COMMONWEALTH OF KENTUCKY BEFORE THE PUBLIC SERVICE COMMISSION

In the matter of:

THE APPLICATION OF EAST KENTUCKY NETWORK,)
LLC FOR THE ISSUANCE OF A CERTIFICATE OF)
PUBLIC CONVENIENCE AND NECESSITY TO) CASE NO. 2020-00223
CONSTRUCT A TOWER IN LETCHER COUNTY,)
KENTUCKY)

East Kentucky Network, LLC, d/b/a Appalachian Wireless, was granted authorization to provide cellular service in the KY-10 Cellular Market Area (CMA452) by the Federal Communications Commission (FCC). FCC license is included as Exhibit 1. East Kentucky Network, LLC merger documents were filed with the Commission on February 2, 2001 in Case No. 2001-022. East Kentucky Network, LLC is a Kentucky Limited Liability Company that was organized on June 16, 1998. East Kentucky Network, LLC is in good standing with the state of Kentucky.

In an effort to improve service in Knott County, pursuant to KRS 278.020 Subsection 1 and 807 KAR 5:001, East Kentucky Network, LLC is seeking the Commission's approval to construct a 100-foot telecommunications tower on a tract of land located at 822 Highway 2034, Whitesburg, Letcher County, Kentucky (37°07'11.1264"N 82°47'36.7491"W). A map and detailed directions to the site can be found in Exhibit 7.

Construction of the proposed tower is required by public convenience and necessity. Due to increasing demand for telecommunications service, the proposed tower is necessary to provide adequate coverage. The proposed tower will improve service in Letcher County by providing an interconnection between East Kentucky Network, LLC's other sites thereby forming a cohesive network.

Exhibit 2 is a list of all Property owners or residents according to the Property Valuation Administrator's record who own property within 500 feet of the proposed tower and all property owners who own property contiguous to the property upon which construction is proposed in accordance with the Public Valuation Administrator's records.

Pursuant to 807 KAR 5:063 Section 1(1)(I), Section 1(1)(m), and Section 2, all affected property owners according to the Property Valuation Administrator's record who own property within 500 feet of the proposed Tower or contiguous to the property upon which construction is proposed were notified by certified mail return receipt requested of East Kentucky Network, LLC's proposed construction and informed of their right to intervene. They were given the docket number under which this application is filed. Enclosed in Exhibit 2 is a copy of that notification.

Letcher County has no formal local planning unit. In absence of this unit, the Letcher County Judge Executive's office was notified by certified mail, return receipt requested of East Kentucky Network, LLC's proposal and informed of their right to intervene. The Letcher County Judge Executive's Office was also given the docket number under which this application is filed. Enclosed in Exhibit 3 is a copy of that notification.

Notice of the location of the proposed construction was published in The Mountain Eagle, July 15, 2020, edition, copies of which are enclosed as Exhibit 3. The Mountain Eagle is the newspaper with the largest circulation in Letcher County.

A geologist was employed to determine soil and rock types and to ascertain the distance to solid bedrock. The geotechnical report is enclosed as Exhibit 4.

A copy of the tower design information is enclosed as Exhibit 5. The proposed tower has been designed by engineers at TAPP, Inc. and will be constructed under their supervision. Their qualifications are evidenced in Exhibit 5 by the seal and signature of the registered professional engineer responsible for this project.

The tower will be erected by S & S Tower Services of St. Albans, West Virginia. S & S Tower Services has vast experience in the erection of communications towers. Their qualifications are described in Exhibit 13.

FAA approval and KAZC confirmation that a permit is not required are included as Exhibit 6.

No Federal Communications Commission approval is required prior to construction of this facility. Once service is established from this tower we must immediately notify the Federal Communications Commission of its operation. Prior approval is needed only if the proposed facility increases the size of the cellular geographic service area. This cell site will not expand the cellular geographic service area.

East Kentucky Network, LLC will finance the subject Construction with earned surplus in its General Fund.

Estimated Cost of Construction \$350,000.00 Annual Operation Expense of Tower \$12,500.00

Two notice signs meeting the requirements prescribed by 807 KAR 5:063, Section 1(2), measuring at least two (2) feet in height and four (4) feet in width and containing all required language in letters of required height, have been posted, one at a visible location on the proposed site and one on the nearest public road. The two signs were posted on July 9, 2020, and will remain posted for at least two weeks after filing of this application as specified.

Enclosed in Exhibit 8 is a copy of East Kentucky Network, LLC's Memorandum of Lease for the site location along with a lot description.

The proposed construction site is on an existing telecommunications site.

East Kentucky Network, LLC's operation will not affect the use of nearby land nor its value. No more suitable site exists in the area. A copy of the search area map is enclosed in Exhibit 7. No other tower capable of supporting East Kentucky Network, LLC's load exists in the general area; therefore, there is no opportunity for co-location of our facilities with anyone else.

Enclosed, and filed as Exhibit 9 is a survey of the proposed tower site signed by a Kentucky registered professional engineer.

Exhibit 10 is a map in one (1) inch equals 200 feet scale identifying every structure and every owner of real estate within 500 feet of the proposed tower and all property owners who own contiguous property to the property upon which construction is proposed.

Exhibit 11 contains a vertical sketch of the tower supplied by James W. Caudill, Kentucky registered professional engineer.

Enclosed as Exhibit 12 is a list of utilities, corporation, or persons with whom the tower is likely to compete.

[THE REMAINDER OF THIS PAGE INTENTIONALLY LEFT BLANK.]

WHEREFORE, Applicant, having met the requirements of KRS 278.020(1), 278.650, and 278.665 and all applicable rules and regulations of the PSC, respectfully requests that the PSC accept the foregoing Application for filing and grant a Certificate of Public Convenience and Necessity to construct and operate the proposed tower.

The foregoing document was prepared by Krystal Branham, Regulatory Compliance Attorney at East Kentucky Network, LLC d/b/a Appalachian Wireless. All related questions or correspondence concerning this filing should be mailed to East Kentucky Network, LLC d/b/a/Appalachian Wireless, 101 Technology Trail, Ivel, KY 41642.

SUBMITTED BY: DATE: 7

Lynn Haney, Regulatory Compliance Director

APPROVED BY: WA Sillum DATE: 7/10/2020

W.A. Gillum, General Manager

ATTORNEY: Kystal Bianham DATE: 7/13/2020

Hon. Krystal Branham, Attorney

CONTACT INFORMATION:

W.A. Gillum, General Manager Phone: (606) 477-2355, Ext. 111 Email: wagillum@ekn.com

Lynn Haney, Regulatory Compliance Director

Phone: (606) 477-2355, Ext. 1007

Email: lhaney@ekn.com

Krystal Branham, Attorney Phone: (606) 477-2355 ext. 1009 Email: kbranham@ekn.com

Mailing Address:

East Kentucky Network, LLC d/b/a Appalachian Wireless 101 Technology Trail Ivel, KY 41642

	FCC License
2	Copies of Cell Site Notices to Land Owners
3	Notifications of County Judge Executive and Newspaper Ad
4	Universal Soil Bearing Analysis
5	Tower Design
6	FAA Determination
7	Driving Directions from County Court House and Map to Suitable Scale
8	Memorandum of Lease for Proposed Site with Legal Description
9	Survey of Site Signed/Sealed by Professional Engineer Registered in State of Kentucky
10	Site Survey Map with Property Owners Identified in Accordance with PVA of County
11	Vertical Profile Sketch of Proposed Tower
12	List of Competitors
13	Qualifications
14	
15	

ULS License

Cellular License - KNKN809 - East Kentucky Network, LLC d/b/a Appalachian Wireless

Call Sign KNKN809 Radio Service CL - Cellular Status Active Auth Type Regular

Market

Market CMA452 - Kentucky 10 - Powell Channel Block B
Submarket 0 Phase 2

Dates

Grant 08/30/2011 Expiration 10/01/2021

Effective 10/10/2014 Cancellation

Five Year Buildout Date

10/17/1996

Control Points

1 US Route 23, FLOYD, Harold, KY

P: (606)478-2355

Licensee

FRN 0001786607 Type Limited Liability Company

Licensee

East Kentucky Network, LLC d/b/a Appalachian P:(606)477-2355

Wireless

101 Technology Trail Ivel, KY 41642

Contact

Lukas, Nace, Gutierrez & Sachs, LLP P:(703)584-8665
Pamela L Gist Esq F:(703)584-8695
8300 Greensboro Drive E:pgist@fcclaw.com

McLean, VA 22102

Ownership and Qualifications

Radio Service Type Mobile

Regulatory Status Common Carrier Interconnected Yes

Alien Ownership

The Applicant answered "No" to each of the Alien Ownership questions.

Basic Qualifications

The Applicant answered "No" to each of the Basic Qualification questions.

1 of 2 3/6/18, 3:33 PM

Demographics

Race

Ethnicity

Gender

EXHIBIT 2 - LIST OF PROPERTY OWNERS

Statement Pursuant to Section 1 (1) (I) 807 KAR 5:063

Section 1 (1)(I) 1. The following is a list of every property owner who according to property valuation administrator's records, owns property within 500 feet of the proposed tower and each have been: notified by certified mail, return receipt requested, of the proposed construction,

Section 1 (1)(1) 2. Every person listed below who, according to the property valuation administrator's records, owns property within 500 feet of the proposed tower has been: Given the Commission docket number under which the application will be processed: and

Section 1 (1)(I) 3. Every person listed below who, according to property valuation administrator's records owns property within 500 feet of the proposed tower has been: Informed of his right to request intervention.

Section 2. If the construction is proposed for an area outside the incorporated boundaries of a city, the application shall state that public notices required by Section 1(1)(L) have been sent to every person who, according to the property valuation administrator, owns property contiguous to the property upon which the construction is proposed

LIST OF PROPERTY OWNERS

Paul D. Brooks P.O. Box 279 Whitesburg, KY 41858

W.D. Childers Realty P.O. Box 430 Whitesburg , KY 41858

Isaac Hall and Ruby White Hall 304 Cedar Creek Road Pikeville, KY 41501

Thacker-Grigsby Telephone Company P.O. Box 789 Hindman, KY 41822

Letcher County Board of Education 224 Park Street Whitesbug, KY 41858 Mae C. Amburgey P.O. Box 54 Ermine, KY 41815

Dan Combs Box 176 Whitesburg , KY 41858

Stephen Gregory Amburgey C/O Melissa Crovetti 1981 Haven Lane Green Oaks, IL 60048

Combs Cemetery Whitesburg , KY 41858

Fultz Properties Ermine, LLC P.O. Box 43 Mayking, KY 41837

> James E. Sturgill Box 28 Ermine, KY 41815

Interproperty Investments, Inc. 2083 Manor Drive Lexington, KY 40502

> Cornerstone Church P.O. Box 904 Whitesburg, KY 41858

> Childers Oil Co. Inc. P.O. Box 430 Whitesburg, KY 41858

> William Ronald Cain II 906 East Mt. Vernon St. Somerset, KY 42501





PUBLIC NOTICE

July 16, 2020

Paul D. Brooks P.O. Box 279 Whitesburg , KY 41858

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2020-00233)

East Kentucky Network, LLC d/b/a Appalachian Wireless has applied to the Public Service Commission of Kentucky for a Certificate of Public Convenience and Necessity to construct and operate a new facility to provide cellular telecommunications service in Letcher County. The facility will include a 100'-foot self-supporting tower with attached antennas extending upwards, and an equipment shelter located on a tract of land at 822 Highway 2034, Whitesburg, Letcher County, Kentucky. A map showing the location of the proposed new facility is enclosed. This notice is being sent to you because you may own property within a 500' radius of the proposed tower or own property contiguous to the property upon which construction is proposed.

The Commission invites your comments regarding the proposed construction. You also have the right to intervene in this matter. The Commission must receive your initial communication within 20 days of the date of this letter as shown above.

Your comments and request for intervention should be addressed to: Executive Director's Office, Public Service Commission of Kentucky, P.O. Box 615, Frankfort, KY 40602. Please refer to Case No. 2020-00233 in your correspondence.

If you have any questions for East Kentucky Network, LLC, please direct them to my attention at the following address: East Kentucky Network, LLC, 101 Technology Trail, Ivel, KY 41642 or call me at 606-477-2355, Ext. 1007.

Sincerely,

Lynn Haney, CPA

Regulatory Compliance Director

Lyun Haney





PUBLIC NOTICE

July 16, 2020

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Sincerely,

Lynn Haney, CPA

Regulatory Compliance Director

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July 16, 2020

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Regulatory Compliance Director





PUBLIC NOTICE

July 16, 2020

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PUBLIC NOTICE

July 16, 2020

Mae C. Amburgey P.O. Box 54 Ermine, KY 41815

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Sincerely,

Lynn Haney, CPA

Regulatory Compliance Director





VIA: <u>U.S. CERTIFIED MAIL</u>

PUBLIC NOTICE

July 16, 2020

Dan Combs Box 176 Whitesburg , KY 41858

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Lynn Haney, CPA

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VIA: <u>U.S. CERTIFIED MAIL</u>

PUBLIC NOTICE

July 16, 2020

Combs Cemetery Whitesburg, KY 41858

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PUBLIC NOTICE

July 16, 2020

Fultz Properties Ermine, LLC P.O. Box 43 Mayking, KY 41837

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PUBLIC NOTICE

July 16, 2020

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Sincerely,

Lynn Haney, CPA

Regulatory Compliance Director

Egur Hancy





PUBLIC NOTICE

July 16, 2020

Cornerstone Church P.O. Box 904 Whitesburg, KY 41858

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Childers Oil Co. Inc. P.O. Box 430 Whitesburg, KY 41858

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William Ronald Cain II 906 East Mt. Vernon St. Somerset, KY 42501

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2020-00233)

East Kentucky Network, LLC d/b/a Appalachian Wireless has applied to the Public Service Commission of Kentucky for a Certificate of Public Convenience and Necessity to construct and operate a new facility to provide cellular telecommunications service in Letcher County. The facility will include a 100'-foot self-supporting tower with attached antennas extending upwards, and an equipment shelter located on a tract of land at 822 Highway 2034, Whitesburg, Letcher County, Kentucky. A map showing the location of the proposed new facility is enclosed. This notice is being sent to you because you may own property within a 500' radius of the proposed tower or own property contiguous to the property upon which construction is proposed.

The Commission invites your comments regarding the proposed construction. You also have the right to intervene in this matter. The Commission must receive your initial communication within 20 days of the date of this letter as shown above.

Your comments and request for intervention should be addressed to: Executive Director's Office, Public Service Commission of Kentucky, P.O. Box 615, Frankfort, KY 40602. Please refer to Case No. 2020-00233 in your correspondence.

If you have any questions for East Kentucky Network, LLC, please direct them to my attention at the following address: East Kentucky Network, LLC, 101 Technology Trail, Ivel, KY 41642 or call me at 606-477-2355, Ext. 1007.

Sincerely.

Lynn Haney, CPA

Regulatory Compliance Director



dba Appalachian Wireless 101 Technology Trail Ivel, KY 41642 Phone: 606-477-2355

Phone: 606-477-2355 Fax: 606-791-2225



To: The Mountain Eagle From: Raina Helton
Attn: Classifieds Regulatory Compliance Assistant

Email: Pwalker_eagle@hotmail.com Date: July 9, 2020

Re: PUBLIC NOTICE ADVERTISEMENT Pages: 1

Please place the following Public Notice Advertisement in The Mountain Eagle to be ran on July 15, 2020.

PUBLIC NOTICE:

RE: Public Service Commission of Kentucky (CASE NO. 2020-00223)

Public Notice is hereby given that East Kentucky Network, LLC, dba Appalachian Wireless has applied to the Kentucky Public Service Commission to construct a cellular telecommunications tower on a tract of land located at 822 Highway 2034, Whitesburg, Letcher County, Kentucky. The proposed tower will be a 100 foot monopole tower with attached antennas. If you would like to respond to this notice, please contact the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to Case No. 2020-00223.

If you have any questions about the placement of the above mentioned notice, please call me at 606-477-2375, ext. 1005.

Thank you,

Raina Helton Regulatory Compliance Assistant

The message above and the information contained in the documents transmitted are confidential and intended only for the person(s) named above. Dissemination, distribution or copying of this communication by anyone other than the person(s) named above is prohibited. If you have received this communication in error, please notify us immediately by telephone and return the original message to us at the address listed above via regular mail. Thank you.





July 16, 2020

Terry Adams, Judge Executive 156 Main Street, Suite 107 Whitesburg, KY 41858

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2020-00233)

East Kentucky Network, LLC d/b/a Appalachian Wireless has applied to the Public Service Commission of Kentucky for a Certificate of Public Convenience and Necessity to construct and operate a new facility to provide cellular telecommunications service in Letcher County. The facility will include a 100-foot monopole tower with attached antennas extending upwards, and an equipment shelter located on a tract of land at 822 Highway 2034, Whitesburg, Letcher County, Kentucky. A map showing the location of the proposed new facility is enclosed. This notice is being sent to you because you are the County Judge Executive of Letcher County.

The Commission invites your comments regarding the proposed construction. You also have the right to intervene in this matter. The Commission must receive your initial communication within 20 days of the date of this letter as shown above.

Your comments and request for intervention should be addressed to: Executive Director's Office, Public Service Commission of Kentucky, P.O. Box 615, Frankfort, KY 40602. Please refer to Case No. 2020-00233 in your correspondence.

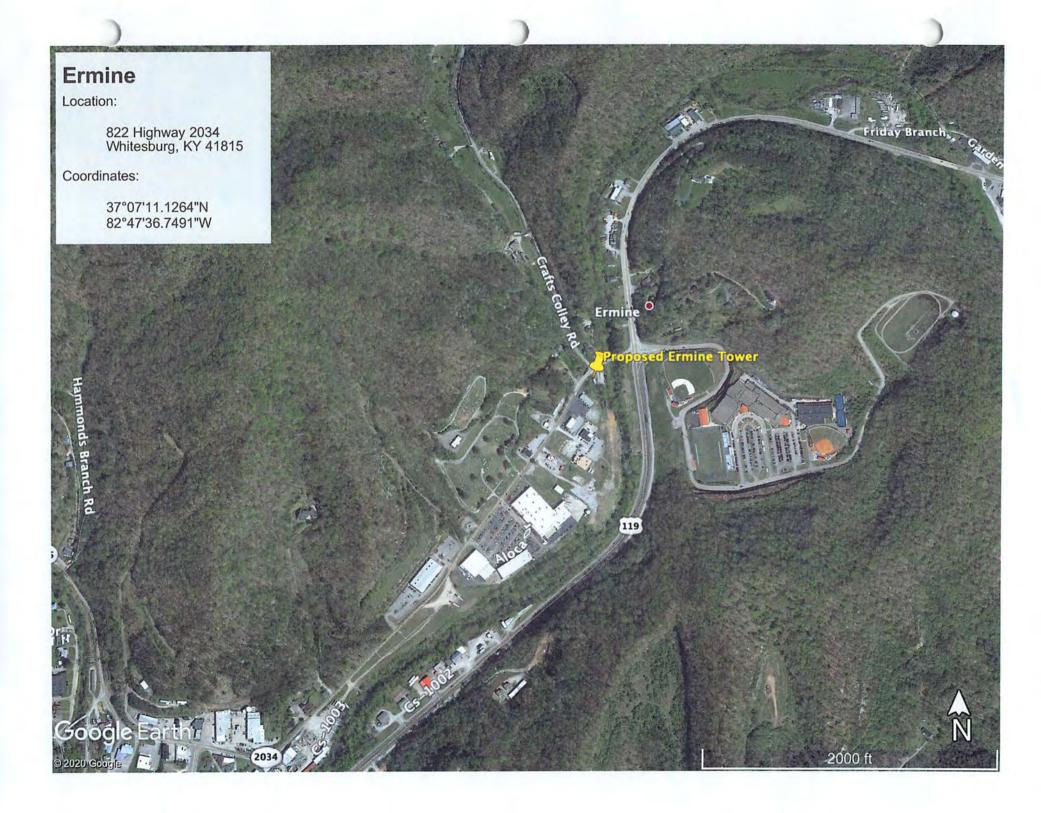
If you have any questions for East Kentucky Network, LLC, please direct them to my attention at the following address: East Kentucky Network, LLC, 101 Technology Trail, Ivel, KY 41642 or call me at 606-477-2355, Ext. 1007.

Sincerely,

Lynn Haney

Regulatory Compliance Director

Lym Henry





230 Swartz Drive • Hazard • Kentucky • 41701 Phone (606) 551-1050

EAST KENTUCKY ENGINEERING, LLC.

APPALACHIAN WIRELESS
Geotechnical Investigation on the
Ermine Tower Site
Letcher County, Kentucky
EKYENG Project No. 165-000-0105

PREPARED FOR: Appalachian Wireless. 101 Technology Trail Ivel, Kentucky 41642

PREPARED BY: Richard Dirk Smith PE, PLS President East Kentucky Engineering 230 Swartz Drive Hazard, Kentucky 41701

MARRITHMAN

, 20215, April 14th, 2020



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- 2.0 PROJECT DESCRIPTION
- 3.0 SITE DESCRIPTION & HISTORICAL MINING
 - 3.1 GENERAL INFORMATION
 - 3.2 SURFACE MINING
 - 3.3 UNDERGROUND MINING
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 - 4.1 SITE INFORMATION
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- I GENERAL
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APPENDIX B - CORE PHOTOGRAPHS

APPENDIX C- SEISMIC DATA

APPENDIX D - PHOTOGRAPHS

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EXECUTIVE SUMMARY

A geotechnical investigation has been performed on the Ermine Tower Site, located in Letcher County, Kentucky. This site is readily accessible. A location map is shown in Figure 1 of this report. One (1) boring was advanced to a maximum depth of 33.0 ft. The following geotechnical considerations were identified:

- The boring utilized for this study encountered fill material to a depth of 14.0, silts and clays to a depth of 23 feet, and sandstone to a depth of 33.0 ft.
- The estimated maximum base elevation of the tower is to be 25 to 32 ft in depth within the sandstone unit.
- This site is in a blacktop parking lot, next to an existing telecommunications building, in Ermine, KY.
- The allowable bearing capacities are estimated at six tsf on this sandstone unit from 1150' to 1143'. See Section 5.2 for additional recommendations.
- The 2015 International Building Code seismic site classification for this site is "A."
- If, during the foundation design, it becomes necessary to lower or raise the footer, alternate design recommendations can be provided by EKYENG.
- Close monitoring of the construction operations discussed herein will be critical in achieving the design subgrade support. We, therefore, recommend that EKYENG is retained to monitor this portion of the work.

This executive summary is included to provide a general overview of the project and should not be relied upon except for the purpose it was prepared. Please rely on the complete report for the information on the findings, recommendations, and all other concerns.



1. INTRODUCTION

East Kentucky Engineering (EKYENG) was retained by Mr. Stanton Neece of Appalachian Wireless to prepare a geotechnical engineering report for the proposed tower site located on the Ermine Property, in Letcher County, Kentucky. A site location map is shown in Figure No. 1.

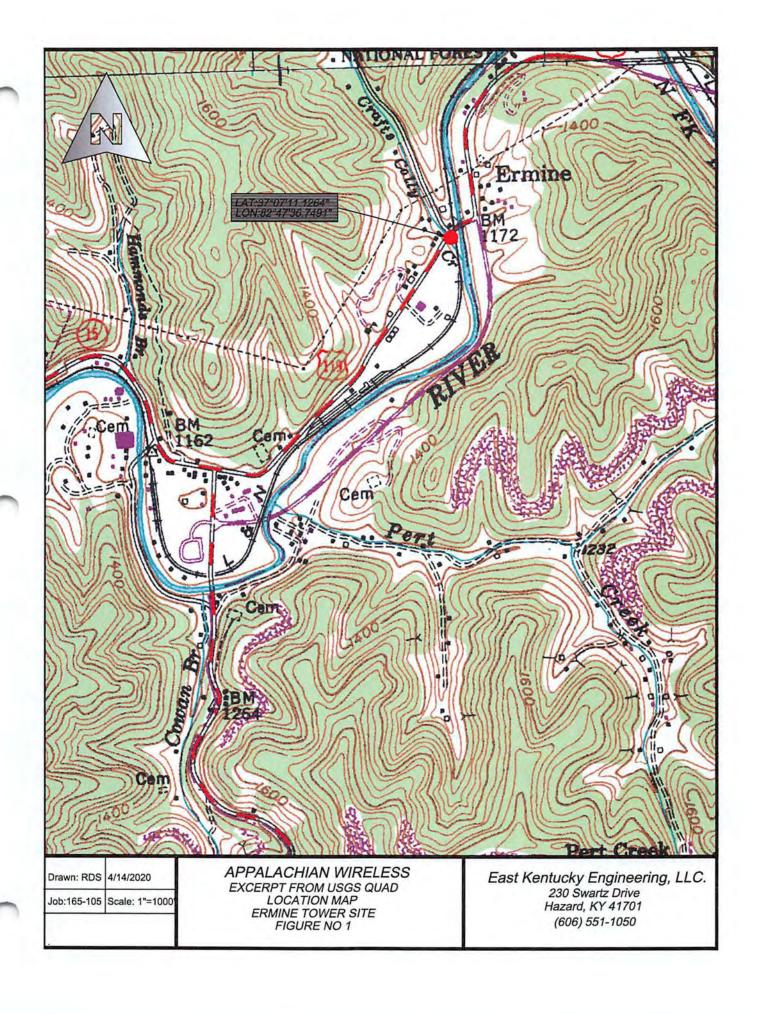
One (1) boring was advanced to a maximum depth of 33.0 ft. Horn and Associates, Inc. provided drilling services to obtain this boring. Logs of the boring along with a boring location plan are included in Appendix A and Appendix D. The purpose of these services is to provide information and geotechnical engineering recommendations about subsurface conditions, earthwork, seismic considerations, groundwater conditions and foundation design.

2.0 PROJECT DESCRIPTION

The proposed communication facility will consist of a self-supporting tower of undetermined height and ancillary support areas. The footing area is estimated to be an 8 ft diameter circular pier, with an estimated base of the tower footer elevation between 1150' to 1143'. Based on the information provided, we estimate the structural loads will be like the following conditions;

CONDITION	LOAD	
Total Shear	40 Kips	
Axial Load	50 Kips	

We anticipate that overturning will govern the structural design. If the loading is significantly different than these expected values, EKYENG should be notified to re-evaluate the recommendations provided in this report.





3.0 SITE DESCRIPTION & HISTORICAL MINING

3.1 GENERAL INFORMATION

The site location is in a blacktop parking lot, next to an existing telecommunications building, in Letcher County, Kentucky. The current surface elevation is approximately 1176 ft. Research on the historical mining was conducted by obtaining previous mine license maps from the "Kentucky Mine Mapping Information System" (KMMIS).

3.2 SURFACE MINING

No issues from surface mining activities are expected at this site location.

3.3 UNDERGROUND MINING

No underground mines were found within the vicinity of this site. Therefore, no subsidence issues are anticipated.

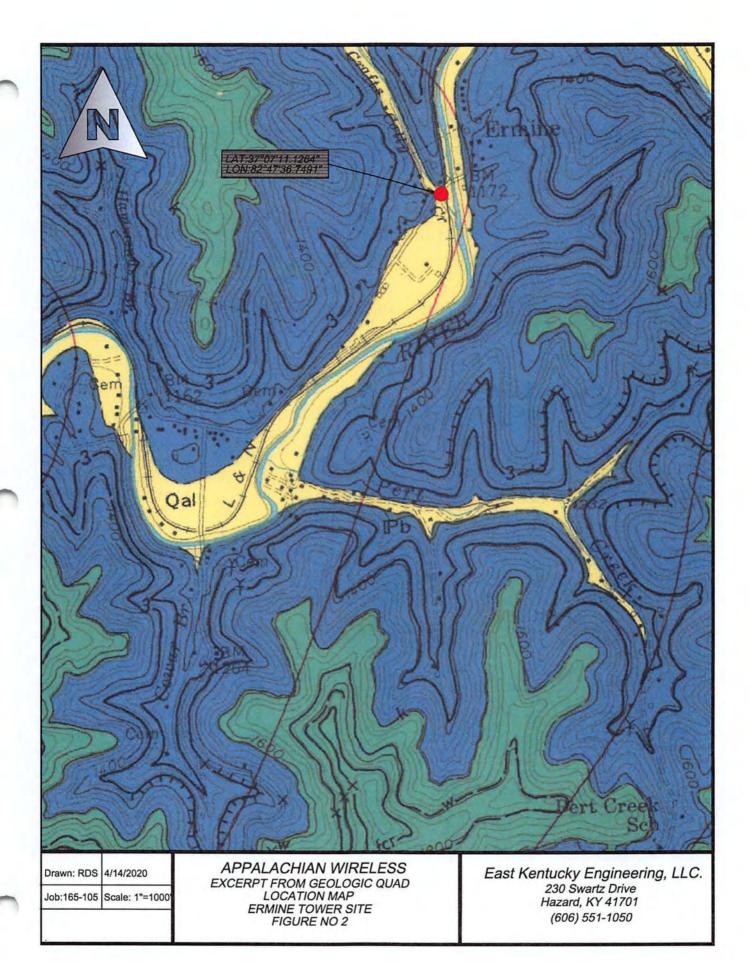
3.4 FLOOD HAZARD

A flood determination was conducted by EKYENG. For this determination, the FEMA Flood Map Service was reviewed for this location. The flood map for the selected area is number 21133C0231C-210140. The flood zone for this area is Zone A and is an area without a base flood elevation. A FIRMette map is included in Appendix C of this report. A permit application to construct along a stream has been submitted to the Kentucky Division of Water to address this construction.

4.0 FIELD EXPLORATION

4.1 SITE INFORMATION

The proposed site is located in a blacktop parking lot in Ermine, KY, next to an existing telecommunications building, in Letcher County, Kentucky. The site lies within the Whitesburg Quadrangle. The site is readily accessible by conventional





exploratory equipment. An estimated pad location was determined based on the information provided. Foundation dimensions were estimated to be an 8 ft. diameter circular pier footer for this report.

4.2 BORING DATA

One (1) boring was made in the relative position shown on the Site Map in Appendix D. The boring logs and resulting data are included in Appendix A. This boring was made with a track mounted boring rig using hollow-stem augers and employing standard penetration resistance methods (ASTM D-1586, which includes 140-pound hammer, 30-inch drop, and two-inch-O.D. split-spoon sampler) at maximum depth intervals of five feet or at major changes in stratum, whichever occurred first. The disturbed split-spoon samples were visually classified, logged, sealed in moisture-proof jars, and taken to the EKYENG laboratory for study. The depths where these "A"-type split-spoon samples were collected are noted on the boring logs. The results of the natural moisture contents by boring and interval are shown in Table 2.

TABLE 2
RESULTS OF NATURAL MOISTURE CONTENT TESTS (ASTM D-4643)

SAMPLE NO.	DEPTH INCREMENT, (FT.)	NATURAL MOISTURE CONTENT, %
B1 S-1	1.5 – 3.0	10.7%
B1 S-2	4.0 – 5.5	36.3%
B1 S-3	6.5 - 8.0	15.8%
B1 S-4	9.0 – 9.4	11.1%
B1 S-5	14.0 – 15.5	23.1%



The position at which the core was taken is indicated on the boring logs and shown on the sitemap in Appendix D. The corresponding Blow counts are shown in Table No. 3.

TABLE NO. 3 STANDARD PENETRATIONS

SAMPLE NO.	DEPTH	BLOW COUNT /	DESCRIPTION
	INCREMENT	RQD *	
B-1 S-1	1.5-3.0	7-11-5	Blacktop, DGA, Fill Material
B-1 S-2	4.0-5.5	0-1-0	Fill Material
B-1 S-3	6.5-8.0	2-10-6	Fill Material
B-1 S-4	9.0-9.4	50/4*	Fill Material
B-1 S-5	14.0-15.5	WH**	Very Soft Silts
B-1 S-6	19.0-20.5	1-1-5	Very Soft Silts
B-1 R-1	23.0-33.0	5.0*	Gray Sandstone

^{**}weight of hammer only.

The boring encountered fill material to a depth of 14 ft, soft silts to a depth of 23.0 ft. The boring was extended by "NX" size rock core that was taken to confirm the presence of rock at the site and to determine its physical characteristics. The core was made with "NX" size diamond coring equipment. This boring is between 23.0 ft. and 33.0 ft in depth and consisted of gray sandstone. The position at which the core was taken is indicated on the boring logs and shown on the boring location map in Appendix D.

4.3 GROUNDWATER

Groundwater in Eastern Kentucky is characterized by water flowing through a system of internal fractures that lead to an alluvial aquifer near the bottom of valley floors. This proposed tower is located in the alluvial valley floor and ground



water was encountered at a depth of 20 feet. This is 3 ft above the underlying sandstone that is the proposed bearing foundation, therefore no issues are expected with groundwater.

4.4 SEISMIC SITE CLASSIFICATION

Based on the encountered soil conditions at the project site, the site classification was determined to be "Site Class A" per the 2015 Kentucky Building Code. In addition, an S_{DS} coefficient of 0.35 g was calculated, and arr S_{D1} coefficient of 0.127 g was also calculated for design based on the aforementioned building code.

5.0 DISCUSSION AND RECOMMENDATIONS

5.1 GENERAL

The structure will be a self-supporting free-standing monopole tower. Due to wind loading, lattice tower foundations can experience both vertical loads and horizontal loads. The vertical loads act in both an upward and downward direction as the tower attempts to overturn and can act in any directions.

5.2 DRILLED PIER FOUNDATION RECOMMENDATIONS

If drilled piers are used for foundation support, we recommend the following design parameters.



TABLE NO. 1

Approx. Depth (ft.)	Allowable Skin Friction (psf.)	Allowable End Bearing Pressure (psf.)	Effective Unit Weight (pcf.)	Cohesion (psf.)	Internal Angle of Friction (Degrees)
Asphalt W/Base 0 - 1.5	Ignore	Ignore	Ignore	Ignore	Ignore
Fill 0.0 - 14.0	150	Ignore	120	Ignore	27
Silts and Sands 14.0 to 23.0	75	Ignore	105	70	22
Gray Sandstone 23.0 – 33.0	2500	12,000	170		33

The skin friction and passive resistance have a factor of safety of 2. The allowable end bearing pressure has an approximate safety factor of 3. If the drilled piers are designed using the above design parameters and socketed into solid bedrock, settlements are not anticipated to exceed ¼ inch.

5.3 BURIED UTILITIES

Excavations for buried utility pipelines should follow the guidelines set forth in this report. Depending on the pipeline material, a minimum thickness of at least 0.5 feet of select fine-grained granular bedding material should be used beneath all below-grade pipes, with a minimum cover thickness of at least 3 feet to afford an "arching" effect and reduce stresses on the pipe. The cover thickness may be reduced if the external loading condition on the pipe is relatively light or if the pipe is designed to withstand the external loading condition. It is not recommended that "pea-gravel" or other "open-work" aggregates be used for trench backfill since these materials are nearly impossible to compact and tend to pond water within their interstices.



6.0 WARRANTY

Our professional services have been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. No other warranty, express or implied, is made.

While the services of EKYENG are a valuable and integral part of the design and construction teams, we do not warrant, guarantee, or insure the quality or completeness of services provided by other members of those teams, the quality, completeness, or satisfactory performance of construction plans and specifications which we have not prepared, nor the ultimate performance of building site materials.

6.1 SUBSURFACE EXPLORATION

Subsurface exploration is normally accomplished by test borings, although test pits are sometimes employed. The method of determining the boring location and the surface elevation at the boring is noted in the report and is presented on the Boring Location Plan or on the boring log. The location and elevation of the boring should be considered accurate only to the degree inherent with the method used.

The boring log includes sampling information, description of the materials recovered, approximate depth of boundaries between soil and rock strata and groundwater data. The boring log represents conditions specifically at the location and time the boring was made. The boundaries between different soil strata are indicated at specific depths; however, these depths are in fact approximate and are somewhat dependent upon the frequency of sampling (The transition between soil strata is often gradual). Free groundwater level readings are made at the times and under conditions stated on the boring logs (Groundwater levels change with time and season). The borehole does not



always remain open sufficiently long enough for the measured water level to coincide with the groundwater table.

6.2 LABORATORY AND FIELD TESTS

Laboratory and field tests are performed by specific ASTM standards unless otherwise indicated. All determinations included in each ASTM standard are not always required and performed. Each test report indicates the measurements and determinations made.

6.3 ANALYSIS AND RECOMMENDATIONS

The geotechnical report is prepared primarily to aid in the engineering design of site work and structural foundations. Although the information in the report is expected to be sufficient for these purposes, it is not intended to determine the cost of construction or to stand alone as a construction specification.

Our engineering report recommendations are based primarily on data from test borings made at the locations shown in a boring location drawing included. Soil variations may exist between borings, and these variations may not become evident until construction. If significant variations are then noted, the geotechnical engineer should be contacted so that field conditions can be examined and recommendations revised if necessary.

The geotechnical engineering report states our understanding as to the location, dimensions and structural features proposed for the site. Any significant changes in the nature, design, or location of the site improvements MUST be communicated to the geotechnical engineer such that the geotechnical analysis, conclusions, and recommendations can be appropriately adjusted. The geotechnical engineer should be given the opportunity to review all drawings that have been prepared based on their recommendations.

6.4 CONSTRUCTION MONITORING

Construction monitoring is a vital element of complete geotechnical services. The field engineer/inspector is the owner's "representative" observing the work of the



contractor, performing tests as required in the specifications, and reporting data developed from such tests and observations. The field engineer or inspector does not direct the contractor's construction means, methods, operations or personnel. The field inspector/engineer does not interfere with the relationship between the owner and the contractor and, except as an observer, does not become a substitute owner on site. The field inspector/engineer is responsible for his own safety but has no responsibility for the safety of other personnel at the site. The field inspector/engineer is an important member of a team whose responsibility is to watch and test the work being done and report to the owner whether that work is being carried out in general conformance with the plans and specifications.

6.5 GENERAL

The scope of our services did not include an environmental assessment for the presence or absence of hazardous or toxic materials in the soil, surface water, groundwater or air, on, within or beyond the site studied. Any statements in the report or on the boring logs regarding odors, staining of soils or other unusual items or conditions observed are strictly for the information of our client.

To evaluate the site for possible environmental liabilities, we recommend an environmental assessment, consisting of a detailed site reconnaissance, a record review, and report of findings. Additional subsurface drilling and samplings, including groundwater sampling, may be required.

This report has been prepared for the exclusive use of Appalachian Wireless, for specific application to the proposed cellular tower located on the Ermine Property located in Letcher County, Kentucky. Specific design and construction recommendations have been provided in the various sections of the report. The report shall, therefore, be used in its entirety. This report is not a bidding document and shall not be used for that purpose. Anyone reviewing this report must interpret and draw their conclusions regarding specific construction techniques and methods that were chosen. EKYENG is not responsible for the



independent conclusions, opinions or recommendations made by others based on the field exploratory and laboratory test data presented in this report.



SPECIFICATIONS

I - GENERAL

1.0 STANDARDS AND DEFINITIONS

- 1.1 STANDARDS All standards refer to latest edition unless otherwise noted.
 - 1.1.1 ASTM D-698-70 (Method C) "Standard Test Methods for Moisture. Density Relations of Soils and Soil Aggregate Mixtures Using 5.5-lb (2.5 kg.) Rammer and 12-inch (305-mm) Drop".
 - 1.1.2 ASTM D-2922 "Standard Test Method for Density of Soil and Soil Aggregate in Place by Nuclear methods (Shallow Depth)".
 - **1.1.3** ASTM D-1556 "Standard Test Method for Density of Soil in place by the Sand-Cone Method".

1.2 DEFINITIONS

- **1.2.1** Owner In these specifications the word "Owner" shall mean Appalachian Wireless.
- **1.2.2** Engineer In these specifications the word "Engineer" shall mean the Owner designated engineer.
- **1.2.3** Design Engineer In these specifications the words "Design Engineer" shall mean the Owner designated design engineer.
- **1.2.4** Contractor In these specifications the word "Contractor" shall mean the firm or corporation undertaking the execution of any work under the terms of these specifications.
- **1.2.5** Approved In these specifications the word "approved" shall refer to the approval of the Engineer or his designated representative.
- **1.2.6** As Directed In these specifications the words "as directed" shall refer to the directions to the Contractor from the Owner or his designated representative.



2.0 GENERAL CONDITIONS

2.1 The Contractor shall furnish all labor, material and equipment and perform all work and services except those set out and furnished by the Owner, necessary to complete in a satisfactory manner the site preparation, excavation, filling, compaction, grading as shown on the plans and as described therein.

This work shall consist of all mobilization clearing and grading, grubbing, stripping, removal of existing material unless otherwise stated, preparation of the land to be filled, filling of the land, spreading and compaction of the fill, and all subsidiary work necessary to complete the grading of the cut and fill areas to conform with the lines, grades, slopes, and specifications.

This work is to be accomplished under the observation of the Owner or his designated representative.

2.2 Prior to bidding the work, the Contractor shall examine, investigate and inspect the construction site as to the nature and location of the work, and the general and local conditions at the construction site, including, without limitation, the character of surface or subsurface conditions and obstacles to be encountered on and around the construction site; and shall make such additional investigation as he may deem necessary for the planning and proper execution of the work.

If conditions other than those indicated are discovered by the Contractor, the Owner should be notified immediately. The material which the Contractor believes to be a changed condition should not be disturbed so that the owner can investigate the condition.

2.3 The construction shall be performed under the direction of an experienced engineer who is familiar with the design plan.



II - ENGINEERED FILL BENEATH STRUCTURES CLEARING AND GRADING SPECIFICATIONS

1.0 **GENERAL CONDITIONS**

The Contractor shall furnish all labor, materials, and equipment, and perform all work and services necessary to complete in a satisfactory manner the site preparation, excavation, filling, compaction and grading as shown on the plans and as described therein.

This work shall consist of all clearing and grading, removal of existing structures unless otherwise stated, preparation of the land to be filled, filling of the land, spreading and compaction of the fill, and all subsidiary work necessary to complete the grading of the cut and fill areas to conform with the lines, grades, slopes, and specifications.

This work is to be accomplished under the constant and continuous supervision of the Owner or his designated representative.

In these specifications, the terms "approved" and "as directed" shall refer to directions to the Contractor from the Owner or his designated representative.

2.0 SUBSURFACE CONDITIONS

Prior to bidding the work, the Contractor shall examine, investigate and inspect the construction site as to the nature and location of the work, and the general and local conditions at the construction site, including without limitation, the character of surface or subsurface conditions and obstacles to be encountered on and around the construction site; and shall make such additional investigation as he may deem necessary for the planning and proper execution of the work. Borings and/or soil investigations shall have been made. Results of these borings and studies will be made available by the Owner to the Contractor upon his request, but the Owner is not responsible for any interpretations or conclusions with respect thereto made by the Contractor based on such information, and the Owner further has no responsibility for the accuracy of the borings and the soil investigations.

If conditions other than those indicated are discovered by the Contractor, the Owner should be notified immediately. The material which the Contractor believes to be a changed condition should not be disturbed so that the Owner can investigate the condition.

3.0 SITE PREPARATION

Within the specified areas, all trees, brush, stumps, logs, tree roots, and structures scheduled for demolition shall be removed and disposed of.

All cut and fill areas shall be properly stripped. Topsoil will be removed to its full depth and stockpiled for use in finish grading. Any rubbish, organic and other objectionable soils, and other deleterious material shall be disposed of off the site,



or as directed by the Owner or his designated representative if on site disposal is provided. In no case shall such objectionable material be allowed in or under the fill unless specifically authorized in writing.

Prior to the addition of fill, the original ground shall be compacted to job specifications as outlined below. Special notice shall be given to the proposed fill area now. If wet spots, spongy conditions, or groundwater seepage is found, corrective measures must be taken before the placement of fill.

4.0 FORMATION OF FILL AREAS

Fills shall be formed of satisfactory materials placed in successive horizontal layers of not more than eight (8) inches in loose depth for the full width of the cross-section. The depth of lift may be increased if the Contractor can demonstrate the ability to compact a larger lift. If compaction is accomplished using hand-tamping equipment, lifts will be limited to 4-inch loose lifts. Engineered fill placed below the structure bearing elevation shall be compacted to at least 95% of the maximum dry unit weight with a moisture content within 2% of the optimum moisture content as determined by the modified Proctor test. The top size of the material placed shall not exceed 4 inches.

All material entering the fill shall be free of organic matter such as leaves, grass, roots, and other objectionable material.

The operations on earth work shall be suspended at any time when satisfactory results cannot be obtained because of rain, freezing weather, or other unsatisfactory conditions. The Contractor shall keep the work areas graded to provide the drainage always.

The fill material shall be of the proper moisture content before compaction efforts are started. Wetting or drying of the material and manipulation to secure a uniform moisture content throughout the layer shall be required. Should the material be too wet to permit proper compaction or rolling, all work thus affected shall be delayed until the material has dried to the required moisture content. The moisture content of the fill material should be no more than two (2) percentage points higher or lower than optimum unless otherwise authorized. Sprinkling shall be done with equipment that will satisfactorily distribute the water over the disced area. Any areas inaccessible to a roller shall be consolidated and compacted by mechanical tampers. The equipment shall be operated in such a manner that hardpan, cemented gravel, clay or other chunky soil material will be broken up into small particles and become incorporated with the other material in the layer.

In the construction of filled areas, starting layers shall be placed in the deepest portion of the fill, and as placement progresses, additional layers shall be constructed in horizontal planes. Original slopes shall be continuously, vertically benched to provide horizontal fill planes. The size of the benches shall be formed so that the base of the bench is horizontal, and the back of the bench is vertical. As many benches as are necessary to bring the site to final grade shall be



constructed. Filling operations shall begin on the lowest bench, with the fill being placed in horizontal eight (8) inch thick loose lifts unless otherwise authorized. The filling shall progress in this manner until the entire first bench has been filled, before any fill is placed on the succeeding benches. Proper drainage shall be maintained always during benching and filling of the benches, to ensure that all water is drained away from the fill area.

Frozen material shall not be placed in the fill nor shall the fill be placed upon frozen material.

The Contractor shall be responsible for the stability of all fills made under the contract, and shall replace any portion, which in the opinion of the Owner or his designated representative, has become displaced due to carelessness or negligence on the part of the Contractor. Fill damaged by inclement weather shall be repaired at the Contractor's expense.

5.0 SLOPE RATIO AND STORM WATER RUN-OFF

Slopes shall not be greater than 2 (horizontal) to 1 (vertical) in both cut and fill, or as illustrated on the construction drawings. Excavations shall be constructed in accordance with all Federal, State and local codes relative to slope geometry.

6.0 GRADING

The Contractor shall furnish, operate, and maintain such equipment as is necessary to construct uniform layers, and control smoothness of grade for maximum compaction and drainage.

7.0 **COMPACTING**

The compaction equipment shall be approved equipment of such design, weight, and quantity to obtain the required density in accordance with these specifications.

8.0 <u>TESTING AND INSPECTION SERVICES</u>

Testing and inspection services will be provided by the Owner.

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EAST KENTUCKY ENGINEERING. LLC.

GUIDELINES FOR EXCAVATIONS AND TRENCHES

The following represents some general guidelines relative to the design and construction of excavations and trenches. It must be emphasized that these guidelines are not intended to represent a "safety plan," but rather are presented herein to provide general guidance regarding the design characteristics and safety measures for excavations and trenches.

- 1. Check with the following utilities prior to breaking ground:
 - Sewer
 - Telephone
 - Fuel
 - Electric
 - Water
 - Gas
 - Cable

When utility companies or owners do not respond to your request within 48 hours, the contractor may only then proceed provided the contractor does so with caution by using detection equipment or other acceptable means to locate utility installations.

Once the excavation is open, the contractor should protect and support the exposed underground utilities or remove installations to safeguard workers and prevent damage to exposed utilities.

- 2. Access and egress ramps must be designed by a "competent person" and structural ramps used for equipment must be designed by a "competent person" with qualified knowledge in structural design. In addition:
 - Ramps must be secured to prevent displacement;
 - Ramps used in lieu of steps must have cleats to prevent slipping; and
 - Trenching excavations four feet or greater in depth must have a stairway, ladder, ramps or other safe means to egress with lateral travel no more than 25 feet.
- 3. Workers must be provided with reflector garments, such as warning orange or red vests, when exposed to vehicular traffic.
- 4. Contractors must not allow workers to work under or near equipment when there is danger of falling debris, spillage or equipment-related injuries.

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- 5. Mobile equipment, operating adjacent to an open excavation or approaching the edge of an excavation, must have one of the following when the operator's view is obstructed:
 - Warning System
 - Mechanical Signals
 - Barricades
 - Stop Logs
 - Hand Signals
- 6. The contractor must check the atmosphere for hazardous gases and oxygen deficiencies when excavating four feet or greater around landfills, or when hazardous substances are stored nearby, and when the contractor expects there could be any exposure to the workers.
- 7. When hazardous atmospheric conditions exist, or when conditions could change, the contractor must make emergency rescue equipment readily available including breathing apparatus, safety harnesses with life lines and a basket stretcher.
- 8. When workers enter bell-bottom pier holes or other deep and confined excavations, the worker must wear (always while performing work in the confined space) a separate life line attached to a harness. The line must be attended by someone above while work is being performed. The worker must check for hazardous atmospheric conditions prior to entry.
- 9. The contractor must ensure that water does not accumulate in open excavations and must inspect the excavation prior to allowing workers to re-enter after heavy rains.
- Adjacent structures (buildings, walls, etc.) must be supported or secured to prevent worker exposure to unsafe conditions and damage to existing structures.
- 11. A registered professional engineer must approve operations when a contractor underpins existing structures to ensure worker safety and prevent damage to existing structures.
- 12. Workers must not be exposed to loose soil and rock or materials in and around excavations. Materials, such as removed soil and rock, must not be stored closer than two feet from the edge of the excavation.
- 13. Daily inspections of the excavation, the adjacent areas and protective systems must be made by a "competent person" for evidence of possible cave-ins, indications of failure of protective systems, hazardous



atmospheres or other hazardous conditions. The "competent person" must stop work immediately and remove workers from the excavation when conditions change and pose a threat to their safety.

- **14.** Workers must not be exposed to fall hazards associated with excavations. Protective walkways or bridges with standard guard rails must be provided.
- **15.** All wells, pits, shafts etc. must be barricaded or covered. After completion of work, all wells, pits, shafts etc. must be backfilled.



IV - DRILLED PIER INSTALLATION

1.0 DRILLING PROCEDURE

- 1.1 Drilled piers will be installed with large caisson drill rigs capable of torque and crowd forces sufficient to install drilled piers at the project site given the in-situ soil conditions.
- 1.2 The drill rig kelly bar and auger will be carefully and accurately placed over the centerline of the drilled pier. The Contractor is responsible for providing necessary surveying to verify drilled pier location before, during, and after the drilled pier installation.
- 1.3 The augers are advanced downwards as they are rotated such that drilling of the soil mass is efficiently accomplished. Depending on the subsurface conditions, and the requirements for the given project, a temporary steel casing should be installed at this time to preclude caving of the soil and/or broken rock mass being penetrated.

2.0 CASING INSTALLATION

- 2.1 The casing will be checked for centerline accuracy and plumbness by the Contractor's survey crew. During casing installation, the Contractors survey crew will verify alignment with instruments. If plumbness and alignment are not within tolerance as determined by the Contractors survey crew, the casing will be extracted and realigned as necessary.
- 2.2 The drill rig will remove soil and bedrock material from within the casing to the drilled pier design tip elevation. A steel casing or



"Sonotube" shall be inserted into the borehole to preclude cave-ins and/or instability in the borehole.

2.3 The bearing surface within the drilled pier will be inspected by a registered Professional Engineer before being approved for structural concreting.

3.0 INSTALLATION OF THE REBAR CAGE

- 3.1 An epoxy coated spiral reinforcing steel cage will be installed while in the drilled pier borehole.
- 3.2 To assist in assuring that the reinforcing steel cage does not settle during concrete pumping, a mat of reinforcing steel bars will be installed across the bottom of the reinforcing steel cage perpendicular to the vertical axis of the cage. The exact number of bars will be determined and installed by the Structural Engineer. The number of rebar boots used on the bottom of the cage will also be determined by the Structural Engineer.
- 3.3 The reinforcing steel cage will be lowered into the drilled pier borehole, while drilled pier spacers are placed at intervals as required by the Structural Engineer. The reinforcing steel cage will be checked for alignment by the Contractors survey crew.
- 3.4 The crane will remain attached to the reinforcing steel cage while the concrete pump outlet pipe is lowered to just above the bottom of the drilled pier. The concrete pump pipe sections will be welded together to assure that do not separate during pumping.



4.0 CONCRETING OF THE DRILLED PIER

- 4.1 Concrete pumping may commence once the bearing surface has been approved in accordance with Clause 2.3
- 4.2 A three-inch trash pump will be used to pump slurry and/or water from within the casing and from above the newly pumped concrete.
- 4.3 The concrete pump outlet pipe will maintain at least ten (10) feet of embedment into the fresh concrete. The concrete level in the casing will be monitored.
- 4.4 The casing will be completely extracted with the crane and/or vibratory hammer. Caisson clamps on the vibratory hammer (if applicable) will be adjusted to the proper dimension to withdrawal the casing.
- 4.5 The concrete will be terminated at the top of drilled pier elevation and screeded flat.
- 4.6 The upper reinforcing steel dowel cage will be lowered into the concrete to the embedment elevation. If necessary, the concrete will be vibrated to assist in placement. Alignment will be verified by the Contractors survey crew and the cage will be sufficiently braced.



V - GENERAL CONCRETE SPECIFICATIONS

1.0 GENERAL

It is the intent of this specification to secure, for every part of the work, concrete of homogenous structure which, when hardened, will have the required strength and resistance to weathering. To this end, the limiting values of concrete and the requirements hereinafter specified must be met. Standard tests of the cement, aggregates, concrete and reinforcement will be made by the Owner as it sees fit. The Contractor shall furnish the material for all required samples plus such labor as required to obtain samples. The Contractor shall provide to authorized representatives of the Owner, convenient access to all parts of the work of all concreting operations for the purpose of sampling and inspection.

2.0 SCOPE

Contractor shall furnish all materials, labor, services, transportation, tools, equipment, and related items required to complete work indicated on the drawings and/or specified.

Unless otherwise noted or as modified by more stringent requirements specified herein, all plain and reinforced concrete work shall be performed in full compliance with applicable requirements of the Building Code Requirements for Reinforced Concrete ACI 318.

Contractor shall obtain Owner's approval of all subgrades, footing bottoms, forms, and reinforcement just prior to placing concrete.

Contractor shall coordinate the work specified in this section with that specified in other sections so that all anchors, pipes and other embedded items are properly installed before concrete is placed.

Contractor shall clean all exposed concrete surfaces and obtain approval of Owner for method of cleaning

3.0 MATERIALS

All materials shall be of the respective quality specified herein, delivered, stored, and handled as to prevent inclusion of foreign matter and damage by dampness or breakage. Packaged material shall be stored in original container until ready for use. Materials showing evidence of dampness or other damage may be rejected.

- A. <u>Fine and Coarse Aggregates:</u> Coarse and fine aggregates shall conform to ASTM Specification C33. The maximum size of aggregate shall not be larger than one-fifth (1/5) of the narrowest dimensions between forms, or larger than three fourths (3/4) of the minimum clear spacing between reinforcement.
 - 1. <u>Fine Aggregate:</u> Sand shall be composed essentially of clear, hard, strong, durable grains free of structurally weak



- grains, organic matter, loam, clay, silt, salt, mica or other fine materials that may affect bonding of the cement paste.
- Coarse Aggregate: Cement concrete shall consist of crushed rock or screened gravel and shall be composed essentially of clean, hard, strong and impermeable particles, resistant to wear and frost and free from deleterious amounts of organic matter, loam, clay, salts, mica, and soft, thin, elongated, laminated or disintegrated stone, and shall be inert to water and cement.
- B. <u>Portland Cement:</u> Portland cement shall conform to ASTM Specification C150. Type I or Type II Portland Cement shall be used provided that they are not intermixed during any one batch. Type II Portland Cement shall <u>not</u> be used unless indicated on the plans.
- C. <u>Water:</u> Water for mixing and curing shall be clean, fresh, and free from deleterious materials.
- D. <u>Metal Reinforcement:</u> Rebar shall be Grade 60 and with deformations conforming to ASTH Specification A305. Welded wire mesh shall conform to W4 x W4 size and be of Grade 60 steel.
- E. <u>Admixtures:</u> Except as herein noted, admixtures shall not be used.
 - 1. Under adverse weather conditions only retarding or accelerating agents containing no chloride may be used.
 - Air-Entraining Agent shall be used for all concrete will give an entrained air range of not less than 4 percent but no greater than 8 percent in the finished product. Under no circumstances shall the air-entraining be interground with cement.
 - 3. Approval in writing shall be required from Owner prior to the use of any admixture.

4.0 FORM

Forms shall be constructed with proper shoring and cross-bracing, safeguarding the total structure and specifically lateral stability and sufficiently strong to stand vibrations of concrete and to carry, without appreciable deflection or displacement, all dead and live loads to which they may be subjected.

5.0 INSERTS, ETC.

Anchors, bolts, dowels, conduit, water stops, vent pipes and other similar built-in or concreted-in items shall be properly located, accurately positioned and secured. The Contractor shall cooperate in placing of such items with other contractors who require a fastening device for their work, and he shall maintain them in proper location during the progress of his work.

ENG

EAST KENTUCKY ENGINEERING, LLC.

6.0 REINFORCEMENT

Reinforcement at the time concrete is placed shall be free from rust, scale or other coatings that will destroy or reduce the bond.

Reinforcement shall be accurately placed and securely tied at intersections and shall be securely held in position during the placing of concrete by pacers, chairs, or other approved supports.

The reinforcement of foundations, footings and other principal structural members in which the concrete is deposited against the ground shall not have less than three (3) inches of concrete between it and the ground contact surface. If concrete surfaces after removal of the forms are to be exposed to the weather or to be in contact with the ground or rock, reinforcement shall be protected with not less than two (2) inches of concrete.

7.0 CONCRETE

Concrete for the various parts of the work shall be of 4000 pounds per square inch compressive strength with a minimum 28-day cure. Contractor is responsible to provide a mix of not less than 6 bags of cement per yard of concrete and not more than 7 gallons of water per bag of cement, producing a minimum slump of 2-1/2 inches and a maximum slump of 4-1/2 inches. Concrete that exceeds the above range of maximum or minimum slump requirements may be rejected by the Owner. All concrete shall be air-entrained. Contractors are required to furnish the name or names of the company(s) that will be providing the mix. The Owner reserves the right to disapprove any concrete supplier that has been known to supply an undesirable material to the Owner on previous occasions.

8.0 DEPOSITING CONCRETE

- 4.1. <u>Preparation for Placing Concrete:</u> Before depositing concrete, the Contractor shall:
- 1. Remove from space to be occupied by concrete all debris, including snow, ice, and water unless otherwise permitted by Owner.
 - 2. Provide diversion, satisfactory to Owner, of any flow of water to an excavation to avoid washing the freshly deposited concrete.
 - 3. Coal the forms prior to placing of reinforcing steel as required in form work.
 - 4. Secure firmly in correct position, all reinforcement and other items to be encased and remove therefrom all coating including ice and frost.
 - B. <u>Transportation of Concrete from Batch Plant:</u> The concrete shall be delivered to the site of the work and discharge shall be completed within 90 minutes after addition of the cement and water to the



aggregates. Each batch of concrete delivered at the job site shall be accompanied by a time slip issued at the batching plant, bearing the time of charging of the mixer drum with the cement and aggregates.

C. Transporting of Concrete from Mixer to Place of Final Deposit:

Transportation shall be done as rapidly as practical by means which shall prevent the separation or loss of the ingredients. If chutes are used, they shall be at a slope not flatter than one vertical to two horizontal. Buggies or carts shall be equipped with pneumatic rubber tires or surfaces of runways shall be sufficiently smooth or both so as not to cause separation or segregation of concrete ingredients. Concrete shall not be allowed to drop freely more than 4 feet. Where greater drops are required, canvas "elephant trunks" or galvanized iron chutes equipped with suitable hopper heads shall be employed and a sufficient number placed to ensure that the concrete may be effectively compacted into horizontal layers not exceeding 12 inches in thickness with minimum lateral movements.

D. <u>Depositing of Concrete:</u> Depositing of concrete shall:

- Proceed continuously after once starting until reaching the end of a section of construction joint location shown on the drawings, or as approved by the Owner. The operations shall be conducted so that no concrete is deposited on concrete sufficiently hardened to cause formation of seams, and planes of weakness.
- 2. Be as near as practical to its final position in the forms.
- 3. Proceed to maintain constantly a top surface which is approximately level.
- 4. Be placed before initial set has occurred, and in no event after it has contained its water content for more than 90 minutes.
- 5. Be thoroughly worked and compacted by means of suitable tools to provide impermeability, durability and strength and shall be thoroughly worked around reinforcements and embedded items and into corners of forms and to be free from voids, pockets or honeycombing. Care shall be taken to provide impermeability.
- E. <u>Vibration Equipment:</u> Vibration equipment shall be of the appropriate type and shall, always, be adequate in number of units and power of each unit to properly consolidate all concrete.



F. <u>Monolithic Pours:</u> Proper delivery of concrete shall be the Contractor's responsibility to make a mono-lithic pour without delays and changes of cold joints.

9.0 CURING

All concrete work shall be protected from injurious action by the sun, rain, flowing water, frost and other injury and shall be covered with plastic after application of curing compound for three (3) days on pours located above ground.

Contractor shall not remove any formwork for a minimum period of 24 hours after a concrete pour without written approval of the Owner.

10.0 CONCRETE FINISHES

Finishes of all exposed concrete shall be free of defects which impair its durability or adversely affect is appearance. All such surfaces when stripped, shall be uniform in appearance and any surfaces displaying any deviations from adjacent uniform surfaces shall be rejected and subject to removal.

Finished work shall be level and plumb, true to lines, and dimensions. Finished plane surfaces shall be smooth, and as nearly perfect as practical; however, deviations from a true plane shall not exceed 1/8 inch when measured from a 6-foot straight edge placed against the surface to any point on the surface and under the straight edge.

All exposed surfaces shall have defects corrects, protrusions removed, and holes filled.



APPENDIX A	BORING LOGS	

HORN AND ASSOCIATES, INC 216 N. Main Street - Winchester, KY 40391 Ph: 800-729-2802 Fax: 859-744-5892

FIELD BORING LOG

Project I	Name Finne Tower	Hole N	Hole Number 8 - 1 Total Depth 33 0				
Federal	Project No.		Location				
State Pr	oject No.	Surfac	e Elevation				
Drilling/S	Sampling Method 5F7/WX	Date S	Started 3-2	小見し Date (Completed	3.24.20	
	Diameter 3 1/4	Driller	Driller Grant Weather				
From To	Soil and Rock Description	Sample/Rui Interval	Blow Counts/RQD	Sample/Run No.	Sample Type	% Recovery	
0,0	6 T	1,5		5 1	SFT		
2.5 1.5	064	1 "	0-1-6	ł ·			
1,5 140 140	Fill		Q-10.6				
140	B. 54,51	9.0 9.7	5%1	5 . 4			
31.15	Br. Si, SA with	14.015.5	WH-WW	5-5			
37 0	Gr, 5 \$, 5.	19.0 20	1-1-5	5-6	SET		
	TH 6033.0	A 3. "33.	50	R-1	NX	7.8	
	180×						
	1 Brackton Entch	<u> </u>					
	1 Bay Concrete						
	·						
			_			<u> </u>	
						<u> </u>	
						<u> </u>	
-							
Water Lev	vel @ Drilling 💮 🛇 🗢 💮 💮	24 Hr. Water Leve	el	7 Day W	ater Level		
Moving/De	elay Time	Hammer Weight	140 lbs.	Hammer D	rop	30 in.	



APPENDIX B	CORE PHOTOG	POV DIE
AFFERINA D	CURE PROTOC	IKAFOJ









APPENDIX C	SEISMIC DATA
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Ermine Tower Location

Latitude, Longitude: 37.11976, -87.79354



900	ogle		Map data ©2020
Date		4/14/2020, 12:27:20) PM
Design Co	de Reference D	Document IBC-2015	
Risk Cate	gory	IV	
Site Class		A - Hard Rock	
Туре	Value	e Description	
SS	0.656	MCE _R ground motion. (for 0.2 second period)	
S ₁	0.239	MCE _R ground motion. (for 1.0s period)	
SMS	0.525	Site-modified spectral acceleration value	
S _{M1}	0.191	Site-modified spectral acceleration value	
SDS	0.35	Numeric seismic design value at 0.2 second SA	
S _{D1}	0.127	Numeric seismic design value at 1.0 second SA	
Туре	Value	Description	
SDC	D	Seismic design category	
Fa	0.8	Site amplification factor at 0.2 second	
F _v	0.8	Site amplification factor at 1.0 second	
PGA	0.352	MCE _G peak ground acceleration	
FPGA	0.8	Site amplification factor at PGA	
PGA _M	0.282	Site modified peak ground acceleration	
TL	12	Long-period transition period in seconds	
SsRT	0.656	Probabilistic risk-targeted ground motion. (0.2 second)	
SsUH	0.775	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration	
SsD	1.5	Factored deterministic acceleration value. (0.2 second)	
SIRT	0.239	Probabilistic risk-targeted ground motion. (1.0 second)	
SIUH	0.289	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration.	
SID	0.6	Factored deterministic acceleration value. (1.0 second)	
PGAd	0,6	Factored deterministic acceleration value. (Peak Ground Acceleration)	
CRS	0.847	Mapped value of the risk coefficient at short periods	
C _{R1}	0.826	Mapped value of the risk coefficient at a period of 1 s	

https://seismicmaps.org

DISCLAIMER

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https://seismicmaps.org 2/2



APPENDIX D PHOTOGRAPHS









APPENDIX E	MAPS	
APPENDIA E	MACO	

National Flood Hazard Layer FIRMette



Without Base Flood Elevation (BFE) SPECIAL FLOOD HAZARD AREAS LETCHER COUNTY 210289 OTHER AREAS OF FLOOD HAZARD Zone A OTHER AREAS AREA OF MINIMAL FLOOD HAZARD 21133 C0231 C **FEATURES** Digital Data Available No Digital Data Available CITY OF WHITESBURG MAP PANELS Unmapped 210140 1176 FEET accuracy standards Zone AE FLOODWAY

1:6,000

Feet

2,000

250

500

1,000

1,500

USGS The National Map: Orthoimagery, Data refreshed April, 2019

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

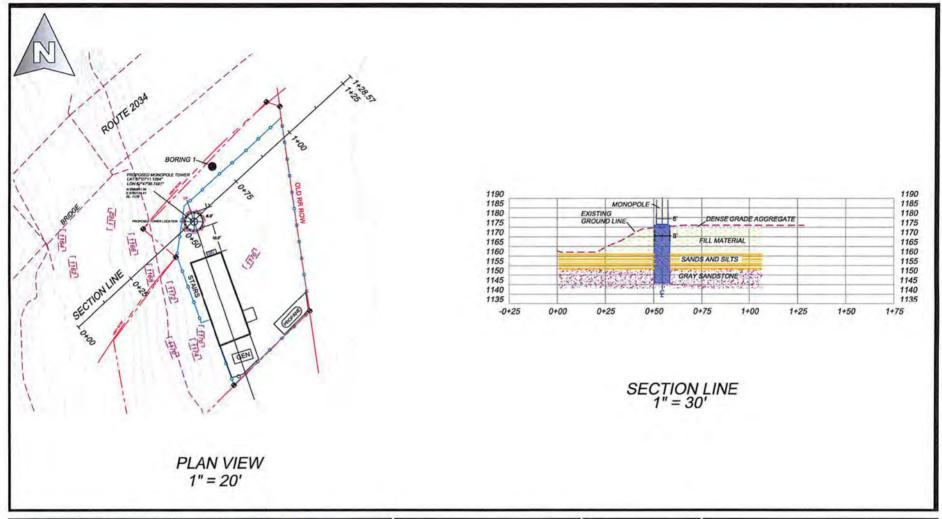
With BFE or Depth Zone AE, AO, AH, VE, AR Regulatory Floodway 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X **Future Conditions 1% Annual** Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee, See Notes, Zone X Area with Flood Risk due to Levee zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs Area of Undetermined Flood Hazard Zone D GENERAL - - - Channel, Culvert, or Storm Sewer STRUCTURES | LITTI Levee, Dike, or Floodwall Cross Sections with 1% Annual Chance 17.5 Water Surface Elevation Coastal Transect Base Flood Elevation Line (BFE) Limit of Study Jurisdiction Boundary Coastal Transect Baseline **Profile Baseline** Hydrographic Feature

> The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/27/2020 at 10:27:32 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



East Kentucky Engineering, LLC

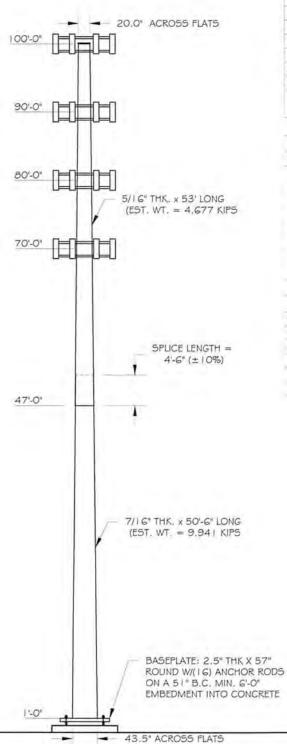
230 Swartz Hazard, KY 41701 (606) 551-1050 Email: ekyeng@ekyeng.net



0,	20' 40'
Drawn by:RDS	4/14/2020
Job #:165-0105 File Location:	Scale:1* = AS NOTED

APPALACHIAN WIRELESS PROPOSED TOWER LOCATION ERMINE SITE LETCHER COUNTY, KENTUCKY





Page of 2	Job Number:	23519-216
Eng:	Customer Ref:	TP-18914
MFP	Date:	5/11/2020
Structure:	100-FT MONOPOLE	
Site:	ERMINE	
Location: LETC	CHER CO., KY / 37°7'12", -82	2°47'34"
Owner:	APPALACHIAN WIRELESS	
Revision No.: Revision D	Date:	
	DESIGN	
Building Code: 2013 KEN	NTUCKY BUILDING CODE	
Design Standard: ANSI/TI	A-222-G	
Wind Speed Load Cases:	ASCE-7-05 WIND S	PEED
Load Case #1: 90 MPH	Design Wind Speed - Vice (Vill	= 116 MPH)
Load Case #2: 30 MPH	Wind with 0.5" Ice Accu	mulation
Load Case #3 60 MPH	Service Wind Speed	
Structure Class Risk Category Expo	osure Cat. Topography Cal	t. Crest Height
II.	C 3	320'

	EQUIPMENT LIST
Elev.	Description
100	(12) NN-G5A-M + (12) RRU
100	12-FT SECTOR MOUNTS
90	(12) NN-G5A-M + (12) RRU
90	12-FT SECTOR MOUNTS
80	(12) NN-G5A-M + (12) RRU
80	12-FT SECTOR MOUNTS
70	(12) NN-G5A-M + (12) RRU
70	12-FT SECTOR MOUNTS

ANTENNA FEED LINES ROUTED ON THE INSIDE OF THE POLE

		STRUCTUR	E PROPER	RTIES	
Cross-S	ection: 18-5	bided	Taper:	0.2436	9 in/ft
Shaft St	ceel: ASTM AS	572 GR 65	Baseplate	Steel: ASTM	A572 GR 50
Anchor I	Rods: 2.25 II	n. AG 15 GR. 75	5 X 7'-0" LON	1G	
Sect.	Length (ft)	Thickness (in)	Splice (ft)	Top Dia. (in)	Bot Dia. (in)
1	53.00	0.3125	4.50	20.00	32.92
2	50.50	0.4375	0.00	31.19	43.50

CERTIFICATION CONCENTION



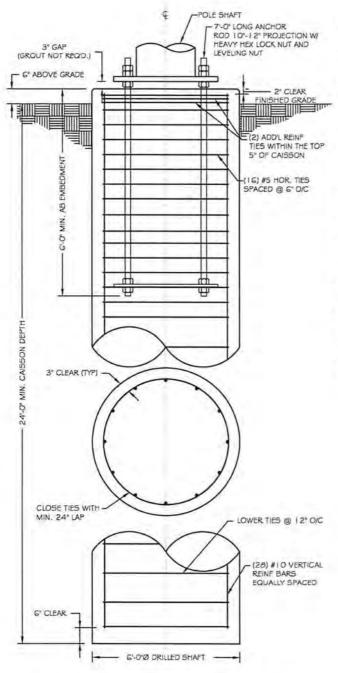
MICHAEL F. PLANOVINSAK, P.E. \$25466 Bols Francistor - Insupersions Engineer 18301 S.R. 161, Plain City, OH 43064 614-398-6250 / mike@mfpeng.com

BASE REACTIONS FOR FOUNDATION DESIGN

Moment: 3887 ft-kp

Shear: 51 kip Axial: 39 kip





Page 2 of 2		Job Number:	23519-216			
Eng: MFP		Customer Ref:	TP-18914			
		Date:	5/11/2020			
Structure:	100-FT MONOPOLE					
Site:		ERMINE				
Location:	LETCHER CO	., KY / 37°7'12", -82"	°47'34"			
Owner:	APPALACHIAN WIRELESS					
Revision No.:	Revision Date:					

FOUNDATION NOTES:

- I. ALL FOUNDATION CONCRETE SHALL USE TYPE II CEMENT AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.46. IN AREAS OF POTENTIAL FREEZING, CONCRETE SHALL BE AIR ENTRAINED G% (\pm 1.5%). ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 3.18. "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION.
- 2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM AG 15 VERTICAL BARS SHALL BE GRADE GO, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 3 15, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES!, LATEST EDITION.
- CAISSON FOUNDATION INSTALLATION SHALL BE IN ACCORDANCE WITH ACI 33G, "STANDARD SPECIFICATIONS FOR THE CONSTRUCTION OF DRILLED PIERS", LATEST EDITION.
- 4. THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.
- 5. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY:
 ENGINEER: EAST KENTUCKY ENGINEERING
 REPORT NO.: 165-000-0105 (DATED 4/14/20)
- 6. ESTIMATED CONCRETE VOLUME = 26 CUBIC YARDS.
- 7. THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED LOADS:

MOMENT: 3887 PT*KIPS SHEAR: 5 | KIPS AXIAL: 39 KIPS

8. GEOTECHNICAL REPORT INDICATES GROUNDWATER MAY BE ENCOUNTERED AT 20'-0' BELOW GRADE.

OF KENT

MICHAEL F. PLAHOVINSAK, P.E. #25466 Sept Property Transparent Papers 18301 S.R. 161, Plain City, OH 43064 614-398-6250 / mike@mfpeng.com

CAISSON FOUNDATION

NOT TO SCALE

Michael F. Plahovinsak, PE 18301 State Route 161 Plain City, OH 4364 Phone: 614-398-6250

FAX: mike@mfpeng.com

Job	100-ft Monopole - MFP #23519-216	Page 1 of 6
Project	Ermine	Date 13:28:21 05/11/20
Client	TP-18914	Designed by JC

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Letcher County, Kentucky.

Basic wind speed of 90 mph.

Structure Class II.

Exposure Category C.

Topographic Category 3.

Crest Height 320.00 ft.

Nominal ice thickness of 0.5000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 30 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

ANSI/TIA-222-G wind speeds are Vasd winds. Refer to IBC Table 1609.3.1 for Vult wind speed conversions..

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
LI	100.00-47.00	53.00	4.50	18	20.0000	32.9200	0,3125	1.2500	A572-65
									(65 ksi)
L2	47.00-1.00	50.50		18	31.1980	43.5000	0.4375	1.7500	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in²	I ini	r	C	I/C in ³	J in	It/Q in²	in	10/1
LI	20.2603	19.5275	956.2382	6.9891	10.1600	94.1179	1913.7347	9.7656	2.9700	9.504
	33.3796	32,3426	4344.5710	11.5757	16.7234	259.7906	8694.8588	16.1744	5.2439	16.781
L2	32.7249	42.7148	5106.2656	10.9200	15,8486	322,1905	10219,2502	21,3615	4.7209	10.791
	44.1036	59,7977	14009.4197	15.2872	22.0980	633.9678	28037.2735	29.9045	6.8860	15.739

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade Adjust. Factor A _f	Adjust. Factor A,	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ſt	Jr	in				in	in	in
L1 100,00-47.00			1	1	1			
L2 47.00-1.00			- 1	1	1			

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Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Total Number		C_AA_A	Weight
	Leg	Limera	Torque Calculation	· jpe	fi	T, MINGE		fr/fi	plf
1 5/8"	C	No	Yes	Inside Pole	100.00 - 1.00	18	No Ice	0.00	0.92
							1/2" Ice	0.00	0.92
1.5/8"	C	No	Yes	Inside Pole	90.00 - 1.00	18	No Ice	0.00	0.92
							1/2" Ice	0.00	0.92
1 5/8"	C	No	Yes	Inside Pole	80.00 - 1.00	18	No Ice	0.00	0.92
4.410			2.55		9700-07600	1	1/2" Ice	0.00	0.92
1 5/8"	C	No	Yes	Inside Pole	70.00 - 1.00	18	No Ice	0.00	0.92
					(4)44	2.0	1/2" Ice	0.00	0.92

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation	Face	A_R	A_F	C ₄ A ₄ In Face	C _A A _A Out Face	Weight	
	ft		ft ²	ft	ft²	fr	K	
L1	100.00-47.00	A	0.000	0.000	0.000	0.000	0.00	
		В	0.000	0.000	0.000	0.000	0.00	
		C	0.000	0.000	0.000	0.000	2.51	
L2	47.00-1.00	A	0.000	0.000	0.000	0.000	0.00	
		В	0.000	0.000	0.000	0.000	0.00	
		C	0.000	0.000	0.000	0.000	3.04	

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation	Face	Ice Thickness	A_{H}	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
	ft	Leg	in	fi ²	ft ²	ft ²	ft²	K
L1	100.00-47.00	A	1.325	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	2.51
L2	47.00-1.00	A	1.258	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	3.04

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			fi fi fi	0	fi		fr	ft²	K
(4) Andrew NN-65A-M w/ mount pipe	Α	From Face	3.00 0.00 0.00	0.0000	100.00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08 0.17
(4) Andrew NN-65A-M w/ mount pipe	В	From Face	3.00 0.00 0.00	0.0000	100,00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08 0.17
(4) Andrew NN-65A-M w/ mount pipe	C	From Face	3.00 0.00 0.00	0.0000	100.00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08 0.17
12) Ericsson Radio 2212-B5	A	From Face	2.00	0.0000	100.00	No Ice	1.86	0.87	0.05

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Description	Face or Leg	Offset Type	Offsets: Hor= Lateral Vert	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weigh
			fi fi fi	•	Ji		ft	fi²	K
			0.00	W. 1945		1/2" Ice	2.03	1.00	0.06
12' Sector Mounts	C	None		0.0000	100.00	No Ice 1/2" Ice	30.00 35.00	30.00 35.00	1.80
**						110-01	4400		
(4) Andrew NN-65A-M w/ mount pipe	٨	From Face	3.00 0.00 0.00	0.0000	90.00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08 0.17
(4) Andrew NN-65A-M w/ mount pipe	В	From Face	3.00 0.00	0,0000	90.00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08 0.17
(4) Andrew NN-65A-M w/	C	From Face	3.00	0.0000	90.00	No Ice	12.41	5.04	0.08
mount pipe		1 form race	0,00	0,000	30.00	1/2" Ice	12.89	5.71	0.17
(12) Ericsson Radio 2212-B5	A	From Face	2.00	0.0000	90.00	No Ice 1/2" Ice	1.86	0.87	0.05
			0.00						
12' Sector Mounts	C	None		0.0000	90.00	No Ice	30.00	30.00	1:80
**						1/2" Ice	35.00	35.00	2.60
(4) Andrew NN-65A-M w/	Α	From Face	3.00	0.0000	80.00	No Ice	12.41	5.04	0.08
mount pipe		1 tom ruce	0,00	0,000	00.00	1/2" Ice	12.89	5.71	0.17
(4) Andrew NN-65A-M w/ mount pipe	В	From Face	3.00 0.00	0.0000	80.00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08
(4) Andrew NN-65A-M w/	0	From Face	0.00	0.0000	80.00	No. ton	12.11	701	0.08
mount pipe	C	From Face	3.00 0.00 0.00	0.0000	80.00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.17
(12) Ericsson Radio 2212-B5	A	From Face	2.00	0.0000	80.00	No Ice	1.86	0.87	0.05
(12/21/10/01/11/11/02/22/22/2		33500 1 335	0.00	3.300	20.00	1/2" Ice	2.03	1.00	0.06
12' Sector Mounts	C	None		0.0000	80.00	No Ice	30,00	30.00	1.80
44						1/2" Ice	35,00	35.00	2.60
(4) Andrew NN-65A-M w/	Á.	From Face	3.00	0.0000	70.00	No Ice	12.41	5.04	0.08
mount pipe	A	From Pace	0.00	0.0000	70,00	1/2" Ice	12.89	5.71	0.17
(4) Andrew NN-65A-M w/ mount pipe	В	From Face	3.00 0.00	0.0000	70.00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08
AN AMERICAN AND GEA AND		Paris Paris	0.00	0.0000	70.00	Mr. Inc.	12.41	501	0.00
(4) Andrew NN-65A-M w/ mount pipe	С	From Face	3.00 0.00 0.00	0.0000	70,00	No Ice 1/2" Ice	12.41 12.89	5.04 5.71	0.08
(12) Ericsson Radio 2212-B5	Α	From Face	2.00 0.00	0.0000	70,00	No Ice 1/2" Ice	1,86 2.03	0.87 1.00	0.05 0.06
12' Sector Mounts	C	None	0.00	0.0000	70.00	No Ice	30.00	30.00	1.80
						1/2" Ice	35.00	35.00	2.60

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Load Combinations

Comb. No.		Description
1	Dead Only	
2	1.2 Dead+1 6 Wind 0 deg - No Ice	
3	0.9 Dead+1.6 Wind 0 deg - No Ice	
4	1.2 Dead+1.6 Wind 90 deg - No Ice	
5	0.9 Dead+1.6 Wind 90 deg - No Ice	
6	1.2 Dead+1.6 Wind 180 deg - No Ice	
7	0.9 Dead+1.6 Wind 180 deg - No Ice	
8	1.2 Dead+1.0 Ice+1.0 Temp	
9	1,2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	
12	Dead+Wind 0 deg - Service	
13	Dead+Wind 90 deg - Service	
14	Dead+Wind 180 deg - Service	

Maximum Member Forces

Section No.	Elevation fi	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
Lì	100 - 47	Pole	Max Tension	1	0.00	0.00	0.00
57/1	000 : 000		Max. Compression	8	-48.73	14.15	8.17
			Max. Mx	-4	-20.66	-1464.82	-37.20
			Max. My	2	-20.85	49.17	1429.37
			Max. Vy	4	45.10	-1464.82	-37.20
			Max. Vx	2	-43.69	49.17	1429.37
			Max. Torque	2		30.0	6.59
L2	47-1	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-67.06	14.53	8.39
			Max, Mx	4	-38.88	-3885.58	-98.59
			Max. My	2	-38.88	110.94	3779.44
			Max. Vv	4	50.66	-3885.58	-98.59
			Max. Vx	2	-49.28	110.94	3779.44
			Max. Torque	2			6.56

Maximum Tower Deflections - Service Wind

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ſŧ	in	Comb.	0	0
Ll	100 - 47	20.235	13	1.6957	0.0119
L2	51.5 - 1	5.430	13	0.9983	0.0034

	-
22234	A141 A14
LILA	ower

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Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	a	0	ft
100.00	(4) Andrew NN-65A-M w/ mount pipe	13	20.235	1.6957	0.0119	19432
90.00	(4) Andrew NN-65A-M w/ mount pipe	13	16.719	1.5613	0.0099	9716
80.00	(4) Andrew NN-65A-M w/ mount pipe	13	13,326	1.4278	0.0080	4857
70.00	(4) Andrew NN-65A-M w/ mount pipe	13	10,180	1.2875	0,0062	3238

Maximum Tower Deflections - Design Wind

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ft	in	Comb.	0.	0
LI	100 - 47	82.441	4	6.9343	0.0479
L2	51.5 - 1	22.057	4	4.0616	0.0136

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
fi		Comb.	in	٩	0	ft
100.00	(4) Andrew NN-65A-M w/ mount pipe	4	82,441	6.9343	0.0479	4855
90.00	(4) Andrew NN-65A-M w/ mount pipe	4	68.095	6.3917	0.0398	2426
80.00	(4) Andrew NN-65A-M w/ mount pipe	4	54,254	5,8359	0.0320	1210
70.00	(4) Andrew NN-65A-M w/ mount pipe	4	41.423	5.2536	0.0247	804

Pole Design Data

Section No.	Elevation	Size	L	L_u	Kl/r	А	P_u	ϕP_n	Ratio Pu
	fi		ft	ft		in ²	K	K	ϕP_n
LI	100 - 47 (1)	TP32.92x20x0.3125	53.00	0.00	0.0	31.2545	-20.66	2317.56	0.009
L2	47 - 1(2)	TP43.5x31.198x0.4375	50.50	0.00	0.0	59.7977	-38.88	4442.67	0.009

Pole Bending Design Data

Section No.	Elevation	Size	M_{ux}	ϕM_{res}	Ratio Mus	M_{u_1}	ϕM_{m}	Ratio Muy
	fi		kip-ft	kip-ft	ϕM_{nx}	kip-ft	kip-ft	ϕM_m
LI	100 - 47 (1)	TP32.92x20x0.3125	1465.29	1498.63	0.978	0.00	1498.63	0.000
L2	47 - 1(2)	TP43.5x31.198x0.4375	3886.82	3925.05	0.990	0.00	3925.05	0.000

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Pole	Shear	Design	Data
I OIC	Olicai	Design	Data

Section No.	Elevation	Size	Actual Vu	ϕV_n	Ratio V _n	Actual T _u	ϕT_n	Ratio T _u
	ft		K	K	φV _n	kip-ft	kip-ft	φT _n
LI	100 - 47 (1)	TP32.92x20x0.3125	45.12	1158.78	0.039	3.79	3005.40	0.001
L2	47 - 1 (2)	TP43.5x31.198x0.4375	50.67	2221.33	0.023	3.76	7871.73	0.000

Pole Interaction Design Data

Section No.	Elevation	Ratio P _u	Ratio M _w	Ratio Muv	Ratio V _u	Ratio T _u	Comb. Stress	Allow. Stress	Criteria	
ft			ft	φ <i>P</i> , φ.	ϕM_{nx} ϕM_{ny}	ϕM_m ϕV_n ϕT_n		Ratio	Ratio	
L1	100 - 47 (1)	0.009	0.978	0.000	0.039	0.001	0.988	1.000	4.8.2	
L2	47 - 1 (2)	0.009	0.990	0.000	0.023	0.000	1.000	1.000	4,8,2	

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	oP _{allow} K	% Capacity	Pass Fail
Lì	100 - 47	Pole	TP32.92x20x0.3125	1	-20.66	2317.56	98.8	Pass
L2 47 - 1	47 - 1	Pole	TP43.5x31.198x0.4375	2	-38.88	4442.67	100.0	Pass
							Summary	
						Pole (L2)	100.0	Pass
						RATING =	100.0	Pass

Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

TIA Rev G Assumption: Clear space between bottom of leveling nut and top of concrete not exceeding (1)*(Rod Diameter)

Site Data

Job # 23519-216 Site Name: ERMINE

LETCHER CO., KY							
=	Pole Manufacturer:	Other					

Anc	hor Rod D	ata
Qty:	16	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	51	in

P	late Data	a
Diam:	57	in
Thick:	2.5	in
Grade:	50	ksi
Single-Rod B-eff:	8.63	în

Stiffener Dat	a (Welding	at both sides)
Config:	0	*
Weld Type:	Fillet	
Groove Depth:	0.25	< Disregard
Groove Angle:	45	< Disregard
Fillet H. Weld:	0.25	in
Fillet V. Weld:	0.3125	in
Width:	5	in
Height:	18	in
Thick:	0.75	in
Notch:	0.5	in
Grade:	36	ksi
Weld str.:	70	ksi

	ole Data	
Diam:	43.5	in
Thick:	0.4375	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf, Fillet Weld	0	"0" if None

Re	Reactions					
Mu:	3887	ft-kips				
Axial, Pu:	39	kips				
Shear, Vu:	51	kips				
Eta Factor, η	0.5	TIA G (Fig. 4-4)				

if No stiffeners, Criteria: AISC LRFD <-Only Applicable to Unstiffened Cases

Anchor Rod Results

Max Rod (Cu+ Vu/n): 237.5 Kips Allowable Axial, Φ*Fu*Anet: 260.0 Kips Anchor Rod Stress Ratio: 91.3% Pass

Base Plate Results Flexural Check Base Plate Stress: 39.4 ksi Allowable Plate Stress: 45.0 ksi Base Plate Stress Ratio: 87.6% Pass

Stiffener Results

Horizontal Weld: n/a Vertical Weld: n/a Plate Flex+Shear, fb/Fb+(fv/Fv)^2: Plate Tension+Shear, ft/Ft+(fv/Fv)^2 n/a Plate Comp. (AISC Bracket):

Pole Results

Pole Punching Shear Check:



n/a

Analysis Date: 5/11/2020

^{* 0 =} none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

^{**} Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

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	TAPP TP-18914	Mike

Caisson Calculation

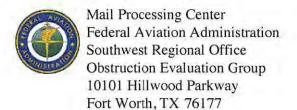
According to ANSI/TIA-222-G

- 1. Foundation overturning resistance calculated with PLS Caisson, for Brom's method for rigid piles. Soil layers modeled after recommendations from the geotechnical report.
- 2. Cohesion strength for the upper 18 ft has been reduced by 50%
- 3. In tieu of a soil resistance factor fs = 0.75 (TIA-9.4.1) an additional safety fator against soil failure of 1.33 has been applied.
- 4. Foundation is designed with a minimum safety factor resisting overturning of 2.0
- 5. Foundation has been designed with factored loads per TIA-222-G.
- 6. Design water table = 20 ft below grade

*** PIER PROPERTIES	CONCRETE STRENGT	H(ksi) = 4	.00		STEEL S	TRENGTH (ksi)	= 60.00
	DIAMETER (ft) =	6.000	DISTANCE FR	OM TOP OF P	IER TO GROU	ND LEVEL (ft)	0.50
*** SOIL PROPERTIES	LAYER TYPE THI		AT TOP OF L		SITY	си кр	PHI
		(ft)				sf)	(degrees)
	1 8	4.00			00.0	1.000	-0.00
	2 5	10.00			20.0	2.663	27.00
	3 8	6.00			05.0	2.198	22.00
	4 S	3.00			42.6	2.198	22.00
	5 8	20.00	2	3.00 1	70.0	3.392	33.00
** DESIGN (FACTORED)	LOADS AT TOP OF PIE	R MOMENT (ft	-k) = 3887.	O VERTICA	L(k) = 39	.O SHEAR ()	k) = 51.0
			SAFETY FACT				•
** CALCULATED PIER LI	ENGTH (ft) = 24.5	00					
	(-2, -111						
			ORCES ALONG	PIER			
** CHECK OF SOILS PRO	OPERTIES AND ULTIMAT	E RESISTING F			v.n	TODO?	N. 1704
** CHECK OF SOILS PRO	OPERTIES AND ULTIMAT	E RESISTING FO	DENSITY	cu	КР	FORCE	
** CHECK OF SOILS PRO TYPE TOP OF LAYER	OPERTIES AND ULTIMAT R BELOW TOP OF PIER (ft)	E RESISTING FO THICKNESS (ft)	DENSITY (pcf)			(k)	(ft)
** CHECK OF SOILS PRO TYPE TOP OF LAYER S	OPERTIES AND ULTIMATE R BELOW TOP OF PIER (ft) 0.50	E RESISTING FO THICKNESS (ft) 4.00	DENSITY (pcf) 100.0	cu	1.000	(k) 14.40	(ft) 3.17
** CHECK OF SOILS PRO TYPE TOP OF LAYER S	OPERTIES AND ULTIMATE R BELOW TOP OF PIER (\$\foatstyle{x}\) 0.50 4.50	E RESISTING FO THICKNESS (ft) 4.00 10.00	DENSITY (pcf) 100.0 120.0	cu	1.000	(k) 14.40 479.34	(ft) 3.17 10.50
** CHECK OF SOILS PROTYPE TOP OF LAYER S S S	OPERTIES AND ULTIMATE R BELOW TOP OF PIER (£t) 0.50 4.50 14.50	THICKNESS (ft) 4.00 10.00 3.22	DENSITY (pcf) 100.0 120.0 105.0	cu	1.000 2.663 2.198	(k) 14.40 479.34 225.33	(ft) 3.17 10.50 16.10
*** CHECK OF SOILS PRO TYPE TOP OF LAYER S S S	OPERTIES AND ULTIMAT R BELOW TOP OF PIER (ft) 0.50 4.50 14.50 17.72	THICKNESS (ft) 4.00 10.00 3.22 2.78	DENSITY (pcf) 100.0 120.0 105.0 105.0	cu	1.000 2.663 2.198 2.198	(k) 14.40 479.34 225.33 -229.26	(ft) 3.17 10.50 16.10 19.14
** CHECK OF SOILS PRO TYPE TOP OF LAYER S S S S	OPERTIES AND ULTIMAT R BELOW TOP OF PIER (ft) 0.50 4.50 14.50 17.72 20.50	E RESISTING FO THICKNESS (ft) 4.00 10.00 3.22 2.78 3.00	DENSITY (pcf) 100.0 120.0 105.0 105.0 42.6	cu	1.000 2.663 2.198 2.198 2.198	(k) 14.40 479.34 225.33 -229.26 -272.27	(ft) 3.17 10.50 16.10 19.14 22.01
** CHECK OF SOILS PRO TYPE TOP OF LAYER S S S	OPERTIES AND ULTIMAT R BELOW TOP OF PIER (ft) 0.50 4.50 14.50 17.72	THICKNESS (ft) 4.00 10.00 3.22 2.78	DENSITY (pcf) 100.0 120.0 105.0 105.0	cu	1.000 2.663 2.198 2.198	(k) 14.40 479.34 225.33 -229.26	ARM (ft) 3.17 10.50 16.16 19.14 22.01 24.01
*** CHECK OF SOILS PROTYPE TOP OF LAYER S S S S S S S S S S S	OPERTIES AND ULTIMAT R BELOW TOP OF PIER (ft) 0.50 4.50 14.50 17.72 20.50 23.50	E RESISTING FO THICKNESS (ft) 4.00 10.00 3.22 2.78 3.00	DENSITY (pcf) 100.0 120.0 105.0 105.0 42.6	cu	1.000 2.663 2.198 2.198 2.198	(k) 14.40 479.34 225.33 -229.26 -272.27	(ft) 3.17 10.50 16.10 19.14 22.01
*** CHECK OF SOILS PROTIFE TOP OF LAYER S S S S S S	OPERTIES AND ULTIMATE R BELOW TOP OF PIER (ft) 0.50 4.50 14.50 17.72 20.50 23.50 ALONG PIER	THICKNESS (ft) 4.00 10.00 3.22 2.78 3.00	DENSITY (pcf) 100.0 120.0 105.0 105.0 42.6 170.0	(paf)	1.000 2.663 2.198 2.198 2.198 3.392	(k) 14.40 479.34 225.33 -229.26 -272.27	(ft) 3.17 10.50 16.16 19.14 22.01 24.01
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	MILL AND ADDITIONAL O	MIL # 11 - 11 - 10 - 10 - 10 - 10 - 10 - 10	42 THOUS 100 DE 1 TOINE	· · · · · · · · · · · · · · · · · · ·
DISTANCE BELOW TOP OF PIER (ft)	SHEAR (k)	MOMENT (ft-k)	SHEAR (k)	MOMENT (ft-k)
0.00	68.4	5242.5	51.3	3932.0
2.45	65.0	5407.8	48.7	4056.0
4.90	45.9	5551.1	34.4	4163.4
7.35	-24.0	5584.9	-18.0	4188.8
9.80	-128.4	5405.2	-96.3	4054.0
12.25	-267.3	4927.4	-200.5	3695.7
14.70	-438.1	4067.2	-328.6	3050.5
17.15	-607.7	2791.2	-455.8	2093.5
19.60	-499.1	1349.6	-374.4	1012.2
22.05	-282,6	388.9	-212.0	291.7
24.50	-0.0	-0.0	-0.0	-0.0

*** TOTAL REINFORCEMENT PCT = 0.82 *** USABLE AXIAL CAP. (k) = 39.0 REINFORCEMENT AREA $(in^2) = 33.39$ USABLE MOMENT CAP. (ft-k) = 4204.0



Issued Date: 03/25/2020

Cindy D. McCarty
East Kentucky Network, LLC
101 Technology Trail
Ivel, KY 41642

** DETERMINATION OF NO HAZARD TO AIR NAVIGATION **

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure: Monopole Ermine Location: Ermine, KY

Latitude: 37-07-11.13N NAD 83

Longitude: 82-47-36.75W

Heights: 1176 feet site elevation (SE)

110 feet above ground level (AGL) 1286 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

It is required that FAA Form 7460-2, Notice of Actual Construction or Alteration, be e-filed any time the project is abandoned or:

	At least 10 days prior to start of construction (7460-2, Part 1)
X	Within 5 days after the construction reaches its greatest height (7460-2, Part 2)

Based on this evaluation, marking and lighting are not necessary for aviation safety. However, if marking/lighting are accomplished on a voluntary basis, we recommend it be installed in accordance with FAA Advisory circular 70/7460-1 L Change 2.

This determination expires on 09/25/2021 unless:

- (a) the construction is started (not necessarily completed) and FAA Form 7460-2, Notice of Actual Construction or Alteration, is received by this office.
- (b) extended, revised, or terminated by the issuing office.
- (c) the construction is subject to the licensing authority of the Federal Communications Commission (FCC) and an application for a construction permit has been filed, as required by the FCC, within 6 months of the date of this determination. In such case, the determination expires on the date prescribed by the FCC for completion of construction, or the date the FCC denies the application.

NOTE: REQUEST FOR EXTENSION OF THE EFFECTIVE PERIOD OF THIS DETERMINATION MUST BE E-FILED AT LEAST 15 DAYS PRIOR TO THE EXPIRATION DATE. AFTER RE-EVALUATION OF CURRENT OPERATIONS IN THE AREA OF THE STRUCTURE TO DETERMINE THAT NO SIGNIFICANT AERONAUTICAL CHANGES HAVE OCCURRED, YOUR DETERMINATION MAY BE ELIGIBLE FOR ONE EXTENSION OF THE EFFECTIVE PERIOD.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power, except those frequencies specified in the Colo Void Clause Coalition; Antenna System Co-Location; Voluntary Best Practices, effective 21 Nov 2007, will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA. This determination includes all previously filed frequencies and power for this structure.

If construction or alteration is dismantled or destroyed, you must submit notice to the FAA within 5 days after the construction or alteration is dismantled or destroyed.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

A copy of this determination will be forwarded to the Federal Communications Commission (FCC) because the structure is subject to their licensing authority.

If we can be of further assistance, please contact our office at (718) 553-2611, or angelique.eersteling@faa.gov. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2020-ASO-6839-OE.

Signature Control No: 432549509-434535540

(DNE)

Angelique Eersteling Technician

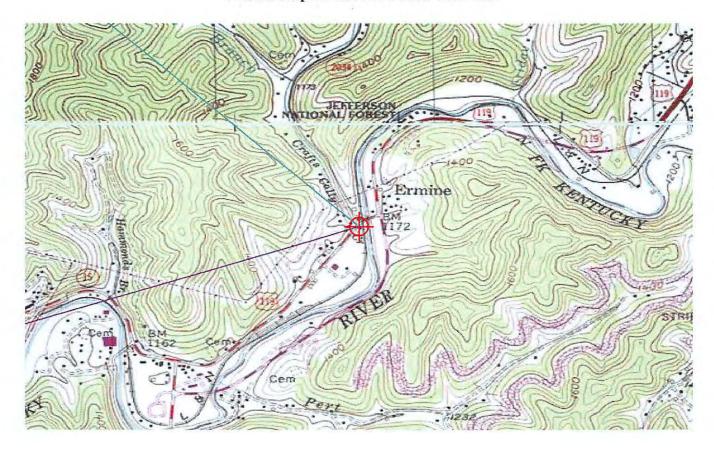
Attachment(s) Frequency Data Map(s)

cc: FCC

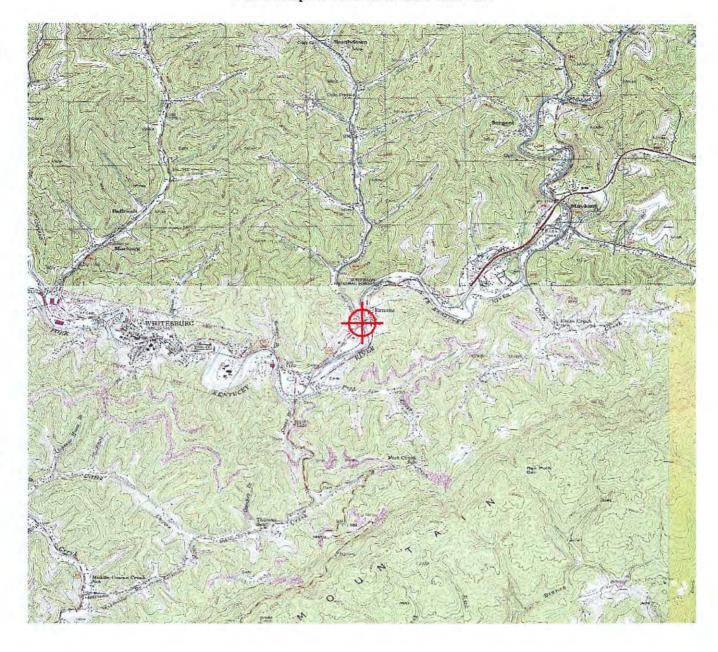
Frequency Data for ASN 2020-ASO-6839-OE

	LOW	HIGH	FREQUENCY		ERP
_	FREQUENCY	FREQUENCY	UNIT	ERP	UNIT
	_	-	O. I.		IDIII.
	6	7	GHz	55	dBW
	6	7	GHz	42	dBW
	10	11.7	GHz	55	dBW
	10	11.7	GHz	42	dBW
	17.7	19.7	GHz	55	dBW
	17.7	19.7	GHz	42	dBW
	21.2	23.6	GHz	55	dBW
	21.2	23.6	GHz	42	dBW
	614	698	MHz	1000	W
	614	698	MHz	2000	W
	698	806	MHz	1000	W
	806	901	MHz	500	W
	806	824	MHz	500	W
	824	849	MHz	500	W
	851	866	MHz	500	W
	869	894	MHz	500	W
	896	901	MHz	500	W
	901	902	MHz	7	W
	929	932	MHz	3500	W
	930	931	MHz	3500	W
	931	932	MHz	3500	W
	932	932.5	MHz	17	dBW
	935	940	MHz	1000	W
	940	941	MHz	3500	W
	1670	1675	MHz	500	W
	1710	1755	MHz	500	W
	1850	1910	MHz	1640	W
	1850	1990	MHz	1640	W
	1930	1990	MHz	1640	W
	1990	2025	MHz	500	W
	2110	2200	MHz	500	W
	2305	2360	MHz	2000	W
	2305	2310	MHz	2000	W
	2345	2360	MHz	2000	W
	2496	2690	MHz	500	W

Verified Map for ASN 2020-ASO-6839-OE



TOPO Map for ASN 2020-ASO-6839-OE



Fwd: Application for Construction - Ermine (Letcher County, KY)

Cindy McCarty

Wed 3/4/2020 12:43 PM FAA/KAZC

To:Raina Helton <rhelton@ekn.com>;

Cindy D. McCarty In-House Counsel East Kentucky Network, LLC d/b/a Appalachian Wireless

(606) 339-1006 (606) 339-1363 (fax) cmccarty@ekn.com

Begin forwarded message:

From: "Houlihan, John F (KYTC)" < <u>John.Houlihan@ky.gov</u>>
Subject: RE: Application for Construction - Ermine (Letcher County, KY)

Date: March 4, 2020 at 11:38:07 AM EST To: Cindy McCarty <<u>cmccarty@ekn.com</u>>

No permit is required from the KAZC. Thank you

Kentucky Airport Zoning Commission (KAZC)
John Houlihan, Administrator
Department of Highways, District 6
421 Buttermilk Pike
Covington, KY 41017
Desk 859-426-6973, Cell 502-330-3955

KAZC webpage: https://transportation.ky.gov/Aviation/Pages/airportzoning.aspx

CONFIDENTIALITY NOTICE: This e-mail message, including any attachments, is for the sole use of the intended recipient(s) and may contain confidential and privileged information. Any unauthorized review, use, disclosure or distribution is prohibited. If you are not the intended recipient, please contact the sender by reply e-mail or call (859) 426-6973 and destroy all copies of the original message.

From: Cindy McCarty < cmccarty@ekn.com Sent: Wednesday, March 4, 2020 10:29 AM

To: Houlihan, John F (KYTC) < John. Houlihan@ky.gov>

Cc: Compliance < compliance@ekn.com>

Subject: Application for Construction - Ermine (Letcher County, KY)

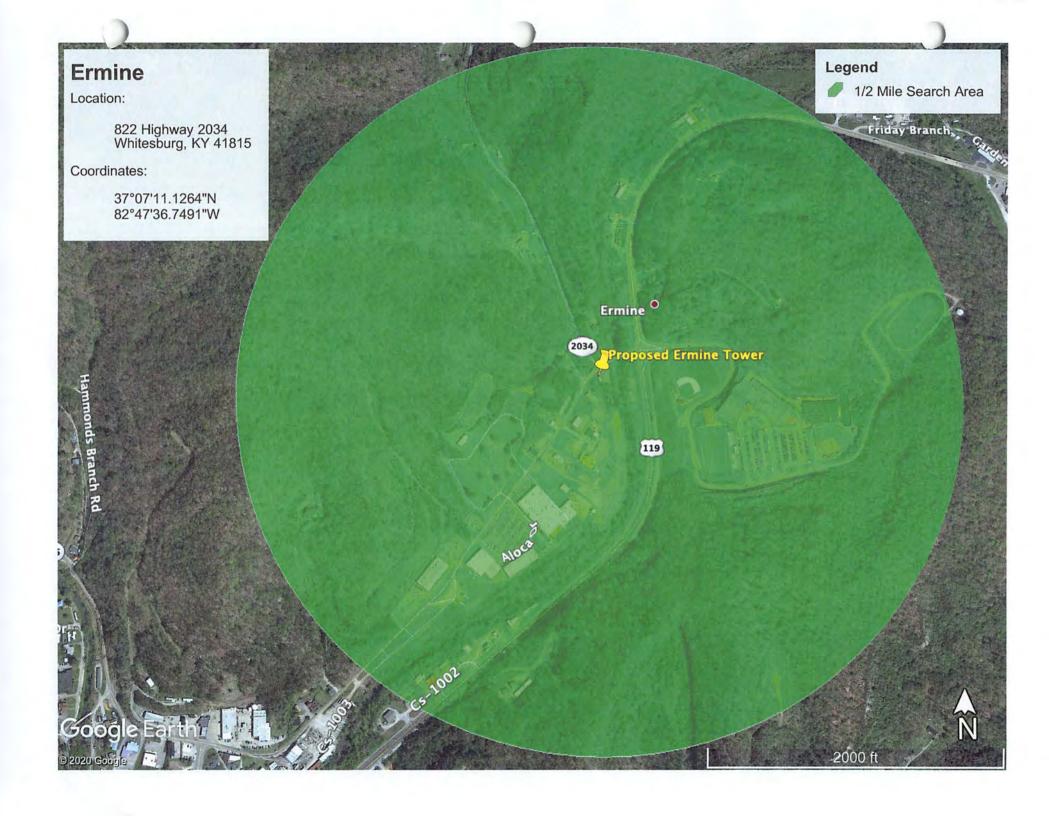
CAUTION PDF attachments may contain links to malicious sites. Please contact the COT Service Desk ServiceCorrespondence@ky.gov for any assistance.

Driving Directions for Ermine

Beginning in front of the Courthouse in Letcher County on Main Street head east toward the intersection of East Main Street and Main Street. Turn right onto Main Street and drive .3 miles to the intersection of Hwy 15 and Main Street. Turn right onto Hwy 15 and drive 1.4 miles until Hwy 15 turns into Hwy 119. Stay straight on Hwy 119. Drive .9 miles to the intersection of Hwy 119 and 2034. Turn left onto 2034 and drive approximately four hundred and fifth feet to the site (signs posted).

Prepared By:

Daryl Bartley Appalachian Wireless (606) 791-0310



MEMORANDUM OF LEASE

WITNESSETH

- 1. Demised Premises. For good and valuable consideration, Lessor leased to Lessee, and Lessee has leased from Lessor that certain tract of real estate located in Letcher County, Kentucky, and being a portion of the same land conveyed to Lessor by Deed dated June 2, 2017, and recorded on June 6, 2017, in Deed Book 436, Page 666, in the Letcher County Clerk's Office. Said property is more particularly described in the description attached hereto and made a part hereof as Exhibit A and the plat attached hereto and made a part hereof as Exhibit B, prepared by James W. Caudill, Licensed Professional Land Surveyor (hereinafter referred to as the "Premises"). The Lessor has also granted unto Lessee full and complete rights of ingress, egress and regress to and from the Premises over any property owned by Lessor and other associated rights for installation of utilities, maintenance, and other purposes.
- 2. Term. The initial term of the Lease is for a period of five (5) years from the Commencement Date set forth above.

- 3. Renewals. The Lease shall automatically renew for an additional seven (7) terms of five (5) years each, unless Lessee provides sixty (60) days written notice prior to the end of the current term that it does not wish to renew.
- 4. Binding Effect. All of the terms, conditions, and covenants hereof shall be binding and inure to the benefit of the parties and their respective heirs, representatives, successors, and assigns.
- 5. Purpose. This Memorandum of Lease is prepared solely for the purpose of recordation, and is not intended to, nor shall it be deemed to, modify any of the terms and conditions set forth in the Lease, nor to construe any of the rights, duties or responsibilities of Lessor and Lessee. In the event of any conflict between the terms and conditions of this Memorandum and the terms and conditions of the Lease, the terms and conditions of the Lease shall supersede and control.

[THE REMAINDER OF THIS PAGE INTENTIONALLY LEFT BLANK.]

IN WITNESS WHEREOF, Lessor and Lessee have caused their names to be signed hereto, as of the date(s) indicated below.

hereto, as of the date(s) indicated below. LESSOR: THACKER-GRIGSBY TELEPHONE CO., INC. Its: COMMONWEALTH OF KENTUCKY COUNTY OF Flood The foregoing instrument was acknowledged before me on this 28th day of February, 2020, by William Congsby of Thacker-Grigsby Telephone Co., Inc., Lessor. Commission No.: KYNP375 My Commission Expires 2-6-2024 LESSEE: EAST KENTUCKY NETWORK, LLC D/B/A APPALACHIAN WIRELESS By: W.A. Gillum Its: CEO/ General Manager COMMONWEALTH OF KENTUCKY COUNTY OF FLOYD The foregoing instrument was acknowledged before me on this 2154 _, 2020, by W.A. Gillum, CEO/General Manager of East Kentucky Network, LLC d/b/a Appalachian Wireless, Lessee. Notary Public Commission No.: KYNP 375 My Commission Expires 2-6-2014.

This instrument was prepared by:

Krystal Branham, Attorney 101 Technology Trail Ivel, Kentucky 41642 (606) 477-2355

LOT DESCRIPTION

Property of
Thacker Grigsby Telephone Company
P.O. Box 789
Hindman, Kentucky 41822
Near Ermine in Letcher County
May 20, 2019

Being a portion of the property conveyed to Thacker-Grigsby Telephone Company, Incorporated from Caudill Lumber Company, Incorporated by Deed of Conveyance dated June 2, 2017 and of record in Deed Book 436 Page 666 of the Letcher County Court Clerk's Office in Whitesburg, Kentucky.

Lot 1A

A circle with a diameter of 9 feet having a center point bearing S 31°19'33" W, a distance of 48.6183 feet, from a set pk nail in pavement being the northern most corner of lot conveyed to Thacker Grigsby Telephone Company by Deed Book 436 Page 666, and also bearing N 81°45'30" E, a distance of 17.94 feet, from a set iron pin with cap marked LS#2259 being the northwest corner of said lot. Containing a calculated area of 63.60 square feet, or 0.001 acres.

Also to be included is a right of way from the highway to the proposed tower site. This is now along the existing access, but if for any reason this access becomes blocked, then additional right of way will be provided for access to the tower site.

Also to be included is a right to utilize area adjacent to the tower site for the purposes of construction and maintenance of the tower and attachment to the existing equipment building.

Unless stated otherwise, any monument referred to herein as "set iron pin with cap" is a set ½" diameter rebar, at least eighteen (18") in length, with a plastic cap stamped "LS-2259". All bearings stated herein are referred to NAD83, KY single zone of the Kentucky state plane system.

This survey was performed on May 20, 2019 by James W. Caudill, a Kentucky Licensed Professional Land Surveyor No. 2259.

STATE OF KENTUCKY COUNTY OF LETCHER

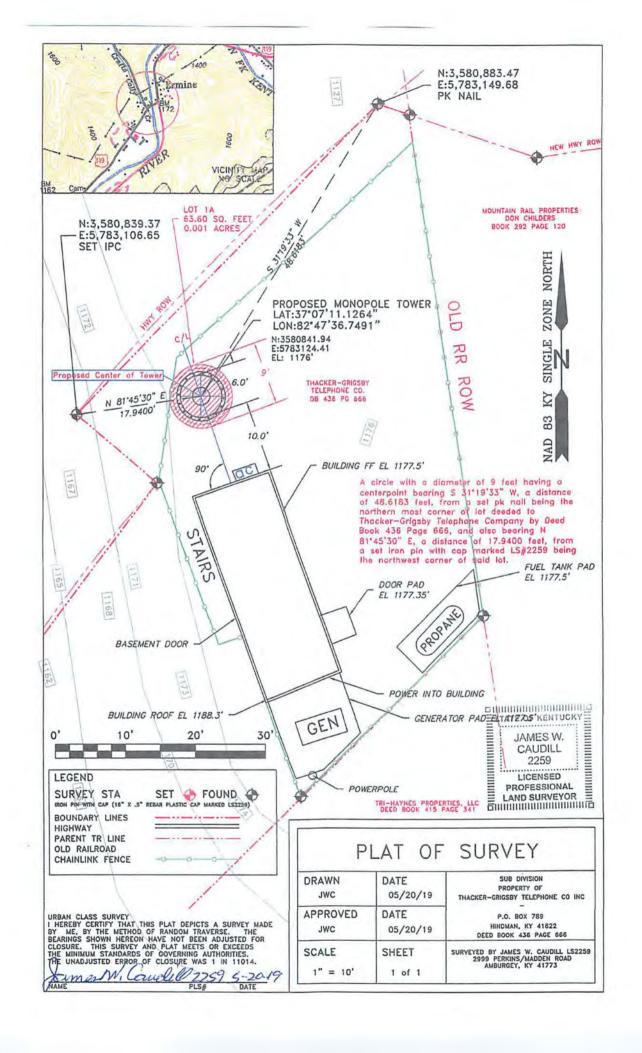
I. WINSTON MEADE. CLERK OF LETCHER COUNTY, DO HEREBY CERTIFY
THAT THE FOREGOING LOCAL WAS ON THIS 10 DAY OF
WAS ON THIS 10 DAY OF THE PROBLEM THAT IT
AND THIS CERTIFICATE HAVE BEEN RECORDED IN LOCAL
BOOK PAGE 52
WITNESS MY HAND THIS 10 DAY OF W CURCLE 2020

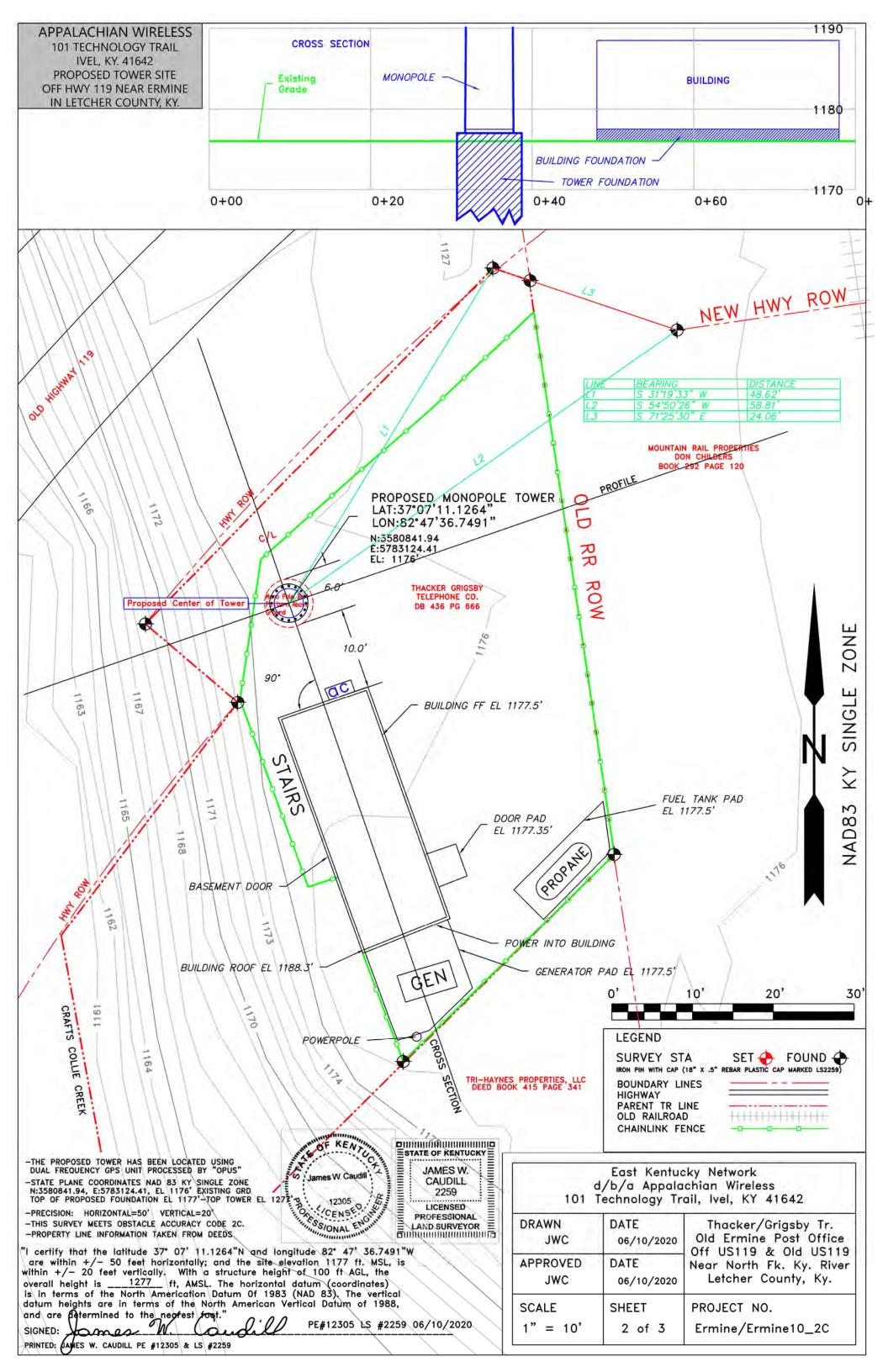
WINSTON MEADE CLERK

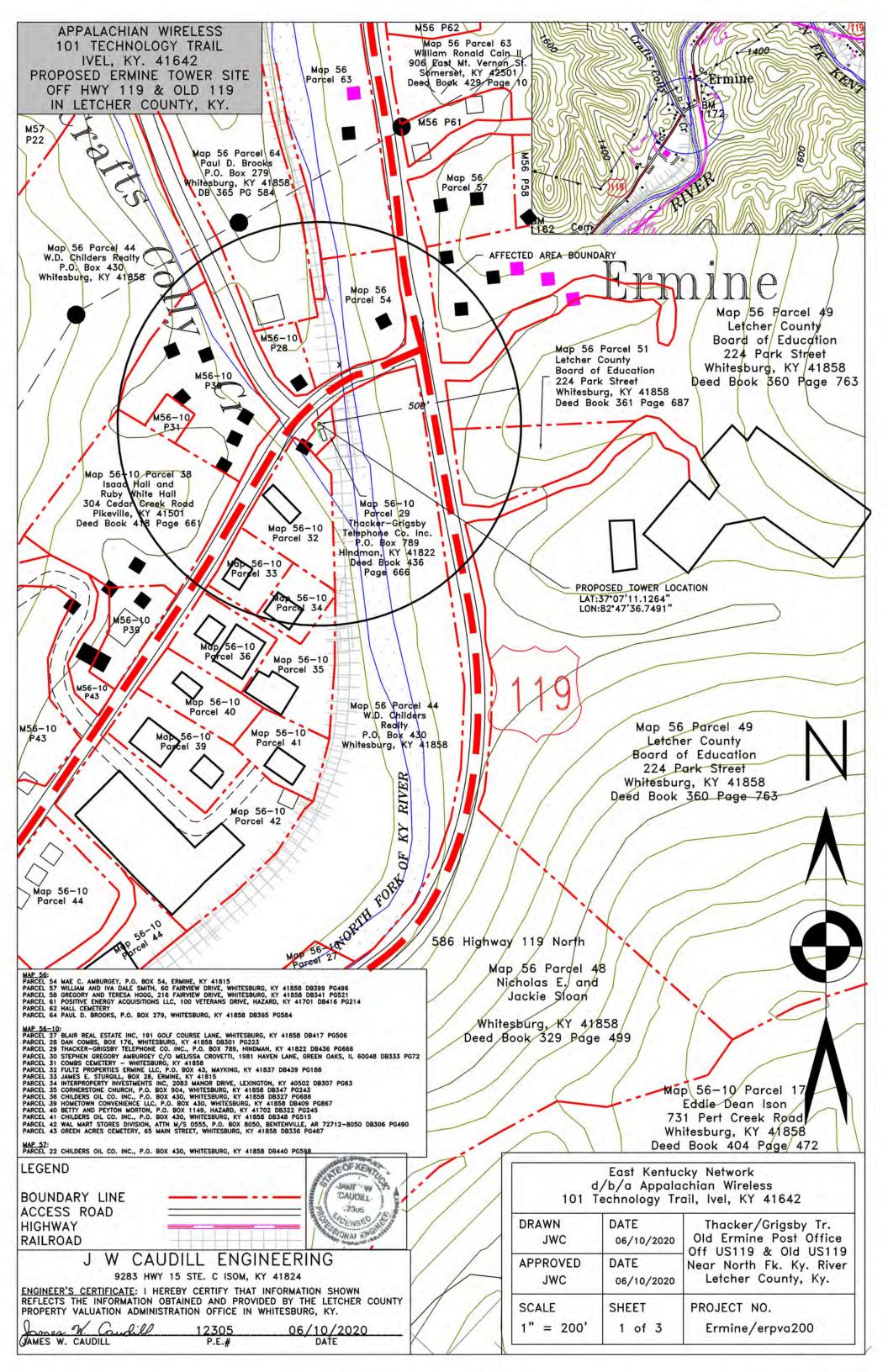
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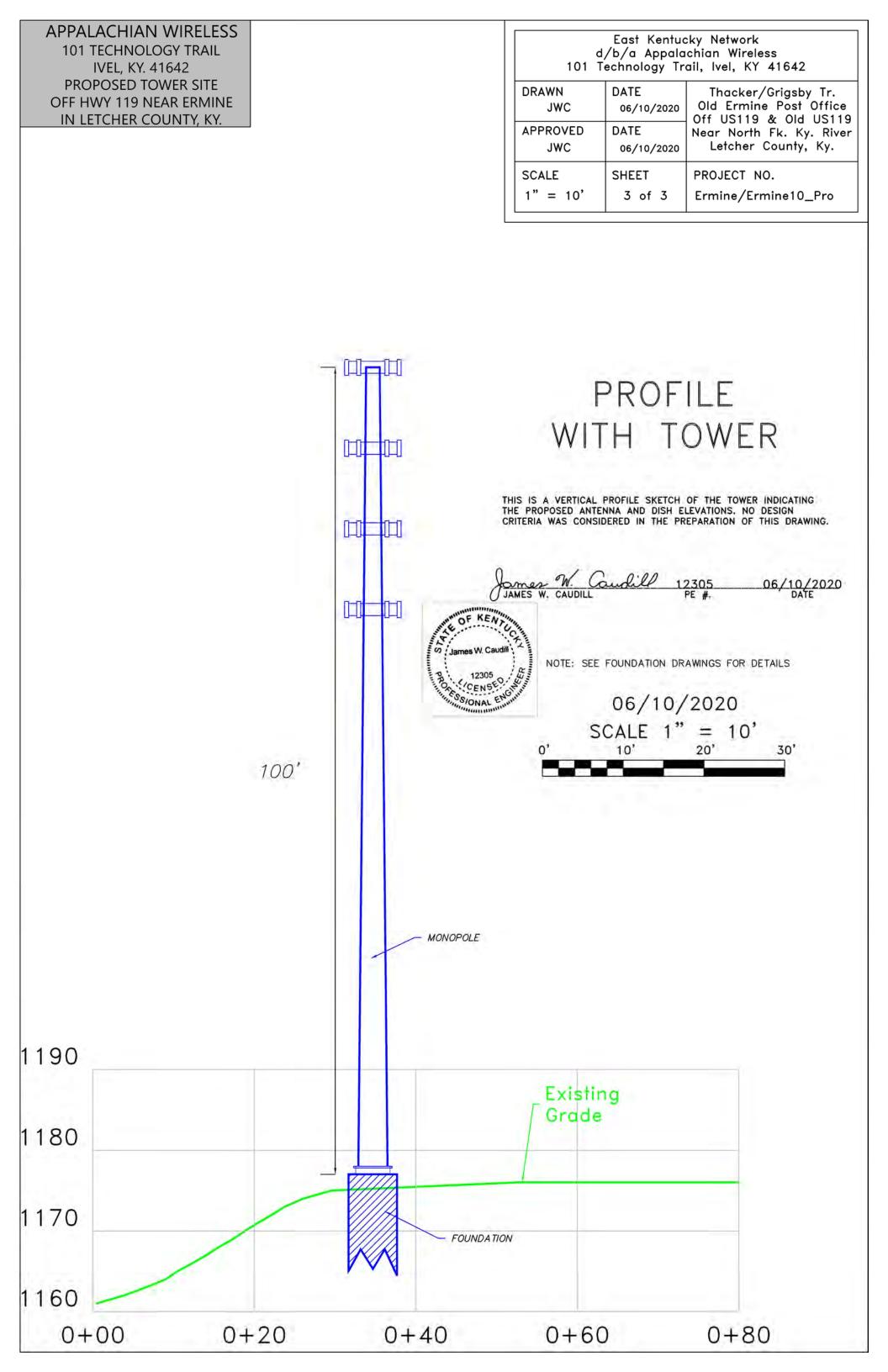
James W. Caudill, PLS #2259 S-20-19

JAMES W.
CAUDILL
2259
LICENSED
PROFESSIONAL
LAND SURVEYOR









Utility ID		Utility Type	Class	City	State
4107900		Celiular	D	Atlanta	GA
4109300	Access Point, Inc.	Cellular	D	Cary	NC
4108300		Cellular	Α		MI
4110650	Alliant Technologies of KY, L.L.C.	Cellular	С		ИJ
44451184	Alltel Communications, LLC	Cellular	Α	Basking Ridge	NJ
4110850	AltaWorx, LLC	Cellular	C	Fairhope	AL
4107800	American Broadband and Telecommunications Company	Cellular	C	Toledo	OH_
4108650	AmeriMex Communications Corp.	Cellular	D	Dunedin	FL
4105100	AmeriVision Communications, Inc. d/b/a Affinity 4	Cellular	٥	Virginia Beach	VA
4110700	Andrew David Balholm dba Norcell	Cellular	C	Clayton	WA
4108600	BCN Telecom, Inc.	Cellular	D	Morristown	NJ
4110550	Blue Casa Mobile, LLC	Cellular	D	Santa Barbara	CA
4108750	Blue Jay Wireless, LLC	Cellular	С	Carrollton	TX
4111050	BlueBird Communications, LLC	Cellular	С	New York	NY
4202300	Bluegrass Wireless, LLC	Cellular	Α	Elizabethtown	KY
4107600	Boomerang Wireless, LLC	Cellular	В	Hiawatha	IA _
4105500	BullsEye Telecom, Inc.	Cellular	D	Southfield	МІ
		Cellular	D	Boston	MA
4100700	Cellco Partnership dba Verizon Wireless	Cellular	Α	Basking Ridge	ИJ
4106600	Cintex Wireless, LLC	Cellular	D	Rockville	MD
4111000	ComApp Technologies LLC	Cellular	С	Melrose	MA
		Cellular	Α	Portland	OR
		Cellular	A	San Francisco	CA
		Cellular	A	San Antonio	TX
4001900		Cellular	D	Grand Rapids	MI
		Cellular	Α	Elizabethtown	KY
		Cellular	Α	ivel	KY
		Cellular	D	Ocala	FL
		Cellular	D	Bartiesville	ОК
		Cellular	D	Chattanooga	TN
		Cellular	С	Concord	NC
		Cellular	D	Oak Hill	VA
		Cellular	D	Norcross	GA
		Cellular	В	Covington	LA
	· · · · · · · · · · · · · · · · · · ·	Cellular	A	Mountain View	CA
		Cellular	D	Quincy	MA
		Cellular	A	San Diego	CA
10630		Cellular	A	Basking Ridge	Ų
		Cellular	С	Atlanta	GA
	<u> </u>	Cellular	A	Newport	KY
		Cellular	D	Tulsa	ОК
		Cellular	D	New York	NY
		Cellular	A	Basking Ridge	NJ
		Cellular	A	Elizabethtown	KY
		Cellular	Α	Elizabethtown	KY
	·	Cellular	D	Johnstown	PA
		Cellular	C	Detroit	MI
		Cellular	D	Newark	NJ
		Cellular	A	Bellevue	WA
		Cellular	D	Mesa	AZ
	^	Cellular	Α	San Antonio	TX_
		Cellular	Α	Basking Ridge	NJ
4000800	Nextel West Corporation	Cellular	D	Overland Park	KS
		Cellular	D	Overland Park	KS

4001800	OnStar, LLC	Cellular	Α	Detroit	MI
	Onvoy Spectrum, LLC	Cellular	c	Plymouth	MN
	Patriot Mobile LLC	Cellular	<u> </u>	Southlake	TX
	Plintron Technologies USA LLC	Cellular	D	Bellevue	WA
	PNG Telecommunications, Inc. dba PowerNet Global Communications	Cellular	D	Cincinnati	ОН
	Powertel/Memphis, Inc. dba T-Mobile	Cellular	A	Bellevue	WA
	Puretalk Holdings, LLC	Cellular	Ā	Covington	GA
	Q Link Wireless, LLC	Cellular	Ā	Dania	FL
	Ready Wireless, LLC	Cellular	В	Hiawatha	ΙA
	Republic Wireless, Inc.	Cellular	D	Raleigh	NC
	ROK Mobile, Inc.	Cellular	c	Culver City	CA
	Rural Cellular Corporation	Cellular	Ā	Basking Ridge	IN]
	Sage Telecom Communications, LLC dba TruConnect	Cellular	<u> </u>	Los Angeles	CA
	SelecTel, Inc. d/b/a SelecTel Wireless	Cellular	D	Freemont	NE
	SI Wireless, LLC	Cellular	Ā	Carbondale	IL.
	Spectrotel, Inc. d/b/a Touch Base Communications	Cellular	D	Neptune	NJ
	Sprint Spectrum, L.P.	Cellular	A	Atlanta	GA
	SprintCom, Inc.	Cellular	A	Atlanta	GA
	Stream Communications, LLC	Cellular	D	Dallas	TX
	T C Telephone LLC d/b/a Horizon Cellular	Cellular	D	Red Bluff	CA
	T-Mobile Central, LLC dba T-Mobile	Cellular	Ā	Bellevue	WA
	TAG Mobile, LLC	Cellular	D	Carroliton	ΤX
	Telecom Management, Inc. dba Pioneer Telephone	Cellular	D	South Portland	ME
	Telefonica USA, Inc.	Cellular	D	Miami	FL
	Telrite Corporation dba Life Wireless	Cellular	D	Covington	GA
	Tempo Telecom, LLC	Cellular	D	Kansas City	МО
4109950	The People's Operator USA, LLC	Cellular	D	New York	NY
4109000	Ting, Inc.	Cellular	Α	Toronto	ON
4110400	Torch Wireless Corp.	Cellular	D	Jacksonville	FL
4103300	Touchtone Communications, Inc.	Cellular	D	Whippany	NJ
4104200	TracFone Wireless, Inc.	Cellular	D	Miami	FL
4002000	Truphone, Inc.	Cellular	D	Durham	NC
	UVNV, Inc.	Cellular	D	Costa Mesa	CA
4105700	Virgin Mobile USA, L.P.	Cellular	Α	Atlanta	GA
	Visible Service LLC	Cellular	C	Lone Tree	œ
4106500	WiMacTel, Inc.	Cellular	D	Palo Alto	CA
	Wing Tel Inc.	Cellular	С	New York	NY
4109900	Wireless Telecom Cooperative, Inc. dba theWirelessFreeway	Cellular	D	Louisville	KY

S & S Tower Services 120 Branden Dr. Mousie, KY 41839

Kentucky Public Service Commission 211 Sower Blvd. P.O. Box 615 Frankfort, KY 40602-0615

Dear Commissioners:

The Construction Manager for the proposed communications facility will be Dave Strausbaugh. His contact information is (606) 497-6730 or dstrausbaugh010@gmail.com.

Dave has been in the industry completing civil construction and constructing towers since 1991. He has worked for S&S Tower Services since 2015 as Construction Manager overseeing the construction of telecommunications towers and sites.

Thank you,

Chris Strausbaugh

Owner

S&S Tower Services

(606) 497-5798