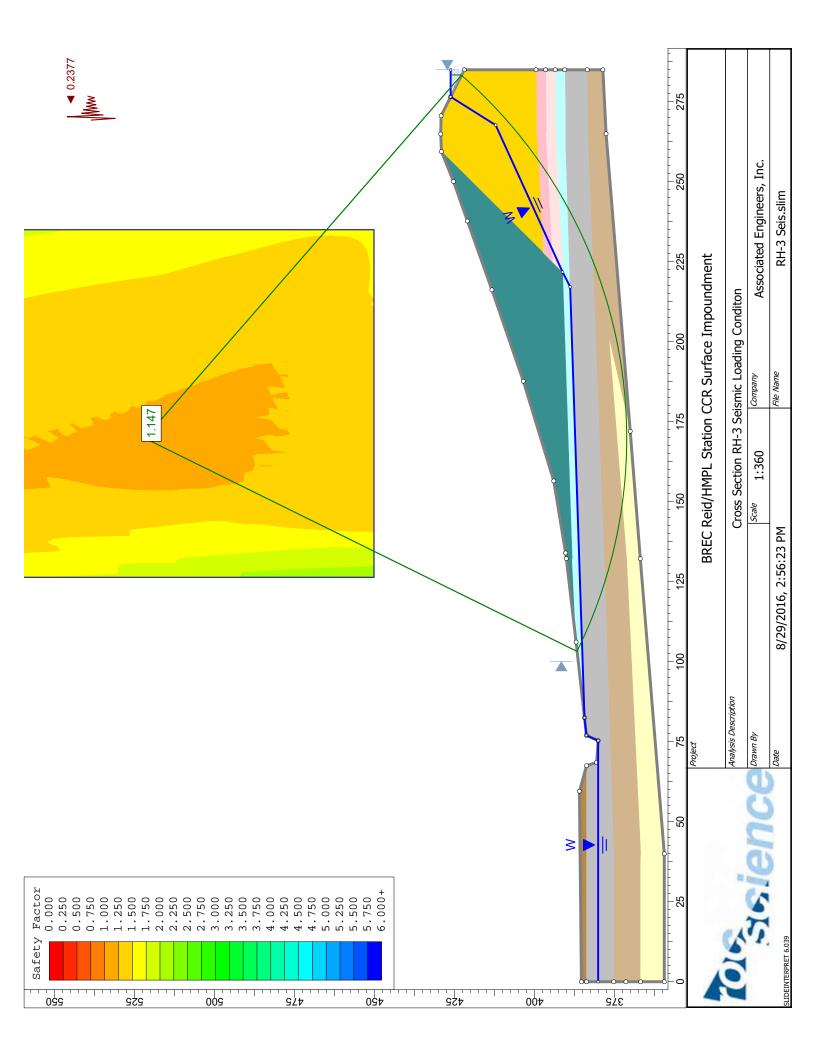
#### **Material Boundary**

X Y

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Conditon					
sience	Drawn By Scale Company Associated Engineers, Inc					
SLIDEINTERPRET 6.039	Date 8/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim		

227.81 397.341 265 399.4 285 399.6

	BREC Reid/HMPL Station CCR Surface Impoundment					
(0)6	Analysis Description Cross Section RH-3 Seismic Loading Condition					
sience	Drawn By Scale Company Associated Engineers, Inc.					
SLIDEINTERPRET 6.039	B/29/2016, 2:56:23	PM	File Name	RH-3 Seis.slim		



## Slide Analysis Information BREC Reid HMPL Station CCR Surface Impoundment

### **Project Summary**

File Name: RH-4

Last saved with Slide version: 6.039

Project Title: BREC Reid HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-4 Maximum Storage Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/4/2016, 12:09:41 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

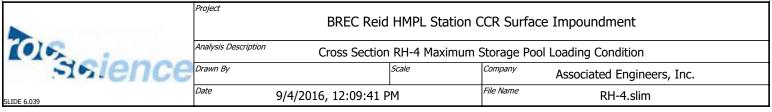
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

## **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

## **Material Properties**

Property	Lean Clay (CL)	Silty Clay (CL-ML)	Lean Clay With Sand (CL) (Dam)	Lean Clay (CL) (Dam)	Silty Sand (SM)	Outslope Material (Dam)	Lean Clay (CL) (1)	Lean Clay (CL)/Clayey Sand (SC)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	134.1	125.8	133.5	125.8	130	128	129.7	125.8
Cohesion [psf]	72	200	260	220	0	200	14.4	80
Friction Angle [deg]	30.4	33.7	30.6	30.4	33	30	28.7	29.6
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

## **List Of Coordinates**

#### **Water Table**

Х	Υ
2.77556e-017	393.033
55.19	393.033
71.2842	395.917
141	397
151.1	397
185.3	405.78
200.877	426.28
215	426.28

	BREC Reid	HMPL Station C	CCR Surface	Impoundment	
(0)6	Analysis Description Cross Section RH-4 Maximum Storage Pool Loading Condition				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	<sup>Date</sup> 9/4/2016, 12:09:41 P	М	File Name	RH-4.slim	

## **External Boundary**

х	٧
215	418.777
213.982	419.318
205.956	423.582
196.055	
	428.841
189.723	429.373
183.027	428.888
164.711	421.366
139.268	413.482
115.733	405.895
88.257	398.958
75.6909	396.706
71.2842	395.917
55.19	393.033
31.267	393.626
21.125	395.865
0	405.212
2.77556e-017	393.033
0	375.2
70.69	375.4
189.72	377.7
215	377.9
215	385.1
215	398.2
215	409.5

## **Material Boundary**

X	Υ
141	400
141	397
145.463	397
151.1	397
163.168	409.053
173.086	418.959
177.455	423.323
183.027	428.888

х	Υ
163.168	409.053
189.8	409.4



	BREC Reid HMPL Station CCR Surface Impoundment				
	Analysis Description	Cross Section	RH-4 Maximum S	Storage Po	ool Loading Condition
e	Drawn By		Scale	Company	Associated Engineers, Inc.
	Date	9/4/2016, 12:09:41 P	······································	File Name	RH-4.slim

Х	Y
151.1	397
189.8	398
215	398.2

## **Material Boundary**

х	Υ
141	397
70.7	392.8
36.8	391
70.7	389.5
79.6711	389.266
189.8	386.4
215	385.1

## **Material Boundary**

Х	Y
71.2842	395.917
141	397

## **Material Boundary**

Х	Υ
88.257	398.958
141	400

## **Material Boundary**

Х	Y
173.086	418.959
189.8	419.1

## **Material Boundary**

Х	Υ
189.8	419.1
213.982	419.318

	BREC Reid HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-4 Maximum Storage Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date 9/4/2016, 12:09:41	PM	File Name	RH-4.slim	

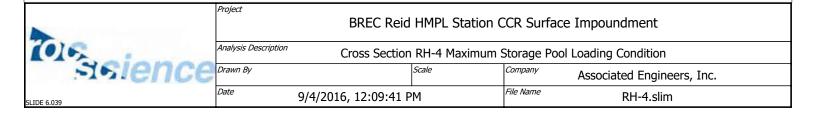
Х	Υ
177.455	423.323
189.8	423.4

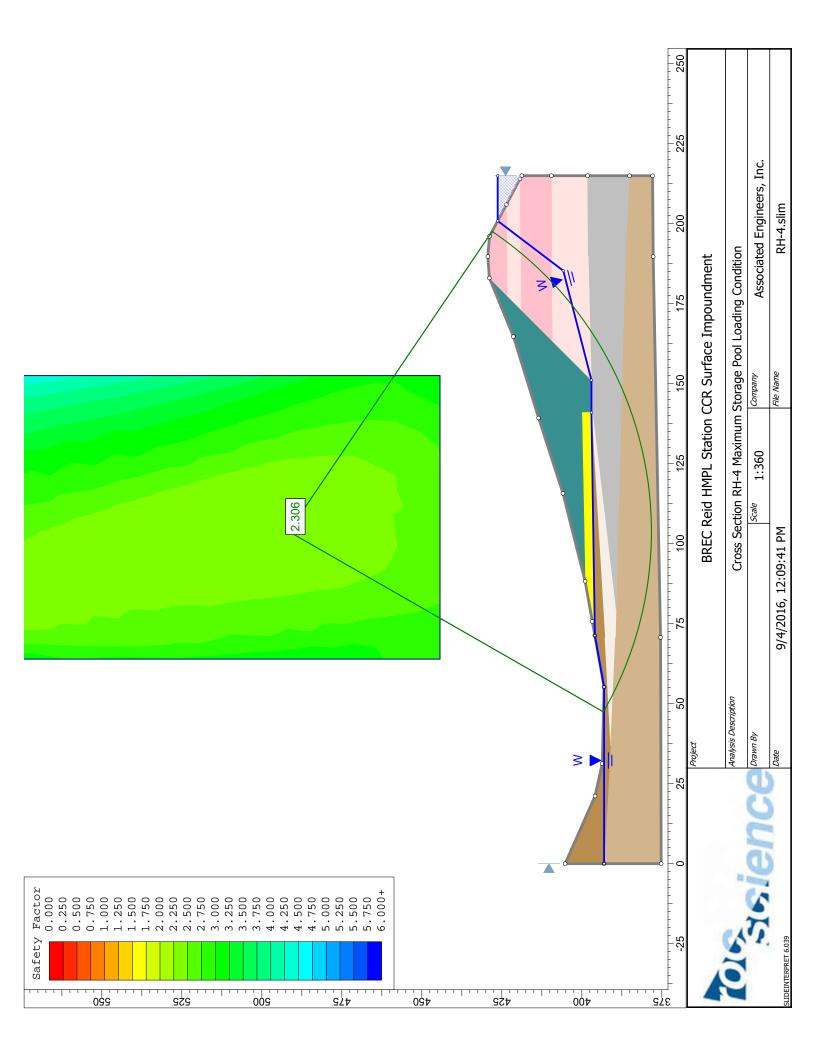
х	Υ
189.8	423.4
205.956	423.582

## **Material Boundary**

Х	Υ
2.77556e-017	393.033
36.8	391

х	Y
79.6711	389.266
145.463	397





# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

### **Project Summary**

File Name: RH-4 Surcharge

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment Analysis: Cross Section RH-4 Maximum Surcharge Pool Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/4/2016, 12:09:41 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

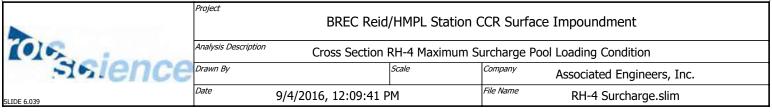
Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116



Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# **Material Properties**

Property	Lean Clay (CL)	Silty Clay (CL-ML)	Lean Clay With Sand (CL) (Dam)	Lean Clay (CL) (Dam)	Silty Sand (SM)	Outslope Material (Dam)	Lean Clay (CL) (1)	Lean Clay (CL)/Clayey Sand (SC)
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	134.1	125.8	133.5	125.8	130	128	129.7	125.8
Cohesion [psf]	72	200	260	220	0	200	14.4	80
Friction Angle [deg]	30.4	33.7	30.6	30.4	33	30	28.7	29.6
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Υ
2.77556e-017	393.033
55.19	393.033
71.2842	395.917
141	397
151.1	397
185.3	405.78
198.372	427.61
215	427.61

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				
sience	Drawn By	Scale	Company	Associated Engineers, Inc.	
SLIDE 6.039	<sup>Date</sup> 9/4/2016, 12:09:41 PM		File Name	RH-4 Surcharge.slim	

#### **External Boundary**

Х	Υ
215	418.777
213.982	419.318
205.956	423.582
196.055	428.841
189.723	429.373
183.027	428.888
164.711	421.366
139.268	413.482
115.733	405.895
88.257	398.958
75.6909	396.706
71.2842	395.917
55.19	393.033
31.267	393.626
21.125	395.865
0	405.212
2.77556e-017	393.033
0	375.2
70.69	375.4
189.72	377.7
215	377.9
215	385.1
215	398.2
215	409.5

#### **Material Boundary**

X	Υ
141	400
141	397
145.463	397
151.1	397
163.168	409.053
173.086	418.959
177.455	423.323
183.027	428.888

х	Υ
163.168	409.053
189.8	409.4



	BREC Reid/HMPL Station CCR Surface Impoundment				
	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				
9	Drawn By Scale Company Associated Engineers, Inc.				
	Date	9/4/2016, 12:09:41 P	· ·M	File Name	RH-4 Surcharge.slim

#### **Material Boundary**

Х	Υ
151.1	397
189.8	398
215	398.2

# **Material Boundary**

х	Y
141	397
70.7	392.8
36.8	391
70.7	389.5
79.6711	389.266
189.8	386.4
215	385.1

#### **Material Boundary**

х	Υ
71.2842	395.917
141	397

# **Material Boundary**

Х	Y
88.257	398.958
141	400

# **Material Boundary**

Х	Y
173.086	418.959
189.8	419.1

#### **Material Boundary**

х	Y
189.8	419.1
213.982	419.318

	Project BREC Reid	/HMPL Station (	CCR Surface	Impoundment	
(0)6	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	<sup>Date</sup> 9/4/2016, 12:09:41 F	PM	File Name	RH-4 Surcharge.slim	

Х	Υ
177.455	423.323
189.8	423.4

# **Material Boundary**

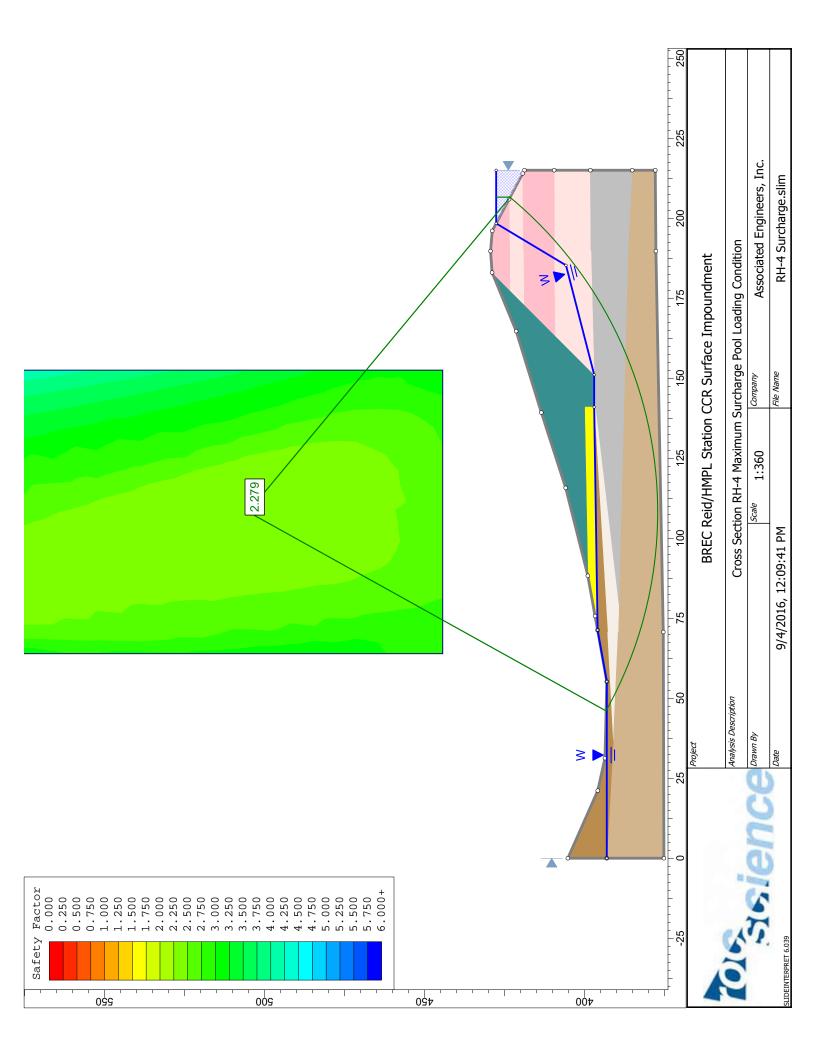
Х	Υ
189.8	423.4
205.956	423.582

# **Material Boundary**

Х	Υ
2.77556e-017	393.033
36.8	391

Х	Y
79.6711	389.266
145.463	397

SLIDE 6.039	Date 9/4/2016, 12:09:	41 PM	File Name	RH-4 Surcharge.slim	
Sience	Drawn By Scale Company Associated Engineers, Inc.				
(0)6	Analysis Description Cross Section RH-4 Maximum Surcharge Pool Loading Condition				
	Project BREC I	Reid/HMPL Stat	ion CCR Surfac	e Impoundment	



# Slide Analysis Information BREC Reid/HMPL Station CCR Surface Impoundment

#### **Project Summary**

File Name: RH-4 Seis 2

Last saved with Slide version: 6.039

Project Title: BREC Reid/HMPL Station CCR Surface Impoundment

Analysis: Cross Section RH-4 Seismic Loading Condition

Company: Associated Engineers, Inc. Date Created: 9/4/2016, 12:09:41 PM

#### **General Settings**

Units of Measurement: Imperial Units

Time Units: days

Permeability Units: feet/second Failure Direction: Right to Left Data Output: Standard

Maximum Material Properties: 20 Maximum Support Properties: 20

#### **Analysis Options**

#### **Analysis Methods Used**

Bishop simplified

Number of slices: 25 Tolerance: 0.005

Maximum number of iterations: 50

Check malpha < 0.2: Yes Initial trial value of FS: 1 Steffensen Iteration: Yes

#### **Groundwater Analysis**

Groundwater Method: Water Surfaces Pore Fluid Unit Weight: 62.4 lbs/ft3 Advanced Groundwater Method: None

#### **Random Numbers**

Pseudo-random Seed: 10116

	e Impoundment				
(0)6	Analysis Description Cross Section RH-4 Seismic Loading Condition				ng Condition
sience	Drawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	Date	9/4/2016, 12:09:41 P	M	File Name	RH-4 Seis 2.slim

Random Number Generation Method: Park and Miller v.3

# **Surface Options**

Surface Type: Circular
Search Method: Grid Search
Radius Increment: 10
Composite Surfaces: Disabled
Reverse Curvature: Invalid Surfaces
Minimum Elevation: Not Defined
Minimum Depth: Not Defined

# Loading

Seismic Load Coefficient (Horizontal): 0.2377

# **Material Properties**

Property	Lean clay	Silty clay	Lean Clay with sand (dike)	Lean clay (dike)	Silty sand (SM)	Sandy wedge	Lean clay OG	Lean clay with sand
Color								
Strength Type	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb	Mohr-Coulomb
Unit Weight [lbs/ft3]	134.1	125.8	133.5	125.8	130	130	129.7	125.8
Cohesion [psf]	72	200	260	220	0	200	14.4	80
Friction Angle [deg]	30.4	33.7	30.6	30.4	33	30	28.7	29.6
Water Surface	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table	Water Table
Hu Value	1	1	1	1	1	1	1	1

# **List Of Coordinates**

#### **Water Table**

Х	Υ
2.77556e-017	393.033
55.19	393.033
71.2842	395.917
141	397
151.1	397
185.3	405.78
200.877	426.28
215	426.28

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description Cross Section RH-4 Seismic Loading Condition				
sience	Orawn By Scale Company Associated Engineers, Inc.				
SLIDE 6.039	9/4/2016, 12:09:41 PM		File Name	RH-4 Seis 2.slim	

#### **External Boundary**

Х	Υ
215	418.777
213.982	419.318
205.956	423.582
196.055	428.841
189.723	429.373
183.027	428.888
164.711	421.366
139.268	413.482
115.733	405.895
88.257	398.958
75.6909	396.706
71.2842	395.917
55.19	393.033
31.267	393.626
21.125	395.865
0	405.212
2.77556e-017	393.033
0	375.2
70.69	375.4
189.72	377.7
215	377.9
215	385.1
215	398.2
215	409.5

#### **Material Boundary**

х	Υ
141	400
141	397
145.463	397
151.1	397
163.168	409.053
173.086	418.959
177.455	423.323
183.027	428.888

х	Y
163.168	409.053



		BREC Reid	/HMPL Station (	CCR Surface	Impoundment
	Analysis Description	Cross	Section RH-4 Sei	smic Loading	Condition
e	Drawn By		Scale	Company	Associated Engineers, Inc.
	Date	9/4/2016, 12:09:41 P	M	File Name	RH-4 Seis 2 slim

189.8	409.4
215	409.5

#### **Material Boundary**

Х	Υ
151.1	397
189.8	398
215	398.2

#### **Material Boundary**

Х	Υ
141	397
70.7	392.8
36.8	391
70.7	389.5
79.6711	389.266
189.8	386.4
215	385.1

#### **Material Boundary**

X	Y
71.2842	395.917
141	397

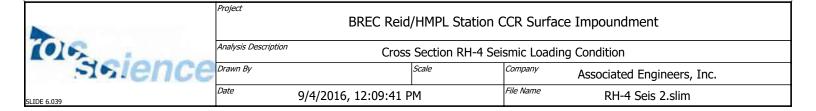
#### **Material Boundary**

X	Y
88.257	398.958
141	400

#### **Material Boundary**

Х	Y
173.086	418.959
189.8	419.1

Х	Y
189.8	419.1
213.982	419.318



# **Material Boundary**

х	Υ
177.455	423.323
189.8	423.4

# **Material Boundary**

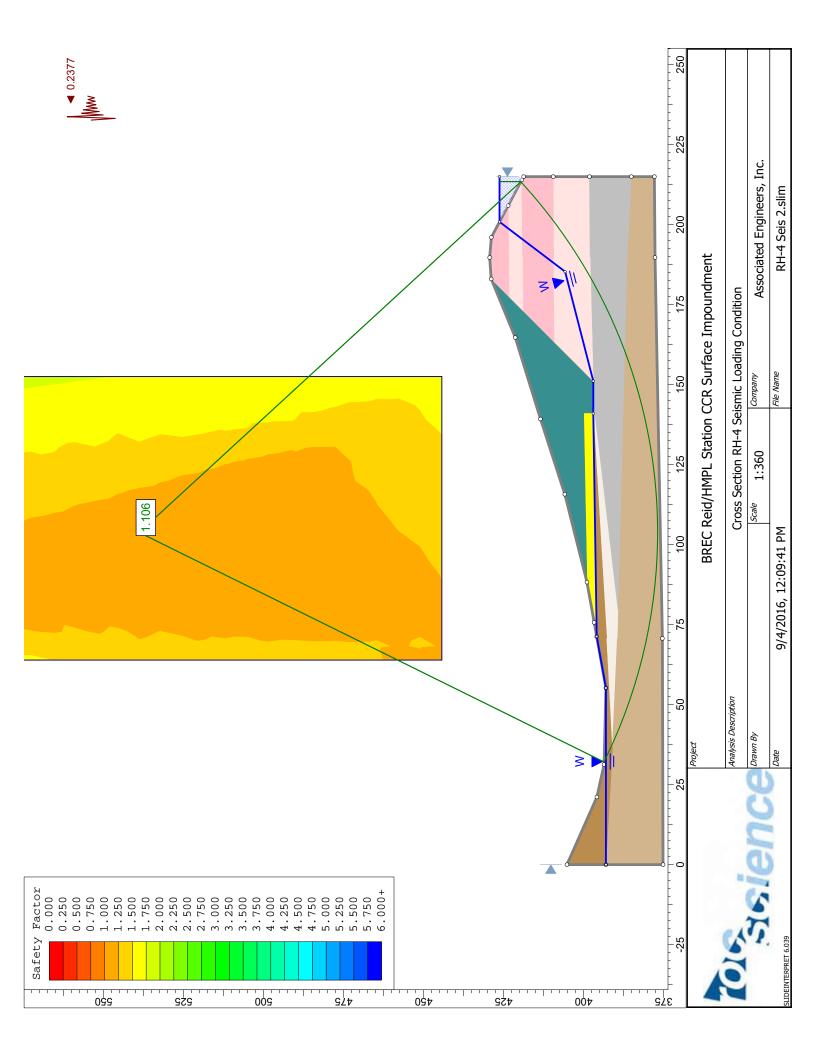
х	Y
189.8	423.4
205.956	423.582

# **Material Boundary**

Х	Υ
2.77556e-017	393.033
36.8	391

х	Y
79.6711	389.266
145.463	397

	BREC Reid/HMPL Station CCR Surface Impoundment				
(0)6	Analysis Description	Cross Section RH-4 Seismic Loading Condition			
sience	Drawn By		Scale	Company	Associated Engineers, Inc.
SLIDE 6.039	Date	9/4/2016, 12:09:41 PM		File Name	RH-4 Seis 2.slim







# Closure Plan for the Green Station CCR Surface Impoundment



# Big Rivers Electric Corporation Robert D. Green Generating Station

**Coal Combustion Residual Rule Compliance** 

# Closure Plan for the Green Station CCR Surface Impoundment

Prepared for

Big Rivers Electric Corporation Robert D. Green Generating Station Roberts, Kentucky

Revision 2 11/24/2020

Prepared by

Burns & McDonnell Engineering Company, Inc. Kansas City, Missouri

#### INDEX AND CERTIFICATION

#### **Big Rivers Electric Corporation** Closure Plan for the Green Station CCR Surface Impoundment

#### Report Index

<u>Chapter</u>	·	<u>Number</u>
Number	Chapter Title	of Pages
1.0	Introduction	1
		1
2.0	Details of Closure	5
3.0	Revisions and Amendments	1
4.0	Record of Revisions and Updates	1
Appendix A	Site Plan	1

#### Certification

I hereby certify, as a Professional Engineer in the State of Kentucky, that the information in this document was assembled under my direct supervisory control. This report is not intended or represented to be suitable for reuse by Big Rivers Electric Corporation or others without specific verification or adaptation by the Engineer.

11/24/2020

uel Yoder, P.E. Kentucky License #31964

Date:

YODER 31964

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2.0	DET	AILS OF	F CLOSURE	2-1
	2.1	Impour	ndment Description	2-1
		2.1.1	CCR Inventory and Extent	2-1
	2.2	Closure	re Method	2-1
			Final Cover System	
			Final Cover Schedule	
3.0	REV	ISIONS	AND AMENDMENTS	3-1
4.0	REC	ORD O	F REVISIONS AND UPDATES	4-1
APPI	ENDIX	( A - SI	TE PLAN	

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Figure 2-2: Typical Alternative Final Cover System	

#### LIST OF ABBREVIATIONS

Abbreviation Term/Phrase/Name

BREC Big Rivers Electric Corporation

CCR Coal Combustion Residuals

CFR Code of Federal Regulations

cm/sec centimeters per second

EPA Environmental Protection Agency

FGD Flue Gas Desulfurization

RCRA Resource Conservation and Recovery Act

USACE United State Army Corps of Engineers

U.S.C. United States Code

WMB Water Mass Balance

#### 1.0 INTRODUCTION

On April 17, 2015, the Environmental Protection Agency (EPA) issued the final version of the federal Coal Combustion Residuals (CCR) Rule to regulate the disposal of coal combustion residual materials generated at coal-fired units. The rule is administered as part of the Resource Conservation and Recovery Act (RCRA, 42 United States Code [U.S.C.] §6901 et seq.), using the Subtitle D approach.

Big Rivers Electric Corporation (BREC) is subject to the CCR Rule and as such must develop a Closure Plan per 40 Code of Federal Regulations (CFR) §257.102. This document serves as BREC's Closure Plan for the Green Station (Green) CCR Surface Impoundment (Ash Pond).

According to §257.102(b)(1), the Closure Plan must contain the following:

- A description of how the CCR unit will be closed.
  - For in-place closure: A description of the final cover system, the methods for installing the final cover system, and the methods for achieving compliance with the standards outlined in \$257.102(d).
  - o For closure by removal: A description of the procedures to remove the CCR and decontaminate the CCR unit as outlined in §257.102(c).
- An estimate of the maximum amount of material ever stored in the CCR unit over its active life.
- An estimate of the largest area of the CCR unit ever requiring a final cover as required by §257.102(d) at any time during the CCR unit's active life.
- A schedule for completing closure activities, including the anticipated year of closure and major milestones for permitting and construction activities.

The seal on this report certifies that this document meets the requirements of 40 CFR §257.102(b). This closure plan is in addition to, not in place of, any other applicable site permits, environmental standards, or work safety practices.

#### 2.0 DETAILS OF CLOSURE

#### 2.1 Impoundment Description

Green is a coal-fueled electric generating station near Robards, Kentucky. The plant consists of Unit 1 and Unit 2 which are respectively 250MW and 242MW (gross) units commercialized in 1979 and 1981 respectively. Units 1 and 2 burn a blend of bituminous coal (pulverized). The plant utilizes the Ash Pond to manage the CCR and non-CCR wastestreams. The Ash Pond was constructed when the plant was built and has been in service for the life of the plant. The CCR wastestreams that are managed in the Ash Pond include sluiced bottom ash, economizer ash and Flue Gas Desulfurization (FGD) wastewater. All fly ash is now handled dry. The various non-CCR wastewaters routed to the Ash Pond originate from the Unit 1 and 2 boiler sumps, metal cleaning wastes, clarifier blowdown, bottom ash hopper seal water, miscellaneous drains including roof drains, landfill leachate, and various stormwater sources. A site plan is included in Appendix A.

#### 2.1.1 CCR Inventory and Extent

Depth of impounded water and CCR is 3 feet and 18 feet (at respective locations of maximum impounded water and CCR depths). Elevation of impounded water and CCR is 396 feet and 400 feet, respectively, above mean sea level. These approximate depths and respective elevations are based on the most recent (October 2018) flight derived topographic contours and bathymetric survey data.

The remaining storage capacity is approximately 230,000 cubic yards (if CCR can be placed to the elevation of the current water surface). This volume was calculated based on the maximum allowable storage volume and the current volume of CCR stored in the facility based on the most recent bathymetric survey.

The approximate volume of CCR currently stored in the Ash Pond is 1,000,000 cubic yards. The maximum storage capacity is 1,230,000 cubic yards. This volume was calculated based on the most recent bathymetric survey, and the best available as-built data for the construction prior to placement of CCR.

#### 2.2 Closure Method

The CCR Rule allows for CCR Units to be closed through removal of CCR or by leaving CCR material in-place. BREC intends to close the existing Ash Pond using a hybrid approach by consolidating CCR material (approximately 1 million cubic yards) within the existing Ash Pond area. The CCR material would be consolidated along the south side of the existing pond, where it will be capped with a CCR compliant system. This area accounts for approximately 16 acres that will require a final cover.

The remaining 10 acres will be closed by removal of CCR. This area will then be used as a water mass balance pond (WMB) for processing and holding of landfill leachate and stormwater.

To facilitate construction of the new water mass balance (WMB) pond and the pond closure, the existing non-CCR wastestreams will need to be managed. The pond water level will be lowered as much as feasible after ceasing the receipt of CCR and the permanent cessation of the coal-fired boilers and prior to the construction contractor coming on site. When the construction contractor begins construction, the remaining non-CCR wastestreams (essentially site stormwater and landfill leachate after the boilers cease coal-fired operations) will be managed using a series of temporary berms, ditches, and pumps to divert site stormwater to other locations. This will likely require KPDES permit modifications following the permanent cessation of the coal-fired boilers operation and the remediation of the coal pile to discharge water from the existing coal pile runoff pond to the Green River. Alternatively, the Contractor may choose to maintain a small portion of the current Ash Pond footprint to continue to receive these flows and pump them through a temporary treatment system to the existing outfall structure. The sequencing of construction and means and methods for the water management will be determined by the construction contractor once a contract is finalized with BREC.

While managing the incoming stormwater and leachate flows, the Contractor will initiate grading and relocating CCR material for the WMB pond and pond closure and continue dewatering and removing the interstitial water in the CCR material (with drainage ditches or potentially an engineered dewatering system) so that the consolidated CCR material is stabilized to allow for the closure in place to be performed in phases to meet the performance standards as required by §257.102(d). The discharge will be directed to the KPDES permitted Outfall #009 (the Ash Pond outfall), with temporary treatment systems installed if required by the permit. As grading is completed in certain areas, the contractor will begin forming a 10-acre WMB Pond to treat remaining process and stormwater flows from Green prior to discharge. The approximate volume of CCR in the ash pond is 1,000,000 cubic yards. Approximately 400,000 cubic yards will be removed for the WMB pond construction. The Contractor will remove the CCR material from the portion of the impoundment that will receive the new WMB pond berm and begin placing fill for the new berm. While this fill is placed in lifts, the Contractor will continue removing CCR from the WMB portion of the pond and compacting it on the outside of the berm in the portion of the pond to be capped in place.

In addition to the new berm, the work for the WMB construction may consist of the installation of a HDPE geomembrane liner, 12-inches of protective cover material, and 18-inches of riprap for the pond side slopes. This liner system will be finalized during detailed design and permitting for the WMB Pond.

Concurrent to the WMB Pond lining work, the contractor will begin placing the final cover system over the closed in place CCR material.

When the grading is complete and the WMB is in service the remaining stormwater and non-CCR wastestreams (landfill leachate) will be rerouted to the new pond and will continue to discharge through the existing KPDES outlet. The contractor will then finalize construction by seeding and stabilizing all disturbed areas.

#### 2.2.1 Final Cover System

Pursuant to §257.102(d)(3)(i), the final cover system must be designed and constructed to meet the following criteria:

- Have a permeability less than or equal to the permeability of any bottom liner system or natural subsoils present, or a permeability no greater than 1x10<sup>-5</sup> centimeters per second (cm/sec), whichever is less.
- The infiltration of liquids through the closed CCR unit must be minimized by use of an infiltration layer that contains a minimum of 18 inches of earthen material.
- The erosion of the final cover system must be minimized by use of an erosion layer that contains a minimum of six inches of earthen material capable of sustaining native plant growth.
- The disruption of the integrity of the final cover system must be minimized through a design that accommodates settling and subsidence.
- The owner or operator may select an alternative final cover system design, provided the alternative final cover system meets the above requirements.

The typical final cover system will consist of an 18-inch thick, earthen (clay) infiltration layer with a 6-inch thick topsoil erosion layer capable of sustaining native plant growth. A cross section of the typical final cover system is shown in Figure 2-1.

6" TOPSOIL AND SEED

18" INFILTRATION LAYER (CLAY)

Figure 2-1: Typical Final Cover System

Because the Ash Pond bottom was not constructed to meet low permeability specifications, the cover system will be designed to meet the permeability limit of 1 x  $10^{-5}$  cm/sec noted in the CCR Rule. The required permeability value will be achieved using clay with properties meeting the developed specification criteria. The infiltration layer will be constructed according to proper quality control methods.

An alternative final cover system may be utilized in lieu of the above described typical final cover system, over the Ash Pond (see Figure 2-2). This alternative system uses a geomembrane component to achieve the minimum permeability requirements of the CCR Rule, rather than relying on the permeability of the 18-inches of infiltration material.

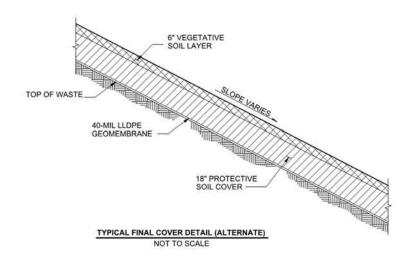


Figure 2-2: Typical Alternative Final Cover System

#### 2.2.1.1 Geometry and Stormwater Management

The geometry and stormwater management controls of the closed impoundment will allow the CCR unit to meet the following requirements as outlined in §257.102(d) of the CCR Rule:

- Control, minimize or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere.
- Prevent future impoundment of water.
- Provide for slope stability to protect against sloughing or movement of the final cover system.

The final closure system grade will slope at a minimum of 2.0 percent over the capped Ash Pond surface to prevent the collection of standing water and limit the velocity of storm water runoff to reduce the

potential for soil erosion. Intermediate swales will be utilized with a minimum slope of 2.0 percent, as appropriate, to limit the maximum overland flow distance, thereby limiting the chance for ponding water, as well as limiting the infiltration of run-off. The intermediate swales will collect area runoff and convey it to the WMB Pond which will discharge through an KPDES permitted outlet.

The period for greatest soil erosion will be immediately after placement of the topsoil material before vegetation is established. Manufactured erosion control products, as well as a seed mix containing quickgrowth seed varieties, will aid in minimizing erosion during this timeframe.

#### 2.2.1.2 Integrity of the Final Cover

Settling and subsidence of the final cover system is expected to be minimal. Settlement would potentially be caused by consolidation of the CCR material or underlying natural subsoils under new loads from construction activities; however, this settlement will likely occur during site grading activities and is expected to be minimal after the cover is installed. CCR material will be placed in a controlled manner to minimize post-construction installation settlement. The underlying natural subsoils at the site are not prone to long-term settlement.

#### 2.2.2 Final Cover Schedule

According to §257.101 of the CCR Rule, closure of the existing impoundment must commence no later than 6 months following the date on which a closure event is triggered, or no later than 30 days following the last known receipt of CCR or non-CCR wastewater by the impoundment. The current schedule is for BREC to cease sending CCR to the Ash Pond in the late spring of 2022. BREC intends to prepare design drawings, obtain the appropriate permits and award the contract for closure prior to removing the pond from service so that closure can commence within 30 days.

The estimated closure schedule is as follows:

Mobilization June 2022

Pond Dewatering June 2022 – August 2022 Cut/Fill and Removal of CCR August 2022 – May 2023

Winter Shutdown December 2022 – March 2023

Install final cover system May 2023 – July 2023 WMB Construction May 2023 – August 2023

Topsoil and Seeding August 2023 – September 2023

Deadline to complete closure October 17, 2023

#### 2.2.2.1 Closure Completion

The CCR Rule does not define "closure complete" for CCR units. For the purposes of this Closure Plan, closure of the impoundment is considered complete when the final cover system is installed, and the applicable construction completion documentation is finalized.

Within 30 days of completion of closure of the impoundment, BREC must prepare a notification of closure of the impoundment and place it in the facility's CCR Operating Record and on BREC's CCR public website. This notification shall include certification by a qualified professional engineer in the State of Kentucky verifying that closure has been completed in accordance with this Closure Plan and the requirements of §257.102. Additionally, BREC must record a notation on the deed to the property following completion of closure of the impoundment in accordance with §257.102(i). The purpose of this notation is to inform any potential future owner of the property of the previous use of the land, and that the land is restricted by post-closure care requirements.

#### 3.0 REVISIONS AND AMENDMENTS

The initial Closure Plan was placed in the CCR Operating Record on October 11, 2016. If the Closure Plan is revised, the written Closure Plan will be amended no later than 30 days following the triggering event. Additionally, the written Closure Plan will be amended at least 60 days prior to a planned change in the operation of the Impoundment, or no later than 60 days after an unanticipated event. The initial Closure Plan and any amendment will be certified by a qualified professional engineer in the State of Kentucky for meeting the requirements of §257.102 of the CCR Rule. All amendments and revisions must be placed on the CCR public website within a reasonable amount of time following placement in the facility's CCR Operating Record. A record of revisions made to this document is included in Section 4.0 of this document.

# 4.0 RECORD OF REVISIONS AND UPDATES

Date	Revisions Made	By Whom
10/11/2016	Initial Issue	Associated Engineers, Inc.
09/13/2017	Revision 1	Associated Engineers, Inc.
11/24/2020	Revision 2 – Updated schedule and added detail to closure method	Burns & McDonnell





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BREC GREEN STATION CCR & ELG COMPLIANCE PROJECT ASH POND CLOSURE SITE PLAN contract drawing SK-C001 sheets file 126878SK-C001.DGN

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