## REGEMED

## VIA HAND DELIVERY

JAN 28006

Kentucky Public Service Commission
PUBLIC SERVICE COLDMSBOH
Attn: Mr. Jeff Cline
211 Sower Blvd.
P.O. Box 615

Frankfort, KY 40602-0615
RE: Application to Construct Wireless Communications Facility
Location: 4625 Ogden Colvin Circle, Kevil, Kentucky 42053
Applicant: Cellco Partnership, d/b/a Verizon Wireless
Site Name: Monkey's Eyebrow
Case No.: 2006-00035
Dear Mr. Cline:
On behalf of our client, Cellco Partnership d/b/a, d/b/a Verizon Wireless, we are submitting the enclosed original and five (5) copies of an Application for Certificate of Public Convenience and Necessity for Construction of a Wireless Communications Facility in an area of Ballard County outside the jurisdiction of a planning commission. We have also enclosed two (2) additional copies of this cover letter. Thank you for your assistance and do not hesitate to contact us if you have any comments or questions concerning this matter.

Sincerely,


Enclosures

In the Matter of:

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THE APPLICATION OF
CELLCO PARTNERSHIP, D/B/A VERIZON WIRELESS
FOR ISSUANCE OF A CERTIFICATE OF PUBLIC
CASE NO.:2006-00035
CONVENIENCE AND NECESSITY TO CONSTRUCT
A WIRELESS COMMUNICATIONS FACILITY AT
4625 OGDEN COLVIN CIRCLE
KEVIL, KENTUCKY 42053
IN THE WIRELESS COMMUNICATIONS LICENSE AREA
IN THE COMMONWEALTH OF KENTUCKY
IN THE COUNTY OF BALLARD
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SITE NAME: MONKEY'S EYEBROW

## APPLICATION FOR

CERTIFICATE OF PUBLIC CONVENIENCE AND NECESSITY FOR CONSTRUCTION OF A WIRELESS COMMUNICATIONS FACILITY

Cellco Partnership, a Delaware General Partnership d/b/a, d/b/a Verizon Wireless ("Applicant"), by counsel, pursuant to (i) KRS $\S \S 278.020,278.040,278.665$ and the rules and regulations applicable thereto, and (ii) the Telecommunications Act of 1996, respectfully submits this Application requesting issuance of a Certificate of Public Convenience and Necessity ("CPCN") from the Kentucky Public Service Commission ("PSC") to construct, maintain, and operate a Wireless Communications Facility ("WCF") to serve the customers of the Applicant with wireless telecommunications services.

In support of this Application, Applicant respectfully provides and states the following information:

1. The complete name and address of the Applicant:

Cellco Partnership, d/b/a Verizon Wireless
180 Washington Valley Road

Bedminster, New Jersey, 07921
2. Applicant proposes construction of an antenna tower for cellular telecommunications services or personal communications services which is to be located in an area outside the jurisdiction of a planning commission, and Applicant submits the within application to the Commission for a certificate of public convenience and necessity pursuant to KRS $\S \S 278.020(1), 278.650$, and 278.665.
3. Applicant entity is not a corporation and, therefore, the requirements of 807 KAR 5:001(8) and 807 KAR 5:001(9) that applicant submit a certified copy of articles of incorporation is inapplicable.
4. The proposed WCF will serve an area completely within the Applicant's Federal Communications Commission ("FCC") licensed service area in the Commonwealth of Kentucky. A copy of the Applicant's FCC license to provide wireless services is attached to this Application or described as part of Exhibit A.
5. The public convenience and necessity require the construction of the proposed WCF. The construction of the WCF will bring or improve the Applicant's services to an area currently not served or not adequately served by the Applicant by increasing coverage and thereby enhancing the public's access to innovative and competitive wireless telecommunications services, including digital services. The WCF will provide a necessary link in the Applicant's telecommunications network that is designed to meet the increasing demands for wireless services in Kentucky's wireless communications licensed area. The WCF is an integral link in the Applicant's network design that must be in place to provide adequate coverage to the service area.
6. To address the above-described service needs, Applicant proposes to construct a WCF at 4625 Ogden Colvin Circle, Kevil, KY 42053 ( $37^{\circ} 10^{\prime} 55.43^{\prime \prime}$ North latitude, $88^{\circ} 56^{\prime} 43.75^{\prime \prime}$ West longitude), in an area located entirely within the county referenced in the caption of this application. The property on which the WCF will be located is owned by Billy Wayne Owsley pursuant to a Deed recorded at Deed Drawer 24, Page 48799 in the office of the Ballard County Clerk. The proposed WCF will consist of a 300 -foot tall tower. The WCF will also include concrete foundations to accommodate the placement of the Applicant's proprietary radio electronics equipment. The equipment will be housed in a prefabricated cabinet or shelter that will contain: (i) the transmitting and receiving equipment required to connect the WCF with the Applicant's users in Kentucky, (ii) telephone lines that will link the WCF with the Applicant's other facilities, (iii) battery back-up that will allow the Applicant to operate even after a loss of outside power, and (iv) all other necessary appurtenances. The Applicant's equipment cabinet or shelter will be approved for use in the Commonwealth of Kentucky by the relevant building inspector. The WCF compound will be fenced and all access gate(s) will be secured. A description of the manner in which the proposed WCF will be constructed is attached as Exhibit B and Exhibit C. Periodic inspections will be performed on the WCF in accordance with the applicable regulations or requirements of the PSC.
7. A list of competing utilities, corporations, or persons is attached as Exhibit D, along with a map showing the location of the proposed new construction as well as the location of any like facilities located anywhere within the map area, along with a map key showing the owner of such other facilities.
8. The site development plan and a vertical profile sketch of the WCF signed and sealed by a professional engineer registered in Kentucky depicting the tower height, as well as a proposed configuration for the antennas of the Applicant and future antenna mounts, has also been included as part of Exhibit B. Foundation design plans and a description of the standards according to which the tower was designed, and which have been signed and sealed by a professional engineer registered in Kentucky, are included as part of Exhibit C.
9. Applicant has considered the likely effects of the installation of the proposed WCF on nearby land uses and values and has concluded that there is no more suitable location reasonably available from which adequate services can be provided, and that there are no reasonably available opportunities to co-locate Applicant's antennas on an existing structure. Applicant has attempted to co-locate on suitable existing structures such as telecommunications towers or other suitable structures capable of supporting Applicant's facilities, and no other suitable or available co-location site was found to be located in the vicinity of the site.
10. FAA notice is required for the proposed construction, and lighting or marking requirements may be applicable to this facility. A copy of the Notice of Proposed Construction or Alteration filed by Applicant with the FAA is attached as Exhibit E. Upon receiving authorization from the FAA, the Applicant will forward a copy of the determination as a supplement to this Application proceeding.
11. A copy of the Kentucky Airport Zoning Commission ("KAZC") Application for the proposed WCF is attached as Exhibit F. Upon receiving authorization from the KAZC,
the Applicant will forward a copy of the determination as a supplement to this Application proceeding.
12. The WCF will be registered with the FCC pursuant to applicable federal requirements. Appropriate required FCC signage will be posted on the site upon receipt of the tower registration number.
13. A geotechnical engineering firm has performed soil boring(s) and subsequent geotechnical engineering studies at the WCF site. A copy of the geotechnical engineering report and evaluation, signed and sealed by a professional engineer registered in the Commonwealth of Kentucky, is attached as Exhibit G. The name and address of the geotechnical engineering firm and the professional engineer registered in the Commonwealth of Kentucky who supervised the examination of this WCF site are included as part of this exhibit.
14. Clear directions to the proposed WCF site from the County seat are included in Exhibit B. The name and telephone number of the preparer of Exhibit $\mathbf{B}$ is included as part of this exhibit.
15. Applicant, pursuant to a written agreement, has acquired the right to use the WCF site and associated property rights. A copy of the agreement or a redacted agreement recorded with the County Clerk is attached as Exhibit H . Also included as part of Exhibit $\mathbf{H}$ is the portion of the full agreement demonstrating that in the case of abandonment a method is provided to dismantle and remove the cellular antenna tower, including a timetable for removal.
16. Personnel directly responsible for the design and construction of the
proposed WCF are well qualified and experienced. FWT, Inc. ("Tower Manufacturer") performed the tower and foundation design. The tower and foundation drawings for the proposed tower submitted as part of Exhibit $C$ bear the signature and stamp of Richard $W$. Hoffman, a professional engineer registered in the Commonwealth of Kentucky. All tower designs meet or exceed applicable laws and regulations.
17. The proposed facility will be constructed under the supervision of Applicant Cellco Partnership, and the identity and qualifications of each person directly responsible for design of the proposed tower are contained in Exhibit C.
18. Based on a review of Federal Emergency Management Agency Flood Insurance Rate Maps, the registered land surveyor has noted in Exhibit B that the proposed WCF is not located within any flood hazard area.
19. The possibility of high winds has been considered in the design of this tower. The tower has been designed and engineered by professional engineers using computer assistance and the same accepted codes and standards as are typically used for high-rise building construction. The tower design is in accordance with EIA/TIA-222-F standards.
20. The site development plan signed and sealed by a professional engineer registered in Kentucky was prepared by Woodrow W. Marcum, Jr. The site survey was performed by Frank L. Sellinger, II. Page C-1 of Exhibit B is drawn to a scale of no less than one (1) inch equals 200 feet, and identifies every owner of real estate within 500 feet of the proposed tower (according to the records maintained by the County Property Valuation Administrator). Every structure and every easement within 500 feet of the proposed tower or within 200 feet of the access road including intersection with the public
street system is illustrated in Exhibit B.
21. Applicant has notified every person who, according to the records of the County Property Valuation Administrator, owns property which is within 500 feet of the proposed tower or contiguous to the site property, by certified mail, return receipt requested, of the proposed construction. Each notified property owner has been given the docket number under which the proposed Application will be processed and has been informed of their right to request intervention. A list of the nearby property owners who received the notices, together with copies of the certified letters, are attached as Exhibit I and Exhibit J, respectively.
22. Applicant has notified the Ballard County Judge/Executive by certified mail, return receipt requested, of the proposed construction. This notice included the PSC docket number under which the application will be processed and informed the Ballard County Judge/Executive of his/her right to request intervention. A copy of this notice is attached as Exhibit K.
23. Two notice signs meeting the requirements prescribed by 807 KAR 5:063 measuring at least two (2) feet in height and four (4) feet in width with all required language in letters of required height have been posted in a visible location on the proposed site and on the nearest public road. Such signs shall remain posted for at least two (2) weeks after filing of the Application, and a copy of the posted text is attached as Exhibit L. Notice of the location of the proposed facility has also been published in a newspaper of general circulation in the county where the WCF is located.
24. The general area where the proposed facility is to be located is rural
farmland. There are no residential structures located within a 500 -foot radius of the proposed tower location.
25. The process that was used by the Applicant's radio frequency engineers in selecting the site for the proposed WCF was consistent with the general process used for selecting all other existing and proposed WCF facilities within the proposed network design area. Applicant's radio frequency engineers have conducted studies and tests in order to develop a highly efficient network that is designed to serve the Federal Communications Commission licensed service area. The engineers determined an optimum area for the placement of the proposed facility in terms of elevation and location to provide the best quality service to customers in the service area. A radio frequency design search area prepared in reference to these radio frequency studies was considered by the Applicant when searching for sites for its antennas that would provide the coverage deemed necessary by the Applicant. Before beginning the site acquisition process, Applicant carefully evaluated locations within the search area for co-location opportunities on existing structures, and no suitable towers or other existing tall structures were found in the immediate area that would meet the technical requirements for the element of the telecommunications network to be provided by the proposed facility. A map of the area in which the tower is proposed to be located which is drawn to scale and clearly depicts the necessary search area within which the site should be located pursuant to radio frequency requirements is attached as Exhibit M.
26. All Exhibits to this Application are hereby incorporated by reference as if fully set out as part of the Application.
27. All responses and requests associated with this Application may be directed
to:

David A. Pike<br>Pike Legal Group, PLLC<br>1578 Highway 44 East, Suite 6<br>P. O. Box 369<br>Shepherdsville, KY 40165-0369<br>Telephone: (502) 955-4400<br>Telefax: (502) 543-4410

WHEREFORE, Applicant respectfully request that the PSC accept the foregoing Application for filing, and having met the requirements of KRS $\S \S 278.020(1), 278.650$, and 278.665 and all applicable rules and regulations of the PSC, grant a Certificate of Public Convenience and Necessity to construct and operate the WCF at the location set forth herein.

Respectfully submitted,


David A. Pike Pike Legal Group, PLLC 1578 Highway 44 East, Suite 6 P. O. Box 369

Shepherdsville, KY 40165-0369
Telephone: (502) 955-4400
Telefax: (502) 543-4410
Attorney for Cello Partnership d/b/a Verizon Wireless

## LIST OF EXHIBITS

A - FCC License Documentation
B - Site Development Plan:
500' Vicinity Map
Legal Descriptions
Flood Plain Certification
Site Plan
Vertical Tower Profile
C - Tower and Foundation Design
D - Competing Utilities, Corporations, or Persons Listand Map of Like Facilities in Vicinity
E - Application to FAA
F - Application to Kentucky Airport Zoning Commission
G - Geotechnical Report
H - Copy of Real Estate Agreement
I - Notification Listing
J - Copy of Property Owner Notification
K - Copy of County Judge/Executive Notice
L - Copy of Posted Notices
M - Copy of Radio Frequency Design Search Area

EXHIBIT A

## Federal Communications Commission Wireless Telecommunications Bureau

Radio Station Authorization (Reference Copy)

This is not an official FCC license. It is a record of public information contained in the FCC's licensing database on the date that this reference copy was generated. In cases where FCC rules require the presentation, posting, or display of an FCC license, this document may not be used in place of an official FCC license.


| Grant Date <br> $11 / 07 / 2000$ | Effective Date <br> $08 / 27 / 2003$ | Expiration Date <br> $10 / 01 / 2010$ | Five Yr Build-Out <br> Date <br> $03 / 25 / 1996$ | Print Date <br> $10 / 20 / 2005$ |
| :---: | :---: | :---: | :---: | :---: |

Site Information

| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | $36-49-50.2$ <br> N | $089-58-20.3 \mathrm{~W}$ |  |  |  |  |
| Address |  |  | City | County | State | Construction Deadline |
| COUNTY ROAD 415, 1.5 MILES N OF |  |  |  |  |  |  |
| HWY. 60 | DEXTER | STODDARD | MO |  |  |  |


| Antenna: 1 Azimuth (degrees from <br> true north) | $\mathbf{0}^{\circ}$ | $\mathbf{4 5}$ | $\mathbf{9 0 ^ { \circ }}$ | $\mathbf{1 3 5}$ | $\mathbf{1 8 0}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0}^{\circ}$ | $\mathbf{3 1 5}^{\circ}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 130.6 | 112.7 | 158.5 | 163.2 | 146.2 | 151.8 | 149.9 | 139.7 |
| Transmitting ERP (watts) | 144.000 | 144.000 | 144.000 | 144.000 | 144.000 | 144.000 | 144.000 | 144.000 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | $36-45-46.2$ <br> N | $090-26-03.4 \mathrm{~W}$ | 130.0 |  |  |
| Address |  | City | County | State | Construction Deadline |
| 2.33 MILES WEST OF |  | POPLAR BLUFF | BUTLER | MO |  |


| Antenna: 1 Azimuth (degrees from true north) | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 141.6 | 150.0 | 167.9 | 165.3 | 169.8 | 148.3 | 150.6 | 122.7 |
| Transmitting ERP (watts) | 127.400 | 126.300 | 124.500 | 168.000 | 55.600 | 27.500 | 38.000 | 40.700 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3 | $36-21-01.2$ <br> N | $089-49-54.3 \mathrm{~W}$ |  |  |  |  |
| Address |  |  |  |  |  |  |
| City |  |  |  |  |  |  |
| 0.8 MILES WEST OF | County | State | Construction Deadline |  |  |  |


| Antenna: 1 Azimuth (degrees from true <br> north) | $0^{\circ}$ | $\mathbf{4 5}$ | $\mathbf{9 0 ^ { \circ }}$ | $\mathbf{1 3 5}$ | $\mathbf{1 8 0}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0}^{\circ}$ | $\mathbf{3 1 5}^{\circ}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 54.7 | 52.9 | 53.1 | 53.9 | 57.3 | 57.8 | 56.2 | 55.0 |
| Transmitting ERP (watts) | 140.100 | 133.800 | 47.500 | 30.000 | 119.300 | 172.400 | 38.600 | 54.500 |
| Antenna: 2 Azimuth (degrees from true <br> north) | $0^{\circ}$ | $45^{\circ}$ | $\mathbf{9 0}$ | $135^{\circ}$ | $180^{\circ}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0}^{\circ}$ | $\mathbf{3 1 5 ^ { \circ }}$ |
| Antenna Height AAT (meters) | 48.9 | 47.1 | 47.3 | 48.1 | 51.5 | 52.0 | 50.4 | 49.3 |
| Transmitting ERP (watts) | 113.900 | 189.000 | 32.100 | 60.900 | 116.500 | 158.600 | 70.200 | 27.300 |


| Location | Latitude | Longitude | Ground Elevation (meters) | Structure Hgt to Tip (meters) |  | Antenna Structure Registration No. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 4 | $\begin{gathered} 36-12-53.2 \\ \mathrm{~N} \end{gathered}$ | 090-03-50.3 W |  |  |  |  |
| Address |  |  | City | County | State | Construction Deadline |
| East side of County Road $5041 / 2$ mile South of |  |  | Kennett | DUNKLIN | MO |  |


| Antenna: 1 Azimuth (degrees from true <br> north) | $\mathbf{0}^{\boldsymbol{o}}$ | $\mathbf{4 5}$ | $\mathbf{9 0}^{\boldsymbol{\circ}}$ | $\mathbf{1 3 5}$ | $\mathbf{1 8 0 ^ { \circ }}$ | $\mathbf{2 2 5}^{\boldsymbol{\circ}}$ | $\mathbf{2 7 0}^{\circ}$ | $\mathbf{3 1 5}^{\boldsymbol{\circ}}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 34.3 | 35.8 | 37.7 | 39.4 | 39.3 | 36.4 | 36.7 | 34.7 |
| Transmitting ERP (watts) | 32.300 | 227.300 | 267.600 | 206.100 | 265.600 | 181.800 | 19.200 | 10.300 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5 | $37-12-06.2$ <br> N | $089-38-07.3 \mathrm{~W}$ | 480.0 |  |  |  |
| Address |  |  |  |  |  |  |
| 0.4 miles east of Route M at Rockview, 1.6 |  |  |  |  |  |  |
| miles NE of | Chaffee | SCOTT | MO |  |  |  |


| Antenna: 1 Azimuth (degrees from true north) | $0^{\circ}$ | $\mathbf{4 5}$ | $\mathbf{9 0}$ | $\mathbf{1 3 5}$ | $\mathbf{1 8 0 ^ { \circ }}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0 ^ { \circ }}$ | $\mathbf{3 1 5 ^ { \circ }}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 97.6 | 107.6 | 96.4 | 89.0 | 85.7 | 114.4 | 102.3 | 90.5 |
| Transmitting ERP (watts) | 24.300 | 2.800 | 3.300 | 27.800 | 86.400 | 95.300 | 95.200 | 76.900 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |
| :---: | :---: | :---: | :---: | :---: | :---: |


| 6 | $\underset{N}{36-32-33.2}$ | 090-01-49.3 W |  | 88.0 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Address |  |  | City |  |  | County | State | Construction Deadline |  |  |
| 150 ' West of end of County Rd. 2083.2 miles Southwest of |  |  | Malden |  | DUNKLIN |  | MO |  |  |  |
| Antenna: 1 Azimuth (degrees from true north) |  |  | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225{ }^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
| Antenna Height AAT (meters) |  |  | 68.5 | 71.4 | 73.0 | 73.6 | 73.1 | 63.7 | 58.7 | 56.5 |
| Transmitting ERP (watts) |  |  | 163.000 | 160.000 | 162.000 | 110.000 | 49.000 | 38.000 | 49.000 | 116.000 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 7 | $36-57-05.2$ <br> N | $089-04-53.2 \mathrm{~W}$ | 137.2 |  |  |  |
| Address |  | City | County | State | Construction Deadline |  |
| Approx. 1 mile SSE of |  |  |  |  |  |  |


| Antenna: 1 Azimuth (degrees from true north) | $0^{\circ}$ | $\mathbf{4 5}$ | $\mathbf{9 0}$ | $\mathbf{9}$ | $\mathbf{1 3 5}$ | $\mathbf{1 8 0}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0}^{\circ}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 315 |  |  |  |  |  |  |  |  |
| Antenna Height AAT (meters) | 69.8 | 44.2 | 51.5 | 60.0 | 45.7 | 78.6 | 77.7 | 79.6 |
| Transmitting ERP (watts) | 0.500 | 33.000 | 283.800 | 425.600 | 77.600 | 2.300 | 0.400 | 1.200 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 8 | $36-10-08.2$ <br> N | $089-38-52.3 \mathrm{~W}$ | 82.0 |  |  |
| Address |  | City | County | State | Construction Deadline |
| $600^{\prime}$ West of end of Route $363,0.6$ miles | Caruthersville | PEMISCOT | MO |  |  |


| Antenna: 1 Azimuth (degrees from true north) | $0^{\circ}$ | $\mathbf{4 5}$ | $\mathbf{9 0}$ | $\mathbf{1 3 5}^{\circ}$ | $\mathbf{1 8 0 ^ { \circ }}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0}^{\circ}$ | $\mathbf{3 1 5}^{\circ}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 43.1 | 43.3 | 43.5 | 44.0 | 45.6 | 44.0 | 44.2 | 41.9 |
| Transmitting ERP (watts) | 38.000 | 9.000 | 2.000 | 3.000 | 23.000 | 56.000 | 57.000 | 57.000 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) | Antenna Structure <br> Registration No. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 9 | $36-38-57.2$ <br> N | $089-32-59.3 \mathrm{~W}$ | 91.0 |  |  |  |
| Address |  |  |  |  |  |  |
| Southwest corner of intersection of US Hwy. <br> $61 / 62$ and, County Rd. 634, north of | New Madrid | NEW <br> MADRID | MO |  |  |  |


| Antenna: 1 Azimuth (degrees from true <br> north) | $\mathbf{0}^{\circ}$ | $\mathbf{4 5}$ | $\mathbf{9 0 ^ { \circ }}$ | $\mathbf{1 3 5}$ | $\mathbf{1 8 0 ^ { \circ }}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0}^{\circ}$ | $\mathbf{3 1 5}^{\circ}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 60.2 | 63.7 | 65.0 | 65.4 | 69.0 | 67.4 | 68.2 | 66.8 |
| Transmitting ERP (watts) | 331.000 | 54.000 | 12.000 | 22.000 | 151.000 | 349.000 | 266.000 | 311.000 |


| Location | Latitude | Longitude | Ground Elevation | Structure Hgt to Tip |
| :--- | :--- | :--- | :--- | :--- | Antenna Structure


|  |  |  | (meters) | (meters) | Registration No. |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 10 | $36-55-17.2$ <br> N | $089-29-57.3 \mathrm{~W}$ |  |  |  |
| Address |  |  | City | County | State |
| 3.3 MILES NE OF |  |  | SIKESTON | SCOTT | MO |


| Antenna: 1 Azimuth (degrees from true north) | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 64.0 | 65.0 | 65.0 | 66.0 | 69.0 | 67.0 | 65.0 | 65.0 |
| Transmitting ERP (watts) | 414.000 | 50.000 | 3.000 | 0.800 | 1.000 | 0.800 | 13.000 | 156.000 |
| Antenna: 2 Azimuth (degrees from true north) | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
| Antenna Height AAT (meters) | 64.0 | 65.0 | 65.0 | 66.0 | 69.0 | 67.0 | 65.0 | 65.0 |
| Transmitting ERP (watts) | 0.700 | 16.000 | 196.000 | 372.000 | 36.000 | 2.000 | 0.700 | 0.800 |
| Antenna: 3 Azimuth (degrees from true north) | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225{ }^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
| Antenna Height AAT (meters) | 64.0 | 65.0 | 65.0 | 66.0 | 69.0 | 67.0 | 65.0 | 65.0 |
| Transmitting ERP (watts) | 0.700 | 1.000 | 0.700 | 2.000 | 37.000 | 364.000 | 223.000 | 14.000 |


| Location | Latitude | Longitude | Ground Elevation (meters) | Structure Hgt to Tip (meters) |  | Antenna Structure Registration No. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 11 | $\begin{gathered} 37-12-25.5 \\ \mathrm{~N} \end{gathered}$ | 089-30-44.0 W | 128.6 | 50.3 |  | 1200145 |
| Address |  |  | City | County | State | Construction Deadline |
| County Road 312 |  |  | Scott City | SCOTT | MO |  |


| Antenna: 1 Azimuth (degrees from true north) | $\mathbf{0}^{\circ}$ | $\mathbf{4 5}$ | $\mathbf{9 0 ^ { \circ }}$ | $\mathbf{1 3 5}$ | $\mathbf{1 8 0 ^ { \circ }}$ | $\mathbf{2 2 5}$ | $\mathbf{2 7 0 ^ { \circ }}$ | $\mathbf{3 1 5}$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 66.2 | 59.1 | 39.9 | 67.8 | 52.3 | 50.5 | 65.7 | 59.6 |
| Transmitting ERP (watts) | 21.800 | 5.200 | 16.200 | 80.900 | 97.700 | 88.900 | 100.000 | 84.700 |


| Location | Latitude | Longitude | Ground Elevation <br> (meters) | Structure Hgt to Tip <br> (meters) |  | Antenna Structure <br> Registration No. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12 | $36-45-47.0$ <br> N | $090-26-05.2 \mathrm{~W}$ | 122.8 | 143.2 |  | 1229586 |
| Address |  |  |  |  |  |  |


| Antenna: 1 Azimuth (degrees from true north) | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Antenna Height AAT (meters) | 133.2 | 142.3 | 160.4 | 157.8 | 162.4 | 140.3 | 122.9 | 115.5 |
| Transmitting ERP (watts) | 150.000 | 109.420 | 29.180 | 3.680 | 0.890 | 3.110 | 27.360 | 112.740 |
| Antenna: 2 Azimuth (degrees from true north) | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225{ }^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
| Antenna Height AAT (meters) | 133.2 | 142.3 | 160.4 | 157.8 | 162.4 | 140.3 | 122.9 | 115.5 |
| Transmitting ERP (watts) | 6.590 | 50.710 | 132.770 | 139.990 | 80.370 | 15.140 | 1.120 | 0.480 |
| Antenna: 3 Azimuth (degrees from true north) | $0^{\circ}$ | $45^{\circ}$ | $90^{\circ}$ | $135^{\circ}$ | $180^{\circ}$ | $225{ }^{\circ}$ | $270^{\circ}$ | $315^{\circ}$ |
|  |  |  |  |  |  |  |  |  |


| Antenna Height AAT (meters) | 133.2 | 142.3 | 160.4 | 157.8 | 162.4 | 140.3 | 122.9 | 115.5 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Transmitting ERP (watts) | 16.500 | 0.310 | 0.300 | 10.170 | 68.980 | 31.590 | 28.500 | 70.890 |

Control Points

| Control Point <br> No. | Address | City | County | State | Telephone Number |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1Verizon Wireless-NOC; 180 <br> Washington Valley Rd. | Bedminster |  | NJ | $(800) 852-2671$ |

## Waivers/Conditions

None

## Conditions

Pursuant to Section 309(h) of the Communications Act of 1934, as amended, 47 U.S.C. Section 309(h), this license is subject to the following conditions: This license shall not vest in the licensee any right to operate the station nor any right in the use of the frequencies designated in the license beyond the term thereof nor in any other manner than authorized herein. Neither the license nor the right granted thereunder shall be assigned or otherwise transferred in violation of the Communications Act of 1934, as amended. See 47 U.S.C. Section 310(d). This license is subject in terms to the right of use or control conferred by Section 706 of the Communications Act of 1934, as amended. See 47 U.S.C. Section 706.

FCC 601 - C
August 2002

## EXHIBIT B

## SITE DEVELOPMENT PLAN:

500' VICINITY MAP<br>LEGAL DESCRIPTIONS<br>FLOOD PLAIN CERTIFICATION SITE PLAN<br>VERTICAL TOWER PROFILE

## MONKEY'S EYEBROW

## CELLCO PARTNERSHIP

D/B/A VERIZON WIRELESS



SITE PLAN NOTES

1. THE PROPOSED DEVELOPMENT IS FOR A 285 FOOT ITS LOCATION IS AT 4625 OGDEN COLVIN CIRCLIE, KEVIII KY 42053
2. THE TOWER WLL BE ACCESSED BY A PROPOSED STABILIZED CIR.) A PUBLIC RIGHT OF WAY. THE ACCESS ROAD IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE LOCAL HIGHWAY DEPARTMENT OF TRANSPORTATION STANDARDS. WATER, SANITARY SEWER, AND WASTE COLLECTIONS SERVICES ARE NOT REQUIRED
FOR THE PROPOSED DEVELOPMENT.
3. CENTERLINE OF PROPOSED TOWER GEOGRAPHIC LOCATIONS: LATITUDE: $37^{\circ} \cdot 10^{\prime} 55.43^{\prime \prime}$ N, NORTHING: 1965173.1687
LONGITUDE: $88^{\circ} 56^{\prime} 43.75^{\prime \prime}$ W, EASTING: 709687.4396
remove all vegetation \& clean area in lease area (where 4. REMOVE
4. FINISH GRADING TO PROVIDE EFFECTIVE DRAINAGE WITH A SLOPE O NO LESS THAN TO PROVIDE EFFECTIVE DRAINAGE WITH A SL AWAY FROM EQUIP. FOR A MIN. DISTANCE OF SIX FEET (G') IN ALL DIRECTIONS.
5. LOCATE ALL U.G. UTLLITIES PRIOR TO ANY CONSTRUCTION
6. COMPOUND FINISHED SURFACE TO BE FENCED

``` BEFORE YOU DIG
```




## LEGEND

| $\overline{-U E}_{-\mathrm{T}--\mathrm{EE}-\mathrm{C}}^{\mathrm{E}}$ | Existng overhead electric EXISTNG OVERHEAD TEIEPHONE |
| :---: | :---: |
|  |  |
|  | EXISTNG UNDERGROUND ELECTRIC |
|  | PRoposed underground electa |
| UT- | PROPOSED UNDERGROUND TELEPHON |
|  | FENCE LINE POWER POLE |
| ${ }^{1 T L E}$ | POWER POLE TEEEPHONE PEDESTAL |
| 20 | water valves |
| \% | FIRE Hydrants |
|  | bollards |









## FWT

5750 EAST 120 FORT WORTH, TEXAS 76119 $\mathrm{PH}:$ (817) 255-3060 FAX: (817) 255-2957
Tower $\frac{285 \text { FT SELF SUPPORT }}{\text { Location MONKEY'S EYEBROW, KENTUCKY }} \frac{\text { Design } \frac{\text { MPH } / 65 \mathrm{MPH}+1 / 2 " \text { RADIAL ICE }}{75 \mathrm{Mc}}}{\text { According to ANSI/EIA 222-F } 1996}$

WfPAUL J. FORD AND COMPANY STRUCTURAL ENGINEERS 250 East Broad Street, Suite 1500, Columbus, Ohio 43215 (614) 221-6679 Fax: (614) 448-4105 www.PJFweb.com

Page $\qquad$ Of 3

By $\qquad$ Date 11-11-2005

Job No. $\qquad$ 1905-029

Revision No. Date $\qquad$


5750 EAST I20 FORT WORTH, TEXAS 76119 PH: (817) 255-3060 FAX: (817) 255-2957
Tower 285 FT SEL.F SUPPORT Location MONKEY'S EYEBROW, KENTUCKY Design $75 \mathrm{MPH} / 65 \mathrm{MPH}+1 / 2^{\prime \prime}$ RADIAL ICE According to ANSI/EIA 222-F 1996

UfPAUL J. FORD AND COMPANY STRUCTURAL ENGINEERS 250 East Broad Street, Suite 1500, Columbus, Ohio 43215 (614) 221-6679 Fax: (614) 448-4105 www.PJFweb.com

| Page | 3 | Of | 3 |
| :---: | :---: | :---: | :---: |
| By | CMM | Dat | 11-11-2005 |
| Job N | 1905-029 |  |  |
| Revisi |  | Dat |  |



NOTES:

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF AT LEAST 3000 PSI AT 28 DAYS.
2. REINFORCING STEEL SHALL CONFORM TO ASTM A615 (GRADE 60) EXCEPT PIER TIES MAY BE ASTM A615 (GRADE 40).
3. CONTRACTOR SHALL CONTACT FWT FOR ANCHOR BOLT SIZE, EMBEDMENT DEPTH AND ORIENTATION.
4. TOTAL CONCRETE $=130$ CUBIC YARDS.
5. FOUNDATION DESIGN BASED UPON GEOTECHNICAL REPORT \#05-3142 BY FSTAN DATED MARCH 23, 2005.

Paul J. Ford and Company
250 East Broad Street, Suite 1500
Columbus, OH 43215
Phone: 614-221-6679
FAX: 614-448-4105

| Job | Nage |  |
| :--- | :--- | :--- |
|  | New 285 Self-Supporting Tower | 1 of 24 |
| Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | Date |
| 14:56:06 11/10/05 |  |  |
| Client | FWT, Inc. | Designed by <br> Craig Meierhoffer |

## Tower Input Data

The main tower is a 3 x free standing tower with an overall height of 285.00 ft above the ground line.
The base of the tower is set at an elevation of 0.00 ft above the ground line.
The face width of the tower is 4.0 ft at the top and 28.0 ft at the base.
This tower is designed using the TIA/EIA-222-F standard.
The following design criteria apply:
Tower is located in Ballard County, Kentucky.
Basic wind speed of 75.00 mph .
Nominal ice thickness of 0.50 in .
Ice density of 56 pcf.
A wind speed of 64.95 mph is used in combination with ice.
Deflections calculated using a wind speed of 50.00 mph .
A non-linear (P-delta) analysis was used.
Pressures are calculated at each section.
Stress ratio used in tower member design is 1.333.
Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.


Triangular Tower

| New 285 Self-Supporting Tower | 2 of 24 |
| :--- | :--- |
| Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) |

Tower Section Geometry

| Tower Section | Tower Elevation | Assembly <br> Database | Description | Section Width | Number of Sections | Section Length |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ft |  |  | ft |  | $f t$ |
| T1 | 285.00-280.00 |  |  | 4.0 | 1 | 500 |
| 12 | 280.00-260.00 |  |  | 4.0 | 1 | 20.00 |
| T3 | 260.00-240.00 |  |  | 4.0 | 1 | 20.00 |
| T4 | 240.00-220.00 |  |  | 4.0 | 1 | 20.00 |
| T5 | 220.00-200.00 |  |  | 6.0 | 1 | 20.00 |
| T6 | 200.00-180.00 |  |  | 8.0 | 1 | 20.00 |
| 17 | 180.00-160.00 |  |  | 10.0 | 1 | 20.00 |
| T8 | 160.00-140.00 |  |  | 12.0 | 1 | 20.00 |
| T9 | 140.00-120.00 |  |  | 14.0 | 1 | 20.00 |
| T10 | 120.00-100.00 |  |  | 16.0 | 1 | 20.00 |
| T11 | 100.00-80.00 |  |  | 18.0 | 1 | 20.00 |
| T12 | 80.00-60.00 |  |  | 20.0 | 1 | 20.00 |
| T1.3 | 60.00-40.00 |  |  | 22.0 | 1 | 20.00 |
| T14 | 40.00-20.00 |  |  | 24.0 | 1 | 2000 |
| T15 | 20.00-0.00 |  |  | 26.0 | 1 | 20.00 |

Tower Section Geometry (cont'd)

| Tower <br> Section | Tower Elevation <br> fit | Diagonal Spacing | Bracing Type | Has KBrace End Panels | Has Horizontals | Top Girt Offset in | Bottom Girt Offset in |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | $28500-280.00$ | 5.0 | X Brace | No | No | 0.00 | 0.00 |
| T2 | 280.00-260.00 | 4.0 | X Brace | No | No | 0.00 | 0.00 |
| T3 | 260.00-240.00 | 50 | X Brace | No | No | 0.00 | 0.00 |
| T4 | 240.00-220.00 | 50 | X Brace | No | No | 0.00 | 000 |
| T5 | 220.00-200.00 | 50 | X Brace | No | Yes | 0.00 | 000 |
| T6 | 200.00-180.00 | 50 | Double K | No | Yes | 0.00 | 0.00 |
| T7 | 180.00-160.00 | 50 | Double K | No | Yes | 0.00 | 0.00 |
| T8 | 160.00-140.00 | 50 | Double K | No | Yes | 0.00 | 0.00 |
| T9 | 140.00-120.00 | 50 | Double K | No | Yes | 0.00 | 0.00 |
| T10 | 120.00-100.00 | 5.0 | Double K | No | Yes | 0.00 | 000 |
| T11 | 100.00-80.00 | 50 | Double K | No | Yes | 0.00 | 0.00 |
| T12 | 80.00-60.00 | 50 | Double K | No | Yes | 0.00 | 000 |
| T13 | 60.00-40.00 | 100 | Double K1 | No | Yes | 0.00 | 000 |
| T14 | 40.00-20.00 | 100 | Double Kl | No | Yes | 0.00 | 000 |
| T15 | 20.00-0.00 | 10.0 | Double K1 | No | Yes | 0.00 | 0.00 |


| Tower Section Geometry (cont'd) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Tower | Leg | Leg | Leg | Diagonal | Diagonal | Diagonal |
| Elevation ft | Type | Size | Grade | Type | Size | Grade |
| $\begin{gathered} \mathrm{T} 1285.00- \\ 280.00 \end{gathered}$ | Solid Round | 2" solid | $\begin{aligned} & \text { A572-50 } \\ & (50 \mathrm{ksi}) \end{aligned}$ | Single Angle | L. $1.75 \times 1.75 \times 3 / 16$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \mathrm{T} 2280.00 \\ 260.00 \end{gathered}$ | Solid Round | 2" solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Single Angle | L. $1.75 \times 1.75 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T3 260.00- | Solid Round | 21/2" solid | A572-50 | Single Angle | L $1.75 \times 1.75 \times 3 / 16$ | A36 |


| Nob | Page 285 Self-Supporting Tower | 3 of 24 |
| :--- | :--- | :--- |
| Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | Date |
| 14:56:06 11/10/05 |  |  |
| Client | FWT, Inc. | Designed by |
|  | Craig Meierhoffer |  |

Phone: 614-221-6679 FAX: 614-448-4105

| Tower Elevation $f t$ | Leg Type | Leg <br> Size | Leg Grade | Diagonal Type | Diagonal Size | Diagonal Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 240.00 |  |  | (50 ksi) |  |  | (36 ksi) |
| $\begin{gathered} \text { T4 } 240.00 \\ 220.00 \end{gathered}$ | Solid Round | $23 / 4^{\prime \prime}$ solid | $\begin{aligned} & \text { A572-50 } \\ & (50 \mathrm{ksi}) \end{aligned}$ | Single Angle | L $1.75 \times 1.75 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T5 } 220.00- \\ 200.00 \end{gathered}$ | Solid Round | 3 " solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $1.75 \times 1.75 \times 3 / 16$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T6 } 200.00- \\ 180.00 \end{gathered}$ | Solid Round | 3 l solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $2.5 \times 2.5 \times 3 / 16$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T7 } 180.00- \\ 160.00 \end{gathered}$ | Solid Round | $31 / 4$ solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $2.5 \times 2.5 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T8 } 160.00- \\ 140.00 \end{gathered}$ | Solid Round | 31/4" solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Single Angle | L. $2.5 \times 2.5 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T9 } 140.00- \\ 120.00 \end{gathered}$ | Solid Round | $31 / 2^{\prime \prime}$ solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $3 \times 3 \times 3 / 16$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \mathrm{T} 1012000- \\ 100.00 \end{gathered}$ | Solid Round | $31 / 2^{\prime \prime}$ solid | $\begin{aligned} & \text { A572-50 } \\ & (50 \mathrm{ksi}) \end{aligned}$ | Single Angle | L $3 \times 3 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{aligned} & \text { T11 } 100.00- \\ & 80.00 \end{aligned}$ | Solid Round | $33 / 4$ " solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Single Angle | L. $3 \times 3 \times 1 / 4$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T12 80.00-60.00 | Solid Round | $33 / 4 \prime$ solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Double Angle | 2L $25 \times 2.5 \times 3 / 16$ (1/2) | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T13 60.00-40.00 | Solid Round | 4 " solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Double Angle | $2 \mathrm{~L} 2.5 \times 2.5 \times 3 / 16(1 / 2)$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T1440.00-20.00 | Solid Round | $4^{\prime \prime}$ solid | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Double Angle | $2 L 25 \times 25 \times 3 / 16$ (1/2) | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T15 2000-0.00 | Solid Round | 4" solid | $\begin{gathered} \mathrm{A} 572-50 \\ (50 \mathrm{ksi}) \\ \hline \end{gathered}$ | Double Angle | $2 \mathrm{~L} 25 \times 25 \times 3 / 16(1 / 2)$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |

## Tower Section Geometry (cont'd)

| Tower <br> Elevation <br> $f l$ | Top Girt <br> Type |  | Top Girt | Size | Top Girt | Bottom Girt |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Grade | Type | Bottom Girt <br> Size | Bottom Girt <br> Grade |  |  |  |
| $\mathrm{T} 128500-$ <br> 280.00 | Single Angle | $\mathrm{L} 2 \times 2 \times 3 / 16$ |  |  |  |  |

Tower Section Geometry (cont'd)

| Tower Elevation <br> ft | No. of Mid Girts | Mid Girt <br> Type | Mid Girt Size | Mid Girt Grade | Horizontal Type | Horizontal Size | Horizontal Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} \text { T6 } 20000- \\ 18000 \end{gathered}$ | None | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L. $2 \times 2 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T } 18000- \\ 160.00 \end{gathered}$ | None | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $2 \times 2 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T8 } 16000- \\ 14000 \end{gathered}$ | None | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $2 \times 2 \times 3 / 16$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \text { T9 } 140.00 \\ 120.00 \end{gathered}$ | None | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $2.5 \times 2.5 \times 3 / 16$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| $\begin{gathered} \mathrm{T} 10120.00- \\ 100.00 \end{gathered}$ | None | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $2.5 \times 2.5 \times 3 / 16$ | $\begin{gathered} A 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T11 10000 | None | Single Angle |  | A36 | Single Angle | L. $3 \times 3 \times 3 / 16$ | A36 |

# RISATower 

Paul J. Ford and Company
250 East Broad Street, Suite 1500
Columbus, OH 43215
Phone: 614-221-6679
FAX 614-448-4105

| Job | New 285 Self-Supporting Tower | Page |
| :--- | :--- | :--- |
| Project | 4 of 24 |  |
| Monkey's Eyebrow, Kentucky (PJF \#01905-029) | Date <br> $14: 56: 06 ~ 11 / 10 / 05 ~$ |  |
| Client | FWT, Inc. | Designed by <br> Craig Meierhoffer |


| Tower Elevation <br> ft | $\begin{gathered} \text { No } \\ \text { of } \\ \text { Mid } \\ \text { Girts } \end{gathered}$ | Mid Girt Type | Mid Girt Size | Mid Girt Grade | Horizontal Type | Horizontal Size | Horizontal Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 80.00 |  |  |  | (36 ksi) |  |  | (36 ksi) |
| T12 80.00-60.00 | None | Single Angle |  | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $3 \times 3 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T1360.00-40.00 | None | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Double Angle | $2 \mathrm{~L} .2 \times 2 \times 3 / 16(1 / 2)$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T14 40.00-20.00 | None | Single Angle |  | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | Double Angle | 2L. $2 \times 2 \times 3 / 16$ (1/2) | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T15 20.00-0.00 | None | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Double Angle | $\begin{gathered} 2 \mathrm{~L} 2.5 \times 2.5 \times 3 / 16 \\ (1 / 2) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A.36 } \\ (36 \mathrm{ksi}) \\ \hline \end{gathered}$ |

## Tower Section Geometry (cont'd)

| Tower Elevation <br> $f t$ | Secondary Horizontal Type | Secondary Horizontal Size | Secondary Horizontal Grade | Inner Bracing Type | Inner Bracing Size | Inner Bracing Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T12 80.00-60.00 | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $3 \times 3 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T13 60.00-40.00 | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $3 \times 3 \times 3 / 16$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T14 40.00-20.00 | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $3 \times 3 \times 3 / 16$ | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T15 20.00-0.00 | Single Angle |  | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | Single Angle | L $3.5 \times 3.5 \times \mathrm{l} / 4$ | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \\ \hline \end{gathered}$ |

## Tower Section Geometry (cont'd)

| Tower <br> Elevation | Redundant <br> Bracing <br> Grade |  | Redundant <br> Type | Redundant <br> Size | K Factor |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $f t$ |  |  |  |  |  |
| T1360.00- | A36 | Horizontal (1) | Single Angle | $\mathrm{L} 2 \times 2 \times 3 / 16$ | 1 |
| 40.00 | $(36 \mathrm{ksi})$ | Diagonal (1) | Single Angle | $\mathrm{L} .2 .5 \times 2.5 \times 3 / 16$ | 1 |
| $\mathrm{~T} 1440.00-$ | A36 | Horizontal (1) | Single Angle | $\mathrm{L} 2.5 \times 2.5 \times 3 / 16$ | 1 |
| 2000 | $(36 \mathrm{ksi})$ | Diagonal (1) | Single Angle | $\mathrm{L} .3 \times 3 \times 3 / 16$ | 1 |
| $\mathrm{~T} 152000-$ | A36 | Horizontal (1) | Single Angle | $\mathrm{L} .25 \times 2.5 \times 3 / 16$ | 1 |
| 0.00 | $(36 \mathrm{ksi})$ | Diagonal (1) | Single Angle | $\mathrm{L} 3 \times 3 \times 3 / 16$ | 1 |



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\left.| Job | New 285 Self-Supporting Tower | Page |
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| Project | 5 of 24 |  |
| Monkey's Eyebrow, Kentucky (PJF \#01905-029) |  |  |$\right]$| Date |
| :---: |
| 14:56:06 11/10/05 |


| Tower Elevation <br> ft | Gusset <br> Area (per face) | Gusset Thickness in | Gusset Grade | Adjust. Factor $A_{f}$ | Adjust <br> Factor <br> $A_{r}$ | Weight Mult | Double Angle Stitch Bolt Spacing Diagonals in | Double Angle Stitch Bolt Spacing Horizontals in |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 260.00 |  |  | (36 ksi) |  |  |  |  |  |
| $\begin{gathered} \text { T3 } 26000 \\ 240.00 \end{gathered}$ | 0.00 | 0.25 | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.1 | 000 | 0.00 |
| $\begin{gathered} \text { T4 } 240.00- \\ 220.00 \end{gathered}$ | 000 | 0.25 | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.1 | 0.00 | 0.00 |
| $\begin{gathered} \text { T5 } 220.00- \\ 200.00 \end{gathered}$ | 0.00 | 0.25 | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.1 | 0.00 | 0.00 |
| $\begin{gathered} \text { T6 } 200.00- \\ 180.00 \end{gathered}$ | 0.00 | 0.38 | $\begin{gathered} \text { A.36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 12 | 0.00 | 0.00 |
| $\begin{gathered} \text { T7 } 180.00- \\ 160.00 \end{gathered}$ | 0.00 | 0.38 | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.2 | 0.00 | 0.00 |
| $\begin{gathered} \text { T8 } 160.00- \\ 140.00 \end{gathered}$ | 0.00 | 0.38 | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.2 | 0.00 | 0.00 |
| $\begin{gathered} \text { T9 } 140.00- \\ 120.00 \end{gathered}$ | 0.00 | 0.38 | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 12 | 0.00 | 0.00 |
| $\begin{gathered} \mathrm{T} 10120.00- \\ 100.00 \end{gathered}$ | 0.00 | 0.38 | $\begin{gathered} \text { A.36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.2 | 0.00 | 0.00 |
| $\begin{gathered} \text { T11 } 100.00- \\ 80.00 \end{gathered}$ | 0.00 | 0.38 | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.2 | 0.00 | 0.00 |
| $\begin{gathered} \text { T12 } 80.00- \\ 60.00 \end{gathered}$ | 000 | 0.38 | $\begin{gathered} \text { A36 } \\ (.36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.2 | 0.00 | 0.00 |
| $\begin{gathered} \text { T13 } 60.00- \\ 40.00 \end{gathered}$ | 0.00 | 0.50 | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.25 | 0.00 | 0.00 |
| $\begin{gathered} \mathrm{T} 1440.00- \\ 20.00 \end{gathered}$ | 0.00 | 0.50 | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ | 1 | 1 | 1.25 | 0.00 | 0.00 |
| T15 20.00-0.00 | 0.00 | 0.50 | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \\ \hline \end{gathered}$ | 1 | 1 | 1.25 | 0.00 | 0.00 |

Tower Section Geometry (cont'd)

| Tower Elevation | Calc K Single Angles | Calc <br> K <br> Solid <br> Rounds | $K$ Factors ${ }^{\prime}$ |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Legs | $X$ | $\begin{gathered} K \\ \text { Brace } \end{gathered}$ | Single <br> Diags | Girts | Horiz. | Sec. Horiz. | Inner <br> Brace |
|  |  |  |  | Brace |  |  |  |  |  |  |
|  |  |  |  | Diags | Diags |  |  |  |  |  |
|  |  |  |  | $X$ | $X$ | $X$ | $X$ | $X$ | $X$ | $X$ |
| $f t$ |  |  |  | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ |
| T1 $285.00-$ | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 280.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T2 280.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 260.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T3 260.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 240.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T424000- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 220.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T5 22000 - | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 200.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T620000. | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 180.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T7180.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 160.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T816000- | Yes | No | I | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 140.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T9 140.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 120.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T10 $12000-$ | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |

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| Job | New 285 Self-Supporting Tower | Page |
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| Project | 6 of 24 |  |
|  | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | Date <br> $14: 56: 06 ~ 11 / 10 / 05 ~$ |
| Client | FWT, Inc. | Designed by <br> Craig Meierhoffer |


| Tower Elevation | Calc K Single Angles | Calc K Solid Rounds | K Factors ${ }^{\text {T }}$ |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Legs | $X$ | $K$ | Single | Girts | Horiz. | Sec Horiz. | Inner <br> Brace |
|  |  |  |  | Brace | Brace | Diags |  |  |  |  |
|  |  |  |  | Diags | Diags |  |  |  |  |  |
|  |  |  |  | $X$ | $X$ | $X$ | $X$ | $X$ | $X$ | $X$ |
| $f t$ |  |  |  | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ |
| 100.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T11 100.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 80.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T1280.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 60.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T1360.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 40.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T1440.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 20.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T15 20.00- | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 0.00 |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |

${ }^{T}$ Note: $K$ factors are applied to member segment lengths. K-braces without inner supporting members will have the $K$ factor in the out-of-plane direction applied to the overall length.

## Tower Section Geometry (cont'd)

| Tower Elevation $f i$ | Leg |  | Diagonal |  | Top Girt |  | Bottom Girt |  | Mid Girt |  | Long Horizontal |  | Short Horizontal |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Net Width Deduct in |  | Net Width Deduct in |  | Net Width Deduct in |  | Net <br> Width <br> Deduct <br> in | $U$ | Net <br> Width <br> Deduct <br> in | $U$ | Net <br> Width <br> Deduct <br> in | $U$ | Net Width Deduct in | $U$ |
| $\begin{gathered} \text { T1 } 285.00- \\ 280.00 \end{gathered}$ | 0.00 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T2 } 280.00- \\ 260.00 \end{gathered}$ | 0.00 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T3 } 260.00- \\ 240.00 \end{gathered}$ | 0.00 | 1 | 000 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T4 } 240.00- \\ 220.00 \end{gathered}$ | 0.00 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 000 | 075 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T5 } 220.00- \\ 200.00 \end{gathered}$ | 0.00 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 075 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T6 } 200.00- \\ 180.00 \end{gathered}$ | 000 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 075 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T7 } 180.00- \\ 16000 \end{gathered}$ | 000 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T8 } 160.00- \\ 140.00 \end{gathered}$ | 000 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T9 } 140.00- \\ 120.00 \end{gathered}$ | 0.00 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 000 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \mathrm{T} 10120.00- \\ 100.00 \end{gathered}$ | 000 | 1 | 000 | 0.75 | 0.00 | 0.75 | 0.00 | 075 | 0.00 | 0.75 | 000 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \text { T11 } 10000- \\ 8000 \end{gathered}$ | 0.00 | 1 | 0.00 | 0.75 | 000 | 0.75 | 0.00 | 075 | 000 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \mathrm{T} 1280.00 \\ 6000 \end{gathered}$ | 000 | 1 | 000 | 0.75 | 0.00 | 0.75 | 0.00 | 075 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \mathrm{T} 1360.00 \\ 40.00 \end{gathered}$ | 000 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 075 | 0.00 | 0.75 | 000 | 0.75 | 0.00 | 0.75 |
| $\begin{gathered} \mathrm{T} 144000- \\ 2000 \end{gathered}$ | 000 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 075 | 0.00 | 0.75 | 000 | 0.75 | 0.00 | 0.75 |
| T15 2000-0.00 | 0.00 | 1 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 | 0.00 | 0.75 |

# RISATower 

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Tower Section Geometry (cont'd)

| Tower Elevation $f t$ | Leg Connection Type | Leg |  | Diagonal |  | Top Girt |  | Bottom Girt |  | Mid Girt |  | Long Horizontal |  | Short Horizontal |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Bolt Size in | No. | Bolt Size in |  | Bolt Size in |  | Bolt Size in |  | Bolt Size in |  | Boll Size in | No. | Boll Size in | No. |
| $\begin{gathered} \text { T1 } 28500- \\ 280.00 \end{gathered}$ | Flange | $\begin{gathered} 0.63 \\ \text { A } 325 \mathrm{~N} \end{gathered}$ | 4 | $\begin{gathered} 0.50 \\ \text { A325N } \end{gathered}$ | 2 | $\begin{gathered} 0.63 \\ \text { A } 325 \mathrm{~N} \end{gathered}$ | 1 | $\begin{gathered} 0.63 \\ \text { A } 325 \mathrm{~N} \end{gathered}$ | 0 | $\begin{gathered} 0.63 \\ \text { A } 325 \mathrm{~N} \end{gathered}$ | 0 | $\begin{gathered} 0.63 \\ \text { A } 325 \mathrm{~N} \end{gathered}$ | 0 | $\begin{gathered} 0.63 \\ \text { A } 325 \mathrm{~N} \end{gathered}$ | 0 |
| $\begin{gathered} \text { T2 } 280.00 \\ 260.00 \end{gathered}$ | Flange | 0.63 A 325 N | 4 | 050 A 325 N | 2 | 0.63 A 325 N | 0 | 063 A 325 N | 0 | 0.63 A 325 N | 0 | 0.63 A 325 N | 0 | $\begin{gathered} 0.63 \\ \text { A325N } \end{gathered}$ | 0 |
| T3 260000 | Flange | 0.75 | 6 | 050 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 |
| 240.00 |  | A325N |  | A 325 N |  | A325N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  |
| T4 240.00- | Flange | 0.75 | 6 | 0.50 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 |
| 220.00 |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  |
| T5 22000 - | Flange | 088 | 6 | 050 | 2 | 0.63 | 0 | 063 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 |
| 200.00 |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A325N |  | A 325 N |  | A 325 N |  |
| T6 $200.00-$ | Flange | 0.88 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 1 | 0.63 | 0 |
| 180.00 |  | A325N |  | A 325 N |  | A 325 N |  | A325N |  | A325N |  | A 325 N |  | A 325 N |  |
| T7 18000- | Flange | 1.00 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 063 | 0 | 0.63 | 1 | 0.63 | 1 |
| 160.00 |  | A. 325 N |  | A325N |  | A 325 N |  | A325N |  | A325N |  | A 325 N |  | A 325 N |  |
| T8 160.00- | Flange | 100 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 063 | 0 | 0.63 | 1 | 0.63 | 1 |
| 14000 |  | A 325 N |  | A 325 N |  | A325N |  | A325N |  | A.325N |  | A 325 N |  | A 325 N |  |
| T9 140.00- | Flange | 100 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 1 | 063 | 1 |
| 120.00 |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  |
| T10 120.00- | Flange | 1.00 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 1 | 063 | 1 |
| 100.00 |  | A 325 N |  | A 325 N |  | A325N |  | A325N |  | A325N |  | A 325 N |  | A 325 N |  |
| T11 100.00- | Flange | 1.13 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 1 | 0.63 | 1 |
| 80.00 |  | A. 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  | A 325 N |  |
| T128000- | Flange | 1.13 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 1 | 0.63 | 1 |
| 60.00 |  | A325N |  | A325N |  | A 325 N |  | A325N |  | A 325 N |  | A 325 N |  | A 325 N |  |
| T13 60.00- | Flange | 1.25 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 1 | 0.63 | 1 |
| 40.00 |  | A 325 N |  | A 325 N |  | A 325 N |  | A325N |  | A325N |  | A 325 N |  | A 325 N |  |
| T14 40.00- | Flange | 1.25 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 063 | 0 | 0.63 | 1 | 0.63 | 1 |
| 20.00 |  | A 325 N |  | A 325 N |  | A 325 N |  | A325N |  | A 325 N |  | A 325 N |  | A 325 N |  |
| T15 20.00-0.00 | Flange | 1.75 | 6 | 0.63 | 2 | 0.63 | 0 | 0.63 | 0 | 0.63 | 0 | 0.63 | 1 | 0.63 | 1 |
|  |  | A 307 |  | A 325 N |  | A 325 N |  | A325N |  | A 325 N |  | A325N |  | A 325 N |  |

Feed Line/Linear Appurtenances - Entered As Round Or Flat

| Description | $\begin{gathered} \text { Face } \\ \text { or } \\ \text { Leg } \end{gathered}$ | Allow <br> Shield | Component Type | Placement <br> ft | Total Number | Number Per Row | Clear Spacing in | Width or Diameter in | Perimeter <br> in | Weight plf |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $15^{\prime \prime}$ flat Cable Ladder Rail | A | Yes | Af (CfAe) | 285.00-6.00 | 2 | 2 | 150 | 150 | 6.00 | 1.8 |
| 15" flat Cable Ladder Rail | B | Yes | Af (CfAe) | 285.00-6.00 | 2 | 2 | 150 | 150 | 6.00 | 18 |
| $15^{\prime \prime}$ flat Cable Ladder Rail | C | Yes | Af(CfAe) | 285.00-6.00 | 2 | 2 | 1.50 | 1.50 | 6.00 | 18 |
| LDF7-50A ( $15 / 8^{\prime \prime}$ foam) | A | Yes | Ar (CfAe) | 285.00-6.00 | 4 | 4 | $\begin{aligned} & 0.52 \\ & 1.98 \end{aligned}$ | 1.98 |  | 0.9 |
| LDF7-50A (1 5/8" foam) | B | Yes | Ar (CfAe) | 285.00-6.00 | 4 | 4 | $\begin{aligned} & 0.52 \\ & 1.98 \end{aligned}$ | 198 |  | 0.9 |
| LDF7-50A (1 5/8" foam) | C | Yes | Ar (ClAe) | 285.00-6.00 | 4 | 4 | $\begin{aligned} & 052 \\ & 1.98 \end{aligned}$ | 198 |  | 09 |
| LDF7-50A (1 5/8"foam) | A | Yes | $\mathrm{Ar}(\mathrm{CfAe})$ | 26500-6.00 | 4 | 2 | 0.52 | 198 |  | 09 |


| RISATower <br> Paul J. Ford and Company 250 East Broad Street, Suite 1500 | Job New 285 Self-Supporting Tower |  | $\begin{array}{ll} \text { Page } \\ 8 \text { of } 24 \end{array}$ |
| :---: | :---: | :---: | :---: |
|  | Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | $\begin{aligned} & \text { Date } \\ & \text { 14:56:06 11/10/05 } \end{aligned}$ |
| Columbus, OH 43215 <br> Phone: 614-221-6679 <br> FAX: 614-448-4105 | Client | FWT, Inc. | Designed by Craig Meierhoffer |


| Description | $\begin{gathered} \text { Face } \\ \text { or } \\ \text { Leg } \end{gathered}$ | Allow Shield | Component Type | Placement <br> $f t$ | Total Number | Number Per Row | Clear Spacing in | Width or Diameter in | Perimeter in | Weight plf |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LDF7-50A (15/8' foam) | B | Yes | Ar (CfAe) | 265.00-6.00 | 4 | 2 | 0.52 | 1.98 |  | 0.9 |
| LDF7-50A ( $15 / 8^{\prime \prime}$ foam) | C | Yes | At (CfAe) | 265.00-6.00 | 4 | 2 | 0.52 | 1.98 |  | 0.9 |
| LDF7-50A (1 5/8" foam) | A | Yes | Ar (CfAe) | 245.00-6.00 | 4 | 2 | 0.52 | 1.98 |  | 0.9 |
| LDF7-50A ( $15 / 8^{\prime \prime}$ foam) | B | Yes | Ar (CfAe) | 245.00-6.00 | 4 | 2 | 0.52 | 1.98 |  | 0.9 |
| LDF7-50A (1 5/8" foam) | C | Yes | Ar (CfAe) | 245.00-6.00 | 4 | 2 | 0.52 | 198 |  | 0.9 |
| LDF7-50A (15/8' foam) | A. | Yes | Ar (CfAe) | 225.00-6.00 | 4 | 2 | 0.52 | 1.98 |  | 0.9 |
| LDF7-50A (15/8" foam) | B | Yes | Ar (CfAe) | 225.00-6.00 | 4 | 2 | 0.52 | 1.98 |  | 0.9 |
| LDF7-50A ( $15 / 8^{\prime \prime}$ foam) | C | Yes | Ar (CfAe) | 225.00-6.00 | 4 | 2 | 0.52 | 1.98 |  | 0.9 |
| EW220 | C | Yes | $\operatorname{Ar}$ (CaAa) | 205.00-6.00 | 1 | 1 | 0.51 | 0.51 |  | 0.1 |

Feed Line/Linear Appurtenances - Entered As Area

| Description | $\begin{gathered} \text { Face } \\ \text { or } \\ \text { Leg } \end{gathered}$ | Allow Shield | Component Type | Placement $f t$ | Total Number |  | $C_{4} A_{A}$ $f^{2} / f i$ | Weight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 " lighting conduit | C | No | CaAa (In Face) | 285.00-6.00 | 1 | No Ice | 0.10 | 2.0 |
|  |  |  |  |  |  | 1/2" Ice | 0.20 | 2.9 |

Feed Line/Linear Appurtenances Section Areas

| Tower <br> Section | Tower Elevation fi | Face | $A_{R}$ <br>  <br>  <br> $t^{2}$ | $A_{F}$ $f t^{2}$ | $C_{A} A_{A}$ In Face $\mathrm{ft}^{\circ}$ | $\qquad$ | Weight $K$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 285.00-28000 | A | 3.300 | 1.250 | 0.000 | 0000 | 0.04 |
|  |  | B | 3.300 | 1.250 | 0.000 | 0.000 | 0.04 |
|  |  | C | 3.300 | 1.250 | 0500 | 0.000 | 005 |
| T2 | 280.00-260.00 | A | 14850 | 5.000 | 0.000 | 0.000 | 0.16 |
|  |  | B | 14850 | 5.000 | 0.000 | 0.000 | 0.16 |
|  |  | C | 14.850 | 5.000 | 2.000 | 0.000 | 0.20 |
| T3 | 260.00-240 00 | A. | 21.450 | 5000 | 0.000 | 0.000 | 0.24 |
|  |  | B | 21.450 | 5.000 | 0.000 | 0.000 | 0.24 |
|  |  | C | 21.450 | 5.000 | 2.000 | 0000 | 028 |
| T4 | $24000-22000$ | A | 28.050 | 5.000 | 0.000 | 0.000 | 031 |
|  |  | B | 28.050 | 5.000 | 0.000 | 0.000 | 031 |
|  |  | C | 28.050 | 5.000 | 2000 | 0000 | 0.35 |
| T5 | 22000-200 00 | A | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
|  |  | B | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
|  |  | C | 33.000 | 5.000 | 2.254 | 0000 | 0.41 |
| T6 | $20000-180.00$ | A | 33000 | 5.000 | 0.000 | 0000 | 0.37 |
|  |  | B | 33.000 | 5.000 | 0.000 | 0.000 | 037 |
|  |  | C | 33.000 | 5.000 | 3.016 | 0.000 | 0.41 |
| T7 | 180.00-160 00 | A | 33.000 | 5.000 | 0.000 | 0.000 | 037 |
|  |  | B | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
|  |  | C | 33000 | 5.000 | 3.016 | 0.000 | 0.41 |
| T8 | 160.00-14000 | A | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
|  |  | B | 33000 | 5000 | 0.000 | 0.000 | 0.37 |
|  |  | C | 33000 | 5000 | 3.016 | 0.000 | 0.41 |
| T9 | 14000-120.00 | A | 33.000 | 5000 | 0.000 | 0.000 | 0.37 |
|  |  | B | 33000 | 5000 | 0.000 | 0.000 | 0.37 |
|  |  | C | 33.000 | 5.000 | 3.016 | 0.000 | 041 |
| T 10 | 120.00-100 00 | A | 33.000 | 5000 | 0.000 | 0.000 | 0.37 |
|  |  | B | 33.000 | 5000 | 0.000 | 0.000 | 0.37 |
|  |  | C | 33.000 | 5.000 | 3.016 | 0.000 | 0.41 |
| T11 | 10000-80.00 | A | 33000 | 5000 | 0.000 | 0.000 | 037 |
|  |  | B | 33.000 | 5000 | 0.000 | 0.000 | 0.37 |



| Tower Section | Tower Elevation $f t$ | Face | $A_{R}$ <br>  <br> $t^{2}$ | $A_{F}$ $f t^{2}$ | $C_{A} A_{A}$ In Face $f t^{2}$ | $C_{A} A_{A}$ Out Face $f t^{2}$ | Weight $K$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T 12 | 80.00-60.00 | C | 33.000 | 5.000 | 3.016 | 0.000 | 0.41 |
|  |  | A | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
|  |  | B | 33.000 | 5000 | 0.000 | 0.000 | 0.37 |
| T13 | $60.00-40.00$ | C | 33.000 | 5.000 | 3.016 | 0.000 | 0.41 |
|  |  | A | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
|  |  | B | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
| T14 | 40.00-20.00 | C | 33.000 | 5.000 | 3.016 | 0.000 | 0.41 |
|  |  | A | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
|  |  | B | 33.000 | 5.000 | 0.000 | 0.000 | 0.37 |
| T15 | 20.00-0.00 | C | 33.000 | 5.000 | 3.016 | 0.000 | 0.41 |
|  |  | A | 23.100 | 3.500 | 0.000 | 0.000 | 0.26 |
|  |  | B | 23.100 | 3.500 | 0.000 | 0.000 | 0.26 |
|  |  | C | 23.100 | 3.500 | 2.111 | 0.000 | 0.29 |

Feed Line/Linear Appurtenances Section Areas - With Ice

\begin{tabular}{|c|c|c|c|c|c|c|c|c|}
\hline Tower Section \& Tower Elevation $f t$ \& $$
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
$$ \& $\qquad$ \& $A_{R}$
$f^{2}$ \& $A_{F}$

$f t^{2}$ \& | $C_{A} A_{A}$ |
| :--- |
| In Face |
| $f t^{2}$ | \& $C_{A} A_{A}$ Out Face $f t^{3}$ \& Weight

$K$ <br>
\hline \multirow[t]{3}{*}{T1} \& \multirow[t]{3}{*}{285.00-280.00} \& A \& \multirow[t]{3}{*}{0500} \& 1.242 \& 4.931 \& 0.000 \& 0.000 \& 0.08 <br>
\hline \& \& B \& \& 1242 \& 4931 \& 0.000 \& 0.000 \& 008 <br>
\hline \& \& C \& \& 1.242 \& 4.931 \& 1.000 \& 0.000 \& 0.10 <br>
\hline \multirow[t]{3}{*}{T2} \& \multirow[t]{3}{*}{280.00-260.00} \& A \& \multirow[t]{3}{*}{0.500} \& 6208 \& 20.764 \& 0.000 \& 0.000 \& 039 <br>
\hline \& \& B \& \& 6.208 \& 20.764 \& 0.000 \& 0.000 \& 0.39 <br>
\hline \& \& C \& \& 6.208 \& 20.764 \& 4.000 \& 0.000 \& 045 <br>
\hline \multirow[t]{3}{*}{T3} \& \multirow[t]{3}{*}{260.00-240.00} \& A \& \multirow[t]{3}{*}{0.500} \& 11.175 \& 24931 \& 0000 \& 0.000 \& 0.58 <br>
\hline \& \& B \& \& 11.175 \& 24.931 \& 0.000 \& 0.000 \& 0.58 <br>
\hline \& \& C \& \& 11175 \& 24.931 \& 4.000 \& 0.000 \& 0.64 <br>
\hline \multirow[t]{3}{*}{T4} \& \multirow[t]{3}{*}{240.00-220.00} \& A \& \multirow[t]{3}{*}{0.500} \& 16.142 \& 29.097 \& 0.000 \& 0000 \& 0.78 <br>
\hline \& \& B \& \& 16.142 \& 29.097 \& 0.000 \& 0000 \& 0.78 <br>
\hline \& \& $C$ \& \& 16.142 \& 29.097 \& 4.000 \& 0.000 \& 0.84 <br>
\hline \multirow[t]{3}{*}{T5} \& \multirow[t]{3}{*}{220.00-200.00} \& A \& \multirow[t]{3}{*}{0.500} \& 19.867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& B \& \& 19.867 \& 32.222 \& 0.000 \& 0.000 \& 093 <br>
\hline \& \& C \& \& 19.867 \& 32.222 \& 4.754 \& 0.000 \& 0.99 <br>
\hline \multirow[t]{3}{*}{T6} \& \multirow[t]{3}{*}{200.00-180.00} \& A \& \multirow[t]{3}{*}{0.500} \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& B \& \& 19867 \& 32.222 \& 0.000 \& 0000 \& 093 <br>
\hline \& \& C \& \& 19867 \& 32.222 \& 7.017 \& 0.000 \& 1.00 <br>
\hline \multirow[t]{3}{*}{T7} \& \multirow[t]{3}{*}{180.00-16000} \& A \& \multirow[t]{3}{*}{0500} \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& B \& \& 19.867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& C \& \& 19.867 \& 32.222 \& 7.017 \& 0.000 \& 1.00 <br>
\hline \multirow[t]{3}{*}{T8} \& \multirow[t]{3}{*}{160.00-140 00} \& A \& \multirow[t]{3}{*}{0500} \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& B \& \& 19.867 \& 32.222 \& 0.000 \& 0.000 \& 093 <br>
\hline \& \& C \& \& 19867 \& 32.222 \& 7.017 \& 0.000 \& 1.00 <br>
\hline \multirow[t]{3}{*}{T9} \& \multirow[t]{3}{*}{140.00-120.00} \& A \& \multirow[t]{3}{*}{0.500} \& 19.867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& B \& \& 19.867 \& 32.222 \& 0.000 \& 0000 \& 0.93 <br>
\hline \& \& C \& \& 19867 \& 32222 \& 7.017 \& 0.000 \& 100 <br>
\hline \multirow[t]{3}{*}{T10} \& \multirow[t]{3}{*}{120.00-100.00} \& A \& \multirow[t]{3}{*}{0.500} \& 19867 \& 32.222 \& 0000 \& 0.000 \& 0.93 <br>
\hline \& \& B \& \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& C \& \& 19.867 \& 32.222 \& 7017 \& 0.000 \& 100 <br>
\hline \multirow[t]{3}{*}{T11} \& \multirow[t]{3}{*}{100.00-80 00} \& A \& \multirow[t]{3}{*}{0.500} \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 093 <br>
\hline \& \& B \& \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 093 <br>
\hline \& \& C \& \& 19867 \& 32.222 \& 7.017 \& 0000 \& 100 <br>
\hline \multirow[t]{3}{*}{T12} \& \multirow[t]{3}{*}{80.00-60 00} \& A \& \multirow[t]{3}{*}{0.500} \& 19867 \& 32.222 \& 0.000 \& 0000 \& 0.93 <br>
\hline \& \& B \& \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& C \& \& 19867 \& 32.222 \& 7.017 \& 0.000 \& 100 <br>
\hline \multirow[t]{3}{*}{T13} \& \multirow[t]{3}{*}{6000-40 00} \& A \& \multirow[t]{3}{*}{0500} \& 19867 \& 32.222 \& 0.000 \& 0.000 \& 093 <br>
\hline \& \& B \& \& 19867 \& 32222 \& 0000 \& 0000 \& 0.93 <br>
\hline \& \& C \& \& 19867 \& 32.222 \& 7.017 \& 0.000 \& 100 <br>
\hline
\end{tabular}

| RISATower <br> Paul J. Ford and Company <br> 250 East Broad Street, Suite 1500 | Job New 285 Self-Supporting Tower |  | $\begin{aligned} & \text { Page } \\ & 10 \text { of } 24 \end{aligned}$ |
| :---: | :---: | :---: | :---: |
|  | Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | $\begin{array}{\|l\|} \text { Date } \\ \text { 14:56:06 11/10/05 } \end{array}$ |
| Columbus, OH 43215 <br> Phone 614-221-6679 <br> FAX: 614-448-4105 | Client | FWT, Inc. | Designed by Craig Meierhoffer |

\begin{tabular}{|c|c|c|c|c|c|c|c|c|}
\hline Tower Section \& Tower Elevation ft \& $$
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
$$ \& Ice Thickness in \& $A_{R}$

$f l^{2}$ \& $A_{F}$

$f t^{2}$ \& $C_{A} A_{A}$ In Face $f t^{2}$ \& \[
$$
\begin{gathered}
C_{A} A_{A} \\
\text { Out Face } \\
f^{2} \\
\hline
\end{gathered}
$$

\] \& | Weight |
| :---: |
| $K$ | <br>

\hline \multirow[t]{3}{*}{T14} \& \multirow[t]{3}{*}{40.00-20.00} \& A \& \multirow[t]{3}{*}{0.500} \& 19.867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& B \& \& 19.867 \& 32.222 \& 0.000 \& 0.000 \& 0.93 <br>
\hline \& \& C \& \& 19.867 \& 32.222 \& 7.017 \& 0.000 \& 1.00 <br>
\hline \multirow[t]{3}{*}{T15} \& \multirow[t]{3}{*}{20.00-0.00} \& A \& \multirow[t]{3}{*}{0.500} \& 13.907 \& 22.556 \& 0.000 \& 0.000 \& 0.65 <br>
\hline \& \& B \& \& 13.907 \& 22.556 \& 0.000 \& 0.000 \& 0.65 <br>
\hline \& \& C \& \& 13.907 \& 22.556 \& 4.912 \& 0.000 \& 0.70 <br>
\hline
\end{tabular}

Feed Line Shielding

\begin{tabular}{|c|c|c|c|c|c|c|}
\hline Section \& Elevation

$f t$ \& Face \& $A_{R}$

$f r^{2}$ \& | $A_{R}$ |
| :--- |
| Ice |
| $f t^{2}$ | \& AF


$\mathrm{ft}^{2}$ \& | $A_{F}$ |
| :--- |
| Ice |
| $f t^{2}$ | <br>

\hline \multirow[t]{3}{*}{Tl} \& \multirow[t]{3}{*}{285.00-280.00} \& A \& 0.000 \& 0.000 \& 0.577 \& 1.118 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 0.577 \& 1.118 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 0.577 \& 1118 <br>
\hline \multirow[t]{3}{*}{T2} \& \multirow[t]{3}{*}{280.00-260.00} \& A \& 0.000 \& 0.000 \& 2.047 \& 3.999 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.047 \& 3.999 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 2.047 \& 3.999 <br>
\hline \multirow[t]{3}{*}{T3} \& \multirow[t]{3}{*}{26000-240.00} \& A \& 0.000 \& 0.000 \& 2.470 \& 4.799 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.470 \& 4.799 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 2.470 \& 4.799 <br>
\hline \multirow[t]{3}{*}{T4} \& \multirow[t]{3}{*}{240.00-220.00} \& A \& 0000 \& 0000 \& 2.748 \& 5.321 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.748 \& 5.321 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 2.748 \& 5.321 <br>
\hline \multirow[t]{3}{*}{T5} \& \multirow[t]{3}{*}{220.00-200.00} \& A \& 0.000 \& 0.000 \& 2.732 \& 5.281 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.732 \& 5.281 <br>
\hline \& \& C \& 0000 \& 0000 \& 2.747 \& 5.344 <br>
\hline \multirow[t]{3}{*}{T6} \& \multirow[t]{3}{*}{200.00-180.00} \& A \& 0000 \& 0.000 \& 3.009 \& 5.396 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 3.009 \& 5.396 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 3.077 \& 5.651 <br>
\hline \multirow[t]{3}{*}{T7} \& \multirow[t]{3}{*}{180.00-160.00} \& A \& 0.000 \& 0.000 \& 2.778 \& 4.985 <br>
\hline \& \& B \& 0.000 \& 0000 \& 2.778 \& 4.985 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 2.840 \& 5.221 <br>
\hline \multirow[t]{3}{*}{T8} \& \multirow[t]{3}{*}{$160.00-140.00$} \& A \& 0.000 \& 0.000 \& 2.634 \& 4.729 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.634 \& 4.729 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 2.692 \& 4.953 <br>
\hline \multirow[t]{3}{*}{T9} \& \multirow[t]{3}{*}{140.00-120.00} \& A \& 0.000 \& 0.000 \& 3.077 \& 5.315 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 3.077 \& 5.315 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 3.146 \& 5.566 <br>
\hline \multirow[t]{3}{*}{T10} \& \multirow[t]{3}{*}{120.00-100.00} \& A \& 0000 \& 0.000 \& 2.997 \& 5.178 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.997 \& 5.178 <br>
\hline \& \& C \& 0.000 \& 0000 \& 3.064 \& 5.423 <br>
\hline \multirow[t]{3}{*}{T11} \& \multirow[t]{3}{*}{100.00-80.00} \& A \& 0.000 \& 0.000 \& 3.098 \& 5.301 <br>
\hline \& \& B \& 0.000 \& 0000 \& 3.098 \& 5.301 <br>
\hline \& \& C \& 0.000 \& 0000 \& 3.167 \& 5.551 <br>
\hline \multirow[t]{3}{*}{T12} \& \multirow[t]{3}{*}{$80.00-60.00$} \& A \& 0.000 \& 0.000 \& 2.704 \& 4.736 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.704 \& 4.736 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 2.764 \& 4960 <br>
\hline \multirow[t]{3}{*}{T13} \& \multirow[t]{3}{*}{$60.00-4000$} \& A \& 0000 \& 0.000 \& 3.050 \& 5.498 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 3050 \& 5498 <br>
\hline \& \& C \& 0.000 \& 0.000 \& 3.118 \& 5.757 <br>
\hline \multirow[t]{3}{*}{T14} \& \multirow[t]{3}{*}{4000-20.00} \& A \& 0.000 \& 0000 \& 3340 \& 5877 <br>
\hline \& \& B \& 0.000 \& 0000 \& 3.340 \& 5877 <br>
\hline \& \& C \& 0000 \& 0.000 \& 3.415 \& 6.155 <br>
\hline \multirow[t]{2}{*}{T1.5} \& \multirow[t]{2}{*}{$2000-0.00$} \& A \& 0.000 \& 0000 \& 2349 \& 4115 <br>
\hline \& \& B \& 0.000 \& 0.000 \& 2.349 \& 4.115 <br>
\hline
\end{tabular}

## RISATower

Paul J. Ford and Company
250 East Broad Street, Suite 1500
Columbus, OH 43215
Phone: 614-221-6679
FAX: 614-448-4105

| Job | New 285 Self-Supporting Tower | Page |
| :--- | :--- | :--- |
| Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) |  |
| Client | Date <br> $14: 56: 06 ~ 11 / 10 / 05 ~$ |  |
| FWT, Inc. | Designed by <br> Craig Meierhoffer |  |

\begin{tabular}{|c|c|c|c|c|c|c|}
\hline Section \& Elevation
ft \& Face \& $A_{R}$

$\mathrm{ft}^{2}$ \& | $A_{R}$ |
| :--- |
| Ice |
| $f t^{2}$ | \& $A_{F}$


$f t^{2}$ \& | $A_{F}$ |
| :--- |
| Ice |
| $f t^{2}$ | <br>

\hline \& \& C \& 0.000 \& 0.000 \& 2.402 \& 4.309 <br>
\hline
\end{tabular}

## Discrete Tower Loads

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \begin{tabular}{l}
Offset \\
Type
\end{tabular} \& \begin{tabular}{l}
Offsets: \\
Horz \\
Lateral \\
Vert \\
\(f t\) \\
ft \\
\(f\)
\end{tabular} \& \begin{tabular}{l}
Azimuth Adjustment \\
-
\end{tabular} \& Placement \& \& \begin{tabular}{l}
\(C_{A} A_{A}\) \\
Front \\
\(f t^{3}\)
\end{tabular} \& \(C_{A} A_{A}\)
Side \& Weight

K <br>
\hline Beacon \& C \& From Leg \& 0.00 \& \multirow[t]{2}{*}{0000} \& 285.00 \& No lce \& 3.60 \& 360 \& 0.10 <br>
\hline \& \& \& 0
0 \& \& \& 1/2" Ice \& 4.00 \& 4.00 \& 0.15 <br>
\hline (4) SC 9014-DIN \& A \& From Leg \& 4.00 \& \multirow[t]{2}{*}{0.000} \& 285.00 \& No Ice \& 6.24 \& 7.29 \& 0.04 <br>
\hline \& \& \& 0 \& \& \& 1/2" Ice \& 6.77 \& 7.82 \& 0.08 <br>
\hline (4) SC $9014 . \mathrm{DIN}$ \& B \& From Leg \& 400 \& \multirow[t]{2}{*}{0.000} \& 285.00 \& No Ice \& 6.24 \& 7.29 \& 0.04 <br>
\hline \& \& \& 0
0 \& \& \& 1/2" Ice \& 6.77 \& 7.82 \& 0.08 <br>
\hline \multirow[t]{2}{*}{(4) SC 9014-DIN} \& \multirow[t]{2}{*}{C} \& From Leg \& 4.00 \& \multirow[t]{2}{*}{0000} \& 285.00 \& No lce \& 6.24 \& 7.29 \& 0.04 <br>

\hline \& \& \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& $1 / 2^{\prime \prime}$ Ice \& 6.77 \& 7.82 \& 0.08 <br>

\hline \multirow[t]{3}{*}{Generic Sector Frame} \& \multirow[t]{2}{*}{A} \& \multirow[t]{2}{*}{From Leg} \& 200 \& \multirow[t]{2}{*}{0.000} \& \multirow[t]{2}{*}{285.00} \& No lce \& 15.00 \& 10.00 \& 0.60 <br>
\hline \& \& \& 0 \& \& \& $1 / 2^{\prime \prime}$ Ice \& 17.50 \& 12.50 \& 0.80 <br>
\hline \& \& \& 0 \& \& \& \& \& \& <br>
\hline \multirow[t]{3}{*}{Generic Sector Frame} \& \multirow[t]{3}{*}{B} \& \multirow[t]{2}{*}{From Leg} \& 2.00 \& \multirow[t]{2}{*}{0000} \& \multirow[t]{2}{*}{28500} \& No Ice \& 15.00 \& 10.00 \& 0.60 <br>
\hline \& \& \& 0 \& \& \& 1/2" lce \& 17.50 \& 12.50 \& 0.80 <br>
\hline \& \& \& 0 \& \& \& \& \& \& <br>

\hline \multirow[t]{2}{*}{Generic Sector Frame} \& \multirow[t]{2}{*}{C} \& \multirow[t]{2}{*}{From Leg} \& 2.00 \& \multirow[t]{2}{*}{0.000} \& \multirow[t]{2}{*}{285.00} \& No Ice \& $$
1500
$$ \& \[

10.00
\] \& 060 <br>

\hline \& \& \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& \[

1 / 2^{\prime \prime} Ice

\] \& \[

17.50

\] \& \[

12.50
\] \& 080 <br>

\hline \multirow[t]{2}{*}{(4) SC 9014-DIN} \& \multirow[t]{2}{*}{A} \& \multirow[t]{2}{*}{From Leg} \& 4.00 \& \multirow[t]{2}{*}{0.000} \& \multirow[t]{2}{*}{265.00} \& No Ice \& 6.24 \& 7.29 \& 0.04 <br>

\hline \& \& \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& $1 / 2^{\prime \prime}$ lce \& 6.77 \& 7.82 \& 0.08 <br>

\hline \multirow[t]{3}{*}{(4) SC $9014-\mathrm{DIN}$} \& \multirow[t]{3}{*}{B} \& \multirow[t]{3}{*}{From Leg} \& 4.00 \& \multirow[t]{3}{*}{0000} \& \multirow[t]{3}{*}{265.00} \& No Ice \& 6.24 \& 7.29 \& 004 <br>
\hline \& \& \& 0 \& \& \& 1/2" Ice \& 6.77 \& 7.82 \& 0.08 <br>
\hline \& \& \& 0 \& \& \& \& \& \& <br>

\hline \multirow[t]{2}{*}{(4) SC 9014-DIN} \& \multirow[t]{2}{*}{C} \& \multirow[t]{2}{*}{From Leg} \& 400 \& \multirow[t]{2}{*}{0.000} \& \multirow[t]{2}{*}{265.00} \& \& $$
6.24
$$ \& \[

7.29

\] \& \[

0.04
\] <br>

\hline \& \& \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& \[

1 / 2^{11} Ice

\] \& \[

6.77

\] \& \[

7.82

\] \& \[

008
\] <br>

\hline \multirow[t]{2}{*}{Generic Sector Frame} \& \multirow[t]{2}{*}{A} \& \multirow[t]{2}{*}{From Leg} \& 2.00 \& \multirow[t]{2}{*}{0000} \& \multirow[t]{2}{*}{265.00} \& No lce \& 15.00 \& 10.00 \& 060 <br>

\hline \& \& \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& $1 / 2^{\prime \prime}$ Ice \& 17.50 \& 12.50 \& 0.80 <br>

\hline \multirow[t]{3}{*}{Generic Sector Frame} \& \multirow[t]{3}{*}{B} \& \multirow[t]{3}{*}{From Leg} \& 200 \& \multirow[t]{3}{*}{0000} \& \multirow[t]{3}{*}{265.00} \& No Ice \& 15.00 \& 10.00 \& 0.60 <br>
\hline \& \& \& 0 \& \& \& 1/2" Ice \& 1750 \& 12.50 \& 0.80 <br>
\hline \& \& \& 0 \& \& \& \& \& \& <br>
\hline \multirow[t]{3}{*}{Generic Sector Frame} \& \multirow[t]{3}{*}{C} \& \multirow[t]{3}{*}{From Leg} \& 2.00 \& \multirow[t]{3}{*}{0.000} \& \multirow[t]{3}{*}{265.00} \& No Ice \& 15.00 \& 10.00 \& 0.60 <br>
\hline \& \& \& 0 \& \& \& 1/2" Ice \& 17.50 \& 12.50 \& 080 <br>
\hline \& \& \& 0 \& \& \& \& \& \& <br>

\hline \multirow[t]{2}{*}{(4) SC 9014-DIN} \& \multirow[t]{2}{*}{A} \& \multirow[t]{2}{*}{From Leg} \& 4.00 \& \multirow[t]{2}{*}{0.000} \& \multirow[t]{2}{*}{24500} \& No Ice \& $$
6.24
$$ \& \[

7.29

\] \& \[

0.04
\] <br>

\hline \& \& \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& \[

1 / 2^{\prime \prime} lce

\] \& \[

6.77

\] \& \[

7.82

\] \& \[

0.08
\] <br>

\hline \multirow[t]{3}{*}{(4) SC 9014-DIN} \& \multirow[t]{3}{*}{B} \& \multirow[t]{3}{*}{From Leg} \& 400 \& \multirow[t]{3}{*}{0000} \& \multirow[t]{3}{*}{24500} \& No Ice \& 6.24 \& 7.29 \& 004 <br>
\hline \& \& \& 0 \& \& \& 1/2" Ice \& 6.77 \& 782 \& 0.08 <br>
\hline \& \& \& 0 \& \& \& \& \& \& <br>
\hline (4) SC 9014-DIN \& C \& From Leg \& 4.00 \& 0.000 \& 24500 \& No Ice \& 6.24 \& 7.29 \& 004 <br>
\hline
\end{tabular}

| Job New 285 Self-Supporting Tower | 12 of 24 |
| :---: | :---: |
| Project <br> Monkey's Eyebrow, Kentucky (PJF \#01905-029) | Date <br> 14:56:06 11/10/05 |
| Client FWT, Inc. | Designed by Craig Meierhoffer |

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& $$
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
$$ \& $$
\begin{aligned}
& \text { Offset } \\
& \text { Type }
\end{aligned}
$$ \& Offsets Horz Lateral Vert $f t$ $f t$ $f t$ \& Azimuth Adjustment \& Placement

$f i$ \& \& $C_{A} A_{A}$ Front $f t^{2}$ \& $C_{A} A_{A}$ Side

$$
f t^{2}
$$ \& Weight <br>

\hline \& \& \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& $1 / 2^{14}$ Ice \& 6.77 \& 7.82 \& 0.08 <br>

\hline Generic Sector Frame \& A \& From Leg \& \[
$$
\begin{gathered}
2.00 \\
0 \\
0
\end{gathered}
$$

\] \& 0.000 \& 245.00 \& | No Ice |
| :--- |
| $1 / 2^{\prime \prime}$ lce | \& \[

$$
\begin{aligned}
& 15.00 \\
& 17.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 10.00 \\
& 12.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.60 \\
& 0.80
\end{aligned}
$$
\] <br>

\hline Generic Sector Frame \& B \& From Leg \& \[
$$
\begin{gathered}
2.00 \\
0 \\
0
\end{gathered}
$$

\] \& 0.000 \& 245.00 \& | No Ice |
| :--- |
| 1/2" Ice | \& \[

$$
\begin{aligned}
& 15.00 \\
& 1750
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 10.00 \\
& 12.50
\end{aligned}
$$

\] \& \[

$$
\begin{gathered}
0.60 \\
0.80
\end{gathered}
$$
\] <br>

\hline Generic Sector Frame \& C \& From Leg \& $$
\begin{gathered}
2.00 \\
0 \\
0
\end{gathered}
$$ \& 0.000 \& 24500 \& No Ice 1/2" Ice \& \[

$$
\begin{aligned}
& 15.00 \\
& 17.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 10.00 \\
& 12.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.60 \\
& 0.80
\end{aligned}
$$
\] <br>

\hline (4) SC 9014-DIN \& A \& From Leg \& $$
\begin{gathered}
4.00 \\
0 \\
0
\end{gathered}
$$ \& 0.000 \& 22500 \& No Ice 1/2" Ice \& \[

$$
\begin{aligned}
& 6.24 \\
& 6.77
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 7.29 \\
& 7.82
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.04 \\
& 0.08
\end{aligned}
$$
\] <br>

\hline (4) SC 9014-DIN \& B \& From Leg \& $$
\begin{gathered}
4.00 \\
0 \\
0
\end{gathered}
$$ \& 0.000 \& 225.00 \& No Ice 1/2" Ice \& \[

$$
\begin{aligned}
& 6.24 \\
& 6.77
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 7.29 \\
& 7.82
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.04 \\
& 0.08
\end{aligned}
$$
\] <br>

\hline (4) SC 9014-DIN \& C \& From Leg \& $$
\begin{gathered}
4.00 \\
0 \\
0
\end{gathered}
$$ \& 0.000 \& 22500 \& \[

$$
\begin{aligned}
& \text { No Ice } \\
& 1 / 2^{\text {" Ice }}
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 6.24 \\
& 6.77
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 7.29 \\
& 7.82
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.04 \\
& 0.08
\end{aligned}
$$
\] <br>

\hline Generic Sector Frame \& A \& From Leg \& $$
\begin{gathered}
2.00 \\
0 \\
0
\end{gathered}
$$ \& 0.000 \& 22500 \& No Ice $1 / 2^{\prime \prime}$ Ice \& \[

$$
\begin{aligned}
& 15.00 \\
& 17.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 10.00 \\
& 12.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.60 \\
& 0.80
\end{aligned}
$$
\] <br>

\hline Generic Sector Frame \& B \& From Leg \& \[
$$
\begin{gathered}
2.00 \\
0 \\
0
\end{gathered}
$$

\] \& 0.000 \& 225.00 \& | No Ice |
| :--- |
| 1/2" Ice | \& \[

$$
\begin{aligned}
& 15.00 \\
& 17.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 10.00 \\
& 12.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.60 \\
& 0.80
\end{aligned}
$$
\] <br>

\hline Generic Sector Frame \& C \& From Leg \& $$
\begin{gathered}
2.00 \\
0 \\
0
\end{gathered}
$$ \& 0.000 \& 225.00 \& No Ice 1/2" Ice \& \[

$$
\begin{aligned}
& 15.00 \\
& 17.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 10.00 \\
& 12.50
\end{aligned}
$$

\] \& \[

$$
\begin{aligned}
& 0.60 \\
& 0.80
\end{aligned}
$$
\] <br>

\hline
\end{tabular}

## Dishes

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& Face or Leg \& \begin{tabular}{l}
Dish \\
Type
\end{tabular} \& \begin{tabular}{l}
Offset \\
Type
\end{tabular} \& \begin{tabular}{l}
Offets: \\
Horz \\
Lateral Vert ft
\end{tabular} \& Azimuth Adjustment \& \begin{tabular}{l}
\(3 d B\) \\
Beam \\
Width
\end{tabular} \& Elevation \& \begin{tabular}{l}
Outside Diameter \\
ft
\end{tabular} \& \& Aperture Area
\[
f i^{2}
\] \& Weight

$K$ <br>
\hline \multirow[t]{2}{*}{8 ft standard} \& \multirow[t]{2}{*}{C} \& Paraboloid w/o \& From \& 1.00 \& 0.000 \& \& 20500 \& 8.00 \& No Ice \& 50.27 \& 0.26 <br>

\hline \& \& Radome \& Leg \& $$
\begin{aligned}
& 0 \\
& 0
\end{aligned}
$$ \& \& \& \& \& 1/2" Ice \& 5132 \& 0.55 <br>

\hline
\end{tabular}

## Load Combinations

| Comb. |  |
| :--- | :--- |
| No. |  |
| 1 | Dead Only |
| 2 | Dead+Wind 0 deg - No Ice |
| 3 | Dead+Wind 30 deg - No Ice |


| RISATower <br> Paul J. Ford and Company 250 East Broad Street, Suite 1500 | Job New 285 Self-Supporting Tower |  | $\begin{aligned} & \text { Page } \\ & 13 \text { of } 24 \end{aligned}$ |
| :---: | :---: | :---: | :---: |
|  | Monkey's Eyebrow, Kentucky (PJF \#01905-029) |  | $\begin{array}{\|l\|} \hline \text { Date } \\ \text { 14:56:06 11/10/05 } \end{array}$ |
| Columbus, OH 43215 <br> Phone: 614-221-6679 <br> FAX 614-448-4105 | Client | FWT, Inc. | Designed by Craig Meierhoffer |


| Comb. No. |  | Description |
| :---: | :---: | :---: |
| 4 | Dead + Wind 60 deg - No Ice |  |
| 5 | Dead+Wind 90 deg - No lce |  |
| 6 | Dead+Wind 120 deg - No Ice |  |
| 7 | Dead + Wind 150 deg - No Ice |  |
| 8 | Dead+Wind 180 deg - No lce |  |
| 9 | Dead+Wind 210 deg - No Ice |  |
| 10 | Dead+Wind 240 deg - No Ice |  |
| 11 | Dead+Wind 270 deg - No Ice |  |
| 12 | Dead + Wind 300 deg - No lce |  |
| 13 | Dead+Wind 330 deg - No lce |  |
| 14 | Dead+Ice |  |
| 15 | Dead+Wind 0 deg+Ice |  |
| 16 | Dead+Wind 30 deg+Ice |  |
| 17 | Dead + Wind $60 \mathrm{deg}+$ Ice |  |
| 18 | Dead+Wind 90 deg+Ice |  |
| 19 | Dead+Wind $120 \mathrm{deg}+$ Ice |  |
| 20 | Dead + Wind $150 \mathrm{deg}+$ Ice |  |
| 21 | Dead+Wind $180 \mathrm{deg}+$ Ice |  |
| 22 | Dead+Wind 210 deg+lce |  |
| 23 | Dead+Wind $240 \mathrm{deg}+$ Ice |  |
| 24 | Dead+Wind 270 deg+lce |  |
| 25 | Dead + Wind $300 \mathrm{deg}+$ Ice |  |
| 26 | Dead+Wind $330 \mathrm{deg}+$ Ice |  |
| 27 | Dead+Wind 0 deg - Service |  |
| 28 | Dead+Wind 30 deg - Service |  |
| 29 | Dead+Wind 60 deg - Service |  |
| 30 | Dead+Wind 90 deg - Service |  |
| 31 | Dead+Wind 120 deg - Service |  |
| 32 | Dead+Wind 150 deg - Service |  |
| 33 | Dead+Wind 180 deg - Service |  |
| 34 | Dead+Wind 210 deg - Service |  |
| 35 | Dead+Wind 240 deg - Service |  |
| 36 | Dead+Wind 270 deg - Service |  |
| 37 | Dead+Wind 300 deg - Service |  |
| 38 | Dead+Wind 330 deg - Service |  |

## Maximum Reactions

| Location | Condition | Gov. <br> Load <br> Comb. | Vertical $K$ | $\begin{gathered} \text { Horizontal, } X \\ K \end{gathered}$ | $\begin{gathered} \text { Horizontal, Z } \\ K \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Leg C | Max Vert | 23 | 381.70 | 3002 | -17.33 |
|  | Max. $\mathrm{H}_{\mathrm{x}}$ | 23 | 381.70 | 30.02 | -17.33 |
|  | Max. $\mathrm{H}_{7}$ | 4 | -317.61 | -24.53 | 14.16 |
|  | Min Vert | 4 | -317.61 | -24.53 | 14.16 |
|  | Min. $\mathrm{H}_{\mathrm{x}}$ | 4 | -317.61 | -24.53 | 14.16 |
|  | Min. $\mathrm{H}_{2}$ | 23 | 381.70 | 30.02 | -17.33 |
| $\operatorname{leg} B$ | Max Vert | 19 | 38131 | -30.04 | -17.26 |
|  | Max $\mathrm{H}_{x}$ | 12 | -307.21 | 23.88 | 13.70 |
|  | Max. $\mathrm{H}_{\mathrm{z}}$ | 12 | -307.21 | 23.88 | 1370 |
|  | Min Vert | 12 | -307.21 | 23.88 | 13.70 |
|  | $\operatorname{Min} \mathrm{H}_{\mathrm{x}}$ | 19 | 381.31 | -30.04 | -17.26 |
|  | Min. H , | 19 | 381.31 | -30.04 | -17.26 |
| $\operatorname{Leg} A$ | Max Vert | 15 | 38131 | -0) 07 | 34.64 |
|  | Max. $\mathrm{H}_{\mathrm{x}}$ | 24 | 37.95 | 2.17 | 2.72 |
|  | Max H, | 15 | 381.31 | -0.07 | 34.64 |
|  | Min. Vert | 8 | -307.21 | 0.08 | -27 53 |
|  | $\mathrm{Min} \mathrm{H}_{x}$ | 18 | 45.39 | -2.24 | 327 |
|  | Min H, | 8 | -307.21 | 008 | -27.53 |


| Job | New 285 Self-Supporting Tower | Page |
| :--- | :--- | :--- |
| Project | 14 of 24 |  |
| Monkey's Eyebrow, Kentucky (PJF \#01905-029) |  |  |$\quad$| Date |
| :--- |
| Client |
|  |
| FWT, Inc. | | Designed by |
| :--- |
| Craig Meierhoffer |


| Location | Condition | Gov | Vertical | Horizontal, $X$ | Horizontal, $Z$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Load | $K$ | $K$ | $K$ |  |

## Tower Mast Reaction Summary

| L.oad Combination | Vertical <br> K | Shear $_{x}$ <br> $K$ | Shear: <br> $K$ | Overturning Moment, $M_{x}$ $k i p-f t$ | Overturning Moment, $M_{z}$ $k i p-f l$ | Torque <br> $k i p-f t$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Dead Only | 75.14 | 0.00 | 0.00 | -1 | 1 | 0 |
| Dead + Wind 0 deg - No Ice | 75.14 | 1.74 | -51.30 | -8556 | -358 | -2 |
| Dead+Wind 30 deg - No Ice | 75.14 | 25.54 | -42.09 | -7118 | -4364 | 0 |
| Dead+Wind 60 deg - No lce | 75.14 | 42.16 | -24.34 | -4155 | -7196 | 0 |
| Dead + Wind 90 deg - No Ice | 75.14 | 49.22 | -1.07 | -220 | -8346 | 0 |
| Dead+Wind 120 deg - No lce | 75.14 | 45.30 | 24.15 | 3968 | -7589 | 2 |
| Dead+Wind 150 deg - No Ice | 75.14 | 23.94 | 41.49 | 6996 | -4034 | 3 |
| Dead+Wind 180 deg - No Ice | 75.14 | -0.22 | 47.45 | 8057 | 47 | 4 |
| Dead+Wind 210 deg - No lce | 75.14 | -2435 | 42.05 | 7112 | 4123 | 3 |
| Dead+Wind 240 deg - No lce | 75.14 | -44.45 | 25.66 | 4282 | 7417 | 0 |
| Dead+Wind 270 deg - No lce | 75.14 | -48.60 | 0.06 | 14 | 8221 | -3 |
| Dead+Wind 300 deg - No Ice | 75.14 | -41.21 | -23.53 | -3987 | 7001 | -4 |
| Dead+Wind 330 deg - No lce | 75.14 | -23.97 | -41.48 | -6991 | 4042 | -3 |
| Dead+Ice | 115.33 | 0.00 | 0.00 | 2 | 3 | 0 |
| Dead+Wind 0 deg+lce | 115.33 | 1.33 | -50.98 | -8314 | -273 | -1 |
| Dead+Wind $30 \mathrm{deg}+$ Ice | 115.33 | 24.37 | -40.57 | -6727 | -4078 | 0 |
| Dead+Wind $60 \mathrm{deg}+$ Ice | 115.33 | 40.03 | -23.11 | -3870 | -6704 | 0 |
| Dead+Wind 90 deg+lce | 115.33 | 47.32 | -0.82 | -169 | -7865 | 0 |
| Dead + Wind 120 deg+lce | 11533 | 44.82 | 24.34 | 3920 | -7337 | 1 |
| Dead + Wind $150 \mathrm{deg}+$ Ice | 115.33 | 23.15 | 40.12 | 6636 | -3824 | 2 |
| Dead+Wind 180 deg+lice | 11533 | -0.17 | 45.28 | 7550 | 38 | 3 |
| Dead+Wind $210 \mathrm{deg}+$ Ice | 11533 | -23.47 | 40.55 | 6725 | 3896 | 2 |
| Dead+Wind $240 \mathrm{deg}+$ Ice | 115.33 | -44.17 | 25.50 | 4162 | 7208 | 0 |
| Dead+Wind 270 deg+Ice | 115.33 | -46.85 | 0.05 | 12 | 7772 | -2 |
| Dead+Wind $300 \mathrm{deg}+$ Ice | 115.33 | -39.30 | -22.49 | -3742 | 6558 | -3 |
| Dead+Wind $330 \mathrm{deg}+$ lce | 115.33 | -23.17 | -40.11 | -6630 | 3835 | -2 |
| Dead+Wind 0 deg - Service | 75.14 | 0.77 | -22.80 | -3802 | -158 | -1 |
| Dead+Wind 30 deg - Service | 75.14 | 11.35 | -18.71 | -3163 | -1939 | 0 |
| Dead+Wind 60 deg - Service | 75.14 | 18.74 | -10.82 | -1846 | -3197 | 0 |
| Dead+Wind 90 deg - Service | 75.14 | 21.87 | -0.48 | -97 | -3709 | 0 |
| Dead+Wind 120 deg - Service | 75.14 | 20.13 | 10.73 | 1764 | -3372 | 1 |
| Dead + Wind 150 deg - Service | 75.14 | 10.63 | 18.45 | 3110 | -1792 | 1 |
| Dead+Wind 180 deg - Service | 75.14 | -0.10 | 21.09 | 3581 | 22 | 2 |
| Dead + Wind 210 deg - Service | 75.14 | -10.82 | 18.69 | 3162 | 1833 | 1 |
| Dead+Wind 240 deg - Service | 75.14 | -19.76 | 11.41 | 1904 | 3297 | 0 |
| Dead+Wind 270 deg - Service | 75.14 | -21.60 | 0.03 | 7 | 3655 | -1 |
| Dead+Wind 300 deg - Service | 75.14 | -18.31 | -10.46 | -1772 | 3112 | -2 |
| Dead+Wind 330 deg - Service | 75.14 | -10.66 | -18.43 | -3107 | 1797 | $-1$ |

## Solution Summary

| Load | Sum of Applied Forces |  |  |  | $P Z$ | $P X$ | Sum of Reactions | $P Z$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |


| RISATower | New 285 Self-Supporting Tower |  | $\begin{aligned} & \text { Page } 15 \text { of } 24 \end{aligned}$ |
| :---: | :---: | :---: | :---: |
| Paul J. Ford and Company 250 East Broad Street, Suite 1500 | Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | $\begin{aligned} & \text { Date } \\ & \text { 14:56:06 11/10/05 } \end{aligned}$ |
| Columbus, OH 43215 <br> Phone: 614-221-6679 <br> FAX: 614-448-4105 | Client | FWT, Inc. | Designed by Craig Meierhoffer |


|  | Stom of Applied Forces |  |  | Sum of Reactions |  |  | \% Error |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Load | PX | PY | $P Z$ | $P X$ | PY | PZ |  |
| Comb. | $K$ | $K$ | $K$ | $K$ | $K$ | $K$ |  |
| 5 | 49.22 | -75.14 | -1.07 | -49.22 | 75.14 | 1.07 | 0.000\% |
| 6 | 45.30 | -75.14 | 24.15 | -45.30 | 75.14 | -24.15 | 0.000\% |
| 7 | 23.94 | -75.14 | 41.49 | -23.94 | 75.14 | -41.49 | 0.000\% |
| 8 | -0.22 | -75.14 | 47.45 | 0.22 | 75.14 | -47.45 | 0.000\% |
| 9 | -24.35 | -75.14 | 42.05 | 24.35 | 75.14 | -42.05 | 0.000\% |
| 10 | -44.45 | -75.14 | 25.66 | 44.45 | 75.14 | -25.66 | 0.000\% |
| 11 | -48.60 | -75.14 | 0.06 | 48.60 | 75.14 | -0.06 | 0.000\% |
| 12 | -4121 | -75.14 | -23.53 | 41.21 | 75.14 | 23.53 | 0.000\% |
| 13 | -2397 | -75.14 | -41.48 | 23.97 | 75.14 | 41.48 | 0.000\% |
| 14 | 0.00 | . 115.33 | 0.00 | 0.00 | 115.33 | 0.00 | 0.000\% |
| 15 | 133 | -115.33 | -50.98 | -1.33 | 115.33 | 50.98 | 0.000\% |
| 16 | 24.37 | -115.33 | -40.57 | -24.37 | 115.33 | 40.57 | 0.000\% |
| 17 | 40.03 | -115.33 | -23.11 | -40.03 | 115.33 | 23.11 | 0000\% |
| 18 | 47.32 | -115.33 | -0.82 | -47.32 | 115.33 | 0.82 | 0.000\% |
| 19 | 4482 | -115.33 | 24.34 | -44.82 | 115.33 | -24.34 | 0.000\% |
| 20 | 2315 | -115.33 | 40.12 | -23.15 | 115.33 | -40.12 | 0.000\% |
| 21 | -0.17 | -115.33 | 45.28 | 0.17 | 115.33 | -45 28 | 0.000\% |
| 22 | -23.47 | -115.33 | 40.55 | 23.47 | 115.33 | -40.55 | 0.000\% |
| 23 | -44.17 | -115.33 | 2550 | 44.17 | 115.33 | -25.50 | 0.000\% |
| 24 | -46.85 | -115.33 | 0.05 | 46.85 | 11533 | -0.05 | 0000\% |
| 25 | -39 30 | -11533 | -22.49 | 39.30 | 115.33 | 22.49 | 0.000\% |
| 26 | -23.17 | -115.33 | -40.11 | 23.17 | 115.33 | 40.11 | 0.000\% |
| 27 | 077 | -75.14 | -22.80 | -0.77 | 75.14 | 22.80 | 0.000\% |
| 28 | 11.35 | -75.14 | -18.71 | -11.35 | 75.14 | 18.71 | 0.000\% |
| 29 | 1874 | -75.14 | -10.82 | -18.74 | 75.14 | 10.82 | 0000\% |
| 30 | 21.87 | -7514 | -0.48 | -21.87 | 75.14 | 0.48 | 0.000\% |
| 31 | 2013 | -75.14 | 10.73 | -20.13 | 75.14 | -10.73 | 0.000\% |
| 32 | 1064 | -75.14 | 18.44 | -10.63 | 75.14 | -18.4.5 | 0.013\% |
| 33 | -0.10 | -75.14 | 21.09 | 0.10 | 75.14 | -21.09 | 0.000\% |
| 34 | -1082 | -75.14 | 1869 | 1082 | 75.14 | -18.69 | 0.000\% |
| 35 | -19.76 | -75.14 | 11.41 | 19.76 | 75.14 | -11.41 | 0.000\% |
| 36 | -21.60 | -75.14 | 0.03 | 21.60 | 75.14 | -0.03 | 0.000\% |
| 37 | -1831 | -75 14 | -10.46 | 18.31 | 75.14 | 10.46 | 0.000\% |
| 38 | -10.65 | -75.14 | -18.43 | 10.66 | 75.14 | 18.43 | 0.013\% |

Non-Linear Convergence Results

| Load <br> Combination | Converged? | Number <br> of Cycles | Displacement <br> Tolerance | Force <br> Tolerance |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Yes | 4 | 0.00000001 | 0.00000001 |
| 2 | Yes | 7 | 0.00000001 | 0.00000720 |
| 3 | Yes | 8 | 0.00000001 | 0.00000974 |
| 4 | Yes | 9 | 0.00000001 | 0.00000917 |
| 5 | Yes | 8 | 0.00000001 | 0.00000974 |
| 6 | Yes | 7 | 0.00000001 | 0.00000720 |
| 7 | Yes | 8 | 0.00000001 | 0.00000660 |
| 8 | Yes | 9 | 0.00000001 | 0.00000687 |
| 9 | Yes | 8 | 0.00000001 | 0.00000777 |
| 10 | Yes | 7 | 000000001 | 0.00000679 |
| 11 | Yes | 8 | 0.00000001 | 0.00000777 |
| 12 | Yes | 9 | 0.00000001 | 0.00000687 |
| 13 | Yes | 8 | 0.00000001 | 0.00000660 |
| 14 | Yes | 4 | 0.00000001 | 0.00000001 |
| 15 | Yes | Yes | 8 | 0.00000001 |


| RISATower <br> Paul J. Ford and Company <br> 250 East Broad Street, Suite 1500 | Job New 285 Self-Supporting Tower |  | $\begin{aligned} & \text { Page } \\ & 16 \text { of } 24 \end{aligned}$ |
| :---: | :---: | :---: | :---: |
|  | Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | $\begin{aligned} & \text { Date } \\ & \text { 14:56:06 11/10/05 } \end{aligned}$ |
| Columbus, OH 43215 <br> Phone: 614-221-6679 <br> FAX 614-448-4105 | Client | FWT, Inc. | Designed by Craig Meierhoffer |


| 18 | Yes | 8 | 0.00000001 | 0.00000932 |
| :--- | :---: | :---: | :---: | :---: |
| 19 | Yes | 6 | 0.00000001 | 0.00000801 |
| 20 | Yes | 8 | 0.00000001 | 0.00000699 |
| 21 | Yes | 9 | 0.00000001 | 0.00000662 |
| 22 | Yes | 8 | 0.00000001 | 0.00000787 |
| 23 | Yes | 6 | 0.00000001 | 0.00000699 |
| 24 | Yes | 8 | 0.00000001 | 0.00000787 |
| 25 | Yes | 9 | 0.00000001 | 0.00000662 |
| 26 | Yes | 8 | 0.00000001 | 0.00000699 |
| 27 | Yes | 4 | 0.00000001 | 0.00000477 |
| 28 | Yes | 5 | 0.00000001 | 0.00000404 |
| 29 | Yes | 5 | 0.00000001 | 0.00000587 |
| 30 | Yes | 5 | 0.00000001 | 0.00000404 |
| 31 | Yes | 4 | 0.00000001 | 0.00000477 |
| 32 | Yes | 14 | 0.00000001 | 0.00000000 |
| 33 | Yes | 5 | 0.00000001 | 0.00000512 |
| 34 | Yes | 5 | 0.00000001 | 0.00000350 |
| 35 | Yes | 4 | 0.00000001 | 0.00000435 |
| 36 | Yes | 5 | 0.00000001 | 0.00000350 |
| 37 | Yes | 5 | 0.00000001 | 0.00000512 |
| 38 | Yes | 14 | 0.00000001 | 0.00000000 |

## Maximum Tower Deflections - Service Wind

| Section <br> No | Elevation | Horz <br> Deflection <br> in | Gov <br> Load <br> Comb. | Tilt | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: |

Critical Deflections and Radius of Curvature - Service Wind

| Elevation <br> ft | Appurtenance | Gov Load Comb. | Deflection in | Tilt | Twist | Radius of Curvature $f t$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 285.00 | Beacon | 35 | 20.44 | 0.818 | 0.025 | 71114 |
| 265.00 | (4) SC 9014-DIN | 35 | 1702 | 0787 | 0.022 | 24602 |
| 245.00 | (4) SC'9014-DIN | 35 | 13.82 | 0.695 | 0.020 | 8889 |
| 225.00 | (4) SC 9014-DIN | 35 | 11.11 | 0.577 | 0.018 | 11134 |
| 205.00 | 8 ft standard | 35 | 8.85 | 0.485 | 0.016 | 13447 |

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$\left.$| Job | New 285 Self-Supporting Tower | Page |
| :--- | :--- | :--- |
| Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) |  |$\quad$| Date |
| :---: |
| $14: 56: 06$ 11/10/05 | \right\rvert\, | Designed by |
| :---: |
| Craig Meierhoffer |

## Maximum Tower Deflections - Design Wind

| Section <br> No. | Elevation | Horz <br> Deflection <br> in | Gov. <br> Load <br> Comb. | Tilt | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: |

Critical Deflections and Radius of Curvature - Design Wind

| Elevation ft | Appurtenance | Gov Load Comb. | Deflection <br> in | Tilt | Twist | Radius of Curvature $f 1$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 285.00 | Beacon. | 10 | 45.93 | 1838 | 0.056 | 32032 |
| 265.00 | (4) SC $9014-\mathrm{DIN}$ | 10 | 3827 | 1.769 | 0.051 | 11008 |
| 24500 | (4) SC $9014-\mathrm{DN}$ | 10 | 31.07 | 1.562 | 0.045 | 3962 |
| 225.00 | (4) SC 9014-DIN | 10 | 24.99 | 1.298 | 0.041 | 4960 |
| 205.00 | 8 ft standard | 10 | 19.91 | 1.091 | 0.036 | 5994 |

## Bolt Design Data

| Section No. | Elevation <br> $f t$ | Component Type | Boll Grade | Bolt Size in | Number Of Bolts | Maximum <br> Lond per Bolt K | Allowable Load K | Ratio <br> LoadAllowable | Allowable Ratio | Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 285 | Leg | A325N | 063 | 4 | 0.65 | 13.48 | 0.048 | 1.333 | Bolt Tension |
|  |  | Diagonal | A 325 N | 0.50 | 2 | 104 | 4.12 | 0252 | 1.333 | Bolt Shear |
|  |  | Top Girt | A 325 N | 0.63 | 1 | 064 | 6.44 | 0.100 | 1.333 | Bolt Shear |
| T2 | 280 | Leg | A 325 N | 0.63 | 4 | 760 | 13.50 | 0.563 | 1.333 | Bolt Tension |
|  |  | Diagonal | A 32.5 N | 0.50 | 2 | 2.18 | 4.12 | 0.529 | 1333 | Bolt Shear |
| T3 | 260 | Leg | A325N | 0.75 | 6 | 14.05 | 19.44 | 0.723 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 050 | 2 | 376 | 4.12 | 0.911 | 1333 | Bolt Shear |
| T4 | 240 | Leg | A 325 N | 075 | 6 | 19.45 | 19.44 | 1.001 | 1333 | Boll Tension |
|  |  | Diagonal | A 325 N | 0.50 | 2 | 187 | 4.12 | 0.454 | 1.333 | Bolt Shear |
| T5 | 220 | Leg | A325N | 0.88 | 6 | 24.01 | 26.46 | 0907 | 1.333 | Bolt Tension |
|  |  | Diagonal | A 325 N | 0.50 | 2 | 196 | 4.12 | 0.476 | 1333 | Bolt Shear |
| T6 | 200 | Leg | A325N | 0.88 | 6 | 27.72 | 26.46 | 1.048 | 1333 | Bolt Tension |
|  |  | Diagonal | A 325 N | 0.63 | 2 | 27.3 | 6.44 | 0.424 | 1.333 | Bolt Shear |
|  |  | Horizontal | A325N | 063 | 1 | 047 | 6.44 | 0074 | 1.333 | Bolt Shear |
| T7 | 180 | Leg | A 32.5 N | 100 | 6 | 3125 | 34.56 | 0904 | 1333 | Bolt Tension |



| Section No. | Elevation <br> $f t$ | Component Type | Bolt Grade | Bolt Size in | Number Of Bolts | Maximum Lond per Bolt K | Allowable load K | Ratio Load Allowable | Allowable Ratio | Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T8 | 160 | Diagonal | A 325 N | 0.63 | 2 | 2.41 | 6.44 | 0.374 | 1.333 | Bolt Shear |
|  |  | Horizontal | A325N | 0.63 | 1 | 046 | 6.44 | 0.071 | 1.333 | Bolt Shear |
|  |  | Leg | A 325 N | 1.00 | 6 | 34.39 | 34.56 | 0.995 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.63 | 2 | 2.44 | 6.44 | 0.379 | 1.333 | Bolt Shear |
| T9 | 140 | Horizontal | A 325 N | 0.63 | 1 | 0.44 | 6.44 | 0.069 | 1.333 | Bolt Shear |
|  |  | Leg | A325N | 100 | 6 | 37.29 | 34.56 | 1.079 | 1.333 | Bolt Tension |
|  |  | Diagonal | A 325 N | 0.63 | 2 | 258 | 6.44 | 0.401 | 1.333 | Bolt Shear |
| T10 | 120 | Horizontal | A325N | 0.63 | 1 | 0.44 | 6.44 | 0.069 | 1.333 | Bolt Shear |
|  |  | Leg | A 325 N | 100 | 6 | 40.08 | 34.56 | 1.160 | 1.333 | Bolt Tension |
|  |  | Diagonal | A 325 N | 0.63 | 2 | 2.75 | 6.44 | 0.426 | 1.333 | Bolt Shear |
| T11 | 100 | Horizontal | A 325 N | 0.63 | 1 | 0.44 | 6.44 | 0.069 | 1.333 | Bolt Shear |
|  |  | Leg | A325N | 1.13 | 6 | 42.76 | 43.74 | 0.978 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.63 | 2 | 3.00 | 6.44 | 0.466 | 1333 | Bolt Shear |
| T 12 | 80 | Horizontal | A325N | 0.63 | 1 | 0.48 | 6.44 | 0.074 | 1.333 | Bolt Shear |
|  |  | Leg | A325N | 113 | 6 | 45.35 | 43.74 | 1.037 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.63 | 2 | 3.43 | 12.89 | 0.266 | 1.333 | Bolt Shear |
| T13 | 60 | Horizontal | A325N | 0.63 | 1 | 0.58 | 6.44 | 0.091 | 1.333 | Bolt Shear |
|  |  | Leg | A325N | 1.25 | 6 | 46.89 | 54.00 | 0.868 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.63 | 2 | 4.59 | 12.89 | 0356 | 1.333 | Bolt Shear |
| T14 | 40 | Horizontal | A325N | 0.63 | 1 | 0.43 | 12.89 | 0.034 | 1.333 | Bolt Shear |
|  |  | Leg | A 325 N | 125 | 6 | 49.52 | 54.00 | 0.917 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.63 | 2 | 4.16 | 12.89 | 0.323 | 1.333 | Bolt Shear |
| T15 | 20 | Horizontal | A 325 N | 0.63 | 1 | 0.49 | 12.89 | 0.038 | 1.333 | Bolt Shear |
|  |  | Leg | A 307 | 1.75 | 6 | 51.68 | 48.11 | 1.074 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.63 | 2 | 462 | 12.89 | 0359 | 1.333 | Bolt Shear |
|  |  | Horizontal | A 325 N | 0.63 | 1 | 038 | 12.89 | 0029 | 1.333 | Bolt Shear |

## Compression Checks

## Leg Design Data (Compression)

| Section No. | Elevation | Size | $L$. | $L_{\text {u }}$ | Kl/r | $F_{n}$ |  | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. <br> $P_{n}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f 1$ | $f 1$ |  | ksi | $i 7^{2}$ | $K$ | K | $P_{4}$ |
| T1 | 285-280 | $2^{\prime \prime}$ solid | 5.00 | 5.00 | $\begin{gathered} 120.0 \\ K=1.00 \end{gathered}$ | 1037 | 314 | -3.65 | 32.58 | 0112 |
| T2 | 280-260 | 2" solid | 20.00 | 4.00 | $\begin{gathered} 96.0 \\ K=1.00 \end{gathered}$ | 1562 | 3.14 | -35.09 | 49.07 | 0.71 .5 |
| T3 | 260-240 | $21 / 2^{\prime \prime}$ solid | 20.00 | 500 | $\begin{gathered} 96.0 \\ K=1.00 \end{gathered}$ | 1562 | 4.91 | -92.66 | 76.67 | 1.209 |
| T4 | 240-220 | $23 / 4^{\prime \prime}$ solid | 20.03 | 501 | $\begin{gathered} 87.4 \\ K=1.00 \end{gathered}$ | 17.49 | 5.94 | -129.40 | 103.87 | 1.246 |
| T5 | 220-200 | 3 " solid | 20.03 | 501 | $\begin{gathered} 80.1 \\ K=1.00 \end{gathered}$ | 18.99 | 7.07 | -159.95 | 134.20 | 1192 |
| T6 | 200-180 | $3^{\prime \prime}$ solid | 20.03 | 501 | $\begin{gathered} 80.1 \\ K=1.00 \end{gathered}$ | 18.99 | 7.07 | -183.69 | 134.20 | 1369 |
| T7 | 180-160 | $\begin{gathered} \text { H1-3 (1.37CR) }-127 \\ 31 / 4^{\prime \prime} \text { solid } \end{gathered}$ | 20.03 | 5.01 | $\begin{gathered} 740 \\ K=100 \end{gathered}$ | 20.19 | 8.30 | -20729 | 16750 | 1238 |
| T8 | 160-140 | $31 / 4^{\prime \prime}$ solid | 20.03 | 501 | $\begin{gathered} 74.0 \\ K=1.00 \end{gathered}$ | 20.19 | 8.30 | $-22909$ | 167.50 | 1368 |

H1-3(1.37CR)-193

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Designed by
FWT, Inc.
Craig Meierhoffer

| Section No. | Elevation <br> $f t$ | Size | $L$ <br> $f t$ | $L_{u}$ <br> fi | $K l / r$ | $F_{a}$ $k s i$ | A $i n^{2}$ | Actual $P$ $K$ | Allow. $P_{a}$ K | $\begin{gathered} \hline \text { Ratio } \\ P \\ \hline P_{a} \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T9 | 140-120 | $31 / 21$ solid | 20.03 | 5.01 | $\begin{gathered} 68.7 \\ K=1.00 \end{gathered}$ | 21.18 | 9.62 | -250.18 | 203.78 | 1.228 |
| T10 | 120-100 | $31 / 2^{\prime \prime}$ solid | 20.03 | 5.01 | $\begin{gathered} 68.7 \\ K=1.00 \end{gathered}$ | 21.18 | 9.62 | $-271.02$ | 203.78 | 1.330 |
| T11 | 100-80 | $33 / 4$ " solid | 20.03 | 5.01 | $\begin{gathered} 64.1 \\ \mathrm{~K}=1.00 \end{gathered}$ | 22.00 | 11.04 | -292.06 | 243.03 | 1.202 |
| T12 | 80-60 | $33 / 4$ " solid | 20.03 | 5.01 | $\begin{gathered} 64.1 \\ \mathrm{~K}=1.00 \end{gathered}$ | 22.00 | 1104 | -313.34 | 243.03 | 1.289 |
| T13 | 60-40 | 4" solid | 20.03 | 5.01 | $\begin{gathered} 60.1 \\ \mathrm{~K}=1.00 \end{gathered}$ | 22.70 | 12.57 | -327.33 | 285.26 | 1147 |
| T14 | 40-20 | 4" solid | 20.03 | 5.01 | $\begin{gathered} 60.1 \\ \mathrm{~K}=1.00 \end{gathered}$ | 22.70 | 12.57 | -351.47 | 285.26 | 1.232 |
| T15 | 20-0 | 4" solid | 20.03 | 5.01 | $\begin{gathered} 60.1 \\ \mathrm{~K}=1.00 \end{gathered}$ | 22.70 | 12.57 | -371.30 | 285.26 | 1.302 |

## Diagonal Design Data (Compression)

| Section No. | Elevation | Size | $\bar{L}$ | $L_{u}$ | $K l / r$ | $F_{a}$ | $A$ | Actual P | Allow $P_{a}$ | Ratio $P$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f 1$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | K | K | $P_{n}$ |
| T1 | 285-280 | L $175 \times 1.75 \times 3 / 16$ | 640 | 284 | $\begin{gathered} 104.4 \\ K=1.05 \end{gathered}$ | 12.41 | 0.62 | -2.07 | 7.71 | 0.269 |
| T2 | 280-260 | L. $1.75 \times 1.75 \times 3 / 16$ | 5.66 | 2.48 | $\begin{gathered} 95.0 \\ K=1.10 \end{gathered}$ | 13.60 | 0.62 | -4.36 | 8.45 | 0.516 |
| T3 | 260-240 | 1. $1.75 \times 1.75 \times 3 / 16$ | 6.40 | 2.81 | $\begin{gathered} 1035 \\ K=1.06 \end{gathered}$ | 12.53 | 0.62 | -7.51 | 7.78 | 0.966 |
| T4 | 240-220 | L. $1.75 \times 1.75 \times 3 / 16$ | 7.62 | 3.60 | $\begin{gathered} 124.3 \\ \mathrm{~K}=0.99 \end{gathered}$ | 9.65 | 062 | -3.75 | 6.00 | 0.625 |
| T5 | 220-200 | L. $1.75 \times 1.75 \times 3 / 16$ | 9.22 | 4.38 | $\begin{gathered} 145.3 \\ K=0.95 \end{gathered}$ | 7.07 | 0.62 | -3.92 | 439 | 0.892 |
| T6 | 200-180 | L. $2.5 \times 2.5 \times 3 / 16$ | 6.73 | 6.06 | $\begin{gathered} 136.6 \\ K=0.93 \end{gathered}$ | 8.01 | 0.90 | -5.23 | 7.22 | 0.725 |
| T7 | 180-160 | L. $2.5 \times 2.5 \times 3 / 16$ | 7.81 | 7.15 | $\begin{gathered} 152.7 \\ K=0.88 \end{gathered}$ | 6.40 | 0.90 | -4.77 | 5.77 | 0.826 |
| 18 | 160-140 | L $2.5 \times 2.5 \times 3 / 16$ | 8.60 | 7.95 | $\begin{gathered} 164.7 \\ K=0.85 \end{gathered}$ | 5.51 | 0.90 | -4.88 | 4.97 | 0.982 |
| T9 | 140-120 | L. $3 \times 3 \times 3 / 16$ | 9.44 | 8.77 | $\begin{gathered} 154.8 \\ \mathrm{~K}=0.88 \end{gathered}$ | 6.23 | 1.09 | -5.17 | 6.79 | 0.760 |
| T10 | 120-100 | L. $3 \times 3 \times 3 / 16$ | 10.30 | 964 | $\begin{gathered} 165.5 \\ K=0.85 \end{gathered}$ | 5.45 | 1.09 | -5.49 | 5.94 | 0.924 |
| T11 | 100-80 | L $3 \times 3 \times 1 / 4$ | 11.18 | 1052 | $\begin{gathered} 177.2 \\ K=0.83 \end{gathered}$ | 4.75 | 1.44 | -5.86 | 683 | 0.857 |
| T12 | 80-60 | 2L. $25 \times 2.5 \times 3 / 16(1 / 2)$ | 11.63 | 10.97 | $\begin{gathered} 150.2 \\ K=0.89 \end{gathered}$ | 6.62 | 180 | -6.85 | 11.94 | 0.574 |
| T13 | 60-40 | 21. $25 \times 2.5 \times 3 / 16(1 / 2)$ | 15.62 | 14.92 | $\begin{gathered} 146.3 \\ K=1.00 \end{gathered}$ | 6.98 | 1.80 | -9.18 | 12.60 | 0.729 |
| T14 | 40-20 | 2L. $25 \times 2.5 \times 3 / 16(1 / 2)$ | 16.40 | 1570 | $\begin{gathered} 1540 \\ \mathrm{~K}=100 \end{gathered}$ | 630 | 180 | -8.01 | 11.36 | 0.705 |
| T15 | 20-0 | 2L. $25 \times 25 \times 3 / 16(1 / 2)$ | 17.21 | 16.51 | $\begin{gathered} 1619 \\ K=1.00 \end{gathered}$ | 5.70 | 180 | -9.25 | 10.28 | 0.899 |

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| Section No | Elevation <br> ft | Size | $\begin{aligned} & \bar{L} \\ & f t \end{aligned}$ | $\begin{gathered} \overline{L_{u}} \\ f t \end{gathered}$ | Kl/r | $F_{a}$ <br> ksi | A <br> $i n^{3}$ | $\begin{gathered} \text { Aclual } \\ P \\ K \end{gathered}$ | $\begin{gathered} \text { Allow. } \\ P_{\pi} \\ K \end{gathered}$ | $\begin{gathered} \text { Ratio } \\ P \\ \hline P_{a} \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T6 | 200-180 | L2 $2 \times 3 / 16$ | 9.50 | 8.92 | $\begin{gathered} 173.4 \\ \mathrm{~K}=1.00 \end{gathered}$ | 4.97 | 0.71 | -0.47 | 3.55 | 0.134 |
| T7 | 180-160 | L $2 \times 2 \times 3 / 16$ | 11.50 | 10.90 | $\begin{gathered} 211.9 \\ \mathrm{~K}=1.00 \end{gathered}$ | 3.33 | 0.71 | -0.46 | 2.38 | 0.193 |
| 18 | 160-140 | L $2 \times 2 \times 3 / 16$ | 13.50 | 12.90 | $\begin{gathered} 250.8 \\ \mathrm{~K}=1.00 \end{gathered}$ | 2.37 | 0.71 | -0.44 | 1.70 | 0.260 |
| T9 | 140-120 | $\begin{gathered} \mathrm{KL} / \mathrm{R}>250(\mathrm{C})-196 \\ \mathrm{~L} .5 \times 2.5 \times 3 / 16 \end{gathered}$ | 15.50 | 14.88 | $\begin{gathered} 229.2 \\ \mathrm{~K}=1.00 \end{gathered}$ | 2.84 | 0.90 | -0.44 | 2.56 | 0.173 |
| T10 | 120-100 | L2.5 $2.5 \times 3 / 16$ | 1750 | 16.88 | $\begin{gathered} 2600 \\ \mathrm{~K}=1.00 \end{gathered}$ | 2.21 | 0.90 | -0.44 | 1.99 | 0.223 |
| T11 | 100-80 | $\begin{gathered} \mathrm{KL} / \mathrm{R}>250(\mathrm{C})-268 \\ \mathrm{~L} 3 \times 3 \times 3 / 16 \end{gathered}$ | 19.50 | 18.85 | $\begin{gathered} 240.9 \\ \mathrm{~K}=1.00 \end{gathered}$ | 2.57 | 1.09 | -0.48 | 2.81 | 0.170 |
| T12 | 80-60 | L $3 \times 3 \times 3 / 16$ | 21.50 | 10.43 | $\begin{gathered} 209.8 \\ \mathrm{~K}=1.00 \end{gathered}$ | 3.39 | 1.09 | -0.58 | 3.70 | 0.158 |
| T13 | 60-40 | $2 \mathrm{~L} 2 \times 2 \times 3 / 16$ (1/2) | 23.00 | 11.17 | $\begin{gathered} 217.1 \\ \mathrm{~K}=1.00 \end{gathered}$ | 3.17 | 1.43 | -0.43 | 4.53 | 0096 |
| T14 | 40-20 | $2 \mathrm{~L} 2 \times 2 \times 3 / 16$ (1/2) | 25.00 | 12.17 | $\begin{gathered} 236.5 \\ \mathrm{~K}=1.00 \end{gathered}$ | 2.67 | 1.43 | -0.49 | 3.82 | 0.127 |
| T15 | 20-0 | $2 \mathrm{~L} 2.5 \times 2.5 \times 3 / 16$ (1/2) | 27.00 | 13.17 | $\begin{gathered} 203.0 \\ \mathrm{~K}=1.00 \end{gathered}$ | 3.62 | 180 | -0.38 | 6.54 | 0.058 |

## Top Girt Design Data (Compression)

| Section No | Elevation | Size | $L$ | $L_{u}$ | $K l / r$ | $F_{i}$ | A | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. <br> $P_{\pi}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f 1$ |  | $f t$ | $f t$ |  | $k s i$ | $i n^{2}$ | $K$ | $K$ | $P_{a}$ |
| T1 | 285-280 | $\mathrm{L} 2 \times 2 \times 3 / 16$ | 4.00 | 3.50 | $\begin{gathered} 1133 \\ \mathrm{~K}=1.06 \end{gathered}$ | 11.22 | 0.71 | -0. 64 | 8.02 | 0.079 |

## Redundant Horizontal (1) Design Data (Compression)

| Section No | Elevation | Size | $L$ | $L_{u}$ | $K l / r$ | $F_{n}$ | $A$ | Actual $P$ | Allow. $P_{a}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f 1$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | $K$ | $K$ | $P_{a}$ |
| T13 | $60-40$ | L $2 \times 2 \times 3 / 16$ | 575 | 5.58 | $\begin{gathered} 170.1 \\ K=1.00 \end{gathered}$ | 516 | 0.71 | -492 | 369 | 1.331 |
| T14 | 40-20 | L $25 \times 25 \times 3 / 16$ | 6.25 | 6.08 | $\begin{gathered} 147.5 \\ \mathrm{~K}=1.00 \end{gathered}$ | 687 | 0.90 | $-5.28$ | 619 | 0.852 |
| T15 | 20-0 | L $25 \times 25 \times 3 / 16$ | 6.75 | 6.58 | $\begin{gathered} 159.6 \\ K=1.00 \end{gathered}$ | 586 | 0.90 | $-5.58$ | 5.29 | 1.054 |

Redundant Diagonal (1) Design Data (Compression)

| Section <br> No | Elevation | Size | $L_{u}$ | $K l / r$ | $F_{a}$ | $A$ | Actual | Allow | Ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f i$ |  | $f t$ | $f t$ |  | $k s i$ | $i n^{2}$ | $P$ | $P_{a}$ |

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Columbus. OH 43215
Phone: 614-221-6679
FAX: 614-448-4105

Job New 285 Self-Supporting Tower

| Project | Monkey's Eyebrow, Kentucky (PJF \#01905-029) |
| :--- | :--- | | Date |
| :--- |
| Client |
|  |


| Section No. | Elevation | Size | $L$ | $L_{u}$ | $K l / r$ | $F_{a}$ | A | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. $P_{a}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | $k s i$ | $i n^{2}$ | $K$ | $K$ | $P_{a}$ |
| T13 | 60-40 | L $2.5 \times 2.5 \times 3 / 16$ | 781 | 7.59 | $\begin{gathered} 183.9 \\ K=1.00 \end{gathered}$ | 4.42 | 0.90 | -3.34 | 3.98 | 0.839 |
| T14 | 40-20 | L $3 \times 3 \times 3 / 16$ | 8.20 | 7.98 | $\begin{gathered} 160.7 \\ K=1.00 \end{gathered}$ | 5.78 | 1.09 | -3.46 | 6.30 | 0550 |
| T15 | 20-0 | L $3 \times 3 \times 3 / 16$ | 8.60 | 8.39 | $\begin{gathered} 168.9 \\ \mathrm{~K}=1.00 \end{gathered}$ | 5.23 | 109 | $-3.55$ | 5.70 | 0.623 |

## Inner Bracing Design Data (Compression)

| Section No. | Elevation | Size | $L$ | $\overline{L_{u}}$ | $K l / r$ | $\overline{F_{u}}$ | $A$ | Actual <br> $P$ | Allow. $P_{a}$ | $\begin{gathered} \hline \text { Ratio } \\ P \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | $i 7^{*}$ | K | K | $P_{a}$ |
| T12 | 80-60 | L3 $\times 3 \times 3 / 16$ | 10.75 | 10.75 | $\begin{gathered} 216.3 \\ \mathrm{~K}=1.00 \end{gathered}$ | 3.19 | 1.09 | -0.01 | 3.48 | 0.004 |
| T13 | 60-40 | L. $3 \times 3 \times 3 / 16$ | 11.50 | 11.50 | $\begin{gathered} 231.4 \\ \mathrm{~K}=1.00 \end{gathered}$ | 279 | 109 | -0.01 | 3.04 | $0.004^{*}$ |
| T14 | 40-20 | L $3 \times 3 \times 3 / 16$ | 12.50 | 12.50 | $\begin{gathered} 251.5 \\ \mathrm{~K}=1.00 \end{gathered}$ | 236 | 109 | -0.02 | 2.57 | 0.007 |
| T15 | 20-0 | $\begin{gathered} \mathrm{KL} / \mathrm{R}>250(\mathrm{C})-435 \\ \mathrm{~L} 3.5 \times 3.5 \times 1 / 4 \end{gathered}$ | 13.50 | 13.50 | $\begin{gathered} 233.4 \\ \mathrm{~K}=100 \end{gathered}$ | 2.74 | 1.69 | -0.01 | 4.63 | $0.003^{*}$ |

* DL controls


## Tension Checks



| Job | New 285 Self-Supporting Tower | Page |
| :--- | :--- | :--- |
| Project <br> Monkey's Eyebrow, Kentucky (PJF \#01905-029) | Date <br> $14: 56: 0611 / 10 / 05$ |  |
| Client | FWT, Inc. | Designed by <br> Craig Meierhoffer |

Diagonal Design Data (Tension)

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | A | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. <br> $P_{\text {a }}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | $K$ | K | $P_{n}$ |
| T1 | 285-280 | L. $175 \times 1.75 \times 3 / 16$ | 6.40 | 2.84 | 68.6 | 29.00 | 0.38 | 2.07 | 10.96 | 0.189 |
| T2 | 280-260 | L. $175 \times 1.75 \times 3 / 16$ | 5.66 | 2.48 | 60.6 | 29.00 | 038 | 430 | 10.96 | 0.392 |
| T3 | 260-240 | L $1.75 \times 1.75 \times 3 / 16$ | 6.40 | 2.81 | 678 | 29.00 | 0.38 | 7.19 | 10.96 | 0.656 |
| T4 | 240-220 | L. $1.75 \times 1.75 \times 3 / 16$ | 7.62 | 3.60 | 85.5 | 29.00 | 0.38 | 3.59 | 10.96 | 0.327 |
| T5 | 220-200 | L. $1.75 \times 1.75 \times 3 / 16$ | 9.22 | 4.38 | 103.1 | 29.00 | 0.38 | 3.92 | 10.96 | 0.358 |
| T6 | 200-180 | L $25 \times 2.5 \times 3 / 16$ | 6.40 | 5.74 | 96.0 | 29.00 | 0.57 | 5.04 | 16.56 | 0.305 |
| T7 | 180-160 | L $2.5 \times 2.5 \times 3 / 16$ | 7.07 | 6.41 | 106.3 | 29.00 | 0.57 | 4.64 | 16.56 | 0.280 |
| T8 | 160-140 | L. $2.5 \times 2.5 \times 3 / 16$ | 8.20 | 7.55 | 123.8 | 29.00 | 0.57 | 4.64 | 1656 | 0.280 |
| T9 | 140-120 | L. $3 \times 3 \times 3 / 16$ | 9.02 | 8.35 | 113.0 | 29.00 | 0.71 | 4.91 | 20.65 | 0.238 |
| T10 | 120-100 | L. $3 \times 3 \times 3 / 16$ | 986 | 9.21 | 123.9 | 29.00 | 071 | 528 | 2065 | 0.256 |
| T11 | 100-80 | L $3 \times 3 \times 1 / 4$ | 10.74 | 10.07 | 136.2 | 29.00 | 0.94 | 5.75 | 2719 | 0.211 |
| T 12 | 80-60 | $2 \mathrm{~L} 25 \times 2.5 \times 3 / 16$ (1/2) | 11.63 | 10.97 | 176.7 | 29.00 | 1.14 | 6.43 | 33.13 | 0.194 |
| T13 | 60-40 | 2L. $2.5 \times 2.5 \times 3 / 16(1 / 2)$ | 15.62 | 14.92 | 151.1 | 29.00 | 1.14 | 7.61 | 33.13 | 0.230 |
| T14 | 40-20 | 2L. $2.5 \times 2.5 \times 3 / 16(1 / 2)$ | 15.62 | 14.92 | 151.1 | 29.00 | 1.14 | 8.33 | 33.13 | 0.251 |
| T15 | 20-0 | 2L. $2.5 \times 25 \times 3 / 16(1 / 2)$ | 16.40 | 15.71 | 158.8 | 29.00 | 1.14 | 7.64 | 33.13 | 0.231 |

## Horizontal Design Data (Tension)

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | A | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow $P_{n}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | K | K | $P_{a}$ |
| T6 | 200-180 | L $2 \times 2 \times 3 / 16$ | 9.50 | 8.92 | 179.9 | 29.00 | 0.43 | 0.33 | 12.49 | 0.026 |
| T7 | 180-160 | L. $2 \times 2 \times 3 / 16$ | 11.50 | 10.90 | 218.4 | 29.00 | 0.43 | 0.33 | 12.49 | 0.027 |
| T8 | 160-140 | L. $2 \times 2 \times 3 / 16$ | 1350 | 12.90 | 2573 | 29.00 | 0.43 | 0.34 | 12.49 | 0.027 |
| T9 | 140-120 | L. $2.5 \times 2.5 \times 3 / 16$ | 15.50 | 14.88 | 234.4 | 29.00 | 0.57 | 0.34 | 16.56 | 0.021 |
| T10 | 120-100 | L $2.5 \times 2.5 \times 3 / 16$ | 17.50 | 16.88 | 2652 | 29.00 | 0.57 | 0.38 | 16.56 | 0.023 |
| Tl1 | 100-80 | L. $3 \times 3 \times 3 / 16$ | 19.50 | 18.85 | 2451 | 29.00 | 0.71 | 0.39 | 20.65 | 0.019 |
| T12 | 80-60 | L. $3 \times 3 \times 3 / 16$ | 2150 | 10.43 | 1353 | 29.00 | 071 | 0.40 | 20.65 | 0.019 |
| T13 | 60-40 | 2L. $2 \times 2 \times 3 / 16(1 / 2)$ | 23.00 | 11.17 | 2203 | 29.00 | 0.86 | 0.10 | 24.98 | 0.004 |
| T14 | 40-20 | 2 L $2 \times 2 \times 3 / 16(1 / 2)$ | 25.00 | 12.17 | 239.7 | 29.00 | 086 | 0.12 | 24.98 | 0.005 |
| T15 | 20-0 | 2L $25 \times 2.5 \times 3 / 16(1 / 2)$ | 27.00 | 13.17 | 205.6 | 29.00 | 1.14 | 0.04 | 33.13 | 0.001 |

Top Girt Design Data (Tension)

| Section No. | Elevation | Size | $L$. | $L_{*}$ | $K l / r$ | $F_{n}$ | A | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. $P_{a}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\cdots$ |  |  | $f t$ | $f t$ |  | ksi | in ${ }^{*}$ | K | $K$ | $P_{a}$ |
| Tl | 285-280 | $1.2 \times 2 \times 3 / 16$ | 4.00 | 3.50 | 74.6 | 29.00 | 0.43 | 0.64 | 12.49 | 0.052 |

Redundant Horizontal (1) Design Data (Tension)

# RISATower 

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| Project | Date |  |
| :--- | :--- | :--- |
|  | Monkey's Eyebrow, Kentucky (PJF \#01905-029) | 14:56:06 11/10/05 |
| Client | FWT, Inc. | Designed by <br> Craig Meierhoffer |
|  |  |  |


| Section No. | Elevation | Size | $L$ | $L_{u}$ | $K l / r$ | $F_{\text {r }}$ | $A$ | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. $P_{a}$ | Ratio $P$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | K | K | $P_{a}$ |
| T13 | 60-40 | L. $2 \times 2 \times 3 / 16$ | 5.75 | 5.58 | 108.6 | 21.60 | 0.71 | 4.92 | 15.44 | 0.318 |
| T14 | 40-20 | L $2.5 \times 2.5 \times 3 / 16$ | 6.25 | 6.08 | 93.7 | 21.60 | 0.90 | 5.28 | 19.48 | 0271 |
| T15 | 20-0 | L $2.5 \times 2.5 \times 3 / 16$ | 6.75 | 6.58 | 101.4 | 21.60 | 0.90 | 5.58 | 19.48 | 0.286 |

## Redundant Diagonal (1) Design Data (Tension)

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{\square}$ | A | Actual $P$ | Allow. <br> $P_{a}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | $k s i$ | $i n^{2}$ | K | K | $P_{\square}$ |
| T13 | 60-40 | $\mathrm{L} 2.5 \times 2.5 \times 3 / 16$ | 781 | 7.59 | 1169 | 21.60 | 0.90 | 334 | 19.48 | 0.171 |
| T14 | 40-20 | L $3 \times 3 \times 3 / 16$ | 8.20 | 7.98 | 102.0 | 21.60 | 1.09 | 3.46 | 23.54 | 0.147 |
| T15 | 20-0 | L. $3 \times 3 \times 3 / 16$ | 8.60 | 8.39 | 107.2 | 21.60 | 1.09 | 3.55 | 23.54 | 0.151 |


| Inner Bracing Design Data (Tension) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section No. | Elevation | Size | $L$ | $L_{u}$ | $K l / r$ | $F_{a}$ | A | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
|  | $f t$ |  | $f t$ | ft |  | ksi | $i n^{2}$ | $K$ | $K$ | $P_{n}$ |
| T12 | 80-60 | L $3 \times 3 \times 3 / 16$ | 10.75 | 10.75 | 137.3 | 21.60 | 109 | 0.01 | 2354 | 0.000 |
| T13 | 60-40 | L. $3 \times 3 \times 3 / 16$ | 11.50 | 11.50 | 146.9 | 21.60 | 1.09 | 0.01 | 23.54 | 0.000 |
| T14 | 40-20 | L. $3 \times 3 \times 3 / 16$ | 12.50 | 12.50 | 159.7 | 21.60 | 1.09 | 0.01 | 2354 | 0.000 |
| T15 | 20-0 | L. $35 \times 35 \times 1 / 4$ | 13.50 | 13.50 | 148.5 | 21.60 | 1.69 | 0.01 | 36.50 | 0.000 |

Section Capacity Table

| Section No. | $\begin{gathered} \text { Elevation } \\ f t \end{gathered}$ | Component Type | Size | Critical Element | $\begin{aligned} & P \\ & K \\ & K \end{aligned}$ | $\begin{gathered} S F^{*} P_{\text {ullow }} \\ K \end{gathered}$ | $\begin{gathered} \% \\ \text { Capacity } \end{gathered}$ | $\begin{gathered} \hline \text { Pass } \\ \text { Fail } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 285-280 | Leg | $2^{\prime \prime}$ solid | 1 | -3.65 | 4343 | 8.4 | Pass |
| T2 | 280-260 | Leg | $2^{\prime \prime}$ solid | 13 | -35.09 | 65.41 | 53.7 | Pass |
| T3 | 260-240 | Leg | $21 / 2^{\prime \prime}$ solid | 46 | -92.66 | 102.20 | 90.7 | Pass |
| T4 | 240-220 | Leg | 23/4" solid | 73 | -129.40 | 13846 | 93.5 | Pass |
| T5 | 220-200 | Leg | 3" solid | 100 | -159.95 | 178.89 | 89.4 | Pass |
| T6 | 200-180 | Leg | 3" solid | 127 | -183.69 | 178.89 | 102.7 | Pass |
| T7 | 180-160 | Leg | $31 / 4$ solid | 160 | -207.29 | 223.28 | 92.8 | Pass |
| T8 | 160-140 | L.eg | $31 / 4{ }^{\text {" }}$ solid | 193 | -22909 | 223.28 | 1026 | Pass |
| T9 | 140-120 | Leg | $31 / 2$ solid | 226 | -250.18 | 271.63 | 92.1 | Pass |
| T10 | 120-100 | L.eg | $31 / 2^{1}$ solid | 259 | -271.02 | 271.63 | 99.8 | Pass |
| T11 | 100-80 | Leg | $33 / 4$ solid | 292 | -292.06 | 323.96 | 90.2 | Pass |
| T12 | 80-60 | Leg | $33 / 4$ " solid | 325 | -313.34 | 323.96 | 96.7 | Pass |
| T13 | 60-40 | Leg | $4 "$ solid | 364 | -327.33 | 380.25 | 86.1 | Pass |
| T14 | 40-20 | Leg | $4^{\prime \prime}$ solid | 409 | -351.47 | 38025 | 92.4 | Pass |
| T15 | 20-0 | Leg | $4{ }^{\prime \prime}$ solid | 454 | -371.30 | 380.25 | 97.6 | Pass |
| T1 | 285-280 | Diagonal | L1 $75 \times 1.75 \times 3 / 16$ | 11 | -2.07 | 10.28 | 20.2 | Pass |
| T2 | 280-260 | Diagonal | L $175 \times 1.75 \times 3 / 16$ | 20 | -4.36 | 11.26 | 38.7 | Pass |
|  |  |  |  |  |  |  | 39.7 (b) 72.5 |  |
| 13 T4 | $260-240$ $240-220$ | Diagonal Diagonal | L $1.75 \times 175 \times 3 / 16$ L. $175 \times 1.75 \times 3 / 16$ | 53 77 | -7.51 -3.75 | 10.37 7.99 | 72.5 46.9 | Pass Pass |
| TS | 220-200 | Diagonal | L1 $75 \times 175 \times 3 / 16$ | 103 | -3.92 | 5.86 | 66.9 | Pass |

## RISATower

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FAX: 6/4-448-4105

$\left.$| Job | New 285 Self-Supporting Tower | Page |
| :--- | :--- | :--- |
| Project | 24 of 24 |  |
| Monkey's Eyebrow, Kentucky (PJF \#01905-029) |  |  |$\quad$| Date |
| :---: |
| $14: 56: 06$ 11/10/05 | \right\rvert\, | Designed by |
| :---: |
| Craig Meierhoffer |


| Section No. | $\begin{gathered} \text { Elevation } \\ f t \end{gathered}$ | Component Type | Size | Critical Element | $\begin{aligned} & P \\ & K \end{aligned}$ | $\begin{gathered} S F^{*} P_{\text {allow }} \\ K \end{gathered}$ | $\%$ <br> Capacity | $\begin{aligned} & \hline \text { Pass } \\ & \text { Fail } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T6 | 200-180 | Diagonal | L $2.5 \times 2.5 \times 3 / 16$ | 147 | $-5.23$ | 9.63 | 54.4 | Pass |
| T7 | 180-160 | Diagonal | L. $2.5 \times 2.5 \times 3 / 16$ | 171 | -4.77 | 7.70 | 62.0 | Pass |
| T8 | 160-140 | Diagonal | L $25 \times 25 \times 3 / 16$ | 204 | -4.88 | 6.62 | 73.7 | Pass |
| T9 | 140-120 | Diagonal | L $3 \times 3 \times 3 / 16$ | 237 | -5.17 | 9.06 | 57.0 | Pass |
| T10 | 120-100 | Diagonal | L. $3 \times 3 \times 3 / 16$ | 270 | -5.49 | 7.92 | 69.3 | Pass |
| T11 | 100-80 | Diagonal | L. $3 \times 3 \times 1 / 4$ | 297 | -5.86 | 9.11 | 64.3 | Pass |
| T12 | 80-60 | Diagonal | 2L $2.5 \times 2.5 \times 3 / 16(1 / 2)$ | 353 | -6.85 | 15.92 | 43.0 | Pass |
| T13 | 60-40 | Diagonal | 2L. $2.5 \times 2.5 \times 3 / 16$ (1/2) | 382 | -9.18 | 16.79 | 54.7 | Pass |
| T14 | 40-20 | Diagonal | 2L. $25 \times 2.5 \times 3 / 16$ (1/2) | 416 | -8.01 | 15.15 | 52.9 | Pass |
| T15 | 20-0 | Diagonal | $2 \mathrm{~L} 2.5 \times 2.5 \times 3 / 16(1 / 2)$ | 472 | $-9.25$ | 13.70 | 67.5 | Pass |
| T6 | 200-180 | Horizontal | L. $2 \times 2 \times 3 / 16$ | 130 | -0.47 | 4.73 | 10.0 | Pass |
| T7 | 180-160 | Horizontal | L $2 \times 2 \times 3 / 16$ | 163 | -0.46 | 3.17 | 14.4 | Pass |
| T8 | 160-140 | Horizontal | L $2 \times 2 \times 3 / 16$ | 196 | -0.44 | 2.26 | 19.5 | Pass |
| T9 | 140-120 | Horizontal | L. $25 \times 25 \times 3 / 16$ | 235 | -0.44 | 3.42 | 13.0 | Pass |
| T10 | 120-100 | Horizontal | L. $2.5 \times 2.5 \times 3 / 16$ | 268 | -0.44 | 2.66 | 16.8 | Pass |
| T11 | 100-80 | Horizontal | L. $3 \times 3 \times 3 / 16$ | 295 | -0.48 | 3.74 | 12.7 | Pass |
| T12 | 80-60 | Horizontal | L $3 \times 3 \times 3 / 16$ | 328 | -0.58 | 4.93 | 119 | Pass |
| T13 | 60-40 | Horizontal | $2 \mathrm{~L} 2 \times 2 \times 3 / 16$ (1/2) | 367 | -0.43 | 6.04 | 7.2 | Pass |
| T14 | 40-20 | Horizontal | $2 \mathrm{~L} 2 \times 2 \times 3 / 16(1 / 2)$ | 412 | -0.49 | 5.09 | 95 | Pass |
| T15 | 20-0 | Horizontal | 2L $2.5 \times 2.5 \times 3 / 16(1 / 2)$ | 471 | -0.38 | 8.72 | 4.3 | Pass |
| T1 | 285-280 | Top Girt | L. $2 \times 2 \times 3 / 16$ | 5 | -0.64 | 10.70 | 5.9 | Pass |
|  |  |  |  |  |  |  | 7.5 (b) |  |
| T13 | 60-40 | Redund Horz 1 Bracing | L. $2 \times 2 \times 3 / 16$ | 369 | -4.92 | 4.92 | 99.9 | Pass |
| T14 | 40-20 | Redund Horz 1 Bracing | L2 $2 \times 25 \times 3 / 16$ | 414 | -5.28 | 8.26 | 63.9 | Pass |
| T15 | 20-0 | Redund Horz 1 Bracing | L $25 \times 25 \times 3 / 16$ | 476 | -5.58 | 7.05 | 79.1 | Pass |
| T13 | 60-40 | Redund Diag 1 Bracing | L $25 \times 25 \times 3 / 16$ | 393 | -3.34 | 5.31 | 62.9 | Pass |
| T14 | 40-20 | Redund Diag 1 Bracing | L. $3 \times 3 \times 3 / 16$ | 438 | -3.46 | 840 | 412 | Pass |
| T15 | 20-0 | Redund Diag 1 Bracing | L. $3 \times 3 \times 3 / 16$ | 498 | -3.55 | 7.60 | 467 | Pass |
| T12 | 80-60 | Inner Bracing | L. $3 \times 3 \times 3 / 16$ | 355 | -0.01 | 3.83 | 0.5 | Pass |
| T13 | 60-40 | Inner Bracing | L $3 \times 3 \times 3 / 16$ | 389 | -0.01 | 3.04 | 05 | Pass |
| T14 | 40-20 | Inner Bracing | L. $3 \times 3 \times 3 / 16$ | 435 | -0.02 | 3.43 | 05 | Pass |
| T15 | 20-0 | Inner Bracing | L $35 \times 35 \times 1 / 4$ | 479 | -0.01 | 4.63 |  | Pass |
|  |  |  |  |  |  | Summary |  |  |
|  |  |  |  |  |  | Leg (T6) | 1027 | Pass |
|  |  |  |  |  |  | Diagonal (T8) | 737 | Pass |
|  |  |  |  |  |  | Horizontal (T8) | 195 | Pass |
|  |  |  |  |  |  | Top Girt (Tl) | 7.5 | Pass |
|  |  |  |  |  |  | Redund Horz I Bracing (T13) | 99.9 | Pass |
|  |  |  |  |  |  | Redund Diag 1 Bracing (T13) | 62.9 | Pass |
|  |  |  |  |  |  | Inner Bracing (T15) | 0.6 | Pass |
|  |  |  |  |  |  | Bolt Checks | 87.0 | Pass |
|  |  |  |  |  |  | RATING = | 102.7 | Pass |

Program Version 4.000-9/23/2005 File:T:/019_FWT_Inc/1905-029 eri



## EXHIBIT D

COMPETING UTILITIES, CORPORATIONS, OR PERSONS LIST AND MAP OF LIKE FACILITIES IN VICINITY

License Search

## Search Results

```
Specified Search
State = Kentucky
County = BALLARD
Radio Service = CL, CW
Status = Active
```

Matches 1-9 (of 9)

| Call Sign | Licensee Name | FRN | Radio Service | Status | Expiration Date |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 KNKN568 | Cellco Partnership | 0003290673 | CL | Active | 10/01/2010 |
| 2 KNKN830 | Orange Licenses Holding, LLC | 0012362919 | CL . | Active | 10/01/2011 |
| 3 KNKQ306 | KENTUCKY RSA NO. 1 PARTNERSHIP | 0001836709 | CL | Active | 10/01/2011 |
| 4 KNLF251 | New Cingular Wireless PCS, LLC | 0003291192 | CW | Active | 06/23/2015 |
| 5 KNL.F252 | WIRELESSCO, L.P. | 0002316545 | CW | Active | 06/23/2015 |
| 6 KNLH404 | POWERTEL KENTUCKY LICENSES, INC. | 0001831189 | CW | Active | 04/28/2007 |
| 7 KNLH405 | POWERTEL KENTUCKY LICENSES, INC. | 0001831189 | CW | Active | 04/28/2007 |
| 8 KNLH653 | Northstar Technology, LLC | 0005869136 | CW | Active | 04/28/2007 |
| 9 WPOI215 | BLUE LICENSES HOLDING, LLC | 0012362869 | CW | Active | 06/23/2015 |
| Call Sign | Licensee Name | FRN | Radio Service | Status | Expiration Date |

# BALLARD COUNTY, KENTUCKY CELLCO PARTNERSHIP SITE NAME: MONKEY'S EYEBROW TOWER LOCATION EXHIBIT 



## EXHIBIT E

## APPLICATION TO FAA

Notice of Proposed Construction or Alteration (7460-1)
Project Name: KENTU-000030436-05 Sponsor: Kentucky RSA 1 Partnership

Details for Case : Monkey's Eyebrow
Show Project Summary

| Case Status |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Status: Accepted |  | Date Submitted: 12/21/2005 |  |  |
| ASN: 2005-ASO-6483-OE |  | Date Accepted: |  |  |
| Next Step: None |  | Date Determined: |  |  |
|  |  | Letter: | ne |  |
| Construction / Alteration Information |  | Structure Summary |  |  |
| Notice Of: Construction |  | Structure Name: Monkey's Eyebrow |  |  |
| Duration: | Permanent | Structure Type: Tower |  |  |
| if Temporary : | Months: Days: | Other: |  |  |
| Work Schedule - Start: |  | FCC Number: |  |  |
| Work Schedule - End: |  | Prior ASN: |  |  |
| Structure Details |  | Common Frequency Bands |  |  |
| Latitude: <br> Longitude: | $37^{\circ} 10^{\circ} 55.43^{\prime \prime} \mathrm{N}$ | Low Freq High Freq <br> 869 894 <br> 1930 1990 <br> 2305 2310 | Freq Unit ERP <br> MHz 500 <br> MHz 1640 <br> MHz 2000 | ERP Unit <br> W <br> W <br> W |
|  | $88^{\circ} 56^{\prime} 43.75^{\prime \prime} \mathrm{W}$ |  |  |  |
|  | NAD83 |  |  |  |
| Horizontal Accuracy: | None | Specific Frequencies |  |  |
|  | 337 (nearest foot) |  |  |  |
| Site Elevation (SE): <br> Structure Height (AGL): | 325 (nearest foot) |  |  |  |  |  |
| Marking/Lighting: <br> Dual-red and medium intensity Other: |  |  |  |  |
|  |  |  |  |  |
| Nearest City: | Kevil |  |  |  |
| Nearest State: | Kentucky |  |  |  |
| Traverseway: | No Traverseway |  |  |  |
| Description of Location: | 4625 Ogden Colvin Circle Kevil, KY 42053 |  |  |  |
| Description of Proposal: | Applicant proposes to construct a 325 ft self support tower. |  |  |  |

## CELLCO PARTNERSHIP

1A Report
Site Name: MONKEY'S EYEBROW
Site No:

Date: February 9, 2005
FSTAN Project No: 05-3141

Site No:
For Aeronautical Study No.

| Location: | City <br> County | Kevll, Ky, <br> Ballard |
| :--- | :--- | :--- |
| U.S.G,S. Quadrangle: | Bandana, Ky, |  |
| (NAD 27) | LATITUDE |  |
|  | LONGITUDE | $37^{\circ} 10^{\prime} 55.24^{\prime \prime}$ |
|  |  | $88^{\circ} 56^{\prime} 43.52^{\prime \prime}$ |
| (NAD 83) | LATITUDE | $37^{\circ} 10^{\prime} 55.43^{\prime \prime}$ |
|  | LONGITUDE | $88^{\circ} 56^{\prime} 43.75^{\prime \prime}$ |

SITE ELEVATION (NAVD 88)
$337^{\prime} \pm$ AMSL
PROPOSED TOWER HEIGHT
PROPOSED LIGHTNING ARRESTOR HEIGHT OVERALL HEIGHT ELEVATION
$300^{\prime} \pm$ FAA AGL
$325^{\prime} \pm$ FAA AGL
$662^{\prime} \pm$ AMSL

I Certify, to the best of my knowledge and belief, that the horizontal and vertical datum as established from the referenced U.S.G.S. Quadrangle, is accurate to 1 A Reporting requirements of $t 20$ feet horizontally and $\pm 3$ feet vertcally.

The horizontal datum (coordinates) are in terms of the North American Datum of 1927 (NAD 27) and 1983 (NAD 83) and expressed as degrees, minutes and seconds.

The vertical datum (helghts) are in terms of the National Geodetic Vertical Datum of 1988 and are determined to the nearest foot.

Kontucky State Plane Coordinates (South Zone) were ostablighed with Trimble Global Positioning Syatems (GPS) reaeivers. This site bas ties to the National Geodetic Reference System established by the National Geodetic Survey, formerly the U.S. Coast \& Geodatic Survey by measurements to PID Station "HB0391", designated as "KEVIL".



## EXHIBIT F

APPLICATION TO KENTUCKY AIRPORT ZONING COMMISSION

## APPLICATION FOR PERMIT TO CONSTRUCT OR ALTER A STRUCTURE

1. APPLICANT - Name, Address, Telephone, Fax, etc.

Kentucky RSA 1 Partnership
30 Independence Blvd.
Warren, NJ 07059
908-607-8132

## 2. Representative of Applicant - Name, Address, Telephone, Fax

## Jennifer Flynn

Verizon Wireless
30 Independence Blvd
Warren, NJ 908-607-8132


FAA Aeronautical Study Number $\qquad$
9. Latitude: $37 \ldots{ }^{\circ} \quad 10 \quad$, $55.43 \ldots$
10. Longitude: 088 $\qquad$ ${ }^{\circ}$ $\qquad$ -' 43.75_"
11. Datum: $\triangle$ NAD $83 \square$ NAD $27 \square$ Other
12. Nearest Kentucky City Kevil County: Ballard
13. Nearest Kentucky public use or Military airport:

Shawnee Community College
14. Distance from \#13 to Structure 6.5818 NM
15. Direction from \#13 to Structure: 320.86 degrees
16. Site Elevation (AMSL):

337 Feet
17. Total Structure Height ( $A G L$ ): 325 _ Feet
18. Overall Height (\#16 + \#17) (AMSL): 662 Feet
19. Previous FAA and/or Kentucky Aeronautical Study Numbers):

Description of Location: (Attach a USGS 7.5 minute Quadrangle Map or an Airport Layout Drawing with the precise site marked and any certified survey)

4625 Ogden Colvin Circle Kevil, KY 42053
20. Description of Proposal:
We are proposing to construct a $\mathbf{3 2 5}$ ft self support tower.
$\square$ No
$X$ Yes, When 12/21/2005
been filed with the Federal Aviation Administration?
Yes, W hen 12/21/2005
CERTIFICATION; I hereby certify that all the above statements made by me are true, complete and correct to the best of my knowledge and belief.


PENALTIES: Persons failing to comply with Kqutućky RevisddStatutes (KRS 183.861 through 183.990) and Kentucky Administrative Regulations (602 KAR 050: Series) are liable for fines and/or indprisonment $d$ set forth in KRS 183.990 (3). Non-compliance with Federal Aviation Administration Regulations may result in further penalties.

## Commission Action:

Chairman, KAZCAdministrator, KAZC
$\square$ Disapproved

## EXHIBIT G

GEOTECHNICAL REPORT


# Land Surveyors \& Consuling Lingineers 

## GEOTECHNICAL ENGINEERING STUDY

Proposed Monkey's Eyebrow, Tower Site 4625 Ogden Colvin Circle, Kevil, Ballard County, Kentucky FStan Project No. 05-3142

FStan Land Surveyors \&<br>Consulting Engineers<br>2315 Crittenden Drive PO Pox 17546<br>Louisville, KY 40217<br>Phome: (502) 636-5111<br>Fax: (502) 636-5263

Prepared For:
Mis. Jama Luecke
Craig \& Associates
2508 Newburg Road
Louisville, KY 40205
Date: March 23, 2005

March 23, 2005
Ms. Jana Luecke
2508 Newburg Road
Louisville, KY 40205-2478
Re: Geotechnical Engineering Study
Proposed 300-foot SST
Cellco Partnership Site Name: Monkeys Eyebrow 4625 Ogden Colvin Circle, Kevil, Ballard County, KY 42053
FStan Project No. 05-3142
Dear Ms. Luecke:
Transmitted herewith is our geotechnical engineering report for the referenced project. This report contains our findings, an engineering interpretation of these findings with respect to the available project characteristics, and recommendations to aid design and construction of the tower foundations. We appreciate the opportunity to be of service to you on this project. If you have any questions regarding this report, please contact our office.


Copies submitted: (3) Ms. Jana Luecke

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## APPENDIX

# GEOTECHNICAL ENGINEERING INVESTIGATION 

Proposed 300-foot Self-Supporting Telecommunications Tower Cellco Partnership - Monkeys Eyebrow<br>4625 Ogden Colvin Circle, Kevil, Ballard County, Kentucky<br>FStan Project No. 05-3142

## 1. PURPOSE AND SCOPE

The purpose of this study was to determine the general subsurface conditions at the location of the proposed tower by drilling four soil test borings and to evaluate this data with respect to foundation concept and design for the proposed self-supported tower. Also included is an evaluation of the site with respect to potential construction problems and recommendations for quality control during construction.

## 2. PROJECT CHARACTERISTICS

Cellco Partnership is proposing to construct a 300 feet tall self-supporting communications tower on property owned by Billy Owsley located on 4625 Ogden Colvin Circle, Kevil, Ballard County, Kentucky. The site explored consists of a plowed agricultural field, located behind several barn structures that front Ogden Colvin Circle. The site topography is generally flat lying with topographic relief of about 3 feet. The site sloped gradually from the northeast property corner downhill to the southwest property corner. The approximate ground elevation at the anticipated tower center was 337 feet msl. An access road runs west from the site to Ogden Colvin Circle. The location of the proposed tower is shown on the Boring Location Plan in the Appendix.

Preliminary information provided us indicates that this project will consist of constructing a self-support communications tower 300 feet tall. We have assumed the following structural information:

- Compression $($ per leg $)=500 \mathrm{kips}$
- Uplift $($ Per Leg $)=400 \mathrm{kips}$
- Total shear $=45 \mathrm{kips}$

The development will also include a small equipment shelter near the base of the tower. The
wall and floor loads for the shelter are assumed to be less than $4 \mathrm{kip} / \mathrm{ln} . \mathrm{ft}$. and $200 \mathrm{lbs} / \mathrm{sq} . \mathrm{ft}$., respectively.

## Site Geology

The 1969 Bandana and Olmsted Geologic Quadrangle map indicates the tower site is underlain by Quaternary aged loess deposits. The loess deposits were formed by windblown soils deposited as dunes on the highest terrace surface in the Ohio River valley during the Illinoian and Wisconsin glaciation. The loess is typically yellowish brown, is unstratified with small amounts of clay and fine to coarse sand. The thickness of the loess over the quadrangle area can vary up to 40 feet thick. Below the Loess, Continental deposits were mapped. The upper portion of the continental deposits consists of yellowish to reddish brown silt and sand.

## 3. SUBSURFACE CONDITIONS

The subsurface conditions were explored by drilling 3 soil test borings near the center of the proposed tower as located and staked on site by the project surveyor. The Geotechnical Boring Logs, which are included in the Appendix, describes the materials and conditions encountered. A reference sheet defining the terms and symbols used on the boring logs has also been included in the Appendix. The general subsurface conditions disclosed by the test borings are discussed in the following paragraphs.

The thickness of the topsoil encountered at the boring locations was about 12 inches thick. Below the topsoil, the borings encountered brown to mottled brown and gray clayey silt to silty lean clay. The standard penetration test values ( N -values) ranged from 14 blows per foot (bpf) to 24 bpf. These values generally represent stiff to very stiff soil conditions. Our engineer using standard soil classification techniques classified the soil as CL/ML according to the unified soil classification system, USCS. The CL/ML soil was encountered to a depth of 33.5 feet in each of the borings.

Below the CL/ML soil, the borings encountered stiff mottled brown to reddish brown lean clay (USCS: CL). N-values of lean clay ranged from 22 bpf to 36 bpf , which represent very stiff to hard cohesive soil consistency. The soil was encountered to the predetermined boring
termination level of 40 feet in borings B-1 and B-2, and was encountered to a depth of 43.5 feet in boring B-3.

Below the lean clay the boring B-3 encountered reddish brown sandy lean clay to clayey sand to the boring termination level of 50 feet. The N -values obtained in this stratum were greater than 50 blows per 6 -inch increment; which represent dense cohesionless soil conditions. Boring B-3 was terminated at a depth of 50.0 feet. Refusal materials were not encountered in the borings advanced at this site.

Water was detected in our borings at depths ranging widely from 4.0 to 20.0 feet 24 hours after drilling. It must be noted however, that short-term water readings in test borings are not necessarily a reliable indication of the actual groundwater level. Furthermore, it must be emphasized that the groundwater level is not stationary, but will fluctuate seasonally.

According to the 2002 Kentucky Building Code, Ballard County, Kentucky is within seismic design category $E$ (an UBC equivalent seismic zone of 4). In this system, Zone $E$ is the most seismically active while Zone B has the lowest earthquake potential. Based on the limited subsurface conditions encountered at the site and using Table 1615.1.1 of the building code, the site class is considered D. Seismic design requirements for telecommunication towers are given in section 1622 of the code. A detailed seismic study was beyond the scope of this report.

## 4. GEOTECHNICAL DESIGN RECOMMENDATIONS

The following geotechnical design recommendations have been developed on the basis of the previously described project characteristics (Section 2.0) and subsurface conditions (Section 3.0). This office must be notified if the project description included herein is incorrect, or if the proposed structure location is changed, to establish if revisions to the following recommendations are necessary.

### 4.1. Tower

### 4.1.1 General

The following design recommendations are based on the previously described project information, the subsurface conditions encountered in our borings, the results of our laboratory testing, empirical correlations for the soil types encountered, our analyses, and our experience. If there is any change in the project criteria or structure location, you should retain us to review our recommendations so that we can determine if any modifications are required. The findings of such a review can then be presented in a supplemental report or addendum.

We recommend FStan be retained to review the near-final project plans and specifications, pertaining to the geotechnical aspects of the project, prior to bidding and construction. We recommend this review to check that our assumptions and evaluations are appropriate based on the current project information provided to us, and to check that our foundation and earthwork recommendations were properly interpreted and implemented.

### 4.1.1 Mat Foundation

Bearing Capacity: A mat foundation is recommended for support of the proposed tower foundation. We recommend the mat foundation be designed to act as a rigid structure. The mat foundation should bear on the stiff clayey silt that was encountered below about 5.0 feet in the borings at an allowable static net bearing pressure of 3000 kips per square foot (ksf). The mat foundation should be buried sufficiently deep to resist uplift and overturning forces. We estimate that the tower mat foundation designed and constructed in accordance with the guides of this report will result in total settlement of about 2.5 , inches and differential settlement of about one inch. If these settlement values are considered unacceptable FStan should be contacted for additional evaluation.

Modulus of Subgrade Reaction: Based on the conditions encountered by the borings and our experience, we recommend sizing the mat foundation for a modulus of subgrade reaction $\left(\mathrm{k}_{\mathrm{s}}\right)$ of 14 kcf . The $\mathrm{k}_{\mathrm{s}}$ value was determined using the estimated total settlement of 2.5 inches and the total contact pressure applied to the foundation subgrade. The total pressure applied to the
foundation subgrade beneath the mat was assumed to be distributed uniformly across the plan dimension of the mat. A more rigorous analysis, such as using the computer program PCAMats, was beyond the scope of our services.

Lateral Load Resistance: Lateral foundation load may be resisted using passive earth pressure. We recommend that the passive resistance of the upper 3 feet of the native silty lean clay to clayey silt stratum be neglected due to environmental effects and lack of confinement. The allowable passive earth pressure to resist lateral loads below this level is calculated as follows:

$$
\mathrm{P}_{\mathrm{p}}=40(\mathrm{D}-3)+1,000 \mathrm{psf}
$$

Where D is the depth to the level of interest.

### 4.2. Equipment Building

The equipment building may be supported on shallow spread footings bearing in the stiff to very stiff native clayey silt to silty lean clay sized for a maximum allowable soil pressure of 2,000 pounds per square foot. The footings should be at least 12 inches wide. The footings should bear at a depth of at least 30 inches. All existing fill, topsoil or soft natural soil should be removed beneath footings.

The floor slab for the new equipment building may be subgrade supported on a properly prepared subgrade. The slab should be designed and adequately reinforced to resist the loads proposed. The exposed subgrade should be carefully inspected by probing and testing as needed. Any organic material still in place, frozen or excessively soft soil and other undesirable materials should be removed.

Once the subgrade has been properly prepared and evaluated, fill may be placed to attain the desired final grade. Any non-organic, naturally occurring, non-expansive soils can be used for structural fill, including those encountered on this site, pending evaluation by the geotechnical engineer. If more than 3 feet of fill is placed below the tower foundation, the geotechnical engineer should be contacted.

### 4.3. Drainage and Groundwater Considerations

Good site drainage must be provided. Surface run-off water should be drained away from the shelter building and not allowed to pond.

At the time of this investigation, groundwater was encountered at depths ranging widely from 4 feet bgs to 38 feet bgs. We believe that ground water could be encountered during the foundation construction for the tower mat foundation; however, because the soils are silty, we believe that any seepage into the foundation excavation will be slow and that seepage water may be removed by pumping from a sump pit adjacent to the excavation.

## 5. GENERAL CONSTRUCTION PROCEDURES AND RECOMMENDATIONS

It is possible that variations in subsurface conditions will be encountered during construction. Although only minor variations that can be readily evaluated and adjusted for during construction are anticipated, it is recommended the geotechnical engineer or a qualified representative be retained to perform continuous inspection and review during construction of the soils-related phases of the work. This will permit correlation between the test boring data and the actual soil conditions encountered during construction.

### 5.1 Shallow Foundations

The following is recommended for the mat and equipment building foundations:

We recommend that foundation inspections be performed at the time of foundation construction in an effort to identify unsuitable soils and remove them prior to foundation construction. We recommend the foundation subgrades be protected from exposure to water. Surface run-off water should be drained away from the excavation and not allowed to pond. If possible, all concrete should be placed that same day the excavation is made. If this is not practical, the excavation should be adequately protected. The following guides address protection of footing subgrades and our recommended remediation for any soft soils encountered.

- Protect foundation support materials exposed in open excavations from freezing weather, severe drying, and water accumulation.
- Remove any soils disturbed by exposure prior to foundation concrete placement.
- Groundwater may be encountered in the tower foundation excavation and equipment building foundation excavations. Protect the silty foundation bearing surface by placing a "lean" concrete mud-mat over the bearing soils.
- Level or suitably bench the foundation bearing area.
- Remove loose soil, debris, and excess surface water from the bearing surface prior to concrete placement.
- Retain the geotechnical engineer to observe all foundation excavations and provide recommendations for treatment of any unsuitable conditions encountered.


### 5.2 Fill Compaction

All engineered fill placed adjacent to and above the tower foundation should be compacted to a dry density of at least 95 percent of the standard Proctor maximum dry density (ASTM D-698). This should be increased to 98 percent for any fill placed below the foundations of equipment building. The compaction should be accomplished by placing the fill in about 8 inch (or less) loose lifts and mechanically compacting each lift to at least the specified minimum dry density. Field density tests should be performed on each lift as necessary to insure that adequate moisture conditioning and compaction is being achieved.

Compaction by flooding is not considered acceptable. This method will generally not achieve the desired compaction and the large quantities of water will tend to soften the foundation soils.

### 5.3 Construction Dewatering

No serious dewatering problems are anticipated for shallow excavations; however 24 hour ground water levels were measured at depths ranging widely from 4 feet bgs to 20 feet bgs. Any seepage encountered should be slow and can be removed by pumping from a sump pit adjacent to the foundation excavation. At the time of our investigation, ground water was not
encountered. Depending upon seasonal conditions, some minor seepage into excavations may be experienced in shallow excavations. It is anticipated that any such seepage into shallow excavations can be handled by conventional dewatering methods such as pumping from sumps.

## 6. FIELD AND LABORATORY INVESTIGATION

The soil test boring was drilled at the tower center location established in the field by the project surveyor. Split-spoon samples were obtained by the Standard Penetration Test (SPT) procedure (ASTM D1586) in the test boring. The boring was extended to refusal materials. The refusal materials were sampled in one boring to the predetermined termination depth of 40.0 feet. The split-spoon and rock core samples were inspected and visually classified by a geotechnical engineer. Representative portions of the soil samples were sealed in glass jars and the rock core were placed in standard sample boxes and returned to our laboratory.

The boring logs are included in the Appendix along with a reference sheet defining the terms and symbols used on the $\log$ and an explanation of the Standard Penetration Test (SPT) procedure. The log presents visual descriptions of the soil strata encountered, Unified Soil Classification System designations, groundwater observations, sampling information, laboratory test results, and other pertinent field data and observations.

## 7. LIMITATIONS OF STUDY

Our professional services have been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. FStan is not responsible for the independent conclusions, opinions or recommendations made by others based on the field exploration and laboratory test data presented in this report.

This geotechnical study is inherently limited since the engineering recommendations are developed from information obtained from test borings that only depict subsurface conditions at that specific location, time and depths shown on the log. Soil conditions at other locations may differ from those encountered in the test borings, and the passage of time may cause the soil conditions to change from those described in this report.

The nature and extent of variation and change in the subsurface conditions at the site may not become evident until the course of construction. Construction monitoring by the geotechnical engineer or a representative is therefore considered necessary to verify the subsurface conditions and to check that the soils connected construction phases are properly completed. If significant variations or changes are in evidence, it may then be necessary to re-evaluate the recommendations of this report. Furthermore, if the project characteristics are altered significantly from those discussed in this report, if the project information contained in this report is incorrect, or if additional information becomes available, a review must be made by this office to determine if any modification in the recommendations will be required.

## APPENDIX

## BORING LOCATION PLAN <br> GEOTECHNICAL BORING LOG <br> SOIL SAMPLE CLASSIFICATION






## SOIL CLASSIFICATION CHART

| MAJOR DIVSIONS |  |  | SYMBOLS |  | TYPICAL DESCRIPTIONS |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | GRAPH | LETIER |  |
| $\begin{aligned} & \text { COARSE } \\ & \text { GRAINED } \\ & \text { SOILS } \end{aligned}$ | GRAVELANDGRAVELLYSOILS | CLEAN GRAVELS | - 0 | GW | WELL-GRADED GRAVELS, GRAVEL SAND MIXTURES, LTTLE OR NO FINES |
|  |  | (LTILE OR NO FINES) | $\left\{\begin{array}{l} 00 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{array} 0\right.$ | GP | POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITILE OR NO FINES |
|  | MORE THAN 50\% OF COARSE FRACTION RETANED ON NO. 4 SIEVE | GRAVELS WITH FINES | $\begin{aligned} & 0000 \\ & 00000 \\ & 009020 \end{aligned}$ | CM | SILTY GRAVELS, GRAVEL - SAND SILT MIXTURES |
|  |  | (APPRECIABIE AMOUNT OF FINES) |  | CC | CLAYEY GRAVELS, GRAVEL - SAND CLAY MIXTURES |
| MORE THAN 50\% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE |  | CLEAN SANDS | $\% \%$ | SW | WEL-GRADED SANDS, GRAVELLY SANDS, LITLE OR NO FINES |
|  |  | (LTTLE OR NO FINES) |  | SP | POORLY-GRADED SANDS, GRAVELY SAND, LTTLE OR NO FINES |
|  | MORE THAN 50\% OF COARSE FRACTION PASSING ON NO.4 SIEVE | $\underset{\text { FINES }}{\text { SANDS WITH }}$ |  | SM | SILTY SANDS, SAND - SLLT MIXTURES |
|  |  | (APPRECLABLE <br> ANOUNT OF FINES) | * | SC | CLAYEY SANDS, SAND-CLAY MEXURES |
| FINE GRAINED SOILS | SILTSANDCLAYS |  |  | ML | INORGNIC SUTS AND YERY FINE SANDS, ROCK FLOUR, SILTY OR SILTS WITH SLGGHT PLASTICTTY |
|  |  |  |  | CL | INORGANIC CLAYS OF LOW TO MEDHU PLASTICITY, GRAVELIY CLAYS, LEAN CLAYS |
|  |  |  |  | OL | ORGANIC SLLTS AND ORGANIC SILTY CLAYS OF LOW PLASTICTTY |
| MORE THAN 50\% OF MATERIAL IS SMALLER THAN NO. 200 SIEVESIZEE | $\begin{aligned} & \text { SILTS } \\ & \text { AND } \\ & \text { CLAYS } \end{aligned}$ | LCOMD Limit GREATER THAN 50 |  | MH | INORGANIC SILTS, MICACEOUS OR DATOMACEOUS FINE SAND OR SIITY SOILS |
|  |  |  |  | CH | MORGANIC CLAYS OF HEH PLASTICTTY |
|  |  |  |  | 匋 OH | ORGAHC CLAYS OF MEDMM TO HIGH PLASTICTY, ORGANIC SILTS |
| HIGHLY ORGANIC SOILS |  |  |  | PT | PEAT, HUMUS, SWAMP SOLLS WITH HIGH ORGANIC CONTENTS |

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOLL CLASSIFICATIONS

## EXHIBIT H COPY OF REAL ESTATE AGREEMENT

Site Name: Monkeys Eyebrow<br>Project No. 2004067191

DOC\# 07-22-96(5) Rev. 11/17/98

## OPTION AND LEASE AGREEMENT


#### Abstract

This Agreement made this $1^{5+}$ day of Che 1 . 2005 , between Billy Wayne Owsley, a single male, with a mailing address of 4625 Ogden Colvin Circle, Kevil, Kgatucky 42053, hereinafter designated LESSOR and Cellco Partnership, a Delaware general partnership, d/b/a Verizon Wireless, with its principal offices located at 180 Washington Valley Road, Bedminster, New Jersey, 07921, hereinafter designated LESSEE. The LESSOR and LESSEE are at times collectively referred to hereinafter as the "Parties" or individually as the "Party".


LESSOR is the owner of that certain real property located on Ogden Colvin Circle in Kevil, Ballard County, State of Kentucky, as shown on the Tax Map of the County of Ballard as Map 53, Lot 2, and being further described in Deed Cabinet 1, Drawer 24, Card 48799, as recorded in the Office of the Ballard County Court Clerk (the entirety of LESSOR's property is referred to hereinafter as the "Property").. LESSEE desires to obtain an option to lease a portion of said Property, with a right-of-way for access thereto (hereinafter referred to as the "Premises"), containing approximately ten thousand $(10,000)$ square feet, more specifically described as a 100 foot by 100 foot parcel and as substantially shown on Exhibit "A" attached hereto and made a part hereof.

## NOW THEREFORE,

, - LESSOR hereby grants to LESSEE the right and option to lease said Premises including a right-of-way for access thereto, for the term and in accordance with the covenants and conditions set forth herein.

The option may be exercised at any time on or prior to one year from the date of execution by Lessor. At LESSEE's election and upon LESSEE's prior written notification to LESSOR, the time during which the option may be exercised may be further extended for one additional period of one year through and including two years from the date of execution by Lessor,

1. The time during which the option may be exercised may be further extended by mutual agreement in writing. If during said option period, or during the term of the lease, if the option is exercised, the LESSOR decides to subdivide, sell or change the status of the Property or his property contiguous thereto he shall immediately notify LESSEE in writing so that LESSEE can take steps necessary to protect LESSEE's interest in the Premises.

This option may be sold, assigned or transferred by the LESSEE without any approval or consent of the LESSOR' to the LESSEE's principal, affiliates, subsidiaries of its principal; to any entity which acquires all or substantially all of LESSEE's assets in the market defined by the Federal Communications Commission in which the Property is located by reason of a merger, acquisition or other business reorganization; or to any entity which acquires or receives an interest in the majority of communication towers of the LESSEE in the market defined by the Federal Communications Commission in which the Property is located. As to other parties, this Agreement may not be sold, assigned or transferred without the written consent of the LESSOR, which such consent will not be unreasonably withheld or delayed.

Should LESSEE fail to exercise this option or any extension thereof within the time herein limited, all rights and privileges granted hereunder shall be deemed completely surrendered, this option terminated, and LESSOR shall retain all money paid for the option, and no additional money shall be payable by either Party to the other.

LESSOR shall cooperate with LESSEE in its effort to obtain all certificates, permits and other approvals that may be required by any Federal, State or Local authorities which will permit LESSEE use of the Premises. LESSOR shall take no action which would adversely affect the status of the Property with respect to the proposed use by LESSEE.

The LESSOR shall permit LESSEE, during the option period, free ingress and egress to the Premises to conduct such surveys, inspections, structural strength analysis, subsurface soil tests, and other activities of a similar nature as LESSEE may deem necessary, at the sole cost of LESSEE.

LESSOR agrees to execute a Memorandum of this Option to Lease Agreement which LESSEE may record with the appropriate Recording Officer. The date set forth in the Memorandum of Option to Lease is for recording purposes only and bears no reference to commencement of either term or rent payments.

Notice of the exercise of the option shall be given by LESSEE to the LESSOR in writing by certified mail, return receipt requested. Notice shall be deemed effective on the date it is posted. On the date of such notice the following agreement shall take effect:

## LEASE AGREEMENT

1. PREMISES. LESSOR hereby leases to LESSEE a portion of that certain parcel of property (the entirety of LESSOR's property is referred to hereinafter as the "Property") containing ten thousand ( 10,000 ) square feet situated on Map 53, Lot 2 all as shown on the Tax Map of the County of Ballard, Kentucky, together with the non-exclusive right for ingress and egress, seven (7) days a week, twenty-four (24) hours a day, on foot or motor vehicle, including trucks, and for the installation and maintenance of utility wires, poles, cables, conduits, and pipes over, under, or along a thirty (30) foot wide right-of-way extending from the nearest public right-of-way, Ogden Colvin Circle, to the demised premises, said demised premises and right-of-way (hereinafter referred to as the "Premises") for access being substantially as described herein in Exhibit "A" attached hereto and made a part hereof.

In the event any public utility is unable to use the aforementioned right-of-way, the LESSOR hereby agrees to grant an additional right-of-way either to the LESSEE or to the public utility at no cost to the LESSEE.
2. SURVEY. LESSOR also hereby grants to LESSEE the right to survey the Property and the Premises, and said survey shall then become Exhibit "B" which shall be attached hereto and made a part hereof, and shall control in the event of boundary and access discrepancies between it and Exhibit "A". Cost for such work shall be borne by the LESSEE.
3. TERM. This Agreement shall be for an initial term of five (5) years, and beginning on the date the option is exercised by LESSEE at an annual rental of : ) to be paid in equal monthly installments on the first day of the month, in advance, to Lessor, or to such other person, firm or place as the LESSOR may, from time to time, designate in writing at least thirty (30) days in advance of any rental payment date. The obligation to pay rent will begin immediately upon the exercise of the option, at which time rental payments and term will begin.
4. EXTENSIONS. This Agreement shall automatically be extended for four (4) additional five (5) year terms unless the LESSEE terminates it at the end of the then current term by giving the LESSOR written notice of the intent to terminate at least six (6) months prior to the end of the then current term.
5. EXTENSION RENTALS.

## Site Name: Monkeys Eyebrow

6. ADDITIONAL EXTENSIONS. If at the end of the fourth (4th) five (5) year extension term this Agreement has not been terminated by either Party by giving to the other written notice of an-intention to terminate it at least six (6) months prior to the end of such term, this Agreement shall continue in force upon the same covenants, terms and conditions for a further term of five (5) years and for five (5) year terms thereafter until terminated by either Party by giving to the other written notice of its intention to sn terminate at least six (6) months prior to the end of such term.
7. USE: GOVERNMENTAL APPROVALS. LESSEE shall use the Premises for the purpose of constructing, maintaining and operating a communications facility and uses incidental and all necessary appurtenances. A security fence consisting of chain link construction or similar but comparable construction may be placed around the perimeter of the Premises at the discretion of LESSEE (not including the access easement). All improvements shall be at LESSEE's expense and the installation of all improvements shall be at the discretion and option of the LESSEE. LESSEE shall have the right to replace, repair, add or otherwise modify its equipment or any portion thereof, whether the equipment is specified or not on any exhibit attached hereto, during the term of this Agreement. LESSEE will maintain the Premises in a good condition reasonable wear and tear excepted. LESSOR will maintain the Property, excluding the Premises, in good condition, reasonable wear and tear excepted. It is understood and agreed that LESSEE's ability to use the Premises is contingent upon its obtaining after the execution date of this Agreement all of the certificates, permits and other approvals that may be required by any Federal, State or Local authorities as well as satisfactory soil boring tests which will permit LESSEE use of the Premises as set forth above. LESSOR shall cooperate with LESSEE in its effort to obtain such approvals and shall take no action which would adversely affect the status of the Property with respect to the proposed use by LESSEE. In the event that any of such applications should be finally rejected or any certificate, permit, license or approval issued to LESSEE is canceled, expires, lapses, or is otherwise withdrawn or terminated by governmental authority or soil boring tests are found to be unsatisfactory so that LESSEE in its sole discretion will be unable to use the Property for its intended purposes or the LESSEE determines that the Premises is no longer technically compatible for its intended use, LESSEE shall have the right to terminate this Agreement. Notice of the LESSEE's exercise of its right to terminate shall be given to LESSOR in writing by certified mail, return receipt requested, and shall be effective upon the mailing of such notice by the LESSEE. All rentals paid to said termination date shall be retained by the LESSOR. Upon such termination, this Agreement shall become null and void and all the Parties shall have no further obligations including the payment of money, to each other.
8. INDEMNIFICATION. Each Party shall indemnify and hold the other harmless against any claim of liability or loss from personal injury or property damage resulting from or arising out of the use and occupancy of the Premises or the Property by the Party, its servants or agents, excepting, however, such claims or damages as may be due to or caused by the acts or omissions of the other Party, or its servants or agents.
9. INSURANCE. The Parties hereby waive any and all rights of action for negligence against the other which may hereafter arise on account of damage to the premises or to property, resulting from any fire, or other casualty of the kind covered by standard fire insurance policies with extended coverage, regardless of whether or not, or in what amounts, such insurance is now or hereafter carried by the Parties, or either of them. LESSOR and LESSEE each agree that at its own cost and expense, each will maintain comprehensive general liability and property liability insurance with liability limits of not less than or injury to or death of one or more persons in any one occurrence for damage or destruction to property in any one occurrence. LESSOR agrees that LESSEE may self-insure against any loss or damage which could be covered by a comprehensive general public liability insurance policy.
10. ANNUAL TERMINATION. Notwithstanding anything to the contrary contained herein, provided LESSEE is not in default hereunder and shall have paid all rents and sums due and payable to the LESSOR by LESSEE, LESSEE shall have the right to terminate this Agreement upon the annual anniversary of this Agreement provided that three (3) months prior notice is given the LESSOR.
11. LNTERFERENCE. LESSOR agrees that LESSOR and/or any other tenants of the Property who currently have or in the future take possession of the Property will be permitted to install only such radio equipment that is of the type and frequency which will not cause measurable interference the existing equipment of the LESSEE. The Parties acknowledge that there will not be an adequate remedy at law for non-compliance with the provisions of this paragraph and therefore, LESSEE shall have the right to specifically enforce the provisions of this paragraph in a court of competent jurisdiction.
12. REMOVAL UPON TERMINATION. LESSEE, upon termination of the Agreement, shall, within ninety (90) days, remove its building(s), antenna structure(s) (except footings), fixtures and all personal property and otherwise restore the Property to its original condition, reasonable wear and tear excepted. If such time for removal causes LESSEE to remain on the Property after termination of this Agreement, LESSEE shall pay rent at the then existing monthly rate or on the existing monthly pro-rata basis if based upon a longer payment term, until such time as the removal of the building, antenna structure, fixtures and all personal property are completed.
13. RIGHT OF FIRST REFUSAL. If the LESSOR during the lease term or any extension of the lease term elects to sell all or any portion of the Property, whether separately or as part of the larger parcel of which the Property are a part, the LESSEE shall have the right of first refusal to meet any bona fide offer of sale on the same terms and conditions of such offer. If LESSEE fails to meet such bona fide offer within thirty (30) days after notice thereof from LESSOR, LESSOR may sell the Property or portion thereof to such third person in accordance with the terms and conditions of his offer. For purposes of this Paragraph, any transfer, bequest or devise of the LESSOR's interest in the Property as a result of the death of the LESSOR, whether by will or intestate succession, shall not be considered a sale of the Property for which the LESSEE has any right of first refusal.
14. RIGHTS UPON SALE. Should the LESSOR, at any time during the term of this Agreement, decide to sell all or any part of the Property to a purchaser other than LESSEE, such sale shall be under and subject to this Agreement and LESSEE's rights hereunder, and any sale by the LESSOR of the portion of this Property underlying the right-of-way herein granted shall be under and subject to the right of the LESSEE in and to such right-of-way.
15. QUIET ENJOYMENT. LESSOR covenants that LESSEE, on paying rent and performing the covenants shall peaceably and quietly have, hold and enjoy the Premises.
16. TITLE. LESSOR covenants that LESSOR is seized of good and sufficient title and interest to the Property and has full authority to enter into and execute this Agreement. LESSOR further covenants that there are no other liens, judgments or impediments of title on the Property, or affecting LESSOR's title to the same and that there are no covenants, easements or restrictions which prevent the use of the Premises by the LESSEE as set forth above.
17. INTEGRATION. It is agreed and understood that this Agreement contains all agreements, promises and understandings between the LESSOR and LESSEE and that no verbal or oral agreements, promises or understandings shall be binding upon either the LESSOR or LESSEE in any dispute, controversy or proceeding at law, and any addition, variation or modification to this Agreement shall be void and ineffective unless made in writing and signed by the Parties. In the event any provision of the Agreement is found to be invalid or unenforceable, such finding shall not effect the validity and enforceability of the remaining provisions of this Agreement. The failure of either Party to insist upon strict performance of any of the terms or conditions of this Agreement or to exercise any of its rights under

## Site Name: Monkeys Eyebrow

the Agreement shall not waive such rights and such Party shall have the right to enforce such rights at any time and take such action as may be lawful and authorized under this Agreement, either in law or in equity.
18. GOVERNINGLAW. This Agreement and the performance thereof shall be governed, interpreted, construed and regulated by the laws of the State in which the Property is located...
19. ASSIGNMENT. This Agreement may be sold, assigned or transferred by the LESSEE without any approval or consent of the LESSOR to the LESSEE's principal, affiliates, subsidiaries of its principal; to any entity which acquires all or substantially all of LESSEE's assets in the market defined by the Federal Communications Commission in which the Property is located by reason of a merger, acquisition or other business reorganization; or to any entity which acquires or receives an interest in the majority of communication towers of the LESSEE in the market defined by the Federal Communications Commission in which the Property is located. As to other parties, this Agreement may not be sold, assigned or transferred without the written consent of the LESSOR, which such consent will not be unreasonably withheld or delayed. LESSEE may sublet the Premises within its sole discretion, upon notice to LESSOR. Any sublease that is entered into by LESSEE shall be subject to the provisions of this Agreement and shall be binding upon the successors, assigns, heirs and legal representatives of the respective parties hereto.
20. NOTICES. All notices hereunder must be in writing and shall be deemed validly given if sent by certified mail, return receipt requested or by commercial courier, provided the courier's regular business is delivery service and provided further that it guarantees delivery to the addressee by the end of the next business day following the courier's receipt from the sender, addressed as follows (or any other address that the Party to be notified may have designated to the sender by like notice):

| LESSOR: | Billy Wayne Owsley <br> 4625 Ogden Colvin Circle <br> Kevil, KY 42053 |
| :--- | :--- |
|  |  |
| LESSEE: | Cellco Partnership <br> d/b/a Verizon Wireless |
|  | 180 Washington Valley Road <br> Bedminster, New Jersey 07921 <br> Attention: Network Real Estate |

Notice shall be effective upon mailing or delivering the same to a commercial courier, as permitted above.
21. SUCCESSORS. This Agreement shall extend to and bind the heirs, personal representatives, successors and assigns of the Parties hereto.
22. SUBORDINATION AND NON-DISTURBANCE. At LESSOR's option, this Agreement shall be subordinate to any mortgage or other security interest or other security interest by LESSOR which from time to time may encumber all or part of the Property or right-of-way; provided, however, every such mortgage or other security interest or other security interest shall recognize the validity of this Agreement in the event of a foreclosure of LESSOR's interest and also LESSEE's right to remain in occupancy of and have access to the Premises as long as LESSEE is not in default of this Agreement. LESSEE shall execute whatever instruments may reasonably be required to evidence this subordination clause. In the event the Property is encumbered by a mortgage or other security interest or other security interest, the LESSOR immediately after this Agreement is executed, will obtain and furnish to LESSEE, a

## Site Name: Monkeys Eyebrow

non-disturbance agreement for each such mortgage or other security interest or other security interest in recordable form. In the event the LESSOR defaults in the payment and/or other performance of any mortgage or other security interest encumbering the Property, LESSEE, may, at its sole option and without obligation, cure or correct LESSOR's default and upon doing so, LESSEE shall be subrogated to any and all rights, titles, liens and equities of the holders of such mortgage or security interest and the LESSEE shall be entitled to deduct and setoff against all rents that may otherwise become due under this Agreement the sums paid by LESSEE to cure or correct such defaults.
23. RECORDING. LESSOR agrees to execute a Memorandum of this Lease Agreement which LESSEE may record with the appropriate Recording Officer. The date set forth in the Memorandum of Lease is for recording purposes only and bears no reference to commencement of either term or rent payments.
24. DEFAULT. In the event there is a default by the LESSEE with respect to any of the provisions of this Agreement or its obligations under it, including the payment of rent, the LESSOR shall give LESSEE written notice of such default. After receipt of such written notice, the LESSEE shall have fifteen (15) days in which to cure any monetary default and thisty (30) days in which to cure any non-monetary default, provided the LESSEE shall have such extended period as may be required beyond the thirty (30) days if the nature of the cure is such that it reasonably requires more than thirty (30) days and the LESSEE commences the cure within the thirty ( 30 ) day period and thereafter continuously and diligently pursues the cure to completion. The LESSOR may not maintain any action or effect any remedies for default against the LESSEE unless and until the LESSEE has failed to cure the same within the time periods provided in this Paragraph.

## 25. ENVIRONMENTAL.

a. LESSOR will be responsible for all obligations of compliance with any and all environmental and industrial hygiene laws, including any regulations, guidelines, standards, or policies of any governmental authorities regulating or imposing standards of liability or standards of conduct with regard to any environmental or industrial hygiene conditions or concerns as may now or at any time hereafter be in effect, that are or were in any way related to activity now conducted in, on, or in any way related to the Property, unless such conditions or concerns are caused by the activities of the LESSEE.
b. LESSOR shall hold LESSEE harmless and indemnify the LESSEE from and assume all duties, responsibility and liability at LESSOR's sole cost and expense, for all duties, responsibilities, and liability (for payment of penalties, sanctions, forfeitures, losses, costs, or damages) and for responding to any action, notice, claim, order, summons, citation, directive, litigation, investigation or proceeding which is in any way related to: a) failure to comply with any environmental or industrial hygiene law, including without limitation any regulations, guidelines, standards, or policies of any governmental authorities regulating or imposing standards of liability or standards of conduct with regard to any environmental or industrial hygiene concerns or conditions as may now or at any time hereafter be in effect, unless such compliance results from conditions caused by the LESSEE; and b) any environmental or industrial hygiene conditions arising out of or in any way related to the condition of the Property or activities conducted thereon, unless such environmental conditions are caused by the LESSEE.
26. CASUALTY. In the event of damage by fire or other casualty to the Premises that cannot reasonably be expected to be repaired within forth-five (45) days following same or, if the Property is damaged by fire or other casualty so that such damage may reasonably be expected to disrupt LESSEE's operations at the Premises for more than forty-five (45) days, then LESSEE may at any time following such fire or other casualty, provided LESSOR has not completed the restoration required to permit LESSEE to resume its operation at the Premises, terminate this Lease upon fifteen (15) days written notice to LESSOR. Any such notice of termination shall cause this Lease to expire with the same force and effect as though the date set forth in such notice were the date originally set as the expiration date of this Lease and the parties shall make an appropriate adjustment, as of such termination date, with respect to
payments due to the other under this Lease. Notwithstanding the foregoing, all rental shall abate during the period of such fire or other casualty.
27. CONDEMNATION. In the event of any condemnation of the Property, LESSEE may terminate this Lease upon fifteen (15) days written notice to LESSOR if such condemnation may reasonably be expected to disrupt LESSEE's operations at the Premises for more than forty-five (45) days. LESSEE may on its own behalf make a claim in any condemnation proceeding involving the Premises for losses related to the antennas, equipment, its relocation costs and its damages and losses (but not for the loss of its leasehold interest). Any such notice of termination shall cause this Lease to expire with the same force and effect as though the date set forth in such notice were the date originally set as the expiration date of this Lease and the parties shall make an appropriate adjustment as of such termination date with respect to payments due to the other under this Lease.
28. SUBMISSION OF LEASE. The submission of this Lease for examination does not constitute an offer to lease the Premises and this Lease becomes effective only upon the full execution of this Lease by the Parties. If any provision herein is invalid, it shall be considered deleted from this Lease and shall not invalidate the remaining provisions of this Lease. Each of the Parties hereto warrants to the other that the person or persons executing this Lease on behalf of such party has the full right, power and authority to enter into and execute this Lease on such Party's behalf and that no consent from any other person or entity is necessary as a condition precedent to the legal effect of this Lease.
29. APPLICABLE LAWS. LESSEE shall use the Premises as may be required or as permitted by applicable laws, rules and regulations. LESSOR agrees to keep the Property in conformance with all applicable, laws, rules and regulations and agrees to reasonably cooperate with the LESSEE regarding any compliance required by the LESSEE in respect to its use of the Premises.
30. SURVIVAL. The provisions of the Agreement relating to indemnification from one Party to the other Party shall survive any termination or expiration of this Agreement. Additionally, any provisions of this Agreement which require performance subsequent to the termination or expiration of this Agreement shall also survive such termination or expiration.
31. CAPTIONS. The captions contained in this Agreement are inserted for convenience only and are not intended to be part of the Agreement. They shall not affect or be utilized in the construction or interpretation of the Agreement.

IN WITNESS WHEREOF, the Parties hereto have set their hands and affixed their respective seals the day and year first above written.

LESSOR: Billy Wayne Owsley


BY:


## LESSOR ACKNOWLEDGEMENT

## STATE OF Kentev $k_{i}$ ) COUNTY OF NT ${ }^{\circ}(\mathrm{Cac}+\mathrm{s})$

This instrument was subscribed, sworn to, and acknowledged before me by Billy Wayne Owsley, Lessor, on this $13^{\text {th }}$ day of $1 / \mathrm{Clc}$, 2005. My commission expires: $\qquad$ Talc, Delano
Notary Public State at Large
STATE OF Irduensa

COUNTY OF DCmidtien )

## LESSEE ACKNOWLEDGEMENT

On this, the St day of Sal y, 2005 before me, the subscriber, a Notary Public, in and for the State of Sustiaina , personally appeared to me Howard H. Bower, as authorized officer and/or agent of Cello Partnership, d/b/a Verizon Wireless, and in due form of law, acknowledged that he or she is authorized on behalf of said entity to execute all documents pertaining hereto and acknowledged to me that he or she executed the same as his or her voluntary act and deed on behalf of said entity.

IN TESTIMONY WHEREOF, I have hereunto set my hand and affixed my seal in said County and State on the day and year last above written. My commission expires:


## EXHIBIT I <br> NOTIFICATION LISTING

Billy Owsley
4625 Ogden Colvin CircleKevil, KY 42053
Nelwyn \& Asleigh Harned Bolin
74 Moonstone Drive
Franklin, NC 28734
Kenneth A. \& Sondra G. Owsley 4668 Monkey's Eyebrow Road
Kevil, KY 42053
Clara T. Randolph Estate
c/o Paul Gene Randolph
919 Sycamore Street
Murray , KY 42071
Louise L. Tilford
10815 Ogden Landing Road
Kevil, KY 42053
Jerry \& Rose Doom
4493 Monkey's Eyebrow Road
Kevil , KY 42053
Gregory Fondaw
802 Marrow Road
Kevil , KY 42053
Providence Missionary Baptist Church
4409 Monkey's Eyebrow Road
La Center, KY 42056
Providence Southern Missionary Baptist Church
4073 Monkey's Eyebrow Road
La Center, KY 42056
Gary \& Nancy Fondaw
3920 Woodville Road
Kevil, KY 42053

EXHIBIT J
COPY OF PROPERTY OWNER NOTIFICATION

# Notice of Proposed Construction of Wireless Communications Facility Site Name: Monkey's Eyebrow 

Dear Landowner:
Cellco Partnership, a Delaware General Partnership d/b/a, d/b/a Verizon Wireless has filed an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located at 4625 Ogden Colvin Circle, Kevil, Kentucky 42053 ( $37^{\circ} 10^{\prime} 55.43^{\prime \prime}$ North latitude, $88^{\circ} 56^{\prime} 43.75^{\prime \prime}$ West longitude). The proposed facility will include a 300 -foot tall antenna tower, plus related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

This notice is being sent to you because the Ballard County Property Valuation Administrator's records indicate that you own property that is within a 500' radius of the proposed tower site or contiguous to the property on which the tower is to be constructed. You have a right to submit testimony to the Kentucky Public Service Commission ("PSC"), either in writing or to request intervention in the PSC's proceedings on the application. You may contact the PSC for additional information concerning this matter at: Kentucky Public Service Commission, Executive Director, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2006-00035 in any correspondence sent in connection with this matter.

We have attached a map showing the site location for the proposed tower. Verizon Wireless' radio frequency engineers assisted in selecting the proposed site for the facility, and they have determined it is the proper location and elevation needed to provide quality service to wireless customers in the area. Please feel free to contact us toll free at (800) 516-4293 if you have any comments or questions about this proposal.

Sincerely,
David A. Pike
Attorney for Verizon Wireless
enclosure

## EXHIBIT K

## COPY OF COUNTY JUDGE/EXECUTIVE NOTICE

## VIA CERTIFIED MAIL

Hon. Bob Buchanan
Ballard County Judge Executive
Ballard County Courthouse
437 Ohio St.
P.O. Box 276

Wickliffe, KY 42087
RE: Notice of Proposal to Construct Wireless Communications Facility Kentucky Public Service Commission Docket No. 2006-00035 Site Name: Monkey's Eyebrow

Dear Judge Buchanan:
Cellco Partnership, a Delaware General Partnership d/b/a, d/b/a Verizon Wireless has filed an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located at 4625 Ogden Colvin Circle, Kevil, Kentucky 42053 ( $37^{\circ} 10^{\prime} 55.43^{\prime \prime}$ North latitude, $88^{\circ} 56^{\prime} 43.75^{\prime \prime}$ West longitude). The proposed facility will include a 300 -foot tall antenna tower, plus related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

You have a right to submit comments to the PSC or to request intervention in the PSC's proceedings on the application. You may contact the PSC at: Executive Director, Public Service Commission, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2006-00035 in any correspondence sent in connection with this matter.

We have attached a map showing the site location for the proposed tower. Cingular's radio frequency engineers assisted in selecting the proposed site for the facility, and they have determined it is the proper location and elevation needed to provide quality service to wireless customers in the area.

Please feel free to contact us with any comments or questions you may have.

Sincerely,

David A. Pike
Attorney for Verizon Wireless
Enclosure

## EXHIBIT L

COPY OF POSTED NOTICES

## MONKEY'S EYEBROW NOTICE SIGNS

Two signs at least (2) feet by four (4) feet in size, of durable material, with the text printed in black letters at least one (1) inch in height against a white background, except for the word "tower," which should be at least four (4) inches in height.

Cellco Partnership, d/b/a Verizon Wireless, proposes to construct a telecommunications tower on this site. If you have questions, please contact Pike Legal Group, PLLC, P.O. Box 369, Shepherdsville, KY 40165. (800) 516-4293, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2006-00035 in your correspondence.

Cellco Partnership, d/b/a Verizon Wireless, proposes to construct a telecommunications tower near this site. If you have questions, please contact Pike Legal Group, PLLC, P.O. Box 369, Shepherdsville, KY 40165 (800) 516-4293, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2006-00035 in your correspondence.

## EXHIBIT M <br> COPY OF RADIO FREQUENCY DESIGN SEARCH AREA



