# RECEIVED

### COMMONWEALTH OF KENTUCKY BEFORE THE PUBLIC SERVICE COMMISSION

FEB 16 2016

PUBLIC SERVICE COMMISSION

In the matter of:

THE APPLICATION OF EAST KENTUCKY NETWORK

LIMITED LIABILITY COMPANY FOR THE ISSUANCE

OF A CERTIFICATE OF PUBLIC CONVENIENCE AND

NECESSITY TO CONSTRUCT A TOWER IN HARLAN

COUNTY, KENTUCKY).

East Kentucky Network, LLC, d/b/a Appalachian Wireless, was granted authorization to provide Personal Communications Service ("PCS") in the Middlesboro-Harlan, KY Basic Trading Area (BTA295) by the Federal Communications Commission (FCC). FCC license is included as Exhibit 1. East Kentucky Network, LLC merger documents were filed with the Commission on February 2, 2001 in Case # 2001-022. East Kentucky Network, LLC is a Kentucky Limited Liability Company that was organized on June 16, 1998. East Kentucky Network, LLC is in good standing with the state of Kentucky.

In an effort to improve service in Harlan County, East Kentucky Network, LLC pursuant to KRS 278.020 Subsection 1 and 807 KAR 5:001 is seeking the Commission's approval to construct a 180 foot monopole tower on a tract of land located at McKnight Road, Benham, Harlan County, Kentucky (36°57'44.9657"N 82°56'58.3310"W). A map and detailed directions to the site can be found in Exhibit 7.

Exhibit 2 is a list of all Property owners according to the property valuation administrator's record who own property within 500 feet of the proposed tower in accordance with the Property Valuation Administrator.

Pursuant to 807 KAR 5:063 Section 1(1)(1) and Section 1(1)(m) all affected property owners according to the property valuation administrator's record who own property within 500 feet of the proposed Tower were notified by certified mail return receipt requested of East Kentucky

Network, LLC's proposed construction and informed of their right to intervene. They were given the docket number under which this application is filed. Enclosed in Exhibit 2 is a copy of that notification.

Harlan County has no formal local planning unit. In absence of this unit the Harlan County Judge Executive's office was notified by certified mail, return receipt requested of East Kentucky Network Limited Liability Company's proposal and informed of their right to intervene. They were given the docket number under which this application is filed. Enclosed in Exhibit 3 is a copy of that notification.

Notice of the location of the proposed construction was published in the Harlan Daily Enterprise, February 16, 2016, edition. Enclosed is a copy of that notice in Exhibit 3. The Harlan Daily Enterprise is the newspaper with the largest circulation in Harlan County.

Environmental Resources Management Consulting Company was employed to determine soil and rock types and to ascertain the distance to solid bedrock. The geotechnical report is enclosed as Exhibit 4.

A copy of the tower design information is enclosed as Exhibit 5. The proposed tower has been designed by engineers at TransAmerican Power Products, Inc. and will be constructed under their supervision. Their qualifications are evidenced in Exhibit 5 by the seal and signature of the registered professional engineer responsible for this project.

The tower will be erected by World Tower Company, Inc. of Mayfield, Kentucky. World Tower Company, Inc. has vast experience in the erection of communications towers.

FAA and Kentucky Airport Zoning Commission applications are included as Exhibit 6.

No Federal Communications Commission approval is required prior to construction of this facility. Once service is established from this tower we must immediately notify the Federal Communications Commission of its operation. Prior approval is needed only if the proposed

facility increases the size of the cellular geographic service area. This cell site will not expand the cellular geographic service area.

East Kentucky Network, LLC will finance the subject Construction with earned surplus in its General Fund.

Estimated Cost of Construction \$ 350,000.00 Annual Operation Expense of Tower \$ 12,500.00

Two notice signs meeting the requirements prescribed by 807 KAR 5:063, Section 1(2), measuring at least two (2) feet in height and four (4) feet in width and containing all required language in letters of required height, have been posted, one at a visible location on the proposed site and one on the nearest public road. The two signs were posted on December 23, 2015, and will remain posted for at least two weeks after filing of this application as specified.

Enclosed in Exhibit 8 is a copy of East Kentucky Network LLC's Lease Agreement for the site location along with a lot description.

The proposed construction site is located in the city limits of Benham on a piece of land previously developed.

East Kentucky Network LLC's operation will not affect the use of nearby land nor its value. No more suitable site exists in the area. A copy of the search area map is enclosed in Exhibit 7. No other tower capable of supporting East Kentucky Network, LLC's load exists in the general area; therefore, there is no opportunity for co-location of our facilities with anyone else.

Enclosed, and filed as Exhibit 9 is a survey of the proposed tower site signed by a Kentucky registered professional engineer.

Exhibit 11 contains a vertical sketch of the tower supplied by James W. Caudill, Kentucky registered professional engineer.

WHEREFORE, Applicant respectfully requests that the PSC accept the foregoing Application for filing, and having met the requirements of KRS [278.020(1), 278.650, and

278.665] and all applicable rules and regulations of the PSC, grant a Certificate of Public Convenience and Necessity to construct and operate the proposed tower.

The foregoing document was prepared by Cindy D. McCarty, Staff Attorney for East Kentucky Network, LLC d/b/a Appalachian Wireless. All related questions or correspondence concerning this filing should be mailed to East Kentucky Network, LLC d/b/a/ Appalachian Wireless, 101 Technology Trail, Ivel, KY 41642.

SUBMITTED BY: Lugar

Havey DATE: 2/12/2016

Lynn Haney, Regulatory Compliance Director

APPROVED BY:

WA Lilleyn

DATE: 2/15/2016

W.A. Gillum, General Manager

ATTORNEY:

Hon. Cindy D. McCarty, Attorney

DATE: 2/15/2016

## **CONTACT INFORMATION:**

W.A. Gillum, General Manager Phone: (606) 477-2355, Ext. 111 Email: wagillum@ekn.com

Lynn Haney, Regulatory Compliance Director

Phone: (606) 477-2355, Ext. 1007

Email: lhaney@ekn.com

Cindy D. McCarty, Staff Attorney Phone: (606) 477-2355, Ext. 1006 Email: cmccarty@ekn.com

## **Mailing Address:**

East Kentucky Network, LLC d/b/a Appalachian Wireless 101 Technology Trail Ivel, KY 41642

1	FCC License
2	Copies of Cell Site Notices to Land Owners
3	Notification of County Judge Executive and Newspaper Advertisement
4	Universal Soil Bearing Analysis
5	Tower Design
6	FAA and KAZC Applications
7	Driving Directions from County Court House and Map to Suitable Scale
8	Lease for Proposed Site with Legal Description
9	Survey of Site Signed/Sealed by Professional Engineer Registered in State of Kentucky
10	Site Survey Map with Property Owners Identified in Accordance with PVA of County
11	Vertical Profile Sketch of Proposed Tower
12	



## PCS Broadband License - WQEF975 - East Kentucky Network, LLC d/b/a Appalachian Wireless

Call Sign

WQEF975

Radio Service

CW - PCS Broadband

Status

Active

Auth Type

Regular

Market

Market

MTA044 - Knoxville

Channel Block

Submarket

12

Associated Frequencies 001850.00000000-001865.00000000 001930.00000000-001945.00000000

(MHz)

Dates

Grant

05/27/2015

Expiration

06/23/2025

Effective

05/27/2015

Cancellation

**Buildout Deadlines** 

1st

2nd

**Notification Dates** 

1st

2nd

Licensee

FRN

0001786607

Type

Limited Liability Company

Licensee

East Kentucky Network, LLC d/b/a Appalachian

Wireless

101 Technology Trail

Ivel, KY 41642

ATTN W.A. Gillum, General Manager/CEO

P:(606)477-2355

Contact

Lukas, Nace, Gutierrez & Sachs, LLP

Pamela L Gist Esq 8300 Greensboro Drive

McLean, VA 22102

P:(703)584-8665 F:(703)584-8695

E:pgist@fcclaw.com

Ownership and Qualifications

Radio Service Type Mobile

Regulatory Status Common Carrier Interconnected

Yes

Alien Ownership

The Applicant answered "No" to each of the Alien Ownership questions.

#### **Basic Qualifications**

The Applicant answered "No" to each of the Basic Qualification questions.

### **Tribal Land Bidding Credits**

This license did not have tribal land bidding credits.

#### **Demographics**

Race

Ethnicity

Gender

#### **EXHIBIT II: LIST OF PROPERTY OWNERS:**

### Statement Pursuant to Section 1 (1) (I) 807 KAR 5:063

**Section 1 (1)(I) 1.** The following is a list of every property owner who according to property valuation administrator's records, owns property within 500 feet of the proposed tower and each have been: notified by certified mail, return receipt requested, of the proposed construction,

**Section 1 (1)(I) 2.** Every person listed below who, according to the property valuation administrator's records, owns property within 500 feet of the proposed tower has been: Given the Commission docket number under which the application will be processed: and

**Section 1 (1)(I) 3.** Every person listed below who, according to property valuation administrator's records owns property within 500 feet of the proposed tower has been: Informed of his right to request intervention.

#### LIST OF PROPERTY OWNERS

Gary D. and Mitzi G. Huff P.O. Box 122 Benham, KY 40807

Pam and Wesley Sheffield P.O. Box 524 Benham , KY 40807

> City of Benham P.O. Box E Benham, KY 40807

Southeast Education Foundation 700 College Road Cumberland, KY 40823

Benham Community Methodist Church P.O. Box 128 Benham, KY 40807

> Joyce Fugate P.O. Box 158 St. Paul, VA 24283

P.O. Box 308 Dungannon, VA 24245

Ernest and Jamie Marsh P.O. Box 361 Benham, KY 40807

Roy Silver and Elaine Conradi P.O. Box G Benham, KY 40807

> Angelo J. Sparacia 7 Brush Hollow Road Holbrook, NY 11741

James Edward and Mary Jane Sneed 242 Mcknight Street Benham, KY 40807

> Herbert Douglas P.O. Box 551 Benham, KY 40807

Shirley D. and Jimmy Hensley P.O. Box 303 Benham, KY 40807

Clyde L. and Nancy Moneyhun P.O. Box 54 Benham, KY 40807

> Billie Jean Franks and Peggy Louis Duncum P.O. Box 403 Benham, KY 40807

Tristan M. Curry 181 Maggard Street Benham, KY 40807 Rebecca Lewis C/O Michael Lewis P.O. Box 405 Benham, KY 40807 Odyssey Inc. P.O. Box 686 Louisa, KY 41230

Michael Hatfield P.O. Box 232 Benham, KY 40807

June Schoonover P.O. Box 322 Benham, KY 40807

Blair Bullock 209 Beale Street Cumberland, KY 40823

Jack Martin P.O. Box 291 Benham, KY 40807

Revelation Energy, LLC 1051 Main Street Milton, WV 25541

Cecil B. and Missy Bowman 182 Maggard Street Benham, KY 40807

Roger and Hilda Faye Raleigh P.O. Box 211 Benham, KY 40807

Gary Lee and Dawne Fields P.O. Box 63 Benham, KY 40807

Nona Cress and Jerry Ann Shoulter 4585 Ward Road Morrow, OH 45152-9709

> ACIN, LLC 5260 Irvine Rd. Huntington, WV 25705

**ARK Land Company** 

City Place One, Suite 300 St. Louis, MO 63141



#### PUBLIC NOTICE

February 15, 2016

Gary D. and Mitzi G. Huff P.O. Box 122 Benham, KY 40807

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

East Kentucky Network, LLC d/b/a Appalachian Wireless has applied to the Public Service Commission of Kentucky for a Certificate of Public Convenience and Necessity to construct and operate a new facility to provide cellular telecommunications service in Harlan County. The facility will include a 180-foot monopole tower with attached antennas extending upwards, and an equipment shelter located on a tract of land at McKnight Road, Benham, Harlan County, Kentucky. A map showing the location of the proposed new facility is enclosed. This notice is being sent to you because you may own property within a 500' radius of the proposed tower.

The Commission invites your comments regarding the proposed construction. You also have the right to intervene in this matter. The Commission must receive your initial communication within 20 days of the date of this letter as shown above.

Your comments and request for intervention should be addressed to: Executive Director's Office, Public Service Commission of Kentucky, P.O. Box 615, Frankfort, KY 40602. Please refer to Case No. 2015-00420 in your correspondence.

If you have any questions for East Kentucky Network, LLC, please direct them to my attention at the following address: East Kentucky Network, LLC, 101 Technology Trail, Ivel, KY 41642 or call me at 606-477-2355, Ext. 1007.

Sincerely,

Lynn Haney

Regulatory Compliance Director



#### PUBLIC NOTICE

February 15, 2016

Pam and Wesley Sheffield P.O. Box 524 Benham, KY 40807

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Joyce Fugate P.O. Box 158 St. Paul, VA 24283

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Lynn Haney

Regulatory Compliance Director

Lyn Henry



#### PUBLIC NOTICE

February 15, 2016

Berchel and Reva H. Caudill P.O. Box 308 Dungannon, VA 24245

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February 15, 2016

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Lynn Haney

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Roy Silver and Elaine Conradi P.O. Box G Benham, KY 40807

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyen Haney





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Angelo J. Sparacia 7 Brush Hollow Road Holbrook, NY 11741

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Lynn Haney

Regulatory Compliance Director

Lyu Haney



#### PUBLIC NOTICE

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James Edward and Mary Jane Sneed 242 Mcknight Street Benham, KY 40807

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyw Haney



#### PUBLIC NOTICE

February 15, 2016

Herbert Douglas P.O. Box 551 Benham, KY 40807

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyur Haney





### **PUBLIC NOTICE**

February 15, 2016

Shirley D. and Jimmy Hensley P.O. Box 303 Benham, KY 40807

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Sincerely,

Lynn Hanev

Regulatory Compliance Director

Lym Haney



#### PUBLIC NOTICE

February 15, 2016

Clyde L. and Nancy Moneyhun P.O. Box 54 Benham, KY 40807

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Regulatory Compliance Director

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Billie Jean Franks and Peggy Louis Duncum P.O. Box 403 Benham, KY 40807

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The Commission invites your comments regarding the proposed construction. You also have the right to intervene in this matter. The Commission must receive your initial communication within 20 days of the date of this letter as shown above.

Your comments and request for intervention should be addressed to: Executive Director's Office, Public Service Commission of Kentucky, P.O. Box 615, Frankfort, KY 40602. Please refer to Case No. 2015-00420 in your correspondence.

If you have any questions for East Kentucky Network, LLC, please direct them to my attention at the following address: East Kentucky Network, LLC, 101 Technology Trail, Ivel, KY 41642 or call me at 606-477-2355, Ext. 1007.

Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyn Haney



#### PUBLIC NOTICE

February 15, 2016

Tristan M. Curry 181 Maggard Street Benham, KY 40807

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

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Sincerely,

Lynn Haney

Regulatory Compliance Director



#### PUBLIC NOTICE

February 15, 2016

Rebecca Lewis C/O Michael Lewis P.O. Box 405 Benham, KY 40807

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lysen Haney

EAST KENTUCKY NETWORK 101 TECHNOLOGY TRAIL IVEL KY 41642 F (606) 874-7550 F (6) 874-7551



### VIA: U.S. CERTIFIED MAIL

#### PUBLIC NOTICE

February 15, 2016

Odyssey Inc. P.O. Box 686 Louisa, KY 41230

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

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Lynn Haney

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### VIA: U.S. CERTIFIED MAIL

#### PUBLIC NOTICE

February 15, 2016

Michael Hatfield P.O. Box 232 Benham, KY 40807

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

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Regulatory Compliance Director

Lyu Haney

EAST KENTUCKY NETWORK 101 TECHNOLOGY TRAIL IVEL, KY 41642 P)\* (606) 874-7550 F- 8) 874-7551



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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyur Hanny



#### **PUBLIC NOTICE**

February 15, 2016

June Schoonover P.O. Box 322 Benham, KY 40807

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Lynn Haney

Regulatory Compliance Director

Lyen Haney

EAST KENTUCKY NETWORK 101 TECHNOLOGY TRAIL IVEL KY 41642 PI (606) 874-7550 EA 5) 874-7551



### VIA: U.S. CERTIFIED MAIL

#### PUBLIC NOTICE

February 15, 2016

Blair Bullock 209 Beale Street Cumberland, KY 40823

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lym Haney



#### PUBLIC NOTICE

February 15, 2016

Jack Martin P.O. Box 291 Benham, KY 40807

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Regulatory Compliance Director

Lyun Haney

EAST KENTUCKY NETWORK 101 TECHNOLOGY TRAIL IVEL, KY 41642 P)\*\* (606) 874 7550 EL 6) 874-7551



### VIA: U.S. CERTIFIED MAIL

#### PUBLIC NOTICE

February 15, 2016

Revelation Energy, LLC 1051 Main Street Milton, WV 25541

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lym Haney



#### PUBLIC NOTICE

February 15, 2016

Cecil B. and Missy Bowman 182 Maggard Street Benham, KY 40807

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lynn Haney



# **PUBLIC NOTICE**

February 15, 2016

Roger and Hilda Faye Raleigh P.O. Box 211 Benham, KY 40807

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lynn Haney

EAST KENTUCKY NETWORK 101 TECHNOLOGY TRAIL IVEL, KY 47642 PHII (606) 874-7550 FA (874-755)



# VIA: U.S. CERTIFIED MAIL

# PUBLIC NOTICE

February 15, 2016

Gary Lee and Dawne Fields P.O. Box 63 Benham, KY 40807

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyn Haney



# **PUBLIC NOTICE**

February 15, 2016

Nona Cress and Jerry Ann Shoulter 4585 Ward Road Morrow, OH 45152-9709

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyen Haney



# PUBLIC NOTICE

February 15, 2016

ACIN, LLC 5260 Irvine Rd. Huntington, WV 25705

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Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyur Haney



# PUBLIC NOTICE

February 15, 2016

ARK Land Company City Place One, Suite 300 St. Louis, MO 63141

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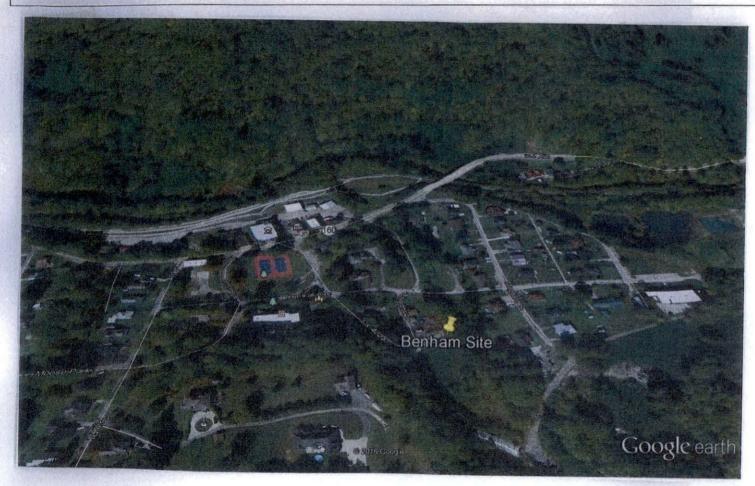
Sincerely,

Lynn Haney

Regulatory Compliance Director

Lyn Haney

# Appalachian Wireless Location Map



Vame

nam Site

Location

McKnight Rd. Benham, KY.

GPS Location

N 36 57 44.99

W 82 56 58.33

dba Appalachian Wireless 101 Technology Trail Ivel, KY 41642

Phone: 606-477-2355 Fax: 606-791-2225



To: The Harlan Daily Enterprise

From:

Raina Helton

Attn: Classifieds

Regulatory Compliance Assistant

Email: ebell@civitasmedia.com

Date:

February 9, 2016

Re:

PUBLIC NOTICE ADVERTISEMENT

Pages:

:

Please place the following Public Notice Advertisement in The Harlan Daily Enterprise to be ran on February 16, 2016.

#### PUBLIC NOTICE:

RE: Public Service Commission of Kentucky (CASE NO. 2015-00420)

Public Notice is hereby given that East Kentucky Network, LLC, dba Appalachian Wireless has applied to the Kentucky Public Service Commission to construct a cellular telecommunications tower on a tract of land located on McKnight Road, Benham, Harlan County, Kentucky. The proposed tower will be a 180 foot monopole tower with attached antennas. If you would like to respond to this notice, please contact the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to Case No. 2015-00420.

If you have any questions about the placement of the above mentioned notice, please call me at 606-477-2375, ext. 1005.

Thank you,

Raina Helton Regulatory Compliance Assistant

The message above and the information contained in the documents transmitted are confidential and intended only for the person(s) named above. Dissemination, distribution or copying of this communication by anyone other than the person(s) named above is prohibited. If you have received this communication in error, please notify us immediately by telephone and return the original message to us at the address listed above via regular mail. Thank you.





February 9, 2016

Dan Mosley, Judge Executive P.O. Box 956 Harlan, KY 40831

RE: Public Notice-Public Service Commission of Kentucky (Case No. 2015-00420)

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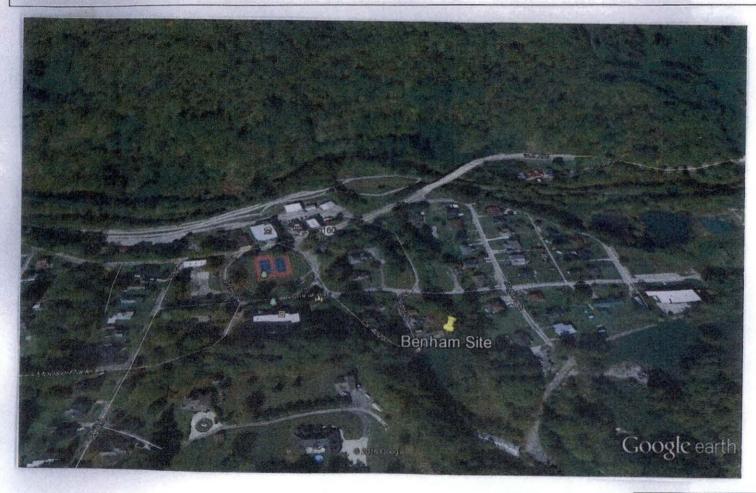
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Lynn Haney

Regulatory Compliance Director

Lyn Haney

# Appalachian Wireless Location Map



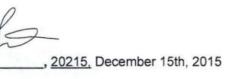
lame am Site

Location McKnight Rd. Benham, KY. GPS Location

N 36 57 44.99 W 82 56 58.33 APPALACHIAN WIRELESS
Geotechnical Investigation on the
Benham Site
Located at McKnight Street,
Benham, Kentucky 40807
ERMC2 Project No. 165-000-0016

PREPARED FOR: Appalachian Wireless. 101 Technology Trail Ivel, Kentucky 41642

PREPARED BY:
Richard Dirk Smith PE, PLS
General Manager Appalachian Region
ENVIRONMENTAL RESOURCES MANAGEMENT
CONSULTING COMPANY
230 Swartz Drive
Hazard, Kentucky 41701





#### **EXECUTIVE SUMMARY**

- 1.0 INTRODUCTION
- 2.0 PROJECT ESCRIPTION
- 3.0 SITE DESCRIPTION
- 4.0 FIELD EXPLORATION
  - 4.1 SITE INFORMATION
  - 4.2 BRING DATA
  - 4.3 GROUNDWATER
  - 4.4 SEISMIC SITE CLASSIFICATION

# 5.0 DISCUSSION AND RECOMMENDATIONS

- 5.1 GENERAL
- 5.2 FOUNDATIONS
- 5.3 SHALLOW FOUNDATONS TOWER
- 5.4 DEEP FOUNDATIONS
- 5.5 ANCILLARY SUPPORT STRUCTURES
- 5.6 BURIED UTILTIES

#### 6.0 DISCUSSION AND RECOMMENDATIONS

- 6.1 SUSURFACE INVESTIGATION
- 6.2 LABATORY AND FIELD TESTING
- 6.3 ANALYSIS AND RECOMMEDATIONS
- 6.4 CONSTRUCTION MONITORING
- 6.5 GENERAL

## **SPECIFICATIONS**

- I GENERAL
- II DRILLED PEIR INSTALATIONS
- III ENGINEERED FILL BENEATH STRUCTURES
- IV GUIDELINES FOR EXCAVATIONS AND TRENCHING
- V GENERAL CONCRETE SPECIFICATIONS

APPENDIX A – BORING DATA SEISMIC DATA

APPENDIX B - SITE MAPS



# **EXECUTIVE SUMMARY**

A geotechnical investigation has been performed on the Benham Property. Located on McKnigth Street, in Benham Kentucky, 40807. This site is located on an empty lot and is readably accessible. A location map is shown in Figure 1 of this report. Four (4) borings were advanced to depths ranging from 15.5 ft. to 46.8 ft. The following geotechnical considerations were identified:

- Borings utilized for this study encountered coal mine reject material to a depth of 35.0 ft. at which point auger refusal was met.
- This area is an empty lot within a residential area.
- The area appears to be used as an underground coal reject dump. The borings encounter 0.7 feet of gravel, then coal mine reject material until auger refusal was met at 35.0 feet. A 10 ft. NX core was taken showing competent sandstone.
- The bearing capacities of the fill material is estimated less than 500 psf. and the underlying rock is 10 tsf. The fill material consist of coal fines with mine reject shales and clays. These materials are highly compressible and therefore shallow foundations is not recommended for this site.
- We recommend deep foundations for this site. Straight shaft drilled piers are recommended.
- The 2009 International Building Code seismic site classification utilizing rock bearing foundations for this site is "C", for shallow foundations the site classification will be "E"
- All support building for this tower will need remediated by undercutting the existing material and then backfilled with graded engineered fill as specified in this report.
- Close monitoring of the construction operations discussed herein will be critical
  in achieving the design subgrade support. We therefore recommend that ErMC2
  be retained to monitor this portion of the work.

This executive summary is included to provide a general overview of the project and should not be relied upon except for the purpose it was prepared. Please rely on the complete report for the information on the findings, recommendation and all other concerns.



# 1. INTRODUCTION

Environmental Resources Management Consultant Company (ErMC2) was retained by Mr. Marty Thacker of Appalachian Wireless to prepare a geotechnical engineering report for the proposed tower site located on the McKnight Street, in Benham, Harlan County. A site location map is shown in Figure No. 1.

Four (4) borings were advanced to depths ranging from 15.5 ft. to 46.8 ft. Logs of the borings along with a boring location plan are included in Appendix A. The purpose of these services is to provide information and geotechnical engineering recommendations relative to subsurface conditions, earthwork, seismic considerations, groundwater conditions and foundation design.

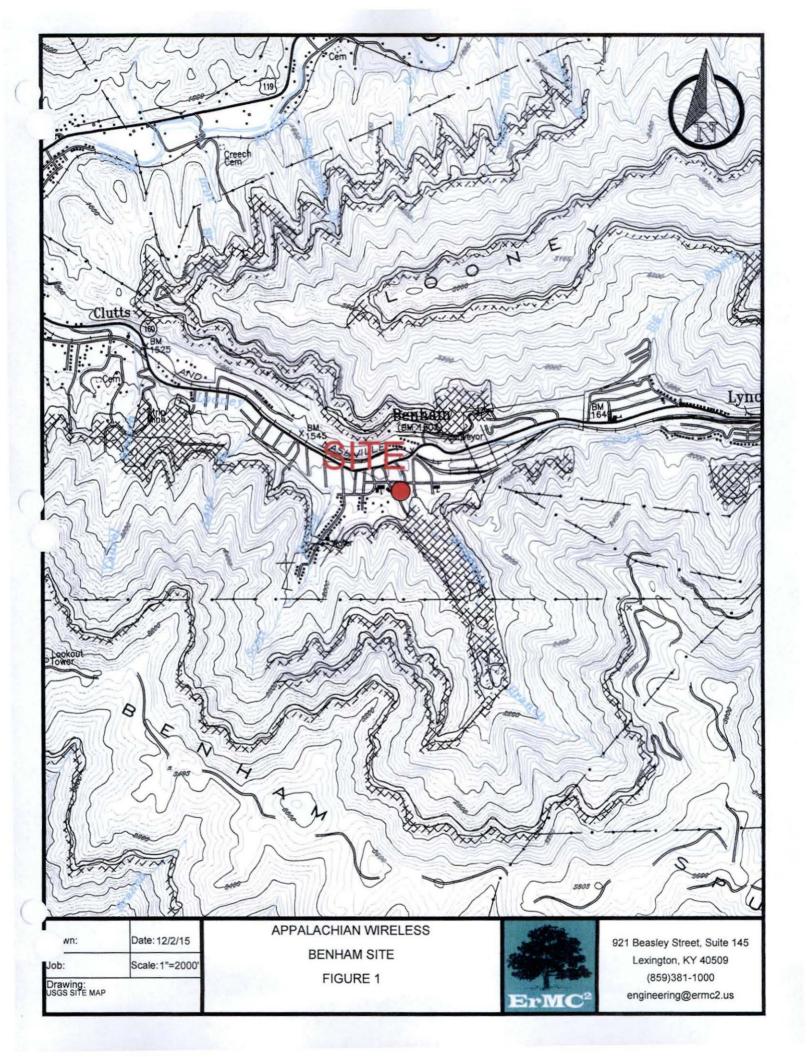
#### 2.0 PROJECT DESCRIPTION

The proposed communication facility will consist of a self-supporting tower of undetermined height and ancillary support areas. The construction area will be approximately 24 ft. x 24 ft. Based upon information provided, we estimate the structural loads will be similar to the following conditions;

CONDITION	LOAD
Total Shear	40 Kips
Axial Load	50 Kips

We anticipate that overturning will govern the structural design. If the loading are significantly different than these expected values, ErMC2 should be notified to revaluate the recommendations provided in this report.





#### 3.0 SITE DESCRIPTION & HISTORICAL MINING

This site is located on an empty lot and is readably accessible. Based upon review of historical documents, this site was once part of a permitted coal preparation facility. The site was last permitted by Harlan Reclamation Services, No. 848-8053. Our borings discovered fill of approximately 35.0 ft. in depth which consists primarily of coal reject materials.

ErMC2 reviewed available historical mine maps from the Kentucky Division of Mine Safety, Kentucky Mine Mapping Information System ("KMMIS"). Based on available data, no historical underground mining has occurred beneath this site..

Previous underground mining has been conducted in area(s) near the proposed tower site, however the site has not been undermined. Previous mining was conducted from the late 1930's through the late 1950's

# 4.0 FIELD EXPLORATION

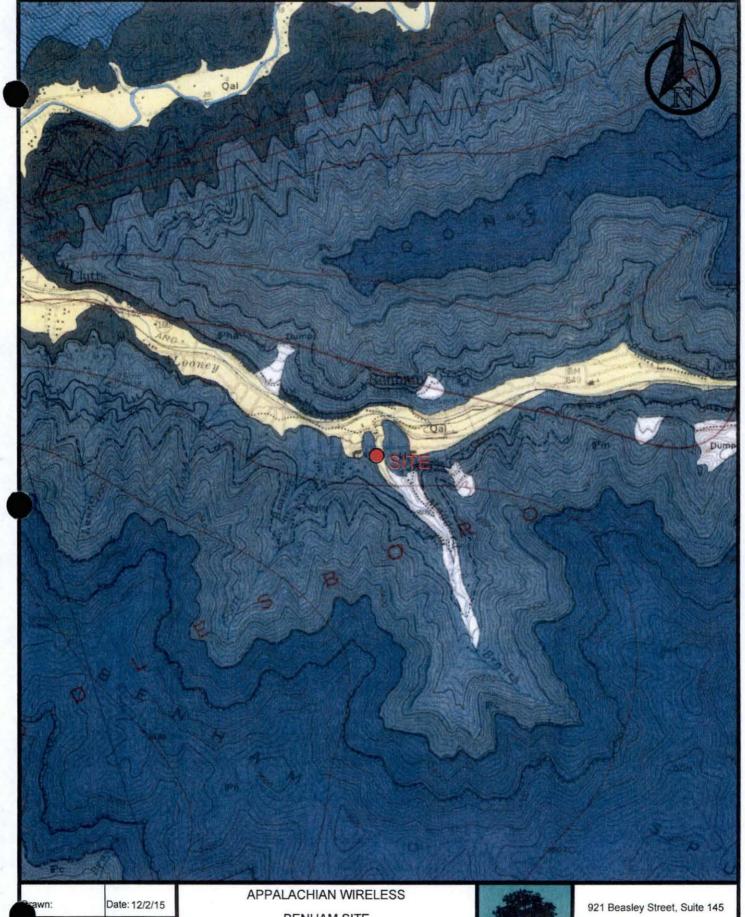
#### 4.1 SITE INFORMATION

This site is located near residential lot on McKnight Street in Benham, Harlan County Kentucky. A boundary retracement survey was conducted on the property and provided to ErMC2. A proposed lot description and drawing is included in Appendix B of this report. The proposed tower lot was established and tied to the existing boundary. An estimated pad location was determined and boring locations were placed at the corners of proposed concrete pad for the towers support.

# 4.2 BORING DATA

Four (4) borings were made in the relative positions shown on the Boring Location Map in Appendix A. The boring logs and resulting data are also included in Appendix A. The borings were made with a track mounted boring rig using hollow-stem augers and employing standard penetration resistance methods (ASTM D-1586, which includes 140-pound hammer, 30-inch drop, and two-inch-O.D. split-spoon sampler) at maximum depth intervals of five feet or at major changes in stratum, whichever occurred first. The disturbed split-spoon samples were visually classified, logged,





Scale: 1"=2000'

Drawing: GQ SITE MAP

**BENHAM SITE** FIGURE 2



Lexington, KY 40509 (859)381-1000 engineering@ermc2.us sealed in moisture-proof jars, and taken to the ERMC2 laboratory for study. The depths where these "A"-type split-spoon samples were collected are noted on the boring logs. The results of the natural moisture contents by boring and interval are shown in Table 1.

TABLE 1

RESULTS OF NATURAL MOISTURE CONTENT TESTS (ASTM D-4643)

BORING NO.	DEPTH INCREMENT, (FT.)	NATURAL MOISTURE CONTENT, %
B1	4.0-5.5	18.8%
B1	9.0-10.5	21.8%
B1	14.5-10.5	14.1%
B1	19.0-20.5	20.4%
B1	24.0-25.5	12.9%
B2	0.0-1.5	10.6%
B2	4.0-5.5	11.%
B2	9.0-10.5	10.3%
B2	14.0-15.5	16.2%
В3	0.0-1.5	7.3%
В3	4.0-5.5	14.1%
В3	9.0-10.5	15.6%
В3	14.0-15.5	15.0%
B4	0.0-1.5	10.5%
B4	4.0-5.5	9.7%
B4	9.0-10.5	22.4%
B4	14.0-15.5	15.0%

In order to determine the depth of the existing fill, boring B-1 was extended to auger refusal to a depth of 35 ft. at which point a ten foot NX core was taken in this rock.



#### 4.3 GROUNDWATER

Groundwater observations were made during the drilling operations (by noting the depth to water on the drilling tools) and in the open boreholes following withdrawal of the drilling augers. No groundwater levels were noted during drilling activities.

#### 4.4 SEISMIC SITE CALSSIFICATION

Based on using underlying rock foundations at the project site, the site classification was determined to be "Site Class C" per the Kentucky Building Code. In addition, a S<sub>DS</sub> coefficient of 0.260g was calculated, and a S<sub>D1</sub> coefficient of 0.107g was also calculated for design based on the aforementioned building code. If shallow foundations with remediated engineered backfill is used the site classification was determined to be "Site Class E" per the Kentucky Building Code. In addition, a S<sub>DS</sub> coefficient of 0.489g was calculated, and a S<sub>D1</sub> coefficient of 0.219g was also calculated for design based on the aforementioned building code.

#### 5.0 DISCUSSION AND RECOMMENDATIONS

# 5.1 GENERAL

The structure will be a self-supporting free standing tri-pole tower. Due to wind loading, lattice tower foundations can experience both vertical loads and horizontal loads. The vertical loads act in both an upward and downward direction as the tower attempt to overturn and can act in any directions.

# 5.2 FOUNDATIONS

It is our understanding that the foundations for these structures can be designed to bear on low bearing pressure soils. This report demonstrates the different expected bearing capacities based upon the type of material encountered from the boring logs and sampling taken at the site.

Approximately 35 feet of fill is present at this proposed location. It consists of mixture of gravel, coal mine reject material and coal fines. Standard penetrations test were conducted on five foot intervals in this material.



The approximate elevation of the surface of the site is 1635 ft. The standard penetration tests were conducted on five foot intervals within the fill material. The blow counts (N) ranged from 3 to 24 to the depth of 35.0 feet.

# 5.3 SHALLOW FOUNDATIONS TOWER

Typically we do not recommend shallow foundations on sites consisting of non-engineered fill material. Foundation design on non-cohesive soil is usually governed by acceptable settlement, and this restriction on bearing pressure is usually much lower than the ultimate bearing capacity divided by the factor of safety of 3. Generally only in the case of narrow strip foundations on loose submerged sands it is vital to determine the ultimate bearing capacity, since this may be more critical than settlement. Settlement can and is likely to occur once the final structure's loading is in place. No settlement calculations have been evaluated for this report. If shallow foundations are used it should be noted that the material type and bearing capacity can vary significantly due to the inconsistency of the underlying material. Based upon the laboratory and field testing, visual inspection of the materials and practical experience we have estimated that the bearing capacity of the soils to be less than 500 psf.

If shallow foundations are used we recommend that site be over excavated to a minimum of (20) twenty ft. below the footing subgrade and (20) twenty ft. outside the footing area. Any large rock and unsuitable material be removed and backfilled with a select backfill or dense grade aggregate. The material is to be placed in 8 inch horizontal lifts, compacted to not less than 95% of the maximum density as determined in accordance with the Standard Proctor dry unit weight (ASTM D-968) and within +2% and -2% of the optimum moisture content. This will not eliminate the potential for settlement but should assist in limiting differential settlement under the proposed foundation. This remediation of the site would increase the bearing capacity to an estimated value of **1500 psf.** 

#### 5.4 DEEP FOUNDATIONS

Auger refusal was encountered at approximately 35 feet below the surface. A rock core was taken of grey sandstone w/ shale streaks with a RQD value of 71. The bearing capacity for this is 10 tsf. Deep foundations are recommended for this site. If



deep foundation are used they should be constructed with 4000 psi reinforced concrete with a minimum of 24 in. diameter piers socketed into competent rock a minimum of 18 in. deep.

# 5.6 ANCILLARY SUPPORT STRUCTUES

All support structures, roadways and concrete pads constructed on this site should be undercut a minimum of 3 ft. and be backfilled and compacted with graded material in 8 inch horizontal lifts.

#### 5.6 BURIED UTILITES

Excavations for buried utility pipelines should follow the guidelines set forth in this report. Depending on the pipeline material, a minimum thickness of at least 0.5 foot of select fine-grained granular bedding material should be used beneath all below-grade pipes, with a minimum cover thickness of at least 3 feet to afford an "arching" effect and reduce stresses on the pipe. The cover thickness may be reduced if the external loading condition on the pipe is relatively light or if the pipe is designed to withstand the external loading condition. It is not recommended that "pea-gravel" or other "open-work" aggregates be used for trench backfill since these materials are nearly impossible to compact and have a tendency to pond water within their interstices.

# 6.0 WARRANTY

Our professional services have been performed, our findings obtained and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. No other warranty, express or implied, is made.

While the services of ErMC2 are a valuable and integral part of the design and construction teams, we do not warrant, guarantee, or insure the quality or completeness of services provided by other members of those teams, the quality, completeness, or satisfactory performance of construction plans and specifications which we have not prepared, nor the ultimate performance of building site materials.



# 6.1 SUBSURFACE EXPLORATION

Subsurface exploration is normally accomplished by test borings, although test pits are sometimes employed. The method of determining the boring location and the surface elevation at the boring is noted in the report, and is presented on the Boring Location Plan or on the boring log. The location and elevation of the boring should be considered accurate only to the degree inherent with the method used.

The boring log includes sampling information, description of the materials recovered, approximate depth of boundaries between soil and rock strata and groundwater data. The boring log represents conditions specifically at the location and time the boring was made. The boundaries between different soil strata are indicated at specific depths; however, these depths are in fact approximate and are somewhat dependent upon the frequency of sampling (The transition between soil strata is often gradual). Free groundwater level reading are made at the times and under conditions stated on the boring logs (Groundwater levels change with time and season). The borehole does not always remain open sufficiently long for the measured water level to coincide with the groundwater table.

# 6.2 LABORATORY AND FIELD TESTS

Laboratory and field tests are performed in accordance with specific ASTM standards unless otherwise indicated. All determinations included in a given ASTM standard are not always required and performed. Each test report indicates the measurements and determinations actually made.

#### 6.3 ANALYSIS AND RECOMMENDATIONS

The geotechnical report is prepared primarily to aid in the engineering design of site work and structural foundations. Although the information in the report is expected to be sufficient for these purposes, it is not intended to determine the cost of construction or to stand alone as a construction specification.

Our engineering report recommendations are based primarily on data from test borings made at the locations shown on a boring location drawing included. Soil variations may exist between borings and these variations may not become evident until construction. If significant variations are then noted, the geotechnical engineer



should be contacted so that field conditions can be examined and recommendations revised if necessary.

The geotechnical engineering report states our understanding as to the location, dimensions and structural features proposed for the site. Any significant changes in the nature, design, or location of the site improvements MUST be communicated to the geotechnical engineer such that the geotechnical analysis, conclusions, and recommendations can be appropriately adjusted. The geotechnical engineer should be given the opportunity to review all drawings that have been prepared based on their recommendations.

#### 6.4 CONSTRUCTION MONITORING

Construction monitoring is a vital element of complete geotechnical services. The field engineer/inspector is the owner's "representative" observing the work of the contractor, performing tests as required in the specifications, and reporting data developed from such tests and observations. The field engineer or inspector does not direct the contractor's construction means, methods, operations or personnel. The field inspector/engineer does not interfere with the relationship between the owner and the contractor and, except as an observer, does not become a substitute owner on site. The field inspector/engineer is responsible for his own safety but has no responsibility for the safety of other personnel at the site. The field inspector/engineer is an important member of a team whose responsibility is to watch and test the work being done and report to the owner whether that work is being carried out in general conformance with the plans and specifications.

#### 6.5 GENERAL

The scope of our services did not include an environmental assessment for the presence or absence of hazardous or toxic materials in the soil, surface water, groundwater or air, on, within or beyond the site studied. Any statements in the report or on the boring logs regarding odors, staining of soils or other unusual items or conditions observed are strictly for the information of our client.

To evaluate the site for possible environmental liabilities, we recommend an environmental assessment, consisting of a detailed site reconnaissance, a record



review, and report of findings. Additional subsurface drilling and samplings, including groundwater sampling, may be required

This report has been prepared for the exclusive use of Appalachian Wireless, for specific application to the proposed cellular tower located on the Benham Property located in Harlan County, Kentucky. Specific design and construction recommendations have been provided in the various sections of the report. The report shall, therefore, be used in its entirety. This report is not a bidding document and shall not be used for that purpose. Anyone reviewing this report must interpret and draw their own conclusions regarding specific construction techniques and methods chosen. ErMC2 is not responsible for the independent conclusions, opinions or recommendations made by others based on the field exploratory and laboratory test data presented in this report.



# **SPECIFICATIONS**

#### I - GENERAL

#### 1.0 STANDARDS AND DEFINITIONS

- 1.1 STANDARDS All standards refer to latest edition unless otherwise noted.
  - 1.1.1 ASTM D-698-70 (Method C) "Standard Test Methods for Moisture. Density Relations of Soils and Soil Aggregate Mixtures Using 5.5-lb (2.5 kg.) Rammer and 12-inch (305-mm) Drop".
  - 1.1.2 ASTM D-2922 "Standard Test Method for Density of Soil and Soil Aggregate in Place by Nuclear methods (Shallow Depth)".
  - 1.1.3 ASTM D-1556 "Standard Test Method for Density of Soil in place by the Sand-Cone Method".

#### 1.2 DEFINITIONS

- **1.2.1** Owner In these specifications the word "Owner" shall mean Appalachian Wireless.
- **1.2.2** Engineer In these specifications the word "Engineer" shall mean the Owner designated engineer.
- 1.2.3 Design Engineer In these specifications the words "Design Engineer" shall mean mean the Owner designated design engineer.
- 1.2.4 Contractor In these specifications the word "Contractor" shall mean the firm or corporation undertaking the execution of any work under the terms of these specifications.
- 1.2.5 Approved In these specifications the word "approved" shall refer to the approval of the Engineer or his designated representative.
- 1.2.6 As Directed In these specifications the words "as directed" shall refer to the directions to the Contractor from the Owner or his designated representative.

# 2.0 GENERAL CONDITIONS

2.1 The Contractor shall furnish all labor, material and equipment and perform all work and services except those set out and furnished by the



Owner, necessary to complete in a satisfactory manner the site preparation, excavation, filling, compaction, grading as shown on the plans and as described therein.

This work shall consist of all mobilization clearing and grading, grubbing, stripping, removal of existing material unless otherwise stated, preparation of the land to be filled, filling of the land, spreading and compaction of the fill, and all subsidiary work necessary to complete the grading of the cut and fill areas to conform with the lines, grades, slopes, and specifications.

This work is to be accomplished under the observation of the Owner or his designated representative.

2.2 Prior to bidding the work, the Contractor shall examine, investigate and inspect the construction site as to the nature and location of the work, and the general and local conditions at the construction site, including, without limitation, the character of surface or subsurface conditions and obstacles to be encountered on and around the construction site; and shall make such additional investigation as he may deem necessary for the planning and proper execution of the work.

If conditions other than those indicated are discovered by the Contractor, the Owner should be notified immediately. The material which the Contractor believes to be a changed condition should not be disturbed so that the owner can investigate the condition.

2.3 The construction shall be performed under the direction of an experienced engineer who is familiar with the design plan.



#### II - DRILLED PIER INSTALLATION

# 1.0 DRILLING PROCEDURE

- 1.1 Drilled piers will be installed with large caisson drill rigs capable of torque and crowd forces sufficient to install drilled piers at the project site given the in-situ soil conditions.
- 1.2 The drill rig kelly bar and auger will be carefully and accurately placed over the centerline of the drilled pier. The Contractor is responsible for providing necessary surveying to verify drilled pier location before, during, and after the drilled pier installation.
- 1.3 The augers are advanced downwards as they are rotated such that drilling of the soil mass is efficiently accomplished. Depending on the subsurface conditions, and the requirements for the given project, a temporary steel casing should be installed at this time to preclude caving of the soil and/or broken rock mass being penetrated.

# 2.0 CASING INSTALLATION

- 2.1 The casing will be checked for centerline accuracy and plumbness by the Contractor's survey crew. During casing installation, the Contractors survey crew will verify alignment with instruments. If plumbness and alignment are not within tolerance as determined by the Contractors survey crew, the casing will be extracted and re-aligned as necessary.
- 2.2 The drill rig will remove soil and bedrock material from within the casing to the drilled pier design tip elevation. A steel casing, or "Sonotube" shall be inserted into the borehole to preclude cave-ins and/or instability in the borehole.



2.3 The bearing surface within the drilled pier will be inspected by a registered Professional Engineer prior to being approved for structural concreting.

# 3.0 INSTALLATION OF THE REBAR CAGE

- 3.1 An epoxy coated spiral reinforcing steel cage will be installed while in the drilled pier borehole.
- 3.2 To assist in assuring that the reinforcing steel cage does not settle during concrete pumping, a mat of reinforcing steel bars will be installed across the bottom of the reinforcing steel cage perpendicular to the vertical axis of the cage. The exact number of bars will be determined and installed by the Structural Engineer. The number of rebar boots used on the bottom of the cage will also be determined by the Structural Engineer.
- 3.3 The reinforcing steel cage will be lowered into the drilled pier borehole, while drilled pier spacers are placed at intervals as required by the Structural Engineer. The reinforcing steel cage will be checked for alignment by the Contractors survey crew.
- 3.4 The crane will remain attached to the reinforcing steel cage while the concrete pump outlet pipe is lowered to just above the bottom of the drilled pier. The concrete pump pipe sections will be welded together to assure that do not separate during pumping.

# 4.0 CONCRETING OF THE DRILLED PIER

- **4.1** Concrete pumping may commence once the bearing surface has been approved in accordance with Clause 2.3
- **4.2** A three inch trash pump will be used to pump slurry and/or water from within the casing and from above the newly pumped concrete.



- 4.3 The concrete pump outlet pipe will maintain at least ten (10) feet of embedment into the fresh concrete. The concrete level in the casing will be monitored.
- 4.4 The casing will be completely extracted with the crane and/or vibratory hammer. Caisson clamps on the vibratory hammer (if applicable) will be adjusted to the proper dimension to withdrawal the casing.
- 4.5 The concrete will be terminated at the top of drilled pier elevation and screeded flat.
- 4.6 The upper reinforcing steel dowel cage will be lowered into the concrete to the embedment elevation. If necessary, the concrete will be vibrated to assist in placement. Alignment will be verified by the Contractors survey crew and the cage will be sufficiently braced.



# III - ENGINEERED FILL BENEATH STRUCTURES CLEARING AND GRADING SPECIFICATIONS

# 1.0 GENERAL CONDITIONS

The Contractor shall furnish all labor, materials, and equipment, and perform all work and services necessary to complete in a satisfactory manner the site preparation, excavation, filling, compaction and grading as shown on the plans and as described therein.

This work shall consist of all clearing and grading, removal of existing structures unless otherwise stated, preparation of the land to be filled, filling of the land, spreading and compaction of the fill, and all subsidiary work necessary to complete the grading of the cut and fill areas to conform with the lines, grades, slopes, and specifications.

This work is to be accomplished under the constant and continuous supervision of the Owner or his designated representative.

In these specifications the terms "approved" and "as directed" shall refer to directions to the Contractor from the Owner or his designated representative.

# 2.0 SUBSURFACE CONDITIONS

Prior to bidding the work, the Contractor shall examine, investigate and inspect the construction site as to the nature and location of the work, and the general and local conditions at the construction site, including without limitation, the character of surface or subsurface conditions and obstacles to be encountered on and around the construction site; and shall make such additional investigation as he may deem necessary for the planning and proper execution of the work. Borings and/or soil investigations shall have been made. Results of these borings and studies will be made available by the Owner to the Contractor upon his request, but the Owner is not responsible for any interpretations or conclusions with respect thereto made by the Contractor on the basis of such information, and the Owner further has no responsibility for the accuracy of the borings and the soil investigations.

If conditions other than those indicated are discovered by the Contractor, the Owner should be notified immediately. The material which the contractor believes to be a



changed condition should not be disturbed so that the Owner can investigate the condition.

# 3.0 SITE PREPARATION

Within the specified areas, all trees, brush, stumps, logs, tree roots, and structures scheduled for demolition shall be removed and disposed of.

All cut and fill areas shall be properly stripped. Topsoil will be removed to its full depth and stockpiled for use in finish grading. Any rubbish, organic and other objectionable soils, and other deleterious material shall be disposed of off the site, or as directed by the Owner or his designated representative if on site disposal is provided. In no case shall such objectionable material be allowed in or under the fill unless specifically authorized in writing.

Prior to the addition of fill, the original ground shall be compacted to job specifications as outlined below. Special notice shall be given to the proposed fill area at this time. If wet spots, spongy conditions, or groundwater seepage is found, corrective measures must be taken before the placement of fill.

# 4.0 FORMATION OF FILL AREAS

Fills shall be formed of satisfactory materials placed in successive horizontal layers of not more than eight (8) inches in loose depth for the full width of the cross-section. The depth of lift may be increased if the Contractor can demonstrate the ability to compact a larger lift. If compaction is accomplished using hand-tamping equipment, lifts will be limited to 4-inch loose lifts. Engineered fill placed below the structure bearing elevation shall be compacted to at least 95% of the maximum dry unit weight with a moisture content within 2% of the optimum moisture content as determined by the modified Proctor test.

All material entering the fill shall be free of organic matter such as leaves, grass, roots, and other objectionable material.

The operations on earth work shall be suspended at any time when satisfactory results cannot be obtained because of rain, freezing weather, or other unsatisfactory conditions. The Contractor shall keep the work areas graded to provide the drainage at all times.



The fill material shall be of the proper moisture content before compaction efforts are started. Wetting or drying of the material and manipulation to secure a uniform moisture content throughout the layer shall be required. Should the material be too wet to permit proper compaction or rolling, all work thus affected shall be delayed until the material has dried to the required moisture content. The moisture content of the fill material should be no more than two (2) percentage points higher or lower than optimum unless otherwise authorized. Sprinkling shall be done with equipment that will satisfactorily distribute the water over the disced area. Any areas inaccessible to a roller shall be consolidated and compacted by mechanical tampers. The equipment shall be operated in such a manner that hardpan, cemented gravel, clay or other chunky soil material will be broken up into small particles and become incorporated with the other material in the layer.

In the construction of filled areas, starting layers shall be placed in the deepest portion of the fill, and as placement progresses, additional layers shall be constructed in horizontal planes. Original slopes shall be continuously, vertically benched to provide horizontal fill planes. The size of the benches shall be formed so that the base of the bench is horizontal and the back of the bench is vertical. As many benches as are necessary to bring the site to final grade shall be constructed. Filling operations shall begin on the lowest bench, with the fill being placed in horizontal eight (8) inch thick loose lifts unless otherwise authorized. The filling shall progress in this manner until the entire first bench has been filled, before any fill is placed on the succeeding benches. Proper drainage shall be maintained at all times during benching and filling of the benches, to insure that all water is drained away from the fill area.

Frozen material shall not be placed in the fill nor shall the fill be placed upon frozen material.

The Contractor shall be responsible for the stability of all fills made under the contract, and shall replace any portion, which in the opinion of the Owner or his designated representative, has become displaced due to carelessness or negligence on the part of the Contractor. Fill damaged by inclement weather shall be repaired at the Contractor's expense.



# 5.0 SLOPE RATIO AND STORM WATER RUN-OFF

Slopes shall not be greater than 2 (horizontal) to 1 (vertical) in both cut and fill, or as illustrated on the construction drawings. Excavations shall be constructed in accordance with all Federal, State and local codes relative to slope geometry.

# 6.0 GRADING

The Contractor shall furnish, operate, and maintain such equipment as is necessary to construct uniform layers, and control smoothness of grade for maximum compaction and drainage.

# 7.0 COMPACTING

The compaction equipment shall be approved equipment of such design, weight, and quantity to obtain the required density in accordance with these specifications.

# 8.0 TESTING AND INSPECTION SERVICES

Testing and inspection services will be provided by the Owner.



# IV GUIDELINES FOR EXCAVATIONS AND TRENCHES

The following represents some general guidelines relative to the design and construction of excavations and trenches. It must be emphasized that these guidelines are not intended to represent a "safety plan," but rather are presented herein to provide general guidance with regard to the design characteristics and safety measures for excavations and trenches.

- 1. Check with the following utilities prior to breaking ground:
  - Sewer
  - Telephone
  - Fuel
  - Electric
  - Water
  - Gas
  - Cable

When utility companies or owners do not respond to your request within 48 hours, the contractor may only then proceed provided the contractor does so with caution by using detection equipment or other acceptable means to locate utility installations.

Once the excavation is open, the contractor should protect and support the exposed underground utilities or remove installations to safeguard workers and prevent damage to exposed utilities.

- 2. Access and egress ramps must be designed by a "competent person" and structural ramps used for equipment must be designed by a "competent person" with qualified knowledge in structural design. In addition:
  - Ramps must be secured to prevent displacement;
  - · Ramps used in lieu of steps must have cleats to prevent slipping; and
  - Trenching excavations four feet or greater in depth must have a stairway, ladder, ramps or other safe means to egress with lateral travel no more than 25 feet.
- Workers must be provided with reflector garments, such as warning orange or red vests, when exposed to vehicular traffic.



- 4. Contractors must not allow workers to work under or near equipment when there is danger of falling debris, spillage or equipment-related injuries.
- Mobile equipment, operating adjacent to an open excavation or approaching the edge of an excavation, must have one of the following when the operator's view is obstructed:
  - Warning System
  - · Mechanical Signals
  - Barricades
  - Stop Logs
  - Hand Signals
- 6. The contractor must check the atmosphere for hazardous gases and oxygen deficiencies when excavating four feet or greater around landfills, or when hazardous substances are stored nearby, and when the contractor expects there could be any exposure to the workers.
- 7. When hazardous atmospheric conditions exist, or when conditions could change, the contractor must make emergency rescue equipment readily available including breathing apparatus, safety harnesses with life lines and a basket stretcher.
- When workers enter bell-bottom pier holes or other deep and confined excavations, the worker must wear (at all times while performing work in the confined space) a separate life line attached to a harness. The line must be attended by someone above while work is being performed. The worker must check for hazardous atmospheric conditions prior to entry.
- 9. The contractor must ensure that water does not accumulate in open excavations and must inspect the excavation prior to allowing workers to reenter after heavy rains.
- 10. Adjacent structures (buildings, walls, etc.) must be supported or secured to prevent worker exposure to unsafe conditions and damage to existing structures.



- 11. A registered professional engineer must approve operations when a contractor underpins existing structures to ensure worker safety and prevent damage to existing structures.
- 12. Workers must not be exposed to loose soil and rock or materials in and around excavations. Materials, such as removed soil and rock, must not be stored closer than two feet from the edge of the excavation.
- Daily inspections of the excavation, the adjacent areas and protective systems must be made by a "competent person" for evidence of possible cave-ins, indications of failure of protective systems, hazardous atmospheres or other hazardous conditions. The "competent person" must stop work immediately and remove workers from the excavation when conditions change and pose a threat to their safety.
- 14. Workers must not be exposed to fall hazards associated with excavations. Protective walkways or bridges with standard guard rails must be provided.
- 15. All wells, pits, shafts etc. must be barricaded or covered. After completion of work, all wells, pits, shafts etc. must be backfilled.



# V - GENERAL CONCRETE SPECIFICATIONS

#### 1.0 GENERAL

It is the intent of this specification to secure, for every part of the work, concrete of homogenous structure which, when hardened, will have the required strength and resistance to weathering. To this end, the limiting values of concrete and the requirements hereinafter specified must be met. Standard tests of the cement, aggregates, concrete and reinforcement will be made by the Owner as it sees fit. The Contractor shall furnish the material for all required samples plus such labor as required to obtain samples. The Contractor shall provide to authorized representatives of the Owner, convenient access to all parts of the work of all concreting operations for the purpose of sampling and inspection.

#### 2.0 SCOPE

Contractor shall furnish all materials, labor, services, transportation, tools, equipment, and related items required to complete work indicated on the drawings and/or specified.

Unless otherwise noted or as modified by more stringent requirements specified herein, all plain and reinforced concrete work shall be performed in full compliance with applicable requirements of the Building Code Requirements for Reinforced Concrete ACI 318.

Contractor shall obtain Owner's approval of all subgrades, footing bottoms, forms, and reinforcement just prior to placing concrete.

Contractor shall coordinate the work specified in this section with that specified in other sections so that all anchors, pipes and other embedded items are properly installed before concrete is placed.

Contractor shall clean all exposed concrete surfaces and obtain approval of Owner for method of cleaning.

# 3.0 MATERIALS

All materials shall be of the respective quality specified herein, delivered, stored, and handles as to prevent inclusion of foreign matter and damage by



dampness or breakage. Packaged material shall be stored in original container until ready for use. Materials showing evidence of dampness or other damage may be rejected.

- A. <u>Fine and Coarse Aggregates:</u> Coarse and fine aggregates shall conform to ASTM Specification C33. The maximum size of aggregate shall not be larger than one-fifth (1/5) of the narrowest dimensions between forms, or larger than three fourths (3/4) of the minimum clear spacing between reinforcement.
  - Fine Aggregate: Sand shall be composed essentially of clean, hard, strong, durable grains free of structurally weak grains, organic matter, loam, clay, silt, salt, mica or other fine materials that may effect bonding of the cement paste.
  - Coarse Aggregate: Cement concrete shall consist of crushed rock or screened gravel and shall be composed essentially of clean, hard, strong and impermeable particles, resistant to wear and frost and free from deleterious amounts of organic matter, loam, clay, salts, mica, and soft, thin, elongated, laminated or disintegrated stone, and shall be inert to water and cement.
- B. <u>Portland Cement:</u> Portland cement shall conform to ASTM Specification C150. Type I or Type II Portland Cement shall be used provided that they are not intermixed during any one batch. Type II Portland Cement shall <u>not</u> be used unless indicated on the plans.
- C. <u>Water:</u> Water for mixing and curing shall be clean, fresh, and free from deleterious materials.
- D. <u>Metal Reinforcement:</u> Rebar shall be Grade 60 and with deformations conforming to ASTH Specification A305. Welded wire mesh shall conform to W4 x W4 size and be of Grade 60 steel.
- E. Admixtures: Except as herein noted, admixtures shall not be used.
  - Under adverse weather conditions only retarding or accelerating agents containing no chloride may be used.
  - Air-Entraining Agent shall be used for all concrete will give an entrained air range of not less than 4 percent but no greater than 8 percent in the finished product. Under no circumstances shall the air-entraining be interground with cement.
  - Approval in writing shall be required from Owner prior to the use of any admixture.



#### 4.0 FORM

Forms shall be constructed with proper shoring and cross-bracing, safeguarding the total structure and specifically lateral stability and sufficiently strong to stand vibrations of concrete and to carry, without appreciable deflection or displacement, all dead and live loads to which they may be subjected.

#### 5.0 INSERTS, ETC.

Anchors, bolts, dowels, conduit, waterstops, vent pipes and other similar builtin or concreted-in items shall be properly located, accurately positioned and secured. The Contractor shall cooperate in placing of such items with other contractors who require a fastening device for their work and he shall maintain them in proper location during the progress of his work.

#### 6.0 REINFORCEMENT

Reinforcement at the time concrete is placed shall be free from rust, scale or other coatings that will destroy or reduce the bond.

Reinforcement shall be accurately placed and securely tied at intersections and shall be securely held in position during the placing of concrete by pacers, chairs, or other approved supports.

The reinforcement of foundations, footings and other principal structural members in which the concrete is deposited against the ground shall not have less than three (3) inches of concrete between it and the ground contact surface. If concrete surfaces after removal of the forms are to be exposed to the weather or to be in contact with the ground or rock, reinforcement shall be protected with not less than two (2) inches of concrete,

#### 7.0 CONCRETE

Concrete for the various parts of the work shall be of 4000 pounds per square inch compressive strength with a minimum 28-day cure. Contractor is responsible to provide a mix of not less than 6 bags of cement per yard of concrete and not more than 7 gallons of water per bag of cement, producing a minimum slump of 2-1/2 inches and a maximum slump of 4-1/2 inches. Concrete that exceeds the above range of maximum or minimum slump



requirements may be rejected by the Owner. All concrete shall be air-entrained. Contractors are required to furnish the name or names of the company(s) that will be providing the mix. The Owner reserves the right to disapprove any concrete supplier that has been known to supply an undesirable material to the Owner on previous occasions.

#### 8.0 DEPOSITING CONCRETE

- 4.1. <u>Preparation for Placing Concrete:</u> Before depositing concrete, the Contractor shall:
  - Remove from space to be occupied by concrete all debris, including snow, ice, and water unless otherwise permitted by Owner.
  - Provide diversion, satisfactory to Owner, of any flow of water to an excavation so as to avoid washing the freshly deposited concrete.
  - Coal the forms prior to placing of reinforcing steel as required in form work.
  - Secure firmly in correct position, all reinforcement and other items to be encased and remove therefrom all coating including ice and frost.
- B. Transportation of Concrete from Batch Plant: The concrete shall be delivered to the site of the work and discharge shall be completed within 90 minutes after addition of the cement and water to the aggregates. Each batch of concrete delivered at the job site shall be accompanied by a time slip issued at the batching plant, bearing the time of charging of the mixer drum with the cement and aggregates.
- C. Transporting of Concrete from Mixer to Place of Final Deposit:

  Transportation shall be done as rapidly as practical by means which shall prevent the separation or loss of the ingredients. If chutes are used, they shall be at a slope not flatter than one vertical to two horizontal. Buggies or carts shall be equipped with pneumatic rubber tires or surfaces of runways shall be sufficiently smooth or both so as not to cause separation or segregation of concrete ingredients. Concrete shall not be allowed to drop freely more than 4 feet. Where greater drops are



required, canvas "elephant trunks" or galvanized iron chutes equipped with suitable hopper heads shall be employed and a sufficient number placed to insure that the concrete may be effectively compacted into horizontal layers not exceeding 12 inches in thickness with minimum lateral movements.

#### D. <u>Depositing of Concrete:</u> Depositing of concrete shall:

- Proceed continuously after once starting until reaching the end of a section of construction joint location shown on the drawings, or as approved by the Owner. The operations shall be conducted so that no concrete is deposited on concrete sufficiently hardened to cause formation of seams, and planes of weakness.
- Be as near as practical to its final position in the forms.
- Proceed so as to maintain constantly a top surface which is approximately level.
- Be placed before initial set has occurred, and in no event after it has contained its water content for more than 90 minutes.
- Be thoroughly worked and compacted by means of suitable tools to provide impermeability, durability and strength and shall be thoroughly worked around reinforcements and embedded items and into corners of forms and so as to be free from voids, pockets or honeycombing. Particular care shall be taken to provide impermeability.
- E. <u>Vibration Equipment:</u> Vibration equipment shall be of the appropriate type and shall, at all times, be adequate in number of units and power of each unit to properly consolidate all concrete.
- F. <u>Monolithic Pours:</u> Proper delivery of concrete shall be the Contractor's responsibility in order to make a mono-lithic pour without delays and changes of cold joints.

#### 9.0 CURING

All concrete work shall be protected from injurious action by the sun, rain, flowing water, frost and other injury and shall be covered with plastic after application of curing compound for three (3) days on pours located above ground.



Contractor shall not remove any formwork for a minimum period of 24 hours after a concrete pour without written approval of the Owner.

#### 10.0 CONCRETE FINISHES

Finishes of all exposed concrete shall be free of defects which impair its durability or adversely affect is appearance. All such surfaces when stripped, shall be uniform in appearance and any surfaces displaying any deviations from adjacent uniform surfaces shall be rejected and subject to removal.

Finished work shall be level and plumb, true to lines, and dimensions. Finished plane surfaces shall be smooth, and as nearly perfect as practical; however deviations from a true plane shall not exceed 1/8 inch when measured from a 6-foot straight edge placed against the surface to any point on the surface and under the straight edge.

All exposed surfaces shall have deflects corrects, protrusions removed, and holes filled.



## APPENDIX A BORING DATA AND SEISMIC INFORMATION



CLIENT:	Appalachian  Benham Tov		S		DATE STD: DRILLERS:	11/18/2015			BORING NO: DATE FINISHED: GROUND ELEV:	B-1 11/18/2015 16
	: As shown or		ation Man		METHOD:	Boring			GROUND ELEV.	10
	TSTRATUM		CLASSI oil Components	Trac Som		SAMPLE	DEPT SAMPI		BLOWS ON SAMPLER PER SPT (6" INTERAL) RQD	SPT "n" OF
				Allu	30-3076				2.2.2	
	0 07	Comment				1	9	5.5		15"
	0 0.7	Gravel				2		10.5		18"
						3	14	15.5		14"
0	7 26.8	Mine Re	etuse		DOD	4	19	20.5		12"
				Recovery	RQD	5	24	25.5	7-7-17	10"
26		Mine Re Gray St	efuse andstone	55/120	22/55					
36	8 46.8			114/120	71/114					
- 55	70.0			114120						- Array
		1								
		1								
		1								
		1								
		1								
		1								
		1								
-		-								
	-	1								
_	_	1				_				
-	-	-								
-	-	-								
	-	-								
	-	-					_			
	+	1								
		-								
-	+	-								
	-	-								
		+					_	_		
	+	1								
	+	1								
		1					_			
		1								
-	-	1				-	-			
		1								
	-	1								
Noted on At comple		ATIONS	HSA Hollow	ous Flight Aug	HOD MD Mud Drilling RC Rock Coring CA Casing Advan	icer	TYPE SA A-Split Sp B-Rock C C-Shelby D-Grab S	oon ore Tube		

2.0	Appalachian				11/18/2015			BORING NO: DATE FINISHED:	
	Benham Tow	ver Site the Location Map		DRILLERS: METHOD:	N/A Boring			GROUND ELEV:	16
	STRATUM	CLASSIFICA  Major Soil Components  Gravel Silt  Sand Clay	Mir Tra So		SAMPLE	SAMPL FROM		BLOWS ON SAMPLER PER SPT (6" INTERAL) RQD	SPT "n" OF RECOVER'
					1	0	1.5	3-7-5	7"
0	0.2	Gravel			2	4	5.5	2-2-3	14"
					3	9	10.5		15"
0.2	15.5	Mine Refuse			4	14	15.5	2-2-2	18"
	15.5	Cutoff							
	10.0	Cuton							
		1							
		1							
		1							
		]							
		1							
		1							
		1							
							-		
		1							
		1							
		1							
		]							
		]							
		1							
		1							
WATER LEV	/EL OBSER\	/ATIONS	BORING ME	ETHOD		TYPE SA	MPLE		
Noted on ro		HSA Hollow Stem	Auger	MD Mud Drilling		A-Split Sp			
At completio		CFA Continuous F	light Aug	RC Rock Coring		B-Rock C	ore		
Afterhr	sft	DC Driven casing		CA Casing Advar	cer	C-Shelby			
						D-Grab S			

CLIENT:	Appalachian	Wireless		REPORT NO				BORING NO:	
	Benham Tow As shown on	ver Site the Location Map		DATE STD: DRILLERS: METHOD:	11/19/2015 N/A Boring			DATE FINISHED: GROUND ELEV:	
	STRATUM	CLASSIFI Major Soil Components Gravel Silt Sand Clay	Tra So		SAMPLE	SAMPL FROM		BLOWS ON SAMPLER PER SPT (6" INTERAL) RQD	SPT "n" OR
			All	u 30-30 %	1	O O	1.5	9-8-9	RECOVERY 15"
0	0.2	Gravel			2	4	5.5		10"
	0.2	O. ave.			3	9	10.5		10"
0.2	15.5	Mine Refuse			4	14	15.5	2-2-2	10"
	15.5	Cutoff							
					-				
					-	_			
					-				-
									<b>†</b>
					-				-
WATER LE Noted on ro At completion Afterhr	on: N/A	HSA Hollow Str CFA Continuou DC Driven cas	s Flight Aug	MD Mud Drilling RC Rock Coring CA Casing Adva		TYPE SAI A-Split Sp B-Rock Co C-Shelby D-Grab Sa	oon ore Tube		

	Appalachian Benham Tow			REPORT NO DATE STD: DRILLERS:	11/19/2015			BORING NO: DATE FINISHED: GROUND ELEV:	B-4 11/19/2015 163
		the Location Map		METHOD:	Boring			GROOND ELEV.	100
	STRATUM	CLAS  Major Soil Components  Gravel Silt  Sand Clay	Tra So	ATERIAL nor Component Tern ace 1-10% ome 11-35%	SAMPLE	SAMPL	E, FT	BLOWS ON SAMPLER PER SPT (6" INTERAL) RQD	SPT "n" OR
			An	d 36-50%	-	FROM	TO	000	RECOVERY
					1	0	1.5		8"
0	0.3	Gravel			2	4	5.5		10"
					3	9	10.5		14"
0.3	15.5	Min Refuse			4	14	15.5	1-1-3	16"
	15.5	Cutoff							
	_								
							_		
						-			
							_		_
					_				_
						-			
					-	-			
	-				+				
	_								
	-								
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						-			
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	-								
	_								
						-			
	VEL OBSERV		BORING M			TYPE SA			
Noted on ro			v Stem Auger	MD Mud Drilling		A-Split Sp			
At completion	on: N/A		nuous Flight Aug	RC Rock Coring		B-Rock C			
Afterhr	sft	DC Driver	n casing	CA Casing Advan	ncer	C-Shelby			
						D-Grab S	ample		

# S Design Maps Summary Report

Iser-Specified Input

Report Title Benham Tower Site

Wed December 16, 2015 21:50:04 UTC

Building Code Reference Document 2006/2009 International Building Code

(which utilizes USGS hazard data available in 2002)

Site Coordinates 36.9625°N, 82.94954°W

Site Soil Classification Site Class E - "Soft Clay Soil"

Occupancy Category IV



USGS-Provided Output

$$S_s = 0.325 g$$

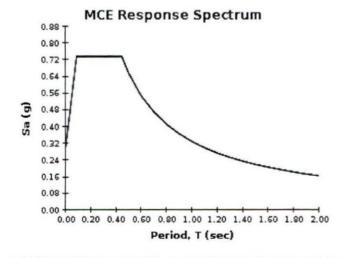
$$S_{MS} = 0.734 g$$

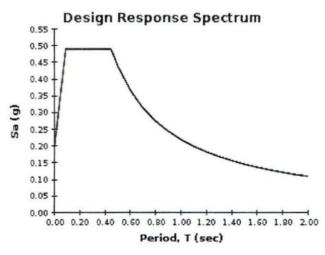
$$S_1 = 0.094 g$$

$$S_{M1} = 0.329 g$$

$$S_{DS} = 0.489 g$$

$$S_{D1} = 0.219 g$$





Although this information is a product of the U.S. Geological Survey, we provide no warranty, expressed or implied, as to the :curacy of the data contained therein. This tool is not a substitute for technical subject-matter knowledge.

# **EIISGS** Design Maps Summary Report

'Jser-Specified Input

Report Title Benham Tower Site Class C

Thu December 17, 2015 19:10:55 UTC

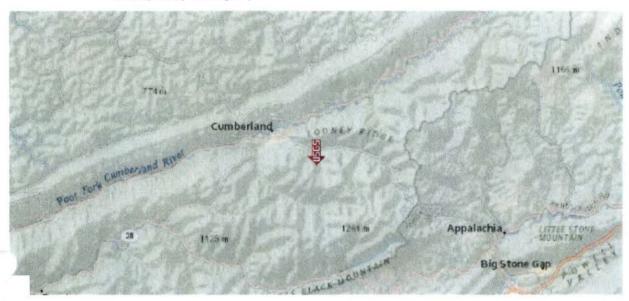
Building Code Reference Document 2006/2009 International Building Code

(which utilizes USGS hazard data available in 2002)

Site Coordinates 36.9625°N, 82.94954°W

Site Soil Classification Site Class C - "Very Dense Soil and Soft Rock"

Occupancy Category IV



#### USGS-Provided Output

$$S_s = 0.325 g$$

0.094 g

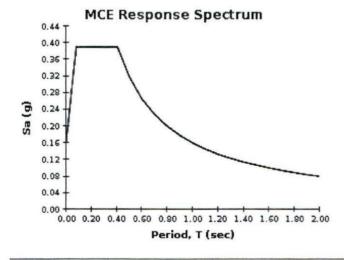
 $S_1 =$ 

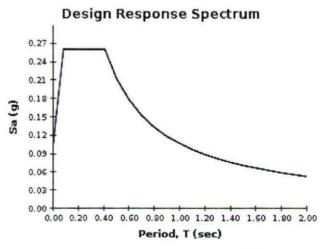
$$S_{MS} = 0.389 g$$

$$S_{M1} = 0.160 g$$

$$S_{DS} = 0.260 g$$

$$S_{D1} = 0.107 g$$

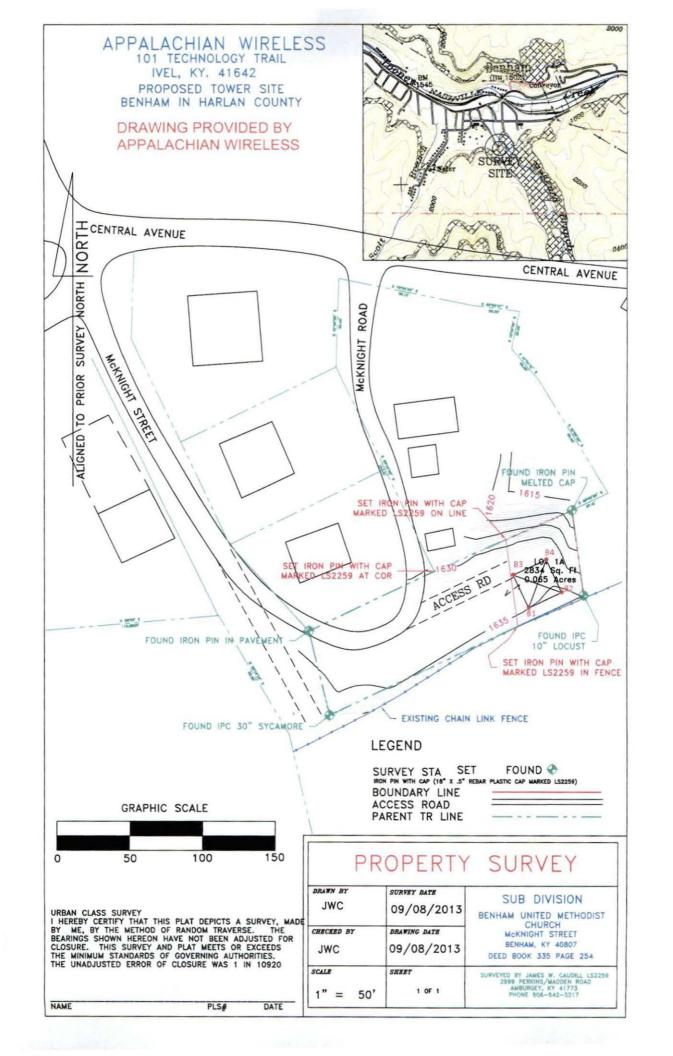




Although this information is a product of the U.S. Geological Survey, we provide no warranty, expressed or implied, as to the ccuracy of the data contained therein. This tool is not a substitute for technical subject-matter knowledge.

## APPENDIX B MAPS



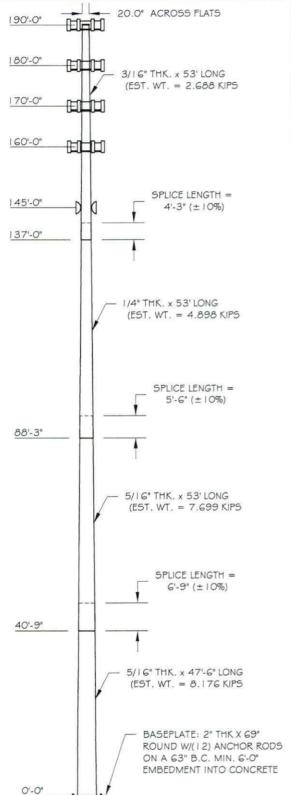




# TransAmerican Power Products, Inc.

2427 Kelly Lane Houston, Texas 77066

PH: 281-444-8277 / FX: 281-444-7270



→ 56.0" ACROSS FLATS

Revision No.:	Revision Date:				
Owner:		WORLD TOWER			
Location:	HARLAN CO	., KY / 36°57'45", -82°	°56'58"		
Site:		BENHAM			
Structure:	190-FT MONOPOLE				
333 1.		Date:	1/26/2016		
Eng:		Customer Ref:	TP-14015		
Page 1 of 2		Job Number:	23516-012		

			None and the last		
		DES	SIGN		
Building Code:	20	3 KENTUCKY BUI	LDING C	ODE	
Design Standard	d: A	NSI/TIA-222-G-2			
Wind Speed Lo.	ad Ca	ses: 3-5	EC. GUS	STED WIND SE	PEED
Load Case #1:	90	MPH Design Win	d Speed	‡	
Load Case #2:	30	MPH Wind with	0.5"	Ice Accumula	ation
Load Case #3	60	MPH Service Wi	nd Spee	d	
Structure Class	Exposure Cat.	Торо	graphy Cat.	Crest Height	

	EQUIPMENT LIST	
Elev.	Description	
190	(9) WPA-800102-4CF + (3) BXA-70063/6CF + (3) RRU	
190	12-FT PLATFORM WITH HANDRAIL	
180	(9) WPA-800102-4CF + (3) BXA-70063/6CF + (3) RRU	
180	I 2-FT PLATFORM WITH HANDRAIL	
170	(9) WPA-800102-4CF + (3) BXA-70063/6CF + (3) RRU	
170	12-FT PLATFORM WITH HANDRAIL	
160	(9) WPA-800102-4CF + (3) BXA-70063/6CF + (3) RRU	
160	12-FT PLATFORM WITH HANDRAIL	
145	(2) 4-FT MICROWAVE DISHES	
145	DUAL MICROWAVE MOUNT	

#### ANTENNA FEED LINES ROUTED ON THE INSIDE OF THE POLE

		STRUCTUR	E PROPER	RTIES					
Cross-S	ection: 18-5	SIDED	Taper:	Taper: 0.19737 in/ft					
Shaft St	Steel: ASTM A572 GR 65 Baseplate Steel: ASTM A572 G								
Anchor F	Rods: 2.25 II	n. AG15 GR. 75	X 7'-0" LON	G					
Sect.	Length (ft)	Thickness (in)	Splice (ft)	Top Dia. (in)	Bot Dia. (in)				
1	53.00	0.1875	4.25	20.00	30.46				
2	53.00	0.2500	5.50	29.25	39.71				
3	53.00	0.3125	6.75	38.12	48.58				
4	47.50	0.3125	0.00	46.63	56.00				



MICHAEL F. PLAHOVINSAK, P.E. #25466 18301 S.R. 161, Plain City, OH 43064 614-398-6250 / mike@mfpeng.com

BASE REACTIONS FOR FOUNDATION DESIGN

Moment: 3589 ft-kip

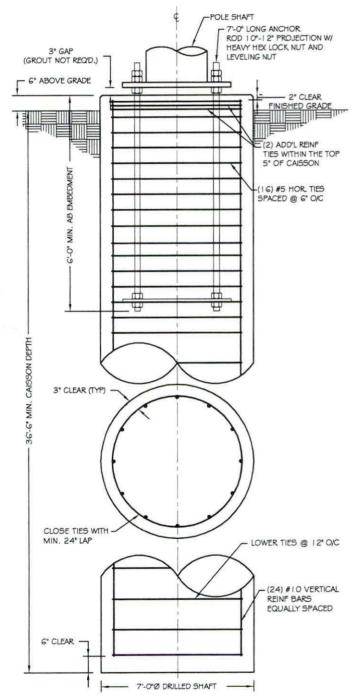
Shear: 25 kip Axial: 55 kip



# TransAmerican Power Products, Inc.

2427 Kelly Lane Houston, Texas 77066

PH: 281-444-8277 / FX: 281-444-7270



Page 2 of 2		Job Number:	23516-012		
Eng:		Customer Ref:	TP-14015		
MFP		Date:	1/26/2016		
Structure:	190-FT MONOPOLE				
Site:		BENHAM			
Location:	HARLAN CO.,	KY / 36°57'45", -82°	56'58"		
Owner:	WORLD TOWER				
Revision No.:	Revision Date:				

#### FOUNDATION NOTES:

- I. ALL FOUNDATION CONCRETE SHALL USE TYPE II CEMENT AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.4G AND SHALL BE AIR ENTRAINED G8 ( $\pm$  1.5%). ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 3 18, "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION.
- 2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM AG 15 VERTICAL BARS SHALL BE GRADE GO, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 3 1 5, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION.
- CAISSON FOUNDATION INSTALLATION SHALL BE IN ACCORDANCE WITH ACI 336, "STANDARD SPECIFICATIONS FOR THE CONSTRUCTION OF DRILLED PIERS", LATEST EDITION.
- 4. THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.
- 5. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY:
  ENGINEER: ENVIRONMENTAL RESOURCES
  REPORT NO.: N/A (DATED | 2/15/15)
- 6. ESTIMATED CONCRETE VOLUME = 53 CUBIC YARDS.
- 7. THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED

LOADS:

MOMENT: 3589 FT\*KIPS SHEAR: 25 KIPS AXIAL: 55 KIPS



MICHAEL F. PLAHOVINSAK, P.E. #25466 18301 S.R. 161. Plain City, OH 43064 614-398-6250 / mike@mfpeng.com

CAISSON FOUNDATION

#### Michael F. Plahovinsak, P.E.

18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com

Job		Page
	190-ft Monopole - MFP #23516-012	1 of 7
Project		Date
	Benham	19:02:52 01/26/16
Client	TARR (TR. 44045)	Designed by
	TAPP (TP-14015)	Mike

## **Tower Input Data**

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Harlan County, Kentucky.

Basic wind speed of 90 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 0.5000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 30 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## **Tapered Pole Section Geometry**

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
ft		ft	ft	Sides	in	in in		in	
L1	190.00-137.00	53.00	4.25	18	20.0000	30.4600	0.1875	0.7500	A572-65 (65 ksi)
L2	137.00-88.25	53.00	5.50	18	29.2462	39.7100	0.2500	1.0000	A572-65 (65 ksi)
L3	88.25-40.75	53.00	6.75	18	38.1241	48.5800	0.3125	1.2500	A572-65 (65 ksi)
L4	40.75-0.00	47.50		18	46.6234	56.0000	0.3125	1.2500	A572-65 (65 ksi)

## **Tapered Pole Properties**

Section	Tip Dia. in	Area in²	I in⁴	r in	C in	I/C in <sup>3</sup>	J in⁴	It/Q in <sup>2</sup>	w in	w/t
L1	20.3085	11.7909	584.7409	7.0334	10.1600	57.5532	1170.2512	5.8966	3.1900	17.013
	30.9299	18.0159	2085.8857	10.7467	15.4737	134.8022	4174.5161	9.0097	5.0310	26.832
L2	30.5494	23.0085	2444.0428	10.2937	14.8571	164.5035	4891.3016	11.5064	4.7073	18,829
	40.3226	31.3115	6159.6442	14.0083	20.1727	305.3459	12327.3933	15.6587	6.5490	26.196
L3	39.8140	37.5044	6774.4000	13,4231	19.3671	349.7898	13557.7139	18.7558	6.1598	19,712
	49.3294	47.8753	14091.5514	17.1350	24.6786	571.0019	28201.6449	23.9422	8.0001	25.6
L4	48.6956	45.9346	12446.3740	16.4404	23.6847	525.5035	24909.1253	22.9717	7.6557	24.498
	56.8639	55.2350	21640.5133	19.7691	28.4480	760.7042	43309.5018	27.6228	9.3060	29.779

Michael F. Plahovinsak, P.E.

18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com

Job		Page
	190-ft Monopole - MFP #23516-012	2 of 7
Project		Date
	Benham	19:02:52 01/26/16
Client	TARR (TR. 11015)	Designed by
	TAPP (TP-14015)	Mike

# Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg			ft			ft²/ft	plf
1 5/8"	С	No	Inside Pole	190.00 - 0.00	18	No Ice	0.00	0.92
						1/2" Ice	0.00	0.92
1 5/8"	C	No	Inside Pole	180.00 - 0.00	18	No Ice	0.00	0.92
						1/2" Ice	0.00	0.92
1 5/8"	C	No	Inside Pole	170.00 - 0.00	18	No Ice	0.00	0.92
						1/2" Ice	0.00	0.92
1 5/8"	C	No	Inside Pole	160.00 - 0.00	18	No Ice	0.00	0.92
						1/2" Ice	0.00	0.92
1 5/8"	C	No	Inside Pole	145.00 - 0.00	2	No Ice	0.00	0.92
						1/2" Ice	0.00	0.92

## **Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	0	ft		ft²	ft²	K
(3) Antel WPA-800102-4CF	Α	From Face	3.00	0.0000	190.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00			1/2" Ice	4.07	4.21	0.06
Antel BXA-70063/6CF w/ mount pipe	A	From Face	3.00 0.00	0.0000	190.00	No Ice 1/2" Ice	7.75 8.29	5.18 6.11	0.04
			0.00					0	0.05
RRU	A	From Face	3.00 0.00	0.0000	190.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.05 0.07
(3) Antel WPA-800102-4CF	В	From Face	3.00	0.0000	190.00	No Ice	3.70	3.66	0.03
w/ mount pipe	В	110m race	0.00	0.0000	190.00	1/2" Ice	4.07	4.21	0.06
Antel BXA-70063/6CF w/	В	From Face	3.00	0.0000	190.00	No Ice	7.75	5.18	0.04
mount pipe			0.00			1/2" Ice	8.29	6.11	0.09
RRU	В	From Face	3.00	0.0000	190.00	No Ice	1.50	1.50	0.05
			0.00			1/2" Ice	2.00	2.00	0.07
(3) Antel WPA-800102-4CF	C	From Face	3.00	0.0000	190.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00			1/2" Ice	4.07	4.21	0.06
Antel BXA-70063/6CF w/	C	From Face	3.00	0.0000	190.00	No Ice	7.75	5.18	0.04
mount pipe			0.00			1/2" Ice	8.29	6.11	0.09
RRU	С	From Face	3.00 0.00 0.00	0.0000	190.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.05 0.07
12' Platform w/ Handrail	C	None	0.00	0.0000	190.00	No Ice 1/2" Ice	24.00 26.00	24.00 26.00	1.80 2.60
**									
(3) Antel WPA-800102-4CF	A	From Face	3.00	0.0000	180.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00			1/2" Ice	4.07	4.21	0.06
Antel BXA-70063/6CF w/	A	From Face	3.00	0.0000	180.00	No Ice	7.75	5.18	0.04
mount pipe			0.00			1/2" Ice	8.29	6.11	0.09

Michael F. Plahovinsak, P.E. 18301 State Route 161

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Project		Date
	Benham	19:02:52 01/26/16
Client	TARR (TR. 44045)	Designed by
	TAPP (TP-14015)	Mike

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weigh
	Leg		Lateral Vert						
			ft ft	0	ft		ft²	ft²	K
			ft						
RRU	A	From Face	3.00	0.0000	180.00	No Ice	1.50	1.50	0.05
			0.00			1/2" Ice	2.00	2.00	0.07
3) Antel WPA-800102-4CF	В	From Face	3.00	0.0000	180.00	No Ice	3.70	3.66	0.03
w/ mount pipe	Б	rioni race	0.00	0.0000	180.00	1/2" Ice	4.07	4.21	0.03
W mount pipe			0.00			112 100	1.01	1,21	0.00
Antel BXA-70063/6CF w/	В	From Face	3.00	0.0000	180.00	No Ice	7.75	5.18	0.04
mount pipe			0.00			1/2" Ice	8.29	6.11	0.09
			0.00						
RRU	В	From Face	3.00	0.0000	180.00	No Ice	1.50	1.50	0.05
			0.00			1/2" Ice	2.00	2.00	0.07
			0.00	0.0000			2.70	2.55	0.00
3) Antel WPA-800102-4CF	C	From Face	3.00	0.0000	180.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00			1/2" Ice	4.07	4.21	0.06
Antel BXA-70063/6CF w/	C	From Face	3.00	0.0000	180.00	No Ice	7.75	5.18	0.04
mount pipe	-	From Face	0.00	0.0000	100.00	1/2" Ice	8.29	6.11	0.04
mount pipe			0.00			1/2 100	0.27	0.11	0.03
RRU	C	From Face	3.00	0.0000	180.00	No Ice	1.50	1.50	0.05
	.55		0.00			1/2" Ice	2.00	2.00	0.07
			0.00						
12' Platform w/ Handrail	C	None		0.0000	180.00	No Ice	24.00	24.00	1.80
						1/2" Ice	26.00	26.00	2.60
**	4							2.22	
3) Antel WPA-800102-4CF	Α	From Face	3.00	0.0000	170.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00			1/2" Ice	4.07	4.21	0.06
Antel BXA-70063/6CF w/	Α	From Face	3.00	0.0000	170.00	No Ice	7.75	5.18	0.04
mount pipe	Α	rioin race	0.00	0.0000	170.00	1/2" Ice	8.29	6.11	0.04
mount pipe			0.00			172 100	0,27	0.11	0.07
RRU	Α	From Face	3.00	0.0000	170.00	No Ice	1.50	1.50	0.05
			0.00			1/2" Ice	2.00	2.00	0.07
			0.00						
3) Antel WPA-800102-4CF	В	From Face	3.00	0.0000	170.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00			1/2" Ice	4.07	4.21	0.06
	-		0.00						
Antel BXA-70063/6CF w/	В	From Face	3.00	0.0000	170.00	No Ice	7.75	5.18	0.04
mount pipe			0.00			1/2" Ice	8.29	6.11	0.09
RRU	В	From Face	3.00	0.0000	170.00	No Ice	1.50	1.50	0.05
RKO	D	riom race	0.00	0.0000	170.00	1/2" Ice	2.00	2.00	0.03
			0.00			1/2 100	2.00	2.00	0.07
3) Antel WPA-800102-4CF	C	From Face	3.00	0.0000	170.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00			1/2" Ice	4.07	4.21	0.06
50% - 50% SA 100 COUNTY - 100 CO			0.00						
Antel BXA-70063/6CF w/	C	From Face	3.00	0.0000	170.00	No Ice	7.75	5.18	0.04
mount pipe			0.00			1/2" Ice	8.29	6.11	0.09
	-	-	0.00	2 2222				12/12/2	21211
RRU	C	From Face	3.00	0.0000	170.00	No Ice	1.50	1.50	0.05
			0.00			1/2" Ice	2.00	2.00	0.07
12' Platform w/ Handrail	С	None	0.00	0.0000	170.00	No Tee	24.00	24.00	1.00
12 Flationiii W/ Handrall	C	None		0.0000	170.00	No Ice 1/2" Ice	24.00 26.00	24.00 26.00	1.80 2.60
**						1/2 100	20.00	20,00	2.00
3) Antel WPA-800102-4CF	A	From Face	3.00	0.0000	160.00	No Ice	3.70	3.66	0.03
w/ mount pipe			0.00	-,0000		1/2" Ice	4.07	4.21	0.06
			0.00				1000		

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Project		Date
	Benham	19:02:52 01/26/16
Client	TARR (TR 44045)	Designed by
	TAPP (TP-14015)	Mike

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	C₄A₄ Side	Weight
			ft ft ft	o	fi		ft²	ft²	K
Antel BXA-70063/6CF w/ mount pipe	A	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	7.75 8.29	5.18 6.11	0.04 0.09
RRU	A	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.05 0.07
(3) Antel WPA-800102-4CF w/ mount pipe	В	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	3.70 4.07	3.66 4.21	0.03 0.06
Antel BXA-70063/6CF w/ mount pipe	В	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	7.75 8.29	5.18 6.11	0.04 0.09
RRU	В	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.05 0.07
(3) Antel WPA-800102-4CF w/ mount pipe	С	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	3.70 4.07	3.66 4.21	0.03 0.06
Antel BXA-70063/6CF w/ mount pipe	С	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	7.75 8.29	5.18 6.11	0.04 0.09
RRU	С	From Face	3.00 0.00 0.00	0.0000	160.00	No Ice 1/2" Ice	1.50 2.00	1.50 2.00	0.05 0.07
12' Platform w/ Handrail	С	None	1202.54	0.0000	160.00	No Ice 1/2" Ice	24.00 26.00	24.00 26.00	1.80 2.60

D		~	h	^	-
8 8	8 8	•			

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				ft	0	0	ft	ft		ft²	K
4 ft standard	A	Paraboloid w/o	From	1.00	0.0000		145.00	4.00	No Ice	12.57	0.10
		Radome	Face	0.00					1/2" Ice	13.10	0.18
ft standard	В	Paraboloid w/o	From	1.00	0.0000		145.00	4.00	No Ice	12.57	0.10
1201-2-000		Radome	Face	0.00					1/2" Ice	13.10	0.18

# **Load Combinations**

Comb. No.		Description	
1	Dead Only		
2	1.2 Dead+1.6 Wind 0 deg - No Ice		
3	0.9 Dead+1.6 Wind 0 deg - No Ice		
4	1.2 Dead+1.6 Wind 90 deg - No Ice		
5	0.9 Dead+1.6 Wind 90 deg - No Ice		
6	1.2 Dead+1.6 Wind 180 deg - No Ice		
7	0.9 Dead+1.6 Wind 180 deg - No Ice		
8	1.2 Dead+1.0 Ice+1.0 Temp		

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Client	TAPP (TP-14015)	Designed by Mike

Comb. No.	Description	
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	
12	Dead+Wind 0 deg - Service	
13	Dead+Wind 90 deg - Service	
14	Dead+Wind 180 deg - Service	

## **Maximum Member Forces**

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Type		Load		Moment	Moment
				Comb.	K	kip-ft	kip-ft
LI	190 - 137	Pole	Max Tension	7	0.00	0.00	0.00
			Max. Compression	8	-32.23	0.00	0.64
			Max. Mx	4	-14.89	-530.82	-2.10
			Max. My	6	-14.91	0.00	-530.18
			Max. Vy	4	17.35	-530.82	-2.10
			Max. Vx	6	17.29	0.00	-530.18
			Max. Torque	5			0.49
L2	137 - 88.25	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-44.03	0.00	0.64
			Max. Mx	4	-24.80	-1429.84	-20.91
			Max. My	6	-24.81	0.00	-1426.02
			Max. Vy	4	20.43	-1429.84	-20.91
			Max. Vx	6	20.36	0.00	-1426.02
			Max. Torque	5			0.49
L3	88.25 - 40.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-59.45	0.00	0.64
			Max. Mx	4	-37.92	-2441.23	-39.07
			Max. My	6	-37.93	0.00	-2434.34
			Max. Vy	4	23.16	-2441.23	-39.07
			Max. Vx	6	23.09	0.00	-2434.34
			Max. Torque	5			0.49
L4	40.75 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-77.78	0.00	0.64
			Max. Mx	4	-53.79	-3588.77	-56.95
			Max. My	6	-53.79	0.00	-3578.85
			Max. Vy	4	24.99	-3588.77	-56.95
			Max. Vx		24.93	0.00	-3578.85
			Max. Torque	6		90700	0.49

## **Maximum Tower Deflections - Service Wind**

Section No.	Elevation	Elevation Horz. Deflection		Tilt	Twist	
	ft	in	Comb.	0	0	
L1	190 - 137	50.123	13	2.4346	0.0000	
L2	141.25 - 88.25	26.925	13	1.9444	0.0000	
L3	93.75 - 40.75	11.261	13	1.1615	0.0000	
L4	47.5 - 0	2.864	13	0.5521	0.0000	

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	Benham	19:02:52 01/26/16
Client	TARR (TR 44045)	Designed by
	TAPP (TP-14015)	Mike

## Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature	
ft		Comb.	in	0	٥	ft	
190.00	(3) Antel WPA-800102-4CF w/ mount pipe	13	50.123	2.4346	0.0007	30767	
180.00	(3) Antel WPA-800102-4CF w/ mount pipe	13	45.068	2.3528	0.0007	15383	
170.00	(3) Antel WPA-800102-4CF w/ mount pipe	13	40.090	2.2661	0.0008	7691	
160.00	(3) Antel WPA-800102-4CF w/ mount pipe	13	35.270	2.1694	0.0008	5126	
145.00	4 ft standard	13	28.503	1.9951	0.0008	3417	

## **Maximum Tower Deflections - Design Wind**

Section No.	Elevation	Elevation Horz.  Deflection		Tilt	Twist	
	ft	in	Comb.	0	٥	
L1	190 - 137	204.394	4	9.9581	0.0010	
L2	141.25 - 88.25	109.899	4	7.9531	0.0000	
L3	93.75 - 40.75	45.976	4	4.7482	0.0000	
L4	47.5 - 0	11.687	4	2,2545	0.0000	

# Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
ft		Load Comb.	in	0	0	Curvature ft
190.00	(3) Antel WPA-800102-4CF w/ mount pipe	4	204.394	9.9581	0.0010	7912
180.00	(3) Antel WPA-800102-4CF w/ mount pipe	4	183.808	9.6237	0.0013	3954
170.00	(3) Antel WPA-800102-4CF w/ mount pipe	4	163.539	9.2690	0.0016	1973
160.00	(3) Antel WPA-800102-4CF w/ mount pipe	4	143.904	8.8738	0.0018	1311
145.00	4 ft standard	4	116.332	8.1607	0.0021	869

## Pole Design Data

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in <sup>2</sup>	K	K	$\phi P_n$
L1	190 - 137 (1)	TP30.46x20x0.1875	53.00	0.00	0.0	17.5167	-14.89	1115.66	0.013
L2	137 - 88.25 (2)	TP39.71x29.2462x0.25	53.00	0.00	0.0	30.4499	-24.80	1959.15	0.013
L3	88.25 - 40.75 (3)	TP48.58x38.1241x0.3125	53.00	0.00	0.0	46.5545	-37.92	3023.96	0.013
L4	40.75 - 0 (4)	TP56x46.6234x0.3125	47.50	0.00	0.0	55.2350	-53.79	3299.62	0.016

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	Benham	19:02:52 01/26/16
Client	TAPP (TP-14015)	Designed by
	IAFF (IF-14015)	Mike

Pole Bending Desi	gn	Data
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Section No.	Elevation	Size	$M_{ux}$	$\phi M_{nx}$	Ratio Mux	$M_{uy}$	$\phi M_{ny}$	Ratio Muy
	ft		kip-ft	kip-ft	$\phi M_{nx}$	kip-ft	kip-ft	$\phi M_{ny}$
Ll	190 - 137 (1)	TP30.46x20x0.1875	530.83	676.26	0.785	0.00	676.26	0.000
L2	137 - 88.25 (2)	TP39.71x29.2462x0.25	1429.99	1548.03	0.924	0.00	1548.03	0.000
L3	88.25 - 40.75 (3)	TP48.58x38.1241x0.3125	2441.54	2922.07	0.836	0.00	2922.07	0.000
L4	40.75 - 0 (4)	TP56x46.6234x0.3125	3589.22	3786.90	0.948	0.00	3786.90	0.000

# Pole Shear Design Data

Section No.	Elevation	Size	Actual V <sub>u</sub>	$\phi V_n$	Ratio V <sub>u</sub>	Actual T <sub>u</sub>	$\phi T_n$	Ratio T <sub>u</sub>
	ft		K	K	$\phi V_n$	kip-ft	kip-ft	$\phi T_n$
Ll	190 - 137 (1)	TP30.46x20x0.1875	17.36	557.83	0.031	0.49	1354.17	0.000
L2	137 - 88.25 (2)	TP39.71x29.2462x0.25	20.43	979.57	0.021	0.48	3099.83	0.000
L3	88.25 - 40.75 (3)	TP48.58x38.1241x0.3125	23.16	1511.98	0.015	0.48	5851.29	0.000
L4	40.75 - 0 (4)	TP56x46.6234x0.3125	24.99	1649.81	0.015	0.48	7583.06	0.000

# Pole Interaction Design Data

Section No.	Elevation	Ratio Pu	Ratio Mux	Ratio Muy	Ratio V <sub>u</sub>	Ratio T <sub>u</sub>	Comb. Stress	Allow. Stress	Criteria
	ft	$\phi P_n$	$\phi M_{nx}$	$\phi M_{ny}$	φ <i>V</i> ,,	φ <i>T</i> ,,	Ratio	Ratio	
Ll	190 - 137 (1)	0.013	0.785	0.000	0.031	0.000	0.799	1.000	4.8.2
L2	137 - 88.25 (2)	0.013	0.924	0.000	0.021	0.000	0.937	1.000	4.8.2
L3	88.25 - 40.75 (3)	0.013	0.836	0.000	0.015	0.000	0.848	1.000	4.8.2
L4	40.75 - 0 (4)	0.016	0.948	0.000	0.015	0.000	0.964	1.000	4.8.2

# **Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\sigma P_{allow}$ $K$	% Capacity	Pass Fail
L1	190 - 137	Pole	TP30.46x20x0.1875	1	-14.89	1115.66	79.9	Pass
L2	137 - 88.25	Pole	TP39.71x29.2462x0.25	2	-24.80	1959.15	93.7	Pass
L3	88.25 - 40.75	Pole	TP48.58x38.1241x0.3125	3	-37.92	3023.96	84.8	Pass
L4 40.75 - 0 P	Pole	TP56x46.6234x0.3125	4	-53.79	3299.62	96.4	Pass	
							Summary	
						Pole (L4)	96.4	Pass
						RATING =	96.4	Pass

#### Michael F. Plahovinsak, P.E.

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190-ft monopole - MFP #23516-012	Page BP-G	
Project Benham	Date 1/26/2016	
Client TAPP TP-14015	Designed by Mike	

## **Anchor Rod and Base Plate Calculation**

### ANSI/TIA-222-G-2

Factored Base Reactions:

Pole Shape:

Anchor Rods:

Base Plate:

Moment:

3589 ft-kips

18-Sided

(12) 2.25 in. A615 GR. 75

2 in. x 69 in. Round

Shear:

25 kips

Pole Dia. (Df):

Anchor Rods Evenly Spaced

Axial:

54 kips

56.00 in

On a 63 in Bolt Circle

fv = 60 ksi

## Anchor Rod Calculation According to TIA-222-G section 4.9.9

 $\phi =$ 

0.80 TIA 499

 $I_{bolts} =$ 

5953.50 in Momet of Inertia

 $P_n =$ 

228 kips Tension Force

2 kips Shear Force

 $\mathbf{R}_{nt} =$ 

325.00 kips Nominal Tensile Strength

 $\eta =$ 

0.50 for detail type (d)

The following Interation Equation Shall Be Satisfied:

$$\left(\frac{P_{u} + \frac{V_{u}}{\eta}}{\phi R_{nt}}\right) \leq 1.0$$

$$0.892 \leq 1$$

#### Base Plate Calculation According to TIA-222-G

0.90 TIA 4.7

551.9 in-kip Plate Moment

14.7 in Section Length

Z =

 $M_p =$ 

14.7 Plastic Section Modulus

879.6 in-kip Plastic Moment

 $\phi M_n =$ 

791.7 in-kip Factored Resistance

Calculated Moment vs Factored Resistance

551.89 in-kip ≤

792 in-kip

Anchor Rods Are Adequate

89.2% ☑

Base Plate is Adequate

69.7% ☑

#### Job Page 190-ft monopole - MFP #23516-012 Michael F. Plahovinsak, P.E. FND 18301 State Route 161 W Project Plain City, OH 43064 Benham 1/26/2016 Phone: 614-398-6250 Client Designed by email: mike@mfpeng.com **TAPP TP-14015** Mike

## **Caisson Calculation**

#### According to ANSI/TIA-222-G-2

- 1. Foundation overturning resistance calculated with PLS Caisson, for Brom's method for rigid piles. Soil layers modeled after recommendations from the geotechnical report.
- 2. Cohesion strength for the upper 21 ft has been reduced by 50%
- 3. In lieu of a soil resistance factor fs = 0.75 (TIA-9.4.1) an additional safey fator against soil failure of 1.33 has been applied.
- 4. Foundation is designed with a minimum safety factor resisting overturning of 2.0
- 5. Foundation has been designed with factored loads per TIA-222-G.
- 6. Geotechnical report indicates groundwater was not encountered within the depth of the foundation.

**	PIER I	PROPERTIES	CONCRETE	STRE	NGTH	(ksi)	= 4	.00				STEEL	STRE	NGTH	(ksi)	=	60.0
			DIAMETER	(ft)	- 1	7.000		DIST	ANCE FR	OM TOP	OF PIER	TO GR	ROUND	LEVEL	(ft)	=	0.5
**	SOIL I	PROPERTIES	LAYER T	YPE	THICE	KNESS	DEPTH	AT T	OP OF L	AYER	DENSIT		CU		KP		PH
						(ft)				(ft)	(pcf	)	(psf)			(de	grees
			1	S		4.00				0.00	0.	0		1	.000		-0.0
			2	S		31.00				4.00	100.	0		1	.420		9.9
			3	C		10.00			3	5.00	130.	0 6	000.0				
**	DESIGN	(FACTORED)	LOADS AT TO	POF	PIER						RTICAL (					) =	25.
**	CALCUI	LATED PIER LE	NGTH (ft)	= 3	1.500	0											
**	CHECK	OF SOILS PRO	PERTIES AND	ULTI	MATE	RESIS	TING F	ORCES	ALONG	PIER							
	TYPE	TOP OF LAYER	BELOW TOP	OF PI	ER	THICK	NESS	DEN	SITY		CU	KF	•	FOR	CE		AR
				(f	t)		(ft)	(	pcf)	(pa	sf)			(	k)		(ft
																	2 4
	S			0.	50		4.00		0.0			1.000		0.			3.1
	S			0. 4.		1											
					50	1	9.38	1	00.0			1.000 1.420 1.420	)	560.	19		17.4
••	S	AND MOMENTS	ALONG PIER	4.	50 88	1	9.38 7.62	1	00.0			1.420	)	560. -526.	19 75		17.4 27.9
	S S SHEAR			4. 23.	50 88	TH THE	9.38 7.62 ADDIT	1 1 IONAL	00.0 00.0 SAFETY		R WI	1.420 1.420	ADDIT	560. -526.	19 75 SAFE	TY	17.4 27.9
	S S SHEAR	AND MOMENTS	OF PIER (f	4. 23.	50 88	TH THE	9.38 7.62 ADDIT	1 1 IONAL k)	00.0	(ft-k)	R WI	1.420 1.420	ADDIT	560. -526. IONAL	19 75	TY	17.4 27.9 FACTO
	S S SHEAR		OF PIER (ff	4. 23.	50 88	TH THE	9.38 7.62 ADDIT HEAR (	1 1 IONAL k)	00.0 00.0 SAFETY	(ft-k) 4936.5	R WI	1.420 1.420	ADDIT	560. -526. IONAL k)	19 75 SAFE	TY	17.4 27.9 FACTO (ft-k 3702.
	S S SHEAR		OF PIER (ff 0.0	4. 23. t) 00	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33	1 1 IONAL k) .4	00.0 00.0 SAFETY	(ft-k) 4936.5 5041.8	R WI	1.420 1.420	ADDIT	560. -526. IONAL k)	19 75 SAFE	TY	17.4 27.9 FACTO (ft-k 3702.
	S S SHEAR		OF PIER (f. 3 6	4. 23. t) 00 15	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28	1 10NAL k) .4 .4	00.0 00.0 SAFETY	(ft-k) 4936.5 5041.8 5144.3	R WI	1.420 1.420	ADDIT HEAR ( 25 25	560. -526. IONAL k) .1	19 75 SAFE	TY NT	17.4 27.9 FACTO (ft-k 3702. 3781. 3858.
	S S SHEAR		OF PIER (f. 3 6 9	4. 23. t) 00 15 30 45	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28	1 1 (IONAL k) .4 .4 .6	00.0 00.0 SAFETY	(ft-k) 4936.5 5041.8 5144.3	R WI	1.420 1.420 THOUT	ADDIT HEAR ( 25 25 21	560. -526. IONAL k) .1 .1 .5	19 75 SAFE	TY NT	17.4 27.9 FACTO (ft-k 3702. 3781. 3858. 3894.
	S S SHEAR		OF PIER (f: 0. 3. 6. 9.	4. 23. t) 000 15 30 45	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28 -3 -64	1 1 IONAL k) .4 .4 .6 .1	00.0 00.0 SAFETY	(ft-k) 4936.5 5041.8 5144.3 5192.3 5093.8	R WI	1.420 1.420 THOUT	ADDIT HEAR ( 25 25 21 -2	560. -526. IONAL k) .1 .5 .3	19 75 SAFE	TY NT	17.4 27.9 FACTO (ft-k 3702. 3781. 3858. 3894. 3820.
	S S SHEAR		OF PIER (f 0. 3. 6. 9. 12.	4. 23. t) 000 15 30 45 60 75	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28 -3 -64 -155	1 1 IONAL k) .4 .4 .6 .1 .4	00.0 00.0 SAFETY	(ft-k) 4936.5 5041.8 5144.3 5192.3 5093.8 4755.6	R WI	1.420 1.420 THOUT SH	ADDIT HEAR ( 25 25 21 -2 -48 -116	560. -526. TONAL k) .1 .1 .5 .3 .4	19 75 SAFE	TY NT	17.4 27.9 FACTO (ft-k 3702. 3781. 3858. 3894. 3820. 3566.
	S S SHEAR		OF PIER (ff 0.0 3 6 9 12 15 18	4. 23. t) 000 15 30 45 60 75 90	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28 -3 -64 -155 -275	1 1 IONAL k) .4 .4 .6 .1 .4 .3	00.0 00.0 SAFETY	(ft-k) 4936.5 5041.8 5144.3 5192.3 5093.8 4755.6	R WI	1.420 1.420 THOUT	ADDIT HEAR ( 25 21 -2 -48 -116	560. -526. IONAL k) .1 .1 .5 .3 .4	19 75 SAFE	TY NT	17.4 27.9 FACTO (ft-k 3702. 3781. 3858. 3858. 3856. 3063.
	S S SHEAR		OF PIER (for 0 3 6 9 12 15 18 22	4. 23. t) 000 15 30 45 60 75 90	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28 -3 -64 -155 -275 -425	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	SAFETY MOMENT	(ft-k) 4936.5 5041.8 5144.3 5192.3 5093.8 4755.6 4084.6 2987.5	R WI	1.420 1.420 THOUT SH	ADDIT HEAR ( 25 21 -2 -48 -116 -206 -319	560. -526. IONAL k) .1 .5 .3 .4 .8	19 75 SAFE	TY	17.4 27.9 FACTO (ft-k 3702. 3781. 3858. 3894. 3820. 3566. 3063. 2240.
	S S SHEAR		OF PIER (fr 0.3. 6. 9. 12. 15. 18. 22. 25.	4. 23. t) 000 15 30 45 60 75 90 05 20	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28 -3 -64 -155 -275 -425 -448	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	00.0 00.0 SAFETY MOMENT	(ft-k) 4936.5 5041.8 5144.3 5192.3 5093.8 4755.6 4084.6 2987.5 1473.5	R WI	1.420 1.420 THOUT SH	ADDIT HEAR ( 25 25 21 -2 -48 -116 -206 -319	560. -526. FIONAL k) .1 .5 .3 .3 .4 .8 .3	19 75 SAFE MOME	TY	17.4 27.9 FACTO (ft-k 3702. 3781. 3858. 3894. 3820. 3566. 3063. 2240.
	S S SHEAR		OF PIER (for 0 3 6 9 12 15 18 22	4. 23. t) 00 15 30 45 60 75 90 005 20 35	50 88	TH THE	9.38 7.62 ADDIT HEAR ( 33 33 28 -3 -64 -155 -275 -425	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	00.0 00.0 SAFETY MOMENT	(ft-k) 4936.5 5041.8 5144.3 5192.3 5093.8 4755.6 4084.6 2987.5	R WI	1.420 1.420 THOUT SH	ADDIT HEAR ( 25 25 21 -2 -48 -116 -206 -319	560. -526. FIONAL k) .1 .5 .3 .3 .4 .8 .3 .1	19 75 SAFE MOME	TY	

USABLE MOMENT CAP. (ft-k) = 3894.8

#### For Design:

\*\*\* USABLE AXIAL CAP. (k) = 55.0

7-ft	Diam	meter cai	sson x	37-ft	long (	6.5-ft	Embeded	with	0.5-ft	above	grade)	
Concr	ete	strength	=4000	PSI @	28 days	. Esti	mated Co	ncrete	Volume	e = 53	CY3.	
(24)	#10	Vertical	Rebar.	Stee	Cross-	Section	n = 30.4	8 in2				



FAA Advisory Circular 70/7460-1L, Obstruction Marking Lighting was published on 12/4/2015, and is effective as of that date. The document may be viewed at

http://www.faa.gov /regulations\_policies /advisory\_circulars/index.cfm /go/document.information /documentID/1028657

## Notice of Proposed Construction or Alteration - Off Airport

Project Name: EAST -000353775-16 Sponsor: East Kentucky Network, LLC

Details for Case: Benham

Show Project Summary

**Case Status** 

ASN:

2016-ASO-762-OE

Status:

Accepted

Date Accepted:

01/14/2

Date Determined: Letters:

Documents:

None

**Public Comments:** 

None

Project E None

01/14/2

Construction / Alteration Information

Notice Of:

Construction

**Duration:** 

Permanent

if Temporary: Months: Days:

Work Schedule - Start:

02/15/2016

Work Schedule - End:

02/20/2016

**Structure Summary** 

Structure Type:

Tower Benham

Structure Name:

FDC NOTAM:

**NOTAM Number:** 

FCC Number:

Prior ASN:

\*For temporary cranes-Does the permanent structure require separate notice to the FAA? To find out, use the Notice Criteria Tool. If separate notice is required, please ensure it is filed. If it is not filed, please state the reason in the Description of Proposal.

State Filing:

Filed with State

#### Structure Details

Latitude:	36° 57' 44.96" N	Common Fre	quency Band
Longitude:	82° 56' 58.33" W	Low Freq	High Freq
Horizontal Datum:	NAD83	698	806
		806	824
Site Elevation (SE):	1634 (nearest foot)	824	849
Structure Height (AGL):	190 (nearest foot)	851	866
Current Height (AGL):	(nearest foot)	869	894
* For notice of alteration or existing provide the current	(1)	896	901
AGL height of the existing structure.		901	902
Include details in the Description of Proposal		930	931
		931	932
Minimum Operating Height (AGL):	(nearest foot)	932	932.5
* For aeronautical study of a crane or construction equipment		935	940
the maximum height should be listed above as the		940	941
Structure Height (AGL). Additionally, provide the minimum		1850	1910
operating height to avoid delays if impacts are identified that require negotiation to a reduced height. If the Structure Height		1930	1990
and minimum operating height are the same enter the same		2305	2310
value in both fields.		2345	2360

Nacelle Height (AGL):

\* For Wind Turbines 500ft AGL or greater

(nearest foot)

Specific Frequencies

Requested Marking/Lighting:

None

	Other :	
Recommended Marking/Lighting:		
Current Marking/Lighting:		N/A Proposed Structure
	Other:	
Nearest City:		Benham
Nearest State:		Kentucky
Description of Location: On the Project Summary page upload any certifie	d survey.	Off of McKnight Rd, Benham (Harlan), KY
Description of Proposal:		A new 180' tower with top-mounted antennas (overall height of 190' AGL)

← Previous

Next

Back to Search Result



#### KENTUCKY TRANSPORTATION CABINET

TC 56-50 Rev. 07/2010 Page 2 of 2

#### KENTUCKY AIRPORT ZONING COMMISSION

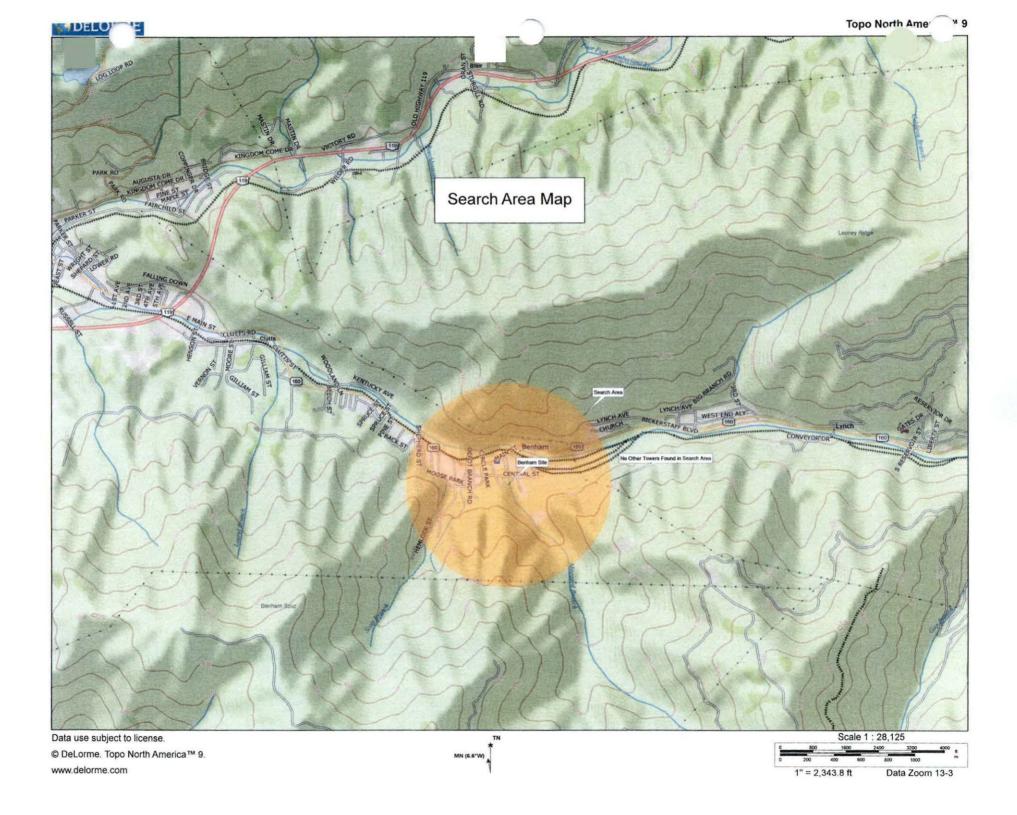
## APPLICATION FOR PERMIT TO CONSTRUCT OR ALTER A STRUCTURE

APPLICANT (name)	PHONE	FAX	KY AERON	IAUTICAL STUDY #		
East Kentucky Network, LLC c/o LNGS	703-584-8667	703-584-8692				
ADDRESS (street)	CITY		STATE	ZIP		
8300 Greensboro Dr, #1200	McLean		VA	22102		
APPLICANT'S REPRESENTATIVE (name)	PHONE	FAX				
Ali Kuzehkanani	703-584-8667	703-584-8692				
ADDRESS (street)	CITY		STATE	ZIP		
8300 Greensboro Dr, #1200	McLean		VA 22102			
APPLICATION FOR New Construct	tion 🔲 Alteration	Existing	WORK SCI	HEDULE		
DURATION Permanent Tem	porary (months	days )	Start 02/1	5/16 End 02/20/16		
TYPE Crane Building	MARKING/PAINTIN	G/LIGHTING PREFE	RRED			
Antenna Tower	Red Lights & Pa	int White- med	ium intensi	ty White-high intensity		
Power Line Water Tank	Dual- red & med	dium intensity white	Dual-	red & high intensity white		
☐ Landfill ☐ Other	Other None					
LATITUDE	LONGITUDE		DATUM	NAD83 ☐ NAD27		
36°57′44.96″	82°56'58.33"		Other	No. of the last of		
NEAREST KENTUCKY	NEAREST KENTUCK	Y PUBLIC USE OR M	ILITARY AIR	RPORT		
City Benham County Harlan	Lonesome Pine Airp	oort				
TE ELEVATION (AMSL, feet)	TOTAL STRUCTURE	HEIGHT (AGL, feet)	CURRENT	(FAA aeronautical study #)		
1634	190					
RALL HEIGHT (site elevation plus to	tal structure height,	feet)	PREVIOUS	(FAA aeronautical study #)		
, l						
DISTANCE (from nearest Kentucky publi	c use or Military airp	oort to structure)	PREVIOUS	(KY aeronautical study #)		
23.2 miles						
<b>DIRECTION</b> (from nearest Kentucky pub	lic use or Military air	port to structure)				
East	149%					
<b>DESCRIPTION OF LOCATION (Attach US</b>	GS 7.5 minute quadr	rangle map or an air <sub>l</sub>	port layout	drawing with the precise site		
marked and any certified survey.)						
Off of McKnight Rad, Benham (Harlan),	KY					
DESCRIPTION OF PROPOSAL						
A new 180' tower with top-mounted an	tennas (overall heig	ht of 190' AGL)				
FAA Form 7460-1 (Has the "Notice of Co	onstruction or Altera	tion" been filed with	the Federa	l Aviation Administration?)		
☐ No  ☐ Yes, when? 01/14/16						
CERTIFICATION (I hereby certify that all	the above entries, n	nade by me, are true	, complete,	and correct to the best of		
my knowledge and belief.)						
PENALITIES (Persons failing to comply w						
imprisonment as set forth in KRS 183.99	0(3). Noncompliance	e with FAA regulation	ns may resu	ılt in further penalties.)		
NAME TITLE	SIGNATURE	11	DATE			
i Kuzehkanani Dir of Engineer	ing Mi XII	3eA/Caman	01/14/16			
1	Chairperson	n, KAZC				
AMISSION ACTION	Administrat					
Approved SIGNATURE	en versita ribile 20 mil	10000000	DATE			
Disapproved			DAIL			
□ ызарргоvец						

## Driving Directions for Benham

From the Harlan County Courthouse located at the intersection of First Street and Central Street. Take Central Street for 1/10 of a mile to the junction of Central Street and 421, and then turn right onto 421. Go 22 miles then exit to the right onto 160. Go 1/10 of a mile; turn left at the light traveling 1.6 miles. Turn right onto Tool House Lane, then go to the end and turn right then left. Go approximately 500 feet and the site will be on your left (signs will be posted here).

Prepared By: Daryl Bartley Appalachian Wireless (606) 791-0310



#### LEASE AGREEMENT

THIS LEASE AGREEMENT is made and entered into on the \_\_\_\_\_\_ day of November, 2013, by and between Benham Community United Methodist Church, of Central Avenue, Benham, KY 40807, LESSORS, and East Kentucky Network, LLC, d/b/a Appalachian Wireless, of 101 Technology Trail, Ivel, Kentucky 41642, LESSEE:

#### WITNESSETH:

That for and in consideration of the rents and other considerations hereinafter set out and subject to the terms and conditions therefore, Lessor(s) do hereby lease, let and demise unto Lessee, its successors and assigns, to have and to hold for the term hereinafter set out and subject to the Lessees right to surrender or terminate this Lease and provided hereinafter, the following described premises (Leased Premises), which term shall include all real property, rights and privileges herein granted:

BEING the same property described by metes and bounds in the description attached hereto and made a part hereof as Exhibit "A", and as shown on the plat dated September 8, 2013, prepared by James W. Caudill, Licensed Professional Land Surveyor, and attached hereto and made a part hereof as Exhibit "B".

The Lessor grants unto Lessee full and complete right of ingress, egress and regress over roads located upon this property controlled by Lessor to and from the Leased Premises, and the nonexclusive right to use any existing road located on this property. In the event the Lessee desires to relocate all or any portion of an existing roadway or to construct another access road to the Leased Premises, the location of such roadway shall be mutually agreed upon by Lessor and Lessee. Lessor further grants to the Lessee a right of way and easement to construct and maintain and operate telephone and power transmission lines over Lessors remaining property to the Leased Premises for service of the tower and related facilities only, said lines to be located where feasible along the access road to the Leased Premises, with Lessor having input as to

location of said power transmission lines in the event Lessee changes the location of its access road. Lessee shall have the right to trim or remove trees, limbs or underbrush which interferes with its access road or power/telephone lines wherever such road and lines are located or may damage tower if they fall. Lessee shall maintain the existing road with gravel and needed repairs.

This Lease is made on the following terms and conditions:

- TERM OF LEASE. The term of this lease shall be for a period of five (5) years from
  the effective date with an allowance of an additional six (6) automatic renewals of five (5) year
  terms unless Lessee gives Lessor written notice at least sixty (60) days prior to expiration of then
  said Term that Lessee does not wish to renew.
- 2. CANCELLATION. Lessee shall have the right to terminate this Lease and abandon (subject to reclamation and removal of all structures erected thereon by Lessee) the Premises at any time under its sole discretion, upon six (6) month written notice to Lessor of its intention to do so. In the event that Leased Premises fail the process for approval as an acceptable cellular tower site by the Federal Communications Commission or any tests or requirements as required for such approval (the "FCC Process") or approval by the Public Service Commission of Kentucky (the "PSC"), or any other regulatory approval required, then in its sole discretion Lessee may terminate this Lease Agreement upon thirty (30) days written notice to Lessor of such intention. In the event of termination by Lessee, the Lessor shall have no obligation to refund all or any portion of the Leasehold rental payment that has been paid through the date of termination. Upon termination of this Lease, Lessee shall have one hundred eighty (180) days thereafter to remove all structures it has erected upon the Leased Premises, and to reclaim the premises. Payment shall continue until said structures are removed.
- 3. RENTAL. As rental for the Leased Premises, Lessee shall pay Lessor \$350.00 per month with an increase of 5% added to the rental payment every five years. Please see table below for scheduled rent payments.

<u>YEARS</u>	<u>AMOUNT</u>
1-5	\$350.00
6-10	367.50
11-15	385.88
16-20	405.17
21-25	425.43
26-30	446.70
31-35	469.04

4. USE OF PREMISES. Lessee shall have the exclusive rights and privileges of the use of the Leased Premises for the purpose of constructing a tower, buildings, and other related facilities, including, but not limited to telephone lines, coaxial lines, power lines and the installation of any and all other equipment deemed necessary by Lessee to receive and transmit any and all electronic signals in the rural service area now or hereafter to be served by the facility. The parties hereto recognize that technology in the communications field is advancing at a rapid rate and that this site may be used for any other purpose now in the development stage or which may later be developed in the communications industry to carry out the objectives of Lessee, that being to transmit and receive signals and communications by wire, fiber optics, radio and satellite. Lessee shall not use the Leased Premises for purposes other than maintenance or use as a site for communications by the use of methods now or hereafter known.

Lessee agrees to maintain the Leased Premises in a neat and orderly manner.

5. INDEMNITY. Lessee agrees to indemnify and save harmless the Lessor from any liability by virtue of Lessee's activities upon the Leased Premises or in the exercise of any rights and privileges granted herein, specifically including but not limited to any claim, loss, fine, penalty and costs (including reasonable attorney's fees) arising out of any violation of any environmental laws or regulations. This provision shall survive the termination of the lease. Lessee shall maintain and keep in full force and effect public liability and property damage insurance in an amount of at least One Million Dollars (\$1,000,000.00). Lessor shall not be held liable for personal injury or property damage on the Leased Premises whether or not associated with Lessee.

6. TAXES. Lessee shall pay all personal property taxes assessed on or any portion of such taxes attributable to the equipment used by Lessee on the Premises. Lessor shall pay when due all real property taxes and all other fees and assessments attributable to the Premises. Lessee shall reimburse the Lessor as additional compensation for any increase in real estate taxes levied against the Lessor (or its successors or assigns) which are directly attributable to or arise as a result of the improvements constructed by the Lessee, its successors or assigns. Payment will be issued after Lessee receives written proof from Lessor that the increase in real estate taxes levied is due to improvements constructed by the Lessee.

7. MISCELLANEOUS PROVISIONS. All notices, demands, or other writings in this Lease Agreement provided to be given, made or sent, or which may be given or made or sent, to either party hereto to the other, shall be deemed to have been fully given or made or sent when made in writing and deposited in the United States Mail, certified and postage prepaid, to Lessor and Lessee at the addresses stated in the caption of this Lease Agreement. Such addresses may be changed by written notice given by such party as above provided.

8. **SUCCESSORS AND ASSIGNS**. This Lease Agreement shall be binding upon the parties hereto, their heirs, executors, administrators and assigns.

WITNESS OUR HANDS, the day and year aforesaid.

LESSORS:

Mitchell Osborne, Church Trustee

Michael Epling, Church Trustee

Robert Falls, Church Trustee

Marcus Ely, Church Trustee Ron Johnson, Church Trustee Mancy Miney hun Nancy Moneyhun, Church Trustee Peggy Buncum, Church Trustee

Joseph VanSickle, Church Trustee

STATE OF KEN	NTUCKY
COUNTY OF_	Flord

The foregoing Lease Agreement was this /s day of \_\_\_\_\_\_\_, 2013, produced and acknowledged before me by Mitchell Osborne, Michael Epling, Robert Falls, Marcus Ely, Nancy Moneyhun, Peggy Duncum, Cindy Humphrey, Joyce Ely, Chris Hockenberry and Joseph VanSickle, Trustees of Benham United Methodist Church, Lessor.

NOTARY PUBLIC

COMMISSION EXPIRES: 7-14-15

COUNTY OF ROPHOKE

The foregoing Lease Agreement was this 24h day of October, 2013, produced and acknowledged before me by Ron Johnson, Trustee of Benham United Methodist Church, Lessor.

NOTARY PUBLIC

COMMISSION EXPIRES: March 31, 2014

LESSEE:

# EAST KENTUCKY NETWORK, LLC d/b/a APPALACHIAN WIRELESS

W. A. Gillum, General Manager

# STATE OF KENTUCKY COUNTY OF FLOYD

The foregoing Lease Agreement was this <u>31</u> day of <u>October</u>, 2013, produced and acknowledged before me by East Kentucky Network, LLC, dba Appalachian Wireless by W. A. Gillum, its General Manager, Lessee.

NOTARY PUBLIC

COMMISSION EXPIRES: 7-14-15

#### LOT DESCRIPTION

Property of Benham United Methodist Church P.O. Box 128 Benham, KY 40807

Maggard Br. Of Looney Creek in Harlan County Near end of McKnight Street September 8, 2013

A portion of the property lying within the tract of land located on Maggard Branch of Looney Creek in Harlan County Kentucky, in the City of Behnam. Being a part of the same land conveyed by deed from Ark Land Company to the Benham United Methodist Church by Deed dated July 18th, 1998 and recorded in Deed Book 335 Page 254 of the Harlan county Court Clerk.

Unless stated otherwise, any monument referred to herein as "set iron pin with cap" is a set ½" diarneter rebar, at least eighteen (18") in length, with a plastic cap stamped "LS-2259". All bearings stated herein are referred to a prior survey of the property. This survey preformed by James W. Caudill, LS2259, on September 8, 2013.

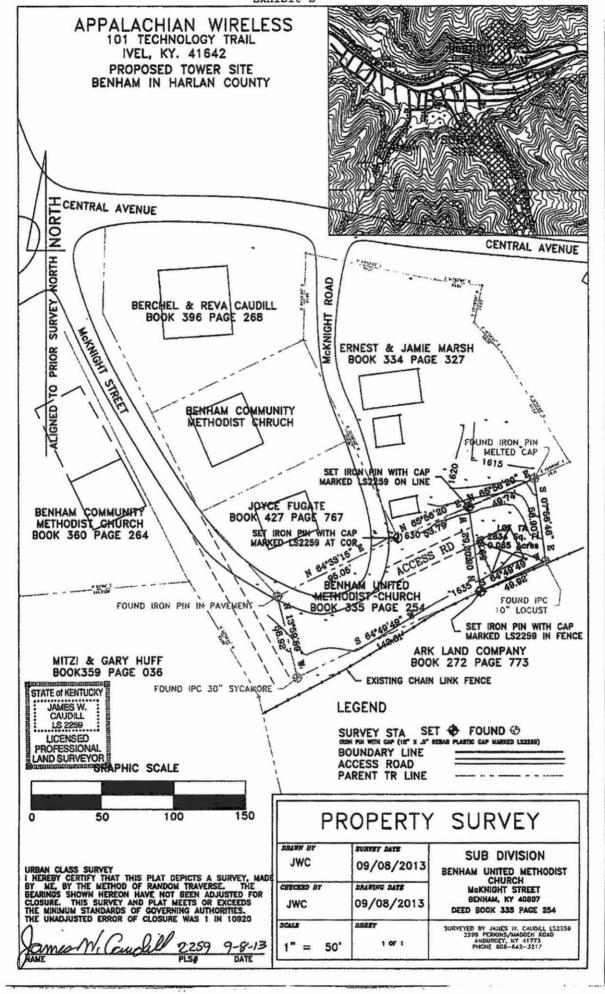
Lot 1A

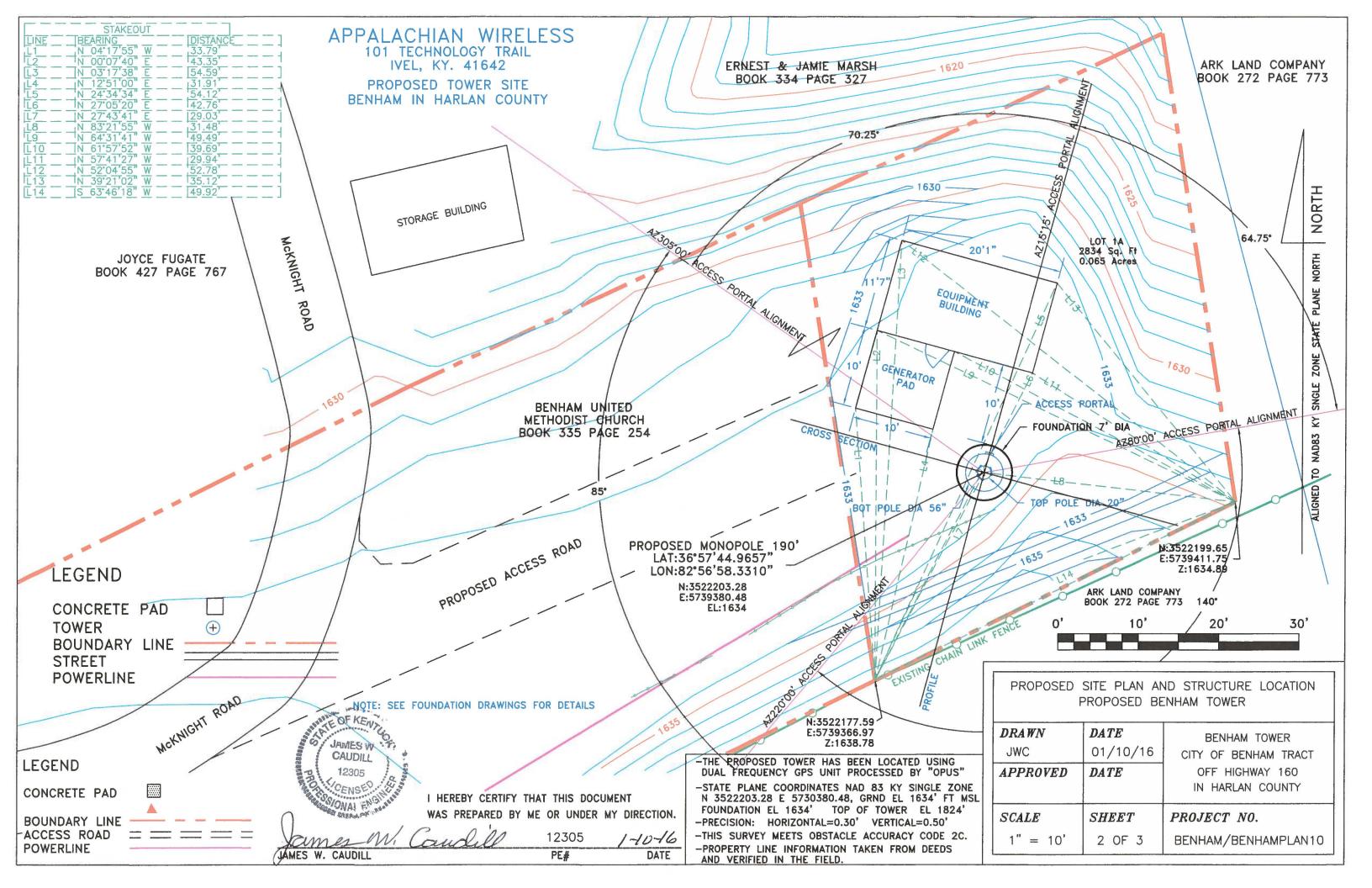
Beginning on a found iron pin with melted cap from original survey, being the 3<sup>rd</sup> corner of the original survey, and near Maggard Branch on the corner between Ernest & Jamie Marsh (book 334 page 327) and Ark Land Company (book 272 page 773); thence running with the dividing line between Ark Land Company and Benham United Methodist Church South 07 deg 56 min 46 sec East, 58.90 feet to a found iron pin with cap at base of 10" locust tree, being the forth corner of the original survey, thence continuing with the line of Ark Land Company South 64 deg 49 min 49 sec West, 49.92 feet to a set iron pin with cap marked ls2259 on the 4<sup>th</sup> line of the original survey; thence leaving the Ark Land company line and running across the lot severing the property of Benham United Methodist Church North 08 deg 02 min 52 sec West, 59.88 feet to a set iron pin with cap marked ls2259 on the 2<sup>nd</sup> line of the original survey and a point on the line of Ernest & Jamie Marsh; thence running with the line of Ernest & Jamie Marsh North 65 deg 56 min 20 sec East, 49.74 feet to the beginning. Containing a calculated area of 2834sq ft or 0.065 acres.

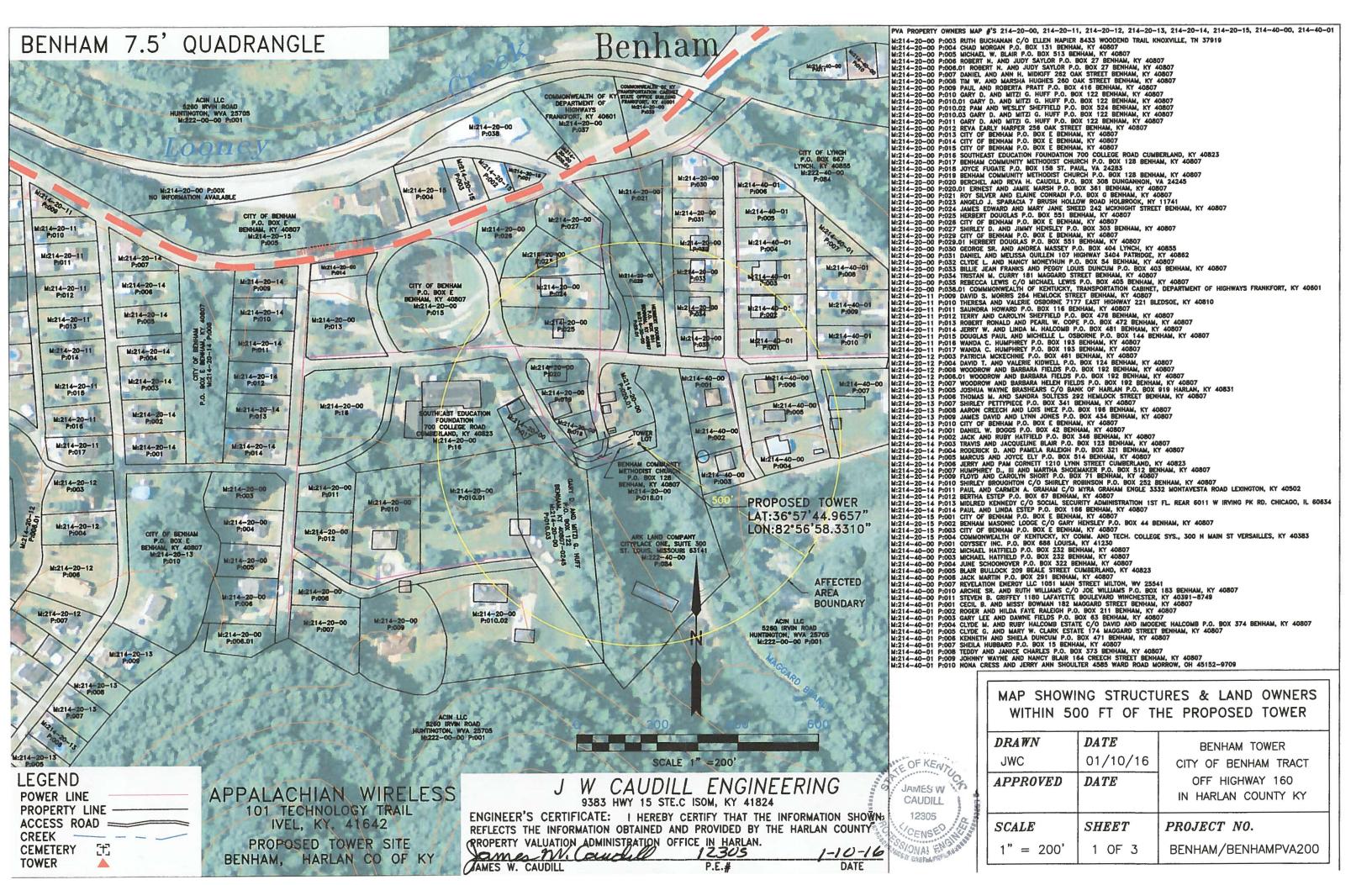
This survey was performed on September 8, 2013 by James W. Caudill, a Kentucky Licensed Professional Land Surveyor No. 2259.

STATE OF KENTUCKY HE JAMES W. GAUDILL HE LS 2259 LICENSED PROFESSIONAL LAND SURVEYOR HE

James W. Caudill, PLS #2259

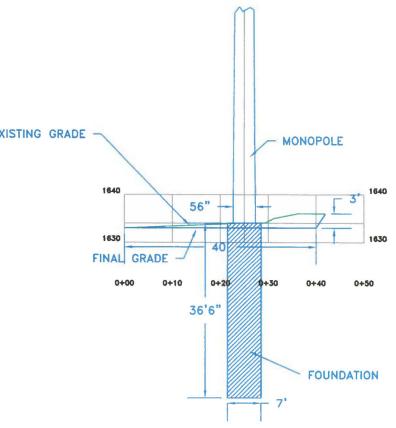






180' 170' 160' 145' 140' EXISTING GRADE -1640 120' 56" FINAL GRADE 0+00 0+10 100' 36'6" 80' 60' 40' 20' MONOPOLE BUILDING 56" BUILDING FOUNDATION EXISTING GRADE FINAL GRADE 1620 36'6" 0+20 0+00 49930 0+10 0+80 FOUNDATION 1" = 20'SITE PROFILE

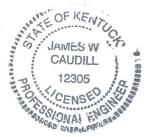
APPALACHIAN WIRELESS
101 TECHNOLOGY TRAIL IVEL, KY. 41642 PROPOSED TOWER SITE BENHAM IN HARLAN CO. KY.



SITE CROSS SECTION

NOTE: SEE FOUNDATION DRAWINGS FOR DETAILS

01/10/16 GRAPHIC SCALE 60



THIS IS A VERTICAL PROFILE SKETCH OF THE TOWER INDICATING THE PROPOSED ANTENNA AND DISH ELEVATIONS. NO DESIGN CRITERIA WAS CONSIDERED IN THE PREPARATION OF THIS DRAWING.

1-10-16 AMES W. CAUDILL DATE

PROPOSED	SITE	PLAN	AND	STF	RUCTURE	LOCATION
	PROP	OSED	BENH	MAI	TOWER	

DRAWN JWC APPROVED	<b>DATE</b> 01/10/16 <b>DATE</b>	BENHAM TOWER CITY OF BENHAM TRACT OFF HIGHWAY 160 IN HARLAN COUNTY
SCALE	SHEET	PROJECT NO.
1" = 20'	3 OF 3	BENHAM/BENHAMPROFILE