RECEIVED

### COMMONWEALTH OF KENTUCKY BEFORE THE PUBLIC SERVICE COMMISSION

FEB 1 2 2016

PUBLIC SERVICE COMMISSION

In the Matter of:

THE APPLICATION OF	)	
NEW CINGULAR WIRELESS PCS, LLC	j	
FOR ISSUANCE OF A CERTIFICATE OF PUBLIC	) CASE	NO.: 2016-00076
CONVENIENCE AND NECESSITY TO CONSTRUCT	)	
A WIRELESS COMMUNICATIONS FACILITY	)	
IN THE COMMONWEALTH OF KENTUCKY	)	
IN THE COUNTY OF MORGAN	)	

SITE NAME: INDEX

# APPLICATION FOR CERTIFICATE OF PUBLIC CONVENIENCE AND NECESSITY FOR CONSTRUCTION OF A WIRELESS COMMUNICATIONS FACILITY

\* \* \* \* \* \*

New Cingular Wireless PCS, LLC, a Delaware limited liability company, d/b/a AT&T Mobility ("Applicant"), by counsel, pursuant to (i) KRS §§ 278.020, 278.040, 278.650, 278.665, and other statutory authority, and the rules and regulations applicable thereto, and (ii) the Telecommunications Act of 1996, respectfully submits this Application requesting issuance of a Certificate of Public Convenience and Necessity ("CPCN") from the Kentucky Public Service Commission ("PSC") to construct, maintain, and operate a Wireless Communications Facility ("WCF") to serve the customers of the Applicant with wireless communications services.

Applicants state that the within Application is substantially similar to the Application filed by Applicants previously in Case Number 2014-00074. A Certificate of Public Convenience and Necessity was issued for Case Number 2014-00074 on

August 14, 2014. The previously proposed tower has not been constructed based upon intervening changes to Applicants' deployment schedule for the subject facility. Applicant AT&T Mobility now requires for the subject facility to be constructed presently to address an existing service need, as discussed further herein.

In support of this Application, Applicant respectfully provides and states the following information:

- The complete name and address of the Applicant: New Cingular Wireless
   PCS, LLC, a Delaware limited liability company, d/b/a AT&T Mobility, having a local address of 601 West Chestnut Street, Louisville, Kentucky 40203.
- 2. Applicant proposes construction of an antenna tower for communications services, which is to be located in an area outside the jurisdiction of a planning commission, and Applicant submits this application to the PSC for a certificate of public convenience and necessity pursuant to KRS §§ 278.020(1), 278.040, 278.650, 278.665, and other statutory authority.
- The Certificate of Authority filed with the Kentucky Secretary of State for the Applicant entity was attached to a prior application and is part of the case record for PSC case number 2011-00473 and is hereby incorporated by reference.
- 4. The Applicant operates on frequencies licensed by the Federal Communications Commission ("FCC") pursuant to applicable FCC requirements. A copy of the Applicant's FCC license to provide wireless services is attached to this Application or described as part of **Exhibit A**, and the facility will be constructed and operated in

accordance with applicable FCC regulations.

- 5. The public convenience and necessity require the construction of the proposed WCF. The construction of the WCF will bring or improve the Applicant's services to an area currently not served or not adequately served by the Applicant by increasing coverage or capacity and thereby enhancing the public's access to innovative and competitive wireless communications services. The WCF will provide a necessary link in the Applicant's communications network that is designed to meet the increasing demands for wireless services in Kentucky's wireless communications service area. The WCF is an integral link in the Applicant's network design that must be in place to provide adequate coverage to the service area.
- 6. To address the above-described service needs, Applicant proposes to construct a WCF at 1999 Highway 460 West, West Liberty, KY 41472 (37° 53' 33.996" North latitude, 83° 17' 14.131" West longitude), on a parcel of land located entirely within the county referenced in the caption of this application. The property on which the WCF will be located is owned by Sarah G. Fannin, Robin Fannin and Farrell Fannin pursuant to a Deed recorded at Deed Book 173, Page 113 in the office of the Morgan County Clerk. The proposed WCF will consist of a 255-foot tall tower, with an approximately 10-foot tall lightning arrestor attached at the top, for a total height of 265-feet. The WCF will also include concrete foundations and a shelter or cabinets to accommodate the placement of the Applicant's radio electronics equipment and appurtenant equipment. The Applicant's equipment cabinet or shelter will be approved for use in the Commonwealth of Kentucky by the relevant building inspector. The WCF compound will be fenced and all access gate(s)

will be secured. A description of the manner in which the proposed WCF will be constructed is attached as **Exhibit B** and **Exhibit C**.

- 7. A list of utilities, corporations, or persons with whom the proposed WCF is likely to compete is attached as **Exhibit D**.
- 8. The site development plan and a vertical profile sketch of the WCF signed and sealed by a professional engineer registered in Kentucky depicting the tower height, as well as a proposed configuration for the antennas of the Applicant has also been included as part of **Exhibit B**.
- Foundation design plans signed and sealed by a professional engineer registered in Kentucky and a description of the standards according to which the tower was designed are included as part of Exhibit C.
- 10. Applicant has considered the likely effects of the installation of the proposed WCF on nearby land uses and values and has concluded that there is no more suitable location reasonably available from which adequate services can be provided, and that there are no reasonably available opportunities to co-locate Applicant's antennas on an existing structure. When suitable towers or structures exist, Applicant attempts to co-locate on existing structures such as communications towers or other structures capable of supporting Applicant's facilities; however, no other suitable or available co-location site was found to be located in the vicinity of the site.
- 11. A copy of the Determination of No Hazard to Air Navigation issued by the Federal Aviation Administration ("FAA") is attached as **Exhibit E**. Please note that the FAA approval documentation reflects a total structure height of 275'. This additional structure

height is based on a varying lightning arrestor height. Prior to construction, a revised FAA application will be filed reflecting a height no greater than 265' as approved by the Kentucky Airport Zoning Commission.

- A copy of the application for Kentucky Airport Zoning Commission ("KAZC")
   Approval to construct the tower is attached as Exhibit F.
- 13. A geotechnical engineering firm has performed soil boring(s) and subsequent geotechnical engineering studies at the WCF site. A copy of the geotechnical engineering report, signed and sealed by a professional engineer registered in the Commonwealth of Kentucky, is attached as **Exhibit G**. The name and address of the geotechnical engineering firm and the professional engineer registered in the Commonwealth of Kentucky who supervised the examination of this WCF site are included as part of this exhibit.
- 14. Clear directions to the proposed WCF site from the County seat are attached as **Exhibit H**. The name and telephone number of the preparer of **Exhibit H** are included as part of this exhibit.
- 15. Applicant, pursuant to a written agreement, has acquired the right to use the WCF site and associated property rights. A copy of the agreement or an abbreviated agreement recorded with the County Clerk is attached as **Exhibit I**.
- 16. Personnel directly responsible for the design and construction of the proposed WCF are well qualified and experienced. The tower and foundation drawings for the proposed tower submitted as part of **Exhibit C** bear the signature and stamp of a professional engineer registered in the Commonwealth of Kentucky. All tower designs

meet or exceed the minimum requirements of applicable laws and regulations.

- 17. The Construction Manager for the proposed facility is Kyle Ballard, and the identity and qualifications of each person directly responsible for design and construction of the proposed tower are contained **Exhibits B & C**.
- 18. As noted on the Survey attached as part of **Exhibit B**, the surveyor has determined that the site is not within any flood hazard area.
- 19. **Exhibit B** includes a map drawn to an appropriate scale that shows the location of the proposed tower and identifies every owner of real estate within 500 feet of the proposed tower (according to the records maintained by the County Property Valuation Administrator). Every structure and every easement within 500 feet of the proposed tower or within 200 feet of the access road including intersection with the public street system is illustrated in **Exhibit B**.
- 20. Applicant has notified every person who, according to the records of the County Property Valuation Administrator, owns property which is within 500 feet of the proposed tower or contiguous to the site property, by certified mail, return receipt requested, of the proposed construction. Each notified property owner has been provided with a map of the location of the proposed construction, the telephone number and address of the PSC, and has been informed of his or her right to request intervention. A list of the notified property owners and a copy of the form of the notice sent by certified mail to each landowner are attached as **Exhibit J** and **Exhibit K**, respectively.
- 21. Applicant has notified the applicable County Judge/Executive by certified mail, return receipt requested, of the proposed construction. This notice included the PSC

docket number under which the application will be processed and informed the County

Judge/Executive of his/her right to request intervention. A copy of this notice is attached as

Exhibit L.

- 22. Notice signs meeting the requirements prescribed by 807 KAR 5:063, Section 1(2) that measure at least 2 feet in height and 4 feet in width and that contain all required language in letters of required height, have been posted, one in a visible location on the proposed site and one on the nearest public road. Such signs shall remain posted for at least two weeks after filing of the Application, and a copy of the posted text is attached as **Exhibit M**. Notice of the location of the proposed facility has been published in a newspaper of general circulation in the county in which the facility is proposed to be located.
- 23. The general area where the proposed facility is to be located is on a mountaintop. No residential structures are located within a 500-foot radius of the proposed tower location.
- 24. The process that was used by the Applicant's radio frequency engineers in selecting the site for the proposed WCF was consistent with the general process used for selecting all other existing and proposed WCF facilities within the proposed network design area. Applicant's radio frequency engineers have conducted studies and tests in order to develop a highly efficient network that is designed to handle voice and data traffic in the service area. The engineers determined an optimum area for the placement of the proposed facility in terms of elevation and location to provide the best quality service to customers in the service area. A radio frequency design search area prepared in reference

to these radio frequency studies was considered by the Applicant when searching for sites for its antennas that would provide the coverage deemed necessary by the Applicant. A map of the area in which the tower is proposed to be located which is drawn to scale and clearly depicts the necessary search area within which the site should be located pursuant to radio frequency requirements is attached as **Exhibit N**.

- 25. All Exhibits to this Application are hereby incorporated by reference as if fully set out as part of the Application.
- 26. All responses and requests associated with this Application may be directed to:

David A. Pike Pike Legal Group, PLLC 1578 Highway 44 East, Suite 6 P. O. Box 369 Shepherdsville, KY 40165-0369

Telephone: (502) 955-4400 Telefax: (502) 543-4410

Email: <u>dpike@pikelegal.com</u>

WHEREFORE, Applicant respectfully request that the PSC accept the foregoing Application for filing, and having met the requirements of KRS §§ 278.020(1), 278.650, and 278.665 and all applicable rules and regulations of the PSC, grant a Certificate of Public Convenience and Necessity to construct and operate the WCF at the location set forth herein.

Respectfully submitted,

David A. Pike

Pike Legal Group, PLLC

1578 Highway 44 East, Suite 6

P. O. Box 369

Shepherdsville, KY 40165-0369

Telephone: (502) 955-4400 Telefax: (502) 543-4410 Email: dpike@pikelegal.com

Attorney for New Cingular Wireless PCS, LLC

d/b/a AT&T Mobility

### LIST OF EXHIBITS

A - FCC License Documentation

B - Site Development Plan:

500' Vicinity Map Legal Descriptions Flood Plain Certification

Site Plan

Vertical Tower Profile

C - Tower and Foundation Design

D - Competing Utilities, Corporations, or Persons List

E - FAA

F - Kentucky Airport Zoning Commission

G - Geotechnical Report

H - Directions to WCF Site

Copy of Real Estate Agreement

J - Notification Listing

K - Copy of Property Owner Notification

L - Copy of County Judge/Executive Notice

M - Copy of Posted Notices

N - Copy of Radio Frequency Design Search Area

# EXHIBIT A FCC LICENSE DOCUMENTATION

**ULS License** 

### AWS (1710-1755 MHz and 2110-2155 MHz) License - WQGD755 - New Cingular Wireless PCS, LLC

Call Sign WQGD755 Radio Service AW - AWS (1710-1755 MHz and

2110-2155 MHz)

Status Active Auth Type Regular

Market

Market BEA047 - Lexington, KY-TN-VA- Channel Block C

WV

Submarket 0 Associated 001730.00000000-

Frequencies 001735.00000000 (MHz) 002130.00000000 002135.00000000

**Dates** 

Grant 12/18/2006 Expiration 12/18/2021

Effective 12/05/2014 Cancellation

**Buildout Deadlines** 

1st 2nd

**Notification Dates** 

1st 2nd

Licensee

FRN 0003291192 Type Limited Liability Company

Licensee

 New Cingular Wireless PCS, LLC
 P:(855)699-7073

 3300 E. Renner Road, B3132
 F:(972)907-1131

 Richardson, TX 75082
 E:FCCMW@att.com

ATTN Reginald Youngblood

Contact

AT&T Mobility LLC P:(202)457-2055 F:(202)457-3073

1120 20th Street, NW - Suite 1000 E:michael.p.goggin@att.com

Washington, DC 20036 ATTN Michael P. Goggin

Ownership and Qualifications

Radio Service Type Mobile

Regulatory Status Common Carrier Interconnected Yes

Alien Ownership

The Applicant answered "No" to each of the Alien Ownership questions.

**Basic Qualifications** 

The Applicant answered "No" to each of the Basic Qualification questions.

### **Tribal Land Bidding Credits**

This license did not have tribal land bidding credits.

Demographics

Race

Ethnicity Gender

**ULS License** 

### PCS Broadband License - WPOI255 - NEW CINGULAR WIRELESS PCS, LLC

Call Sign WPOI255 Radio Service CW - PCS Broadband

Status Active Auth Type Regular

Market

Market MTA026 - Louisville-Lexington- Channel Block A

Evansvill

Submarket 19 Associated 001850.00000000-

Frequencies 001865.00000000 (MHz) 001930.00000000 001945.00000000

**Dates** 

Grant 05/27/2015 Expiration 06/23/2025

Effective 05/27/2015 Cancellation

**Buildout Deadlines** 

1st 06/23/2000 2nd 06/23/2005

**Notification Dates** 

1st 07/07/2000 2nd 02/17/2005

Licensee

FRN 0003291192 Type Limited Liability Company

Licensee

 NEW CINGULAR WIRELESS PCS, LLC
 P:(855)699-7073

 3300 E. Renner Road, B3132
 F:(972)907-1131

 Richardson, TX 75082
 E:FCCMW@att.com

ATTN Reginald Youngblood

Contact

AT&T MOBILITY LLC P:(202)457-2055
Michael P Goggin F:(202)457-3073

1120 20th Street, NW - Suite 1000 E:michael.p.goggin@att.com

Washington, DC 20036 ATTN Michael P. Goggin

Ownership and Qualifications

Radio Service Type Mobile

Regulatory Status Common Carrier Interconnected Yes

Alien Ownership

The Applicant answered "No" to each of the Alien Ownership questions.

**Basic Qualifications** 

The Applicant answered "No" to each of the Basic Qualification questions.

### **Tribal Land Bidding Credits**

This license did not have tribal land bidding credits.

Demographics

Race

Ethnicity Gender

**ULS** License

### AWS (1710-1755 MHz and 2110-2155 MHz) License - WQGA822 - New Cingular Wireless PCS, LLC

Call Sign WQGA822 Radio Service AW - AWS (1710-1755 MHz and

2110-2155 MHz)

Status Active Auth Type Regular

Market

Market CMA451 - Kentucky 9 - Elliott Channel Block A

Submarket 0 Associated 001710.0000000000-

Frequencies 001720.00000000 (MHz) 002110.00000000 002120.00000000

**Dates** 

Grant 11/29/2006 Expiration 11/29/2021

Effective 02/12/2014 Cancellation

**Buildout Deadlines** 

1st 2nd

**Notification Dates** 

1st 2nd

Licensee

FRN 0003291192 Type Limited Liability Company

Licensee

New Cingular Wireless PCS, LLC P:(855)699-7073
3300 E. Renner Road, B3132 F:(972)907-1131
Richardson, TX 75082 E:FCCMW@att.com

ATTN Reginald Youngblood

Contact

AT&T Mobility LLC P:(202)457-2055 Michael P Goggin F:(202)457-3073

1120 20th Street, NW - Suite 1000 E:michael.p.goggin@att.com

Washington, DC 20036 ATTN Michael P. Goggin

Ownership and Qualifications

Radio Service Type Mobile

Regulatory Status Common Carrier Interconnected Yes

Alien Ownership

The Applicant answered "No" to each of the Alien Ownership questions.

**Basic Qualifications** 

The Applicant answered "No" to each of the Basic Qualification questions.

### **Tribal Land Bidding Credits**

This license did not have tribal land bidding credits.

Demographics

Race

Ethnicity

Gender

#### **ULS License**

### Cellular License - KNKN861 - NEW CINGULAR WIRELESS PCS, LLC

Call Sign KNKN861 Radio Service CL - Cellular

Status Active Auth Type Regular

Market

Market CMA451 - Kentucky 9 - Elliott Channel Block A
Submarket 0 Phase 2

Dates

Grant 08/30/2011 Expiration 10/01/2021

Effective 06/09/2014 Cancellation

**Five Year Buildout Date** 

02/04/1997

**Control Points** 

1 1650 Lyndon Farms Court, LOUISVILLE, KY

P: (502)329-4700

2 707 CONCORD ROAD, KNOXVILLE, TN

Licensee

FRN 0003291192 Type Limited Liability Company

Licensee

 NEW CINGULAR WIRELESS PCS, LLC
 P:(855)699-7073

 3300 E. Renner Road, B3132
 F:(972)907-1131

 Richardson, TX 75082
 E:FCCMW@att.com

ATTN Reginald Youngblood

Contact

AT&T MOBILITY LLC P:(202)457-2055 Michael P Goggin F:(202)457-3073

1120 20th Street, NW - Suite 1000 E:michael.p.goggin@att.com

Washington, DC 20036 ATTN Michael P. Goggin

**Ownership and Qualifications** 

Radio Service Type Mobile

Regulatory Status Common Carrier Interconnected Yes

Alien Ownership

The Applicant answered "No" to each of the Alien Ownership questions.

**Basic Qualifications** 

The Applicant answered "No" to each of the Basic Qualification questions.

Demographics

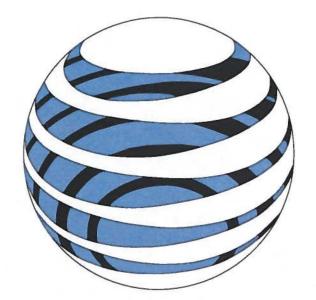
Race

Ethnicity Gender

### **EXHIBIT B**

### SITE DEVELOPMENT PLAN:

500' VICINITY MAP
LEGAL DESCRIPTIONS
FLOOD PLAIN CERTIFICATION
SITE PLAN
VERTICAL TOWER PROFILE



# at&t

# INDEX SITE ID: KYALU6170

1999 HWY 460 WEST MORGAN COUNTY WEST LIBERTY, KENTUCKY 41472

PROPOSED 255' SELF-SUPPORT
WITH MULTIPLE EQUIPMENT LOCATIONS

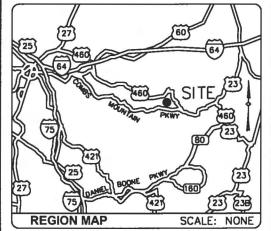
### SCOPE OF WORK:

CONSTRUCTION DRAWINGS FOR:
CONSTRUCTION OF A NEW UNMANNED TELECOMMUNICATIONS FACILITY.

SITE WORK: NEW SELF-SUPPORT TOWER, UNMANNED EQUIPMENT SHELTER AND GENERATOR ON A CONCRETE FOUNDATION, AND UTILITY INSTALLATIONS.

### UTILITY PROTECTION NOTE

THE CONTRACTOR'S ATTENTION IS DIRECTED TO THE UTILITY PROTECTION CENTER, WHICH WAS ESTABLISHED TO PROVIDE ACCURATE LOCATIONS OF UNDERGROUND UTILITIES. THE CONTRACTOR SHALL NOTIFY THE UTILITY PROTECTION CENTER 48 HOURS IN ADVANCE OF ANY CONSTRUCTION ON THIS PROJECT. ALL NEW SERVICE AND GROUNDING TRENCHES PROVIDE A WARNING TAPE 12 INCHES ABOVE THE UNDERGROUND INSTALLATION (SEE NEC 300.5).



# Engineering, Inc.

Consulting Engineers, Landscape Architects, Planners & Surveyors
"Serving the Bluegrass and Beyond"

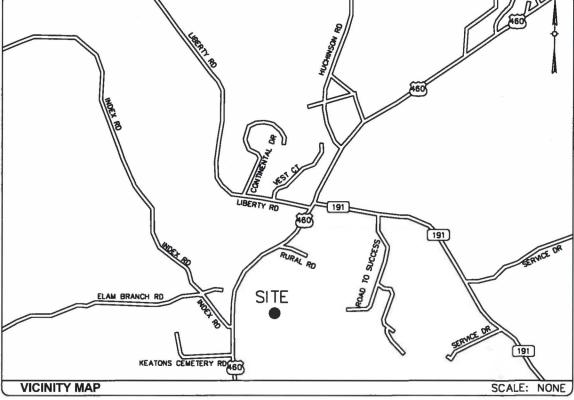
3001 TAYLOR SPRINGS DRIVE LOUISVILLE, KENTUCKY 40220 (502) 459-8402 PHONE (502) 459-8427 FAY

**DESIGN ENGINEER** 



(502) 459-8427 FAX

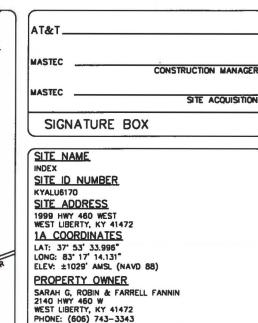
SURVEYOR



DIRECTIONS FROM COUNTY SEAT: FROM WEST LIBERTY AT THE CORNER OF US 460 (MAIN ST) AND US 460/SR 7 (PRESTONSBURG AVE), PROCEED SOUTH ON US 460 (MAIN ST) APRX 2.5 MILES TO ACCESS RD AND TURN LEFT. FOLLOW ACCESS RD TO SITE APRX .30 MILES ON THE RIGHT.

DIRECTIONS FROM WINCHESTER AT&T MTSO: STARTING AT 3800 CORPORATE DR WINCHESTER, KY, PRCEED TO THE COMBS MOUNTAIN PKWY FOR APRX 56.5 MILES TO EXIT 57 (SR 191/205) AND TURN LEFT. PROCEED ON SR 191/205 APRX 9.0 MILES TO US 460 AND TURN RIGHT. CONTINUE ON US 460 APRX 2.20 MILES TO ACCESS RD AND TURN RIGHT. FOLLOW ACCESS RD TO SITE APRX .30 MILES ON THE RIGHT.

**DIRECTIONS TO SITE** 



#### **APPLICANT**

AT&T 601 W. CHESTNUT ST. 1 EAST LOUISVILLE, KENTUCKY 40203 CONTACT: MICHELLE WARD PHONE: (502) 779-5950

### TAX MAP NUMBER

PARCEL NUMBER 089-00-00-017.00 SOURCE OF TITLE DEED BOOK 173, PAGE 113

LEASE AREA

PROJECT INFORMATION

SHT_NO.	DESCRIPTION
T-1	TITLE SHEET
C-1	500' RADIUS VICINITY MAP
C-1A	500' RADIUS VICINITY MAP
C-2	COMMUNICATIONS SITE SURVEY
C-2A	COMMUNICATIONS SITE SURVEY
Z-2	OVERALL SITE PLAN
Z-2A	OVERALL SITE PLAN (DIMENSIONS)
Z-3	SITE LAYOUT
Z-4	AT&T SHELTER LAYOUT
Z-5	NORTH/SOUTH TOWER ELEVATIONS
Z-6	EAST/WEST TOWER ELEVATIONS

### SHEET INDEX

POLICE DEPARTMENT MORGAN CO SHERIFF PHONE: (606) 743-9935

### FIRE DEPARTMENT WHITE OAK VOL FD

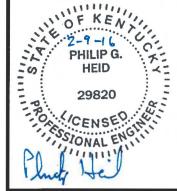
PHONE: (606) 743-5200 ELECTRIC COMPANY

#### LICKING RIVER RECC CONTACT: CUSTOMER SERVICE PHONE: (606) 743-3179

TELEPHONE COMPANY
MOUNTAIN TELEPHONE COMPANY
CONTACT: CUSTOMER SERVICE
PHONE: (606) 743-3121

CONTACT INFORMATION

BTM Engineering, Inc. consulting engineering and engineers. LANDSCAPE ARCHITECTS, PLANNERS & SURNETORS Bene CONSULE, KENTILCKY 40220 PHONE: (502) 459-8402 PHONE: (502) 459-8402

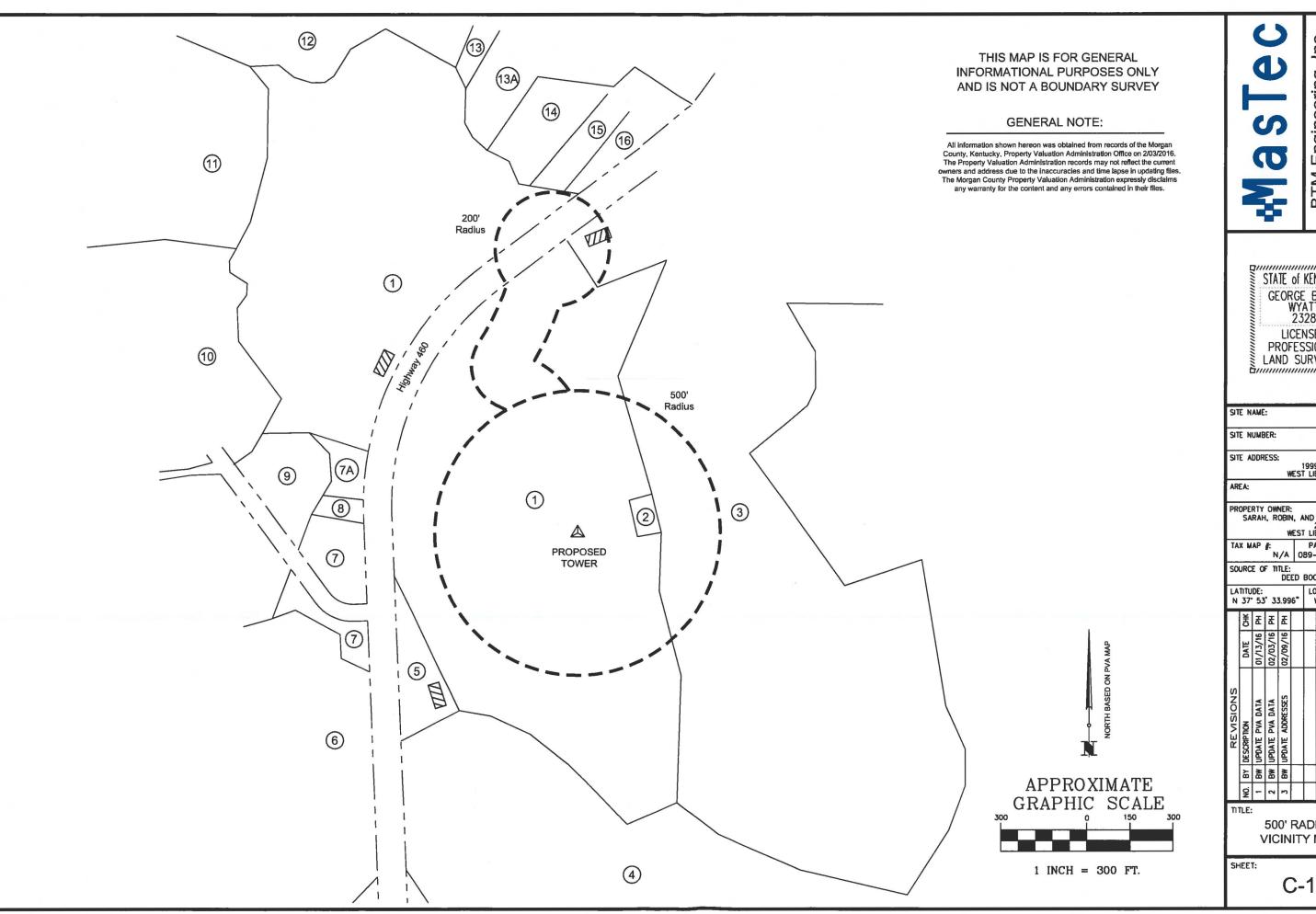


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TITLE SHEET



STATE of KENTUCKY GEORGE BRIAN WYATT 2328 LICENSED PROFESSIONAL LAND SURVEYOR

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	DATE	01/13/16	02/03/16	02/09/16							
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- 1 PARCEL NUMBER: 089-00-00-017.00 Sarah G., Robin & Farrell Fannin 2140 Highway 460 W West Liberty, Kentucky 41472
- 2 PARCEL NUMBER: 089-00-00-017.01 Appalachian Wireless East KY Network 101 Technology Trail Ivel, Kentucky 41642
- (3) PARCEL NUMBER: 089-00-00-025.00 Samuel Long P.O. Box 456 West Liberty, Kentucky 41472
- 4 PARCEL NUMBER: 089-00-00-021.00
  Alex Goodpaster & Hillary Murray
  c/o Allan Goodpaster
  P.O. Box 503
  West Liberty, Kentucky 41472
- 5 PARCEL NUMBER: 089-00-00-016.01 Sarah & Robin Fannin 2140 Highway 460 W West Liberty, Kentucky 41472
- 6 PARCEL NUMBER: 089-00-00-019.00 William G. Holbrook DVM P.O. Box 66 West Liberty, Kentucky 41472
- 7 PARCEL NUMBER: 089-00-00-016.00 Sharlene Copas & Walter & George Elam c/o George Elam 3832 Highway 711 West Liberty, Kentucky 41472
- (7A) PARCEL NUMBER: 089-00-00-014.00 Sharlene Copas & Walter & George Elam c/o George Elam 3832 Highway 711 West Liberty, Kentucky 41472
- 8 PARCEL NUMBER: 089-00-00-015.00 David Stacy 2144 Highway 460 W West Liberty, Kentucky 41472

- 9 PARCEL NUMBER: 089-00-00-009.00
  Betty Lou Elam & Linda Blackburn
  309 Larkwood Drive
  Lexington, Kentucky 40509
- PARCEL NUMBER: 089-00-00-008.00
  Woodford B. Gevedon & Mary Beth Popplewell
  173 Index Road
  West Liberty, Kentucky 41472

and

Fairanna Nickell 173 Index Road West Liberty, Kentucky 41472

- PARCEL NUMBER: 089-00-00-007.00
  Caney Farms c/o Buford Sherman
  12094 Highway 437
  West Liberty, Kentucky 41472
- PARCEL NUMBER: 089-00-00-024.00
  David Earl & Susan May
  1042 Liberty Road
  West Liberty, Kentucky 41472
- (13) PARCEL NUMBER: 089-00-00-024.01 K & M Rentals P.O. Box 273 West Liberty, Kentucky 41472
- PARCEL NUMBER: 089-03-00-002.00 K & M Rentals (Tim Keller & John Motley) P.O. Box 273 West Liberty, Kentucky 41472
- PARCEL NUMBER: 089-03-00-011.00 Mt. Holiness Kentucky Box 2 VanCleave, Kentucky 41385

and

Ky, Mt. Holiness c/o Index Community Church 1749 W. Main St. West Liberty, KY 41472

- PARCEL NUMBER: 089-03-00-012.00
  Anthony Frederick
  2919 Highway 1000
  West Liberty, Kentucky 41472
- PARCEL NUMBER: 089-03-00-013.00
  No online PVA data found for this parcel

### THIS MAP IS FOR GENERAL INFORMATIONAL PURPOSES ONLY AND IS NOT A BOUNDARY SURVEY

### **GENERAL NOTE:**

All information shown hereon was obtained from records of the Morgan County, Kentucky, Property Valuation Administration Office on 2/03/2016. The Property Valuation Administration records may not reflect the current owners and address due to the inaccuracies and time lapse in updaining files. The Morgan County Property Valuation Administration expressly disclaims any warranty for the content and any errors contained in their files.



STATE of KENTUCKY

GEORGE BRIAN
WYATT
2328

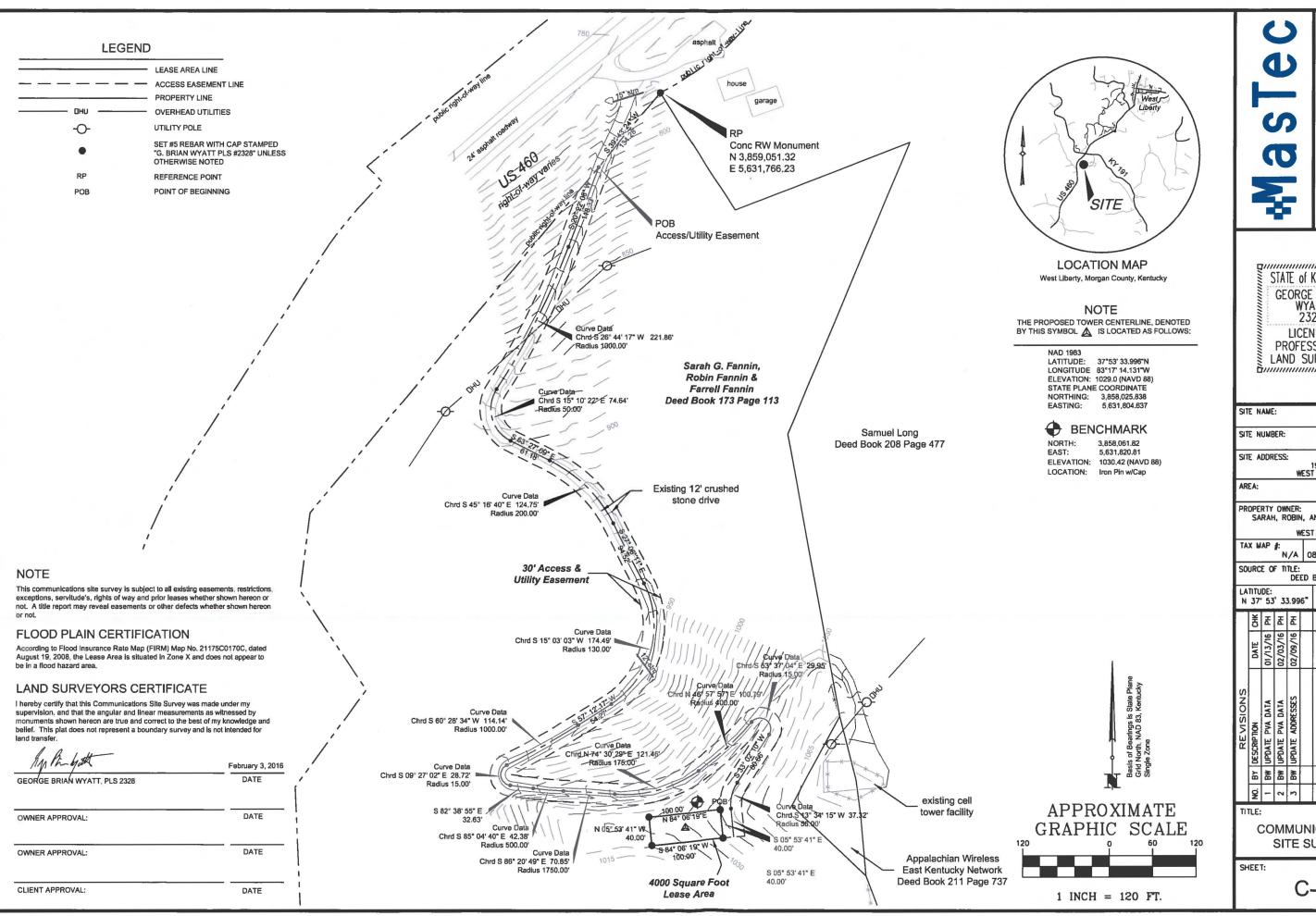
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**VICINITY MAP** 

C-1A

SHEET:



Engineering, languers, Landscape arcute. Jankeyors, Sankeyors, Sankeyors, Stankeyors, Stan

STATE of KENTUCKY GEORGE BRIAN WYATT 2328 LICENSED **PROFESSIONAL** LAND SURVEYOR

INDEX KYALU6170 1999 HWY 460 WEST WEST LIBERTY, KY\_41472 4,000 SF PROPERTY OWNER:
SARAH, ROBIN, AND FARRELL FANNIN
2140 HWY 460 W
WEST LIBERTY, KY 41472 PARCEL NUMBER: N/A 089-00-00-017.00 DEED BOOK 173 PAGE 113 LONGITUDE: W 83' 17' 14.131" COMMUNICATIONS

SITE SURVEY

### LEGAL DESCRIPTIONS

These are the descriptions of a Lease Area for telecommunications equipment, and an Access/Utility easement, located entirely on the tract of land conveyed to Sarah G. Fannin, Robin Fannin, and Farrell Fannin by deed of record in Deed Book 173 at Page 113 in the Office of the County Clerk of Morgan County, Kentucky and further described as follows:

### **DESCRIPTION OF GRANTOR'S TRACT**

As recorded in Deed Book 173 at page 113, in the Office of the County Clerk of Morgan County, Kentucky,

Beginning at the mouth of Little Caney creek; thence up Little Caney creek with its meanders to the line and land of Bill Elam (formerly Kola Noble); thence with the line of Bill Elam to the line of Kola Noble; thence with Kola Noble's line to the line of J.T. Thomas' thence with J.T. Thomas' line to the Lewis Henry line; thence with Lewis Henry's line around to Big Caney Creek just above the ford opposite Isaac Henry's (now Henry's Heirs) line; thence down Big Caney creek with its meanders to the place of beginning, containing 40 acres, more or less, and to contain and include all of the land in the above described boundary with the exception of two lots that have been previously deeded to S.S. Oldfield and wife, of Index, Kentucky.

There is excepted from the foregoing described tract of land a tract of land heretofore conveyed by Stella D. Fannin and others to L. Clifford Long and Aleene F. Long, by deed dated October 19, 1956, and recorded in Deed Book 93, Page 204, Morgan County Court Clerk's records, and reference is hereby made to said deed of conveyance for a more particular description of the portion of land excepted from the above described tract.

LESS AND EXCEPT Deed of Conveyance dated February 27,2007, from Sarah Fannin-Holliday and Noah Shane Holliday, Robin L. Fannin, Erma Fannin, and Grover Farrell Fannin, as Grantor to East Kentucky Network, LLC, d/b/a Appalachian Wireless, as Grantee, recorded in Volume 211, Page 737 of the Official Public Records of Morgan County, Kentucky.

### DESCRIPTION OF LEASE AREA FOR TELECOMMUNICATIONS EQUIPMENT

NOTE: All bearings and distances are based on grid north Kentucky State Plane Coordinate System Single Zone NAD 1983

A Lease Area for telecommunications equipment, described as follows:

Beginning at a point in the Grantor's tract, as recorded in Deed Book 173 at Page 113 in the Morgan County Court Clerk's Office, said point being a set iron pin with cap stamped G. Brian Wyatt PLS 2328, having NAD 83 Single Zone coordinates of: North 3,858,050.87 and East 5,631,849.82; thence South 05 degrees 53 minutes 41 seconds East, a distance of 40.00 feet to a set iron pin; thence South 84 degrees 06 minutes 19 seconds West, a distance of 100.00 feet to a set iron pin; thence North 05 degrees 53 minutes 41 seconds West, a distance of 40.00 feet to a set iron pin; thence North 84 degrees 06 seconds 19 seconds East, a distance of 100.00 feet to the point of beginning containing 4000 square feet, or 0.092 acres.

### **DESCRIPTION OF ACCESS/UTILITY EASEMENT**

NOTE: All bearings and distances are based on grid north Kentucky State Plane Coordinate System Single Zone NAD 1983

The right to use for Access and Utilities, to the above-described Telecommunications Lease Area, an easement, the centerline of which is described as follows:

Beginning, for reference, at a point in the Grantor's northerly property line, as recorded in Deed Book 173 at Page 113 in the Morgan County Court Clerk's Office, said point being the northwesterly corner of the Samuel Long tract, as recorded in Deed Book 208 at Page 477 in the aforesaid Clerk's office, and further being a found concrete right-of-way marker located in the southeasterly line of Highway 460, having NAD 83 Single Zone coordinates of: North 3,859,051.32 and East 5,631,766.23; thence running with the aforesaid line of US 460, South 39 degrees 43 minutes 24 seconds West, a distance of 134.28 feet to the TRUE POINT OF BEGINNING in the centerline of the 30-foot wide Access/Utility Easement herein described; thence on, over and across lands of the grantor for the following nineteen (19) calls: 1) South 20 degrees 22 minutes 08 seconds West, a distance of 118.33 feet to a point; 2) with a curve to the right of radius 1000.00 feet, the chord of which bears South 26 degrees 44 minutes 17 seconds West, a distance of 221.86 feet to a point; 3) with a curve to the left of radius 50.00 feet, the chord of which bears South 15 degrees 10 minutes 22 seconds East, a distance of 74.64 feet to a point; 4) South 63 degrees 27 minutes 09 seconds East, a distance of 61.18 feet to a point; 5) with a curve to the right of radius 200.00 feet, the chord of which bears South 45 degrees 16 minutes 40 seconds East, a distance of 124.75 feet to a point; 6) South 27 degrees 06 minutes 11 seconds East, a distance of 94.52 feet to a point; 7) with a curve to the right of radius 130.00 feet, the chord of which bears South 15 degrees 03minutes 03 seconds West, a distance of 174.49 feet; 8) South 57 degrees 12 minutes 17 seconds West, a distance of 54.27 feet to a point; 9) with a curve to the right of radius 1000.00 feet, the chord of which bears South 60 degrees 28 minutes 34 seconds West, a distance of 114.14 feet to a point; 10) with a curve to the left of radius 15.00 feet, the chord of which bears South 09 degrees 27 minutes 02 seconds East, a distance of 28.72 feet; 11) South 82 degrees 38 minutes 55 seconds East, a distance of 32.63 feet to a point; 12) with a curve to the left of radius 500,00 feet, the chord of which bears South 85 degrees 04 minutes 40 seconds, a distance of 42,38 feet to a point; 13) with a curve to the right of radius 1750.00 feet, the chord of which bears South 86 degrees 20 minutes 49 seconds East, a distance of 70.85 feet to a point; 14) with a curve to the left of radius 175.00 feet, the chord of which bears North 74 degrees 30 minutes 29 seconds East, a distance of 121.46 feet to a point; 15) with a curve to the left of radius 400.00 feet, the chord of which bears North 46 degrees 57 minutes 57 seconds East, a distance of 100.79 feet to a point; 16) with a curve to the right of radius 15.00 feet, the chord of which bears South 53 degrees 37 minutes 04 seconds East, a distance of 29.95 feet to a point; 17) South 33 degrees 02 minutes 10 seconds West, a distance of 80.66 feet to a point; 18) with a curve to the left of radius 56.00 feet, the chord of which bears South 13 degrees 34 minutes 15 seconds West, a distance of 37.32 feet to a point; 19) South 05 degrees 53 minutes 41 seconds East, a distance of 40.00 feet to a point.

### TITLE COMMITMENT

Schedule B - Section II

- Item 1. Defects, liens, encumbrances, adverse claims or other matters, if any, created, first appearing in the public records or attaching subsequent to the Effective Date but prior to the date the proposed Insured acquires for value of record the estate or interest or mortgage thereon covered by the Commitment. **BTM Engineering, Inc did not examine or address this item.**
- Item 2. Facts which would be disclosed by an accurate and comprehensive survey ofthe premises herein described. *The land title lines shown hereon represent those called for in Item* 2
- Item 3. Rights or claims of parties in possession. BTM Engineering, Inc did not examine or address these items.
- Item 4. Construction, mechanic's, contractors' or materialmen's lien claims, if any, where no notice thereof appears of record. **BTM Engineering, Inc did not examine or address this item.**
- Item 5. Easements or claims of easements not shown by the public records. BTM Engineering, Inc did not examine or address this item.
- Item 6. Any adverse ownership claim by the state of KY by right of sovereignty to any portion of the lands insured hereunder, including submerged, filled and artificially exposed lands and lands accredited to such lands. **BTM Engineering, Inc did not examine or address this item.**
- Item 7. State road right reservation(s), if any. BTM Engineering, Inc did not examine or address this Item.
- Item 8. Oil, gas and mineral right reservations, if any. BTM Engineering, Inc did not examine or address this item.
- Item 9. Any lien arising, in favor of any city, town, village or port authority for unpaid service charges for service by any water system, sewer system or gas system servicing the lands described herein. **BTM Engineering, Inc did not examine or address** this item.
- Item 10. Lands lie within various county special assessment districts and municipal taxing districts and are subject to liens for any unpaid special assessments by virtue of the ordinances and resolutions creating these districts. The special assessments are payable with the ad valorem taxes. **BTM Engineering, Inc did not examine or address this item.**
- Item 11. Covenants, conditions, restrictions, easements and reservations or leases of minerals or mineral rights, if any, appearing of public record. This policy insures that the use of the land for residential one to four family dwelling purposes is not affected or impaired by reason of the aforementioned matters. **BTM Engineering, Inc did not examine or address this item.**
- Item 12. Subject to Memorandum of Lease between Sarah George Fannin, unmarried, Robin Fannin, unmarried, Erma Fannin, unmarried, Farrell Fannin, unmarried and Kelly Kristen Fannin Koenig and Chris Koenig, husband and wife and New Cingular Wireless PCS, LLC, a Delaware limited liability company dated October 15,2010 and filed on January 24, 2014 in (book) 55, (page) 155, of the official property records of Morgan County, Kentucky. Affects the subject property, and is shown hereon as 4000 SF Lease Area and 30' Access & Utility Easement.
- Item 13. Subject to Articles of Merger between East Kentucky Network, LLC and Mountaineer Cellular, LLC and Appalachian Cellular, LLC dated December 15, 1999 and filed on March 7, 2000 in (book) Al5, (page) 11, of the official property records of Morgan County, Kentucky. Affects the subject property, but not the 4000 SF Lease Area, and is shown hereon.
- Item 14. Subject to Order Appointing Fiduciary dated May 16, 1995 and filed on May 16, 1995 in (book) IUB7, (page) 773, of the official property records of Morgan County, Kentucky. **BTM Engineering, Inc did not examine or address this item.**



BTM Engineering, Inconsulting Engineering, Inconsulting Engineering, LANDSCAPE ARCHITECTS, PLANNERS & SIGNETORS, LANDSCAPE (SOO) 1410-1410 (1972) (1972) (1973) (19

STATE of KENTUCKY

GEORGE BRIAN
WYATT
2328

LICENSED
PROFESSIONAL
LAND SURVEYOR

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COMMUNICATIONS
SITE SURVEY

SHEET:

C-2A

### SITE PLAN NOTES

- THE PROPOSED DEVELOPMENT IS FOR A 255 FOOT SELF-SUPPORT TOWER AND MULTIPLE EQUIPMENT LOCATIONS. THE LOCATION IS 1999 HIGHWAY 460 WEST, WEST LIBERTY, KY 41472.
- 2. THE TOWER WILL BE ACCESSED BY A PROPOSED STABILIZE DRIVE FROM AN EXISTING GRAVEL ROADWAY (HWY 460) WHICH IS A PUBLIC RIGHT-OF-WAY. THE ACCESS ROAD IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE LOCAL HIGHWAY DEPARTMENT OF TRANSPORTATION STANDARDS. WATER, SANITARY SEWER AND WASTE COLLECTIONS SERVICES ARE NOT REQUIRED FOR THE PROPOSED DEVELOPMENT.
- 3. CENTERLINE OF PROPOSED TOWER GEOGRAPHIC LOCATIONS

LATITUDE:

37" 53" 33.996" N LONGITUDE: 83° 17' 14.131" W

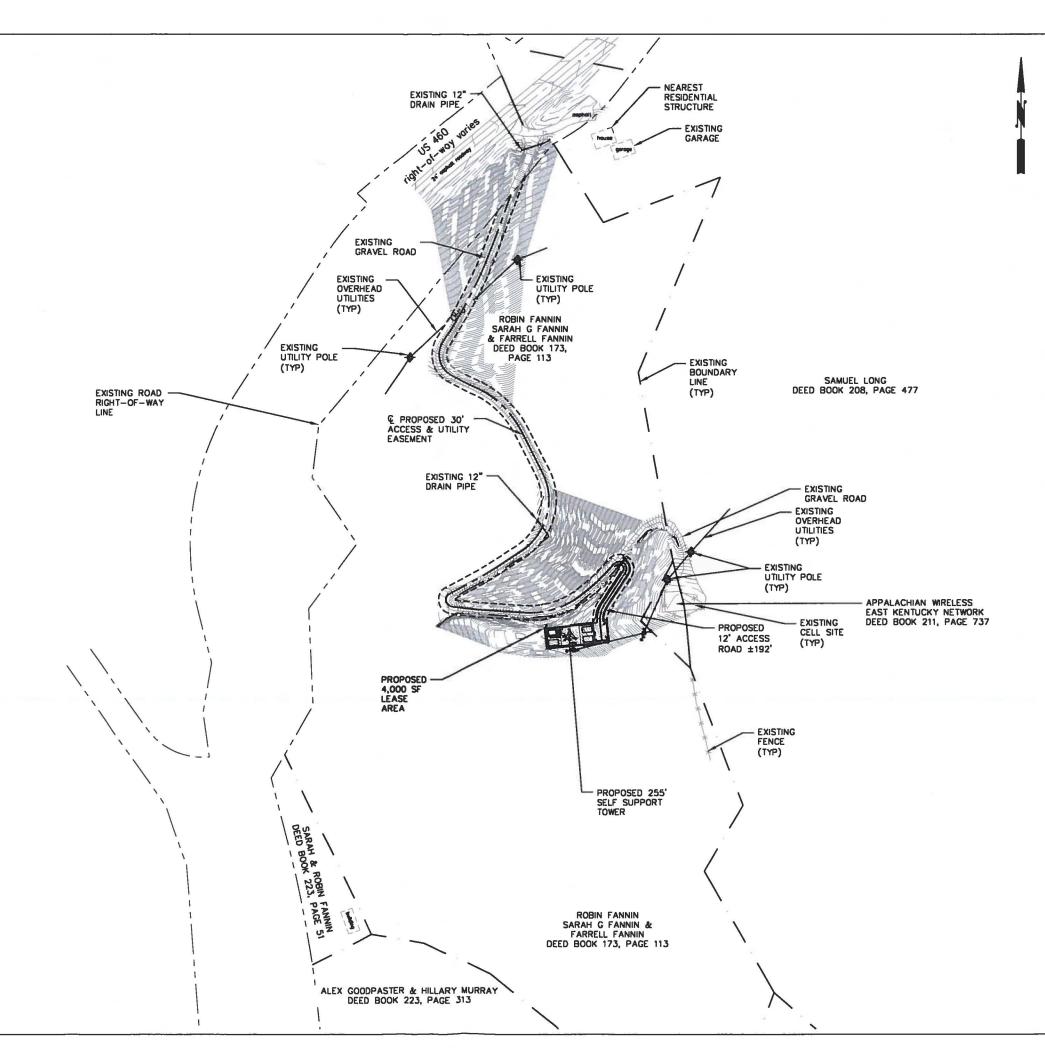
- 4. REMOVE ALL VEGETATION & CLEAN LEASE AREA (WHERE REQUIRED)
- 5. FINISH GRADING TO PROVIDE EFFECTIVE DRAINAGE WITH A SLOPE OF NO LESS THAN ONE EIGHTH INCH (1/8") PER FOOT FLOWING AWAY FROM EQUIPMENT FOR A MINIMUM DISTANCE OF SIX FEET (6') IN ALL
- 6. LOCATE ALL U.G. UTILITIES PRIOR TO ANY CONSTRUCTION.
- 7. COMPOUND FINISHED SURFACE TO BE FENCED.

### **LEGEND**

OHE	EXISTING OVERHEAD ELECTRIC
ОНТ	EXISTING OVERHEAD TELEPHONE
UGE	EXISTING UNDERGROUND ELECTRIC
UGT	EXISTING UNDERGROUND TELEPHONE
UGE	PROPOSED UNDERGROUND ELECTRIC
UGT	PROPOSED UNDERGROUND TELEPHONE
$-\!\!\!-\!\!\!\!-\!\!\!\!-\!\!\!\!\!-\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!$	FENCE LINE
മ	POWER POLE
■ TELE.	TELEPHONE PEDESTAL
⊠' <sup>™</sup>	WATER VALVES
ü	FIRE HYDRANTS
•	BOLLARDS
<b>∞</b>	GAS VALVES

### **GRAPHIC SCALE**





Engineering, BTM
CONSULTING E
PLANNERS &
3001 TAYLOR
LOUISVILLE, KI
PHONE: (502) A
FAX: (502) A



SITE NAME: INDEX SITE NUMBER: KYALU6170 SITE ADDRESS: 1999 HWY 460 WEST WEST LIBERTY, KY 41472 AREA: 4,000 SF PROPERTY OWNER: SARAH, ROBIN, AND FARRELL FANNIN 2140 HWY 460 W WEST LIBERTY, KY 41472 PARCEL NUMBER: N/A 089-00-00-017.00 SOURCE OF TITLE: DEED BOOK 173 PAGE 113 LONGITUDE: N 37° 53' 33.996" W 83' 17' 14.131

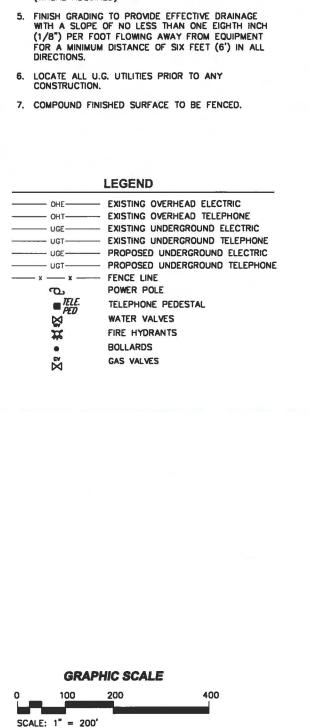
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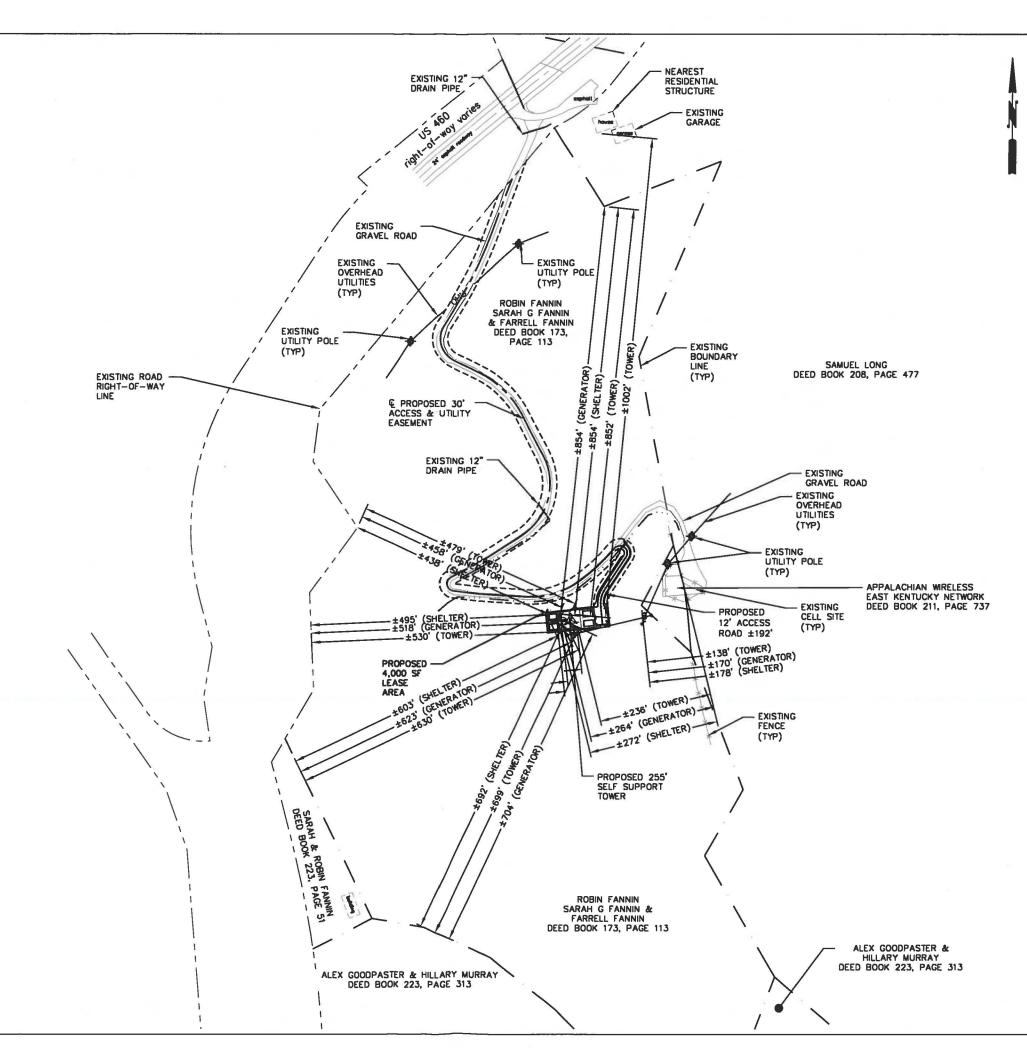
**OVERALL** SITE PLAN

SHEET:

Z-2

### SITE PLAN NOTES THE PROPOSED DEVELOPMENT IS FOR A 255 FOOT SELF-SUPPORT TOWER AND MULTIPLE EQUIPMENT LOCATIONS. THE LOCATION IS 1999 HIGHWAY 460 WEST, WEST LIBERTY, KY 41472. 2. THE TOWER WILL BE ACCESSED BY A PROPOSED STABILIZE DRIVE FROM AN EXISTING GRAVEL ROADWAY (HWY 460) WHICH IS A PUBLIC RIGHT-OF-WAY. THE ACCESS ROAD IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE LOCAL HIGHWAY DEPARTMENT OF TRANSPORTATION STANDARDS. WATER, SANITARY SEWER AND WASTE COLLECTIONS SERVICES ARE NOT REQUIRED FOR THE PROPOSED DEVELOPMENT. 3. CENTERLINE OF PROPOSED TOWER GEOGRAPHIC LOCATIONS 37° 53' 33.996" N LATITUDE: 83° 17' 14.131" W LONGITUDE: 4. REMOVE ALL VEGETATION & CLEAN LEASE AREA (WHERE REQUIRED) 6. LOCATE ALL U.G. UTILITIES PRIOR TO ANY CONSTRUCTION. 7. COMPOUND FINISHED SURFACE TO BE FENCED. **LEGEND** - EXISTING OVERHEAD ELECTRIC - EXISTING OVERHEAD TELEPHONE - UGT-Ф POWER POLE ■ TELE. PED TELEPHONE PEDESTAL WATER VALVES FIRE HYDRANTS







Engineering, Inc BTM ECONSULTING EI PLANNERS & 3001 TAYLOR LOUISVILE, KE PHONE: (502) 45

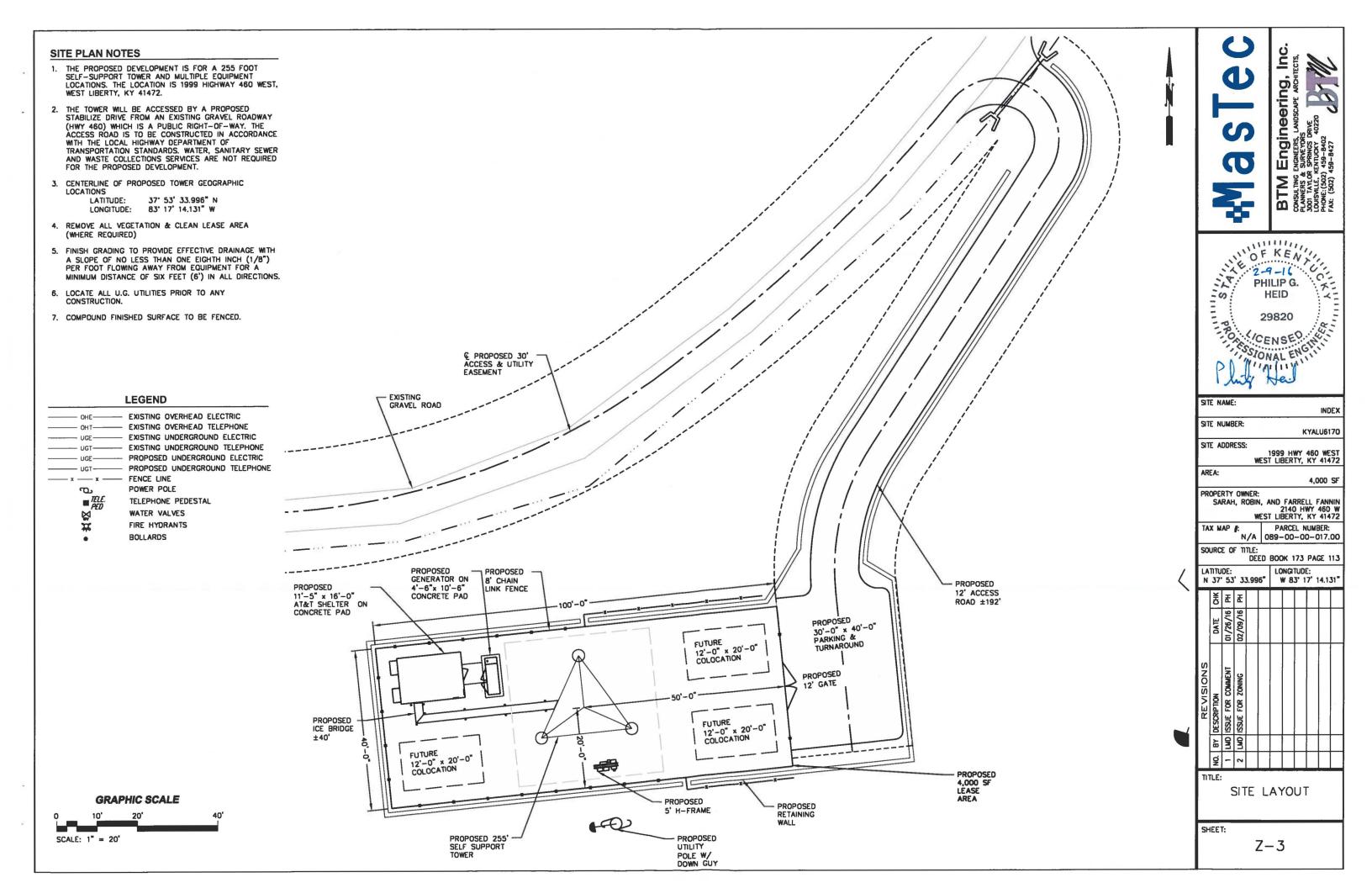


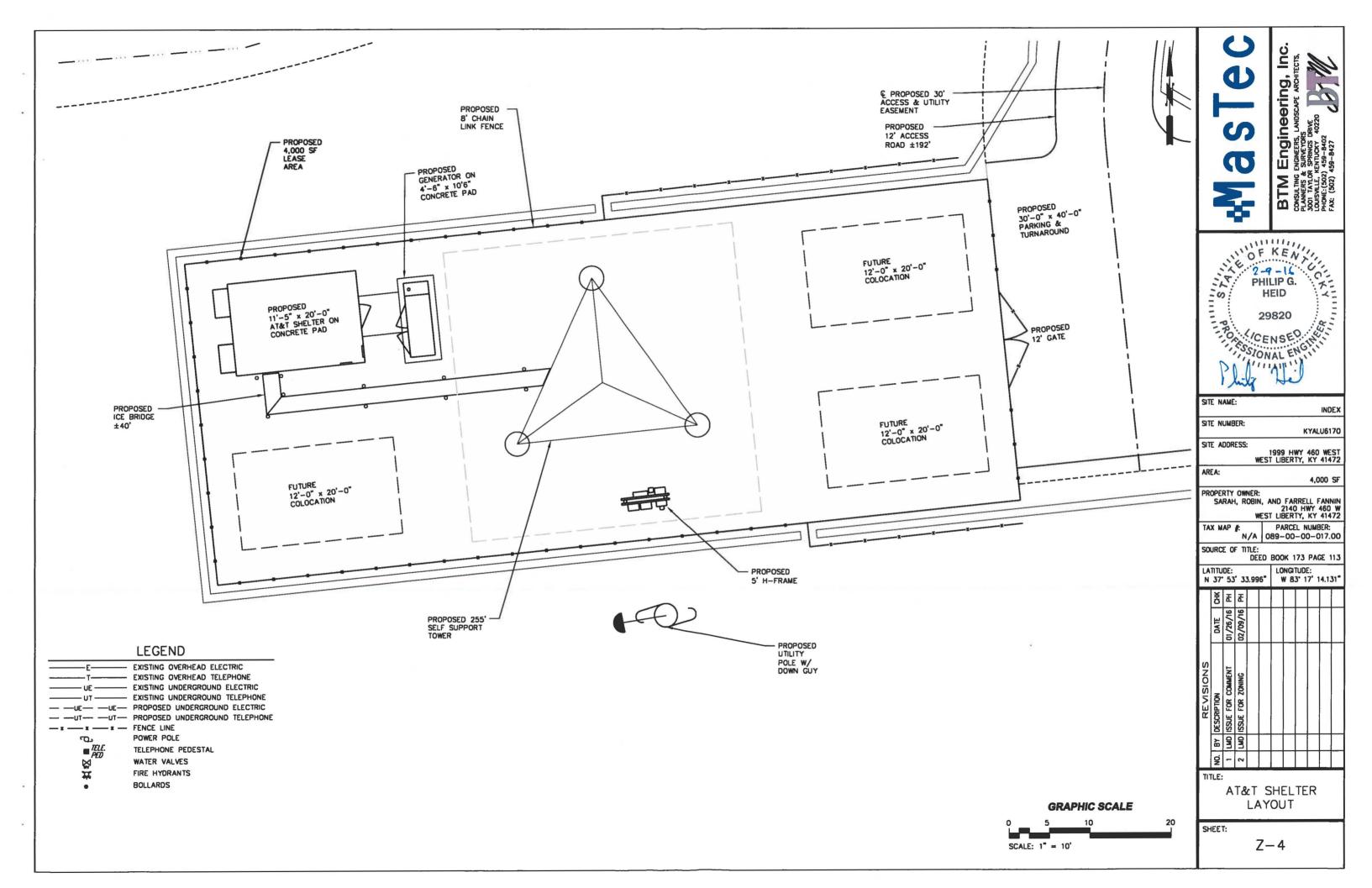
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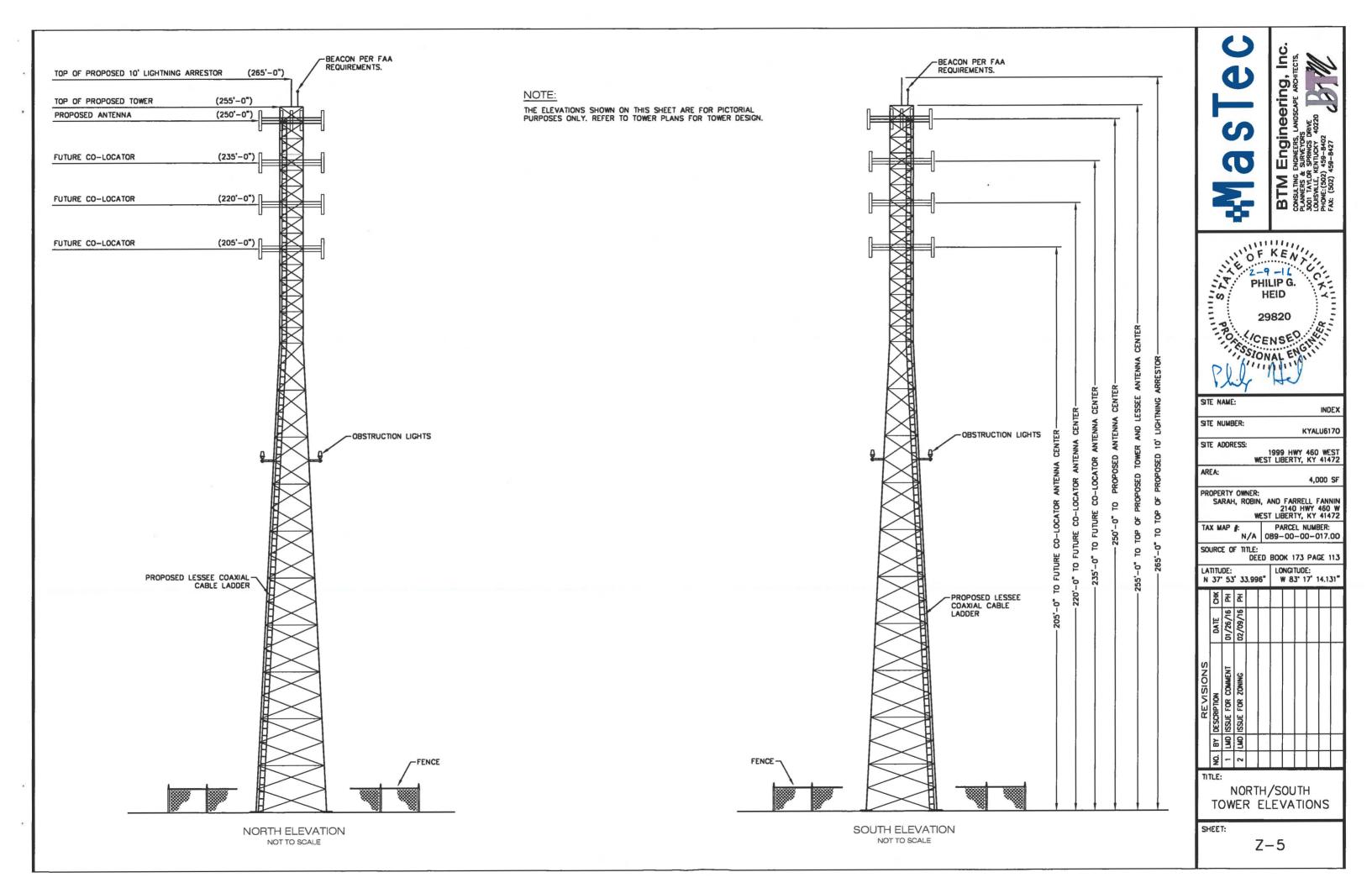
OVERALL SITE PLAN (DIMENSIONS)

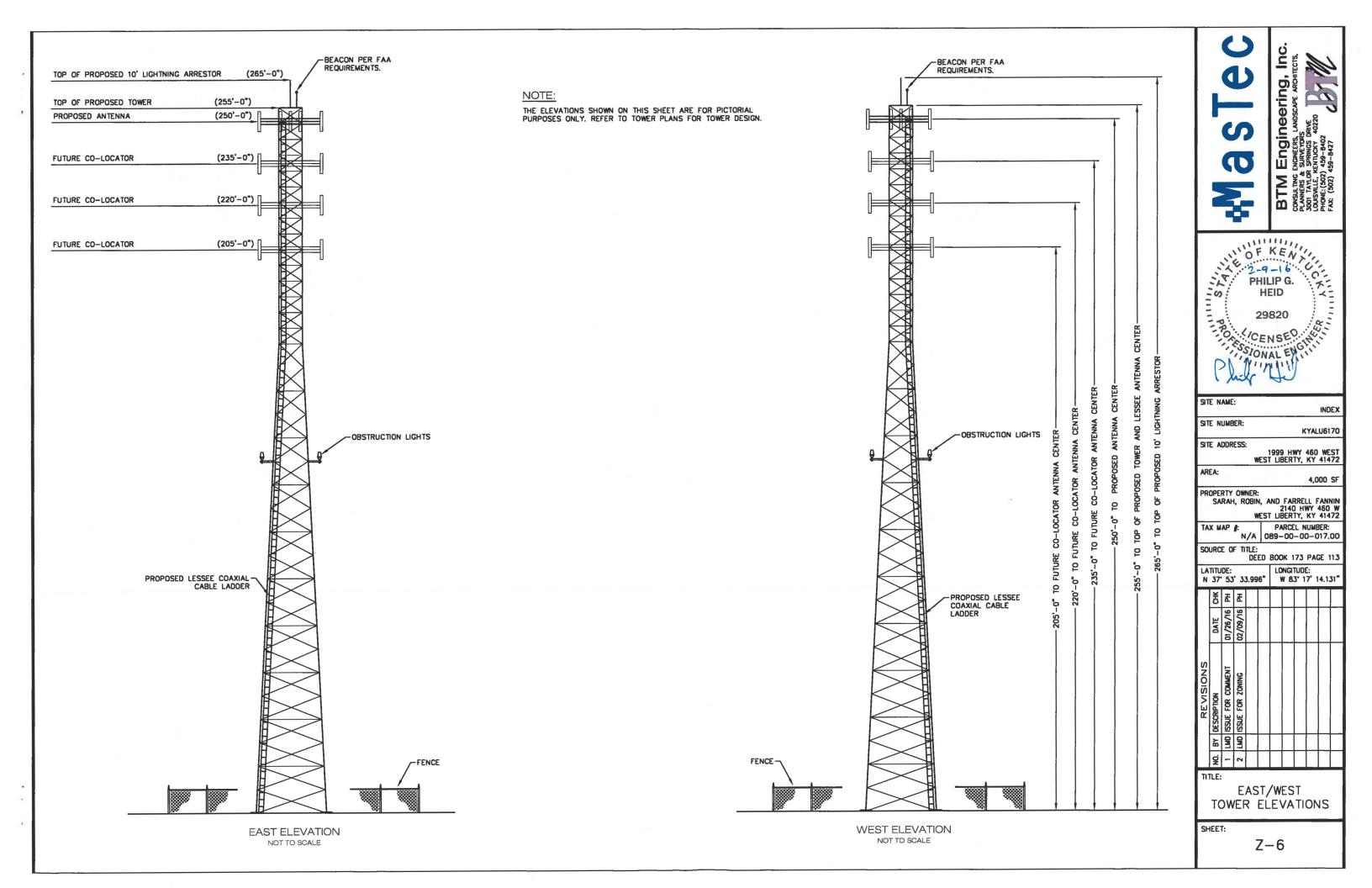
SHEET:

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### EXHIBIT C TOWER AND FOUNDATION DESIGN



December 3, 2013

American Tower Corp.

Attn: Mr. Ron Rohr

SUBJECT: Valmont File #239816 Model V-29.0 x 255' Self Supporting Tower

Site: #282100 Index - Index, KY

Thank you for your inquiry concerning tower design codes and practices as they relate to your requested tower designs.

Valmont Structures has been designing and building guyed and self-supporting towers and monopoles since the early 1950's. During this time, we have sold thousands of towers ranging in height form as little as 50' high to in excess of 1400'. These towers were individually engineered to accommodate the loading requirements imparted by the design wind speed, ice considerations, antenna loading, and other factors dictated by the national code requirements existing at the time the tower was built.

The present National Tower code, the TIA-222-G, represents the latest refinement of specific minimum requirements for tower engineers and manufacturers to follow to help assure that the tower structure and its foundation are designed to meet the most realistic conditions for local weather while assuring that the tower is designed to stringent factors of safety.

The TIA-222-G code incorporates an escalating wind factor based on tower height. If 90 MPH 3 second gust is the basic design wind speed at the 10 meter height, then per the specification, this speed is then increased in stages up the tower. "Meeting the code" implies that the design will have all of the code requirements for safety factors intact at the wind speed specified. Thus, the ultimate survival speed would be considerably higher.

While failure is extremely rare in any kind of tower, it is especially so for self supported towers and monopoles. In fact, only if a tower or monopole were subjected to a direct hit from a tornado or the severest of hurricanes would failure be predicted, and then usually only if hit by flying debris.

We are aware of only a very few documented instances of a self supporting tower or monopole failure. Self supporting towers and monopoles can be designed such that the most common mode of failure is in the upper middle region of the tower, with the upper portion of the tower remaining connected and "bending and bowing over" against the base of the tower or pole. The fact that the wind is normally greater on the upper portion of the structure contributes to the likelihood of this type of failure.





This particular Tower is designed such that its first point of predicted failure is in the region above the 180' level. The predicted mode of wind induced failure would be a buckling of the tower legs above the 180' level with the top sections of the tower folding over on to the intact base sections. This would then affect a "zero fall zone" at ground level.

As Chief Engineer of the company and a registered P.E. in 49 states, I oversee all engineering and application of our towers. I am a graduate engineer from Purdue University and am assisted by other registered professional engineers on our staff.

Valmont Structures is an AISC approved shop. All Valmont Structures welders are AWS and CWB qualified. Mathematical and physical tests are performed routinely on tower sections and designs as required. Our total design, engineer and build process has been quality audited by our customers including public utilities, telephone companies, government agencies, and of course AISC.

We trust the above and the attached will be helpful to you. If you should need anything else, please let us know at your convenience.

Sincerely,

William Heiden, P.E. Senior Engineer Ext. #5243





### **UNIT BASE FOUNDATION SUMMARY**

### ATC Index, KY

Pad width, W:	41.0	ft
Depth, D:	7.0	ft
Ext. above grade, E:	0.5	ft
Pier diameter, d <sub>i</sub> :	6.5	ft
Pad thickness, T:	1.50	ft
Depth neglected, N:	7.0	ft
Volume, Vo:	115.51	су

pad, m_p:	72	bars *
size, <b>s</b> _p:	9	17.
vertical, m_c:	31	verticals
size, <b>s</b> _c:	8	6' cage
ties, m_t:	7	ties
size, S t:	4	w/ overlap

<sup>\*</sup> Rebar to be equally spaced, both ways, top & bottom

\* Use standees to support top rebar above bottom rebar in mat

A- 239816

V- 29.0

255

Soil Information Per:						
Assumed	as Clay Per TIA-222-G Annex F.					

Soil unit weight, γ:	110	pcf
Ultimate Bearing, Bc:	5.000	ksf
Cohesion, Col	1.000	Ksf
Friction angle, $\varphi$ :	0.0	degrees
Ult. Passive P., Pp:	0.396	pcf
Base sliding, µ:	0.20	
Seismic Zone:	1	
Water at:	none	ft

Anchor Steel Selection		
Part Number, P/N:	109881	Dia = 1.25
1 000, 1 100, 110, 000, 1 1 110,	100001	Langth = SO

Material Properties		
Steel tensile str, Fy:	60000	psi
Conc. Comp. str, F'e:	4000	psi
Conc. Density, δ:	150	pcf
Clear cover, cc:	3.00	in

V 2.0

Backfill Compaction		
Lift thickness:	12	in
Compaction:	97	%
Standard Proctor:	ASTM	D698

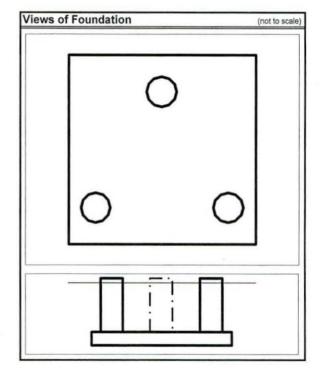
### Tower design conforms to the following:

- \* 1997 Uniform Building Code (UBC)
- \* 2000 & 2003 International Building Code (IBC)
- \* ANSI TIA-222-G
- \* Building Code Requirements for Reinforced Concrete (ACI 318-05)

Note: The centroid of the tower is offset from the centroid of the foundation

Load Case 1		stress rat	io: 99.6%	mark up:	0.4%
Shear (total), S:	142.00	kips	x 1.004 =	142.57	kips
Moment, M:	18730.00	ft-kips	x 1.004 =	18804.92	ft-kips
Compression/Leg, C:	782.00	kips	x 1.004 =	785.13	kips
Uplift/Leg, U:	698.00	kips	x 1.004 =	700.79	kips
Tower Weight, Wt:	107.00	kips	=	107.00	kips
Load Case 2		stress rat	io: 99.6%	mark up:	0.4%
Shear (total), S:	142.00	kips	x 1.004 =	142.57	kips
Moment, M:	18730.00	ft-kips	x 1.004 =	18804.92	ft-kips
Compression/Leg, C:	782.00	kips	x 1.004 =	785.13	kips
Uplift/Leg, U:	698.00	kips	x 1.004 =	700.79	kips
Tower Weight, Wt:	107.00	kips	=	107.00	kips





#### Additional Notes:

- \* No foundation modifications listed.
- \* See attached "Foundation Notes" for further information.

### **UNIT BASE FOUNDATION SUMMARY**

### ATC Index, KY

V- 29.0 255 A- 239816

V 2.0

Pad width, W:	41.0	ft
Depth, D:	7.0	ft
Ext. above grade, E:	0.5	ft
Pier diameter, d <sub>i</sub> :	6.5	ft
Pad thickness, T:	1.50	ft
Depth neglected, N:	7.0	ft
Volume, Vo:	115.51	су

pad, m_p:	72	bars *
size, <b>s</b> _p:	9	
vertical, m_c:	31	verticals
size, <b>s</b> _c:	8	6' cage
ties, m_t	7	ties
size, S <sub>f</sub> :	4	w/ overlap

<sup>\*</sup> Rebar to be equally spaced, both ways, top & bottom

Soil Information Per:	
Assumed as Clay Per TIA-22	2-G Annex F.

Soil unit weight, γ:	110	pcf
Ultimate Bearing, B <sub>c</sub> :	5.000	ksf
Cohesion, C <sub>a</sub>	1.000	Ksf
= Friction arigle, ⊋;	0.0	degrees
Ult. Passive P., Pp:	0.396	pcf
Base sliding, μ:	0.20	
Seismic Zone:	1	
Water at:	none	ft

Anchor Steel Sele	ction	
Part Number, P/N:	109881	Dia = 1.25"

Material Properties		
Steel tensile str, Fy:	60000	psi
Conc. Comp. str, F'e:	4000	psi
Conc. Density, δ:	150	pcf
Clear cover, cc:	3.00	in

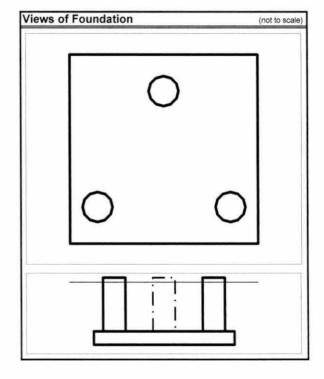
Backfill Compaction		
Lift thickness:	12	in
Compaction:	97	%
Standard Proctor:	ASTM	D698

#### Tower design conforms to the following:

- \* 1997 Uniform Building Code (UBC)
- \* 2000 & 2003 International Building Code (IBC)
- \* ANSI TIA-222-G
- \* Building Code Requirements for Reinforced Concrete (ACI 318-05)

Note: The centroid of the tower is offset from the centroid of the foundation

Foundation Loadi	ng				
Load Case 1					
		in is a		14.4	
Load Case 2	stress ratio: 99.6%			mark up:	0.4%
Shear (total), S:	142.00	kips	x 1.004 =	142.57	kips
Moment, M:	18730.00	ft-kips	x 1.004 =	18804.92	ft-kips
Compression/Leg, C:	782.00	kips	x 1.004 =	785.13	kips
Uplift/Leg, U:	698.00	kips	x 1.004 =	700.79	kips
Tower Weight, Wt:	107.00	kips	=	107.00	kips



#### Additional Notes:

- \* No foundation modifications listed.
- \* See attached "Foundation Notes" for further information.

<sup>\*</sup> Use standees to support top rebar above bottom rebar in mat

#### **FOUNDATION NOTES**

1 IN THE ABSENCE OF A GEOTECHNICAL REPORT, THE FOLLOWING PRESUMPTIVE SOIL PARAMETERS WERE USED: AN ULTIMATE BEARING PRESSURE OF 5000 PSF, A COHESION OF 1000 PSF, A SOIL UNIT WEIGHT OF 110 PCF, AN ANGLE OF INTERNAL FRICTION OF 0 DEGREES AND NO GROUNDWATER ENCOUNTERED. THESE SOIL PARAMETERS ARE IN COMPLIANCE WITH THE REQUIREMENTS OF ANSI/TIA-222-G-2005 AND CAN BE FOUND IN ANNEX F OF THIS STANDARD.

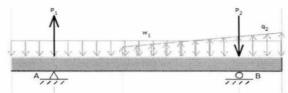
## **UNIT BASE FOUNDATION (Load Case 2)**

ATC Index, KY						29.0 39816	255			V 2.0
Reactions	stress ratio 99.6%	mark up:	0.4%	Soi	l per: As	sumed as	Clay Per T	IA-222-G An	nex F.	
Shear, S:	142.00 kips x 1.004 =	142.57 kij	os							
Moment, M: 18	3730.00 ft-kips x 1.004 =	18804.92 ft-	kips							
Compression / leg, C:	782.00 kips x 1.004 =	785.13 kij	os							
Uplift / leg, U:	698.00 kips x 1.004 =	700.79 kij	ps			Ultimate	e bearing:	5.000	ksf	
	107.00 kips =	107.00 kij					timate Pp:	0.396	kcf	
	Loa	d Case 2 =	0.9*D + 1	1.0*Dg + 1.6*	'Wo					
Physical Parameters:										
Concrete volume:	V = T * W2 +	3 * (di² / 4 * m	) * (D + E -	T)			V =	115.5	су	
Concrete weight:	W <sub>c</sub> = ∨ * δ						W <sub>c</sub> =	467.8	kips	
Soil weight:	$W_s = (D - T) *$	(W2 - 3 * (di2 /	4 * π)) * y				W, =	956.8	kips	
Total weight:	P = Wc + Ws						P =	1531.60	kips	
Passive Pressure:										
Pp coefficient:	$K_p = TAN(45)$						$K_p =$	1.000		
	$P_{pn} = Kp \cdot \gamma \cdot$	N + 2 * Co * √	(Kp)				P <sub>pn</sub> =	2.770	ksf	
	$P_{pt} = Kp * \gamma *$	(D - T) + 2 * Co	o * √(Kp)				P <sub>pt</sub> =	2.605	ksf	
	P <sub>pb</sub> = Kp * y *	D + 2 * Co * V	(Kp)				P <sub>pb</sub> =	2.770	ksf	
	$P_{ptop} = IF(N < (I))$	) - T), Ppt, Ppr	1)				P <sub>ptop</sub> =	2.8	ksf	
	Pp' = (Pptop +						Pp' =	2.770	ksf	
Shear area:	$T_{pp} = 0$						T <sub>pp</sub> =	0.0	ft	
	App = Tpp * W						A <sub>pp</sub> =	0.00	ft <sup>2</sup>	
Shear Capacity: $\varphi r = 0.75$	S <sub>actual</sub> = (Pp' * Ap						S <sub>actual</sub> =	229.740	kips	
φr = 0.73		Check	S <sub>actual</sub> =	229.74 kips		>=	S =	142.57	kips	ОК
Overturning Moment Res	The second secon	T					101			
Wt of soil wedge:	W <sub>sw</sub> = D * (D *		ν - γ				W <sub>sw</sub> =	0.0	kips	
Dist. from leg to edge:	O = (W - 0.8)						0 =	7.943	ft	
Additional offset of Wt:		.866 * w' + O)	- VV / 2				O <sub>a</sub> =	4.186	ft	
Resisting moments:	$M_{\text{rwt}} = P \cdot W / 2$						M <sub>rwt</sub> =	30949.87	ft-kips	
	$M_{rp} = Pp' * App$	p*(D - N)/3					$M_{rp} =$	0.00	ft-kips	
	$M_{rsw} = Wsw * ()$	W + D * TAN(4	0) / 3)				M <sub>rsw</sub> =	0.00	ft-kips	
Total resisting:	$M_{rt} = (Mrwt + 1)$	Mrp + Mrsw) *	фг				$M_{rt} =$	23212.40	ft-kips	
$\varphi r = 0.75$ Total overturning:	M <sub>o</sub> = M + S *	(D + E)					M <sub>o</sub> =	19874.18	ft-kips	
Total overturning.	W <sub>0</sub> - W · S	Check	M <sub>rt</sub> = 2	23212.40 ft-kip:	s	>=	M <sub>o</sub> =	19874.18	ft-kips	ОК
Bearing Resistance due	to Pressure Distribution:									
	area = W2						area =	1681.0	ft <sup>2</sup>	
Area of mat:	alea - VV							444000	ft <sup>3</sup>	
Area of mat: Section modulus:	$SM = W^3 / 6$						SM =	11486.8		
Section modulus:	SM = W <sup>a</sup> / 6	) * (Wc + Ws)						11486.8	kin	
Section modulus: Factored total weight:	$SM = W^3 / 6$ P' = Wt + 0.9						P' =	1389.1	kip ksf	
Section modulus:	SM = W <sup>a</sup> / 6	+ Mo / SM					P' = P <sub>pos</sub> =		kip ksf ksf	
Section modulus: Factored total weight: Pressure exerted:	$SM = W^3 / 6$ P' = Wt + 0.9 $P_{pos} = P' / area$	+ Mo / SM - Mo / SM	kern. Beari	ng area has be	een adju	sted belo	P' = P <sub>pos</sub> = P <sub>neg</sub> =	1389.1 2.557	ksf	
Section modulus: Factored total weight: Pressure exerted:	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$	+ Mo / SM - Mo / SM	kern. Beari	ng area has be	een adju	sted belo	P' = P <sub>pos</sub> = P <sub>neg</sub> =	1389.1 2.557	ksf	
Section modulus: Factored total weight: Pressure exerted: No.	$SM = W^3 / 6$ P' = Wt + 0.9 $P_{pos} = P' / area$ $P_{neg} = P' / area$ te: The stress resultant is NO $e_c = Mo / P'$	+ Mo / SM - Mo / SM OT within the		ng area has be	een adju	sted belo	$P' = P_{pos} = P_{neg} =$	1389.1 2.557 -0.904	ksf ksf ft	
Section modulus: Factored total weight: Pressure exerted: No. Load eccentricity:	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_c = Mo / P'$ $P_{adj} = 2 * P' / (3)$	+ Mo / SM - Mo / SM OT within the i	- ec))	ng area has be	een adju	sted belo	$P' =$ $P_{pos} =$ $P_{neg} =$ $P_{neg} =$ $P_{neg} =$ $P_{neg} =$ $P_{neg} =$	1389.1 2.557 -0.904 14.31 3.6	ksf ksf ft ksf	
Section modulus: Factored total weight: Pressure exerted: No.	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_c = Mo / P'$ $P_{adj} = 2 * P' / (3)$	+ Mo / SM - Mo / SM OT within the	- ec))	ing area has be	een adju	sted belo	$P' = P_{pos} = P_{neg} =$	1389.1 2.557 -0.904	ksf ksf ft	OHO
Section modulus: Factored total weight: Pressure exerted:  No Load eccentricity:  Adj. applied pressure: $\varphi r = 0.75$ Concrete Shear Strength	$SM = W^{3} / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_{c} = Mo / P'$ $P_{adj} = 2 * P' / (3)$ $q_{a} = IF(Pneg)$	+ Mo / SM - Mo / SM OT within the is 3 * W * (W / 2 >= 0, Ppos, Po	- ec)) adj)		een adju		$P' = P_{pos} = P_{neg} =$	1389.1 2.557 -0.904 14.31 3.6 3.647	ksf ksf ft ksf ksf	OH
Section modulus: Factored total weight: Pressure exerted:  No Load eccentricity:  Adj. applied pressure: $\varphi r = 0.75$ Concrete Shear Strength	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_c = Mo / P'$ $P_{adj} = 2 * P' / (0)$ $q_s = IF(Pneg)$ $E'$ rom tower	+ Mo / SM - Mo / SM DT within the I 3 * W * (W / 2 >= 0, Ppos, Pi Check	- ec)) adj)		een adju		$P' = P_{pos} = P_{neg} = P_{neg} = P_{neg} = P_{adj} = Q_{adj} = Q_{adj} = P_{adj} = Q_{adj} = P_{adj} =$	1389.1 2.557 -0.904 14.31 3.6 3.647 3.750	ksf ksf ft ksf ksf	OH
Section modulus: Factored total weight: Pressure exerted:  No Load eccentricity:  Adj. applied pressure: $\varphi r = 0.75$ Concrete Shear Strength	$SM = W^{3} / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_{c} = Mo / P'$ $P_{adj} = 2 * P' / (3)$ $q_{a} = IF(Pneg)$	+ Mo / SM - Mo / SM DT within the I 3 * W * (W / 2 >= 0, Ppos, Pi Check	- ec)) adj)		een adju		$P' = P_{pos} = P_{neg} =$	1389.1 2.557 -0.904 14.31 3.6 3.647	ksf ksf ft ksf ksf	Он
Section modulus: Factored total weight: Pressure exerted:  No Load eccentricity:  Adj. applied pressure:  \( \psi = 0.75 \)  Concrete Shear Strength One way beam action at d, fi	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_c = Mo / P'$ $P_{adj} = 2 * P' / (0)$ $q_s = IF(Pneg)$ $E'$ rom tower	+ Mo / SM - Mo / SM DT within the I 3 * W * (W / 2 >= 0, Ppos, Pi Check	- ec)) adj)		een adju		$P' = P_{pos} = P_{neg} = P_{neg} = P_{neg} = P_{adj} = Q_{adj} = Q_{adj} = P_{adj} = Q_{adj} = P_{adj} =$	1389.1 2.557 -0.904 14.31 3.6 3.647 3.750	ksf ksf ft ksf ksf	Он
Section modulus: Factored total weight: Pressure exerted:  No Load eccentricity:  Adj. applied pressure:  \( \pi = 0.75 \)  Concrete Shear Strength  Die way beam action at d, fi  Effective depth:	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_c = Mo / P'$ $P_{adj} = 2 * P' / (0)$ $q_s = IF(Pneg)$ $C$ $rom tower$ $d_c = T - cc - C$	+ Mo / SM - Mo / SM DT within the I 3 * W * (W / 2 >= 0, Ppos, Pi Check	- ec)) adj) q <sub>a</sub> =		een adju		$P' = P_{pos} = P_{neg} =$	1389.1 2.557 -0.904 14.31 3.6 3.647 3.750	ksf ksf ft ksf ksf ksf	OF
Section modulus: Factored total weight: Pressure exerted:  No Load eccentricity:  Adj. applied pressure: $\varphi r = 0.75$ Concrete Shear Strength One way beam action at d, fi Effective depth: Factored Intensity:	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = $	+ Mo / SM - Mo / SM DT within the ! 3 * W * (W / 2 >= 0, Ppos, Pi Check db_p / 2 - di / 2 - dc) * \	- ec)) adj) q <sub>a</sub> =		een adju		$P' = P_{pos} = P_{reg} =$	1389.1 2.557 -0.904 14.31 3.6 3.647 3.750 14.436 0.467 89.11	ksf ksf ft ksf ksf ksf in ksf kips	OF
Section modulus: Factored total weight: Pressure exerted:  No. Load eccentricity:  Adj. applied pressure: $\varphi r = 0.75$ Concrete Shear Strength One way beam action at d, fi Effective depth: Factored Intensity: Required shear:	$SM = W^3 / 6$ $P' = Wt + 0.9$ $P_{pos} = P' / area$ $P_{neg} = P' / area$ $te: The stress resultant is NO$ $e_c = Mo / P'$ $P_{adj} = 2 * P' / (0)$ $q_s = IF(Pneg)$ $C$ $Trom tower d_c = T - cc - c q_s = C / area V_{n1} = qs * (0)$	+ Mo / SM - Mo / SM DT within the ! 3 * W * (W / 2 >= 0, Ppos, Pi Check db_p / 2 - di / 2 - dc) * \	- ec)) adj) q <sub>a</sub> =		een adju		$P' = P_{pos} = P_{reg} =$	1389.1 2.557 -0.904 14.31 3.6 3.647 3.750	ksf ksf ft ksf ksf ksf	OF

	Po = (di + dc)	•π					P <sub>o</sub> =	24.20	ft	
Required shear: φs = 0.75 [ACI 9.3.2.3]	$V_{n2} = qs/\phi s^*$	(area - (di + d	dc)² * π / 4	)			V <sub>n2</sub> =	1017.82	kips	
Available shear: [ACI 12.2.2]	V <sub>c2</sub> = 4 * √(F'c)	* Po * dc					V <sub>c2</sub> =	1060.54	kips	
		Check	V <sub>c2</sub> =	1060.54	kips	>=	V <sub>n2</sub> =	1017.82	kips	OK
Column Compression Capacity:										
Compression reaction: $\varphi c = 0.65 [ACI 9.3.2.2]$	$P_{c} = \varphi c * 0.8$	* F'c * (di² / 4	* п)				P <sub>c</sub> =	9939.0	kips	
		Check	P <sub>c</sub> =	9938.99	kips	>=	C =	785.13	kips	OK
Pier Reinforcement:										
Cross-sectional area:	$A_g = di^2 * \pi / 4$						A <sub>g</sub> =	4778.36	in <sup>2</sup>	
Min. area of steel (pier):	$A_{st_c} = Ag \cdot 0.00$	)5					$A_{st_c} =$	23.89	in <sup>2</sup>	
[ACI 10.9.1] & [ACI 10.8.4]										
Cage circle:	d <sub>o</sub> = di - 2 * c	0					d <sub>o</sub> =	72.00	in	
Rebar:	s_c = 8				d <sub>b_c</sub> =	1	in			
	m_c = 31				A <sub>b_c</sub> =	0.79	in <sup>2</sup>		. 2	
	$A_{s_c} = Ab_c \cdot n$				1-2		A <sub>s_c</sub> =	24.49	in <sup>2</sup>	
		Check	A <sub>s_c</sub> =	24.49	in <sup>2</sup>	>=	A <sub>st_c</sub> =	23.89	in <sup>2</sup>	OK
Actual moment:	$M_{max} = (D - T +$	E)*S/2					M <sub>max</sub> =	427.70	ft-kips	
Pier moment capacity:	M <sub>allow</sub> per Maxmor	nnt.xls (see atta	ched)				M <sub>allow</sub> =	775.25	ft-kips	
		Check	M <sub>allow</sub> =	775.25	ft-kips	>=	M <sub>max</sub> =	427.70	ft-kips	OK
Bar separation:	$B_{s_c} = (do * \pi)$	m_c - db_c					B <sub>s_c</sub> =	6.30	in	
		Check	11	>=	B <sub>s_c</sub> =	6.30	in	>=	4.5"	OK
Reinforcement location: [ACI 12.2.4] Epoxy coating:	$\psi_{\text{t,c}}$ = if the space $\psi_{\text{e,c}}$ = if epoxy-co					pated	$\psi_{t,c} = \psi_{\bullet,c} = 0$	1.3		
[ACI 12.2.4]	bars are u	sed, then if B	s < 6 * db d	or cc < 3 *	db, use 1.5,	else 1.2				
Max term: [ACI 12.2.4]	$\psi_t \psi_{e_c} = \text{the produc}$	t of ψt & ψe,	need not b	e taken la	ger than 1.7		21.21 =	4.0		
Reinforcement size:		$\psi_{\rm s\_c}$ = if the bar size is 6 or less, then use 0.8, else use 1.0								
	$\psi_{s_c}$ = if the bar s	ize is 6 or les	s, then use	0.8, else			$\psi_i \psi_{e,c} = \psi_{s,c} = 0$	1.3		
[ACI 12.2.4] Light weight concrete:	$\psi_{s_c}$ = if the bar s $\lambda_c = \text{if lightwieg}$				use 1.0					
[ACI 12.2.4]		ht concrete is	used, 1.3,	else use	use 1.0	fistace	ψ <sub>s_0</sub> =	1	in	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Specing/cover:	$\lambda_{_{g}} = \text{if lightwieg}$	ht concrete is	used, 1.3, par spacing	else use	use 1.0	listace	$\psi_{s_{-c}} = \lambda_{-c} =$	1 1.0	in in	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover: [ACI 12.2.4] Transverse bars:	$\lambda_{c} = \text{if lightwiseg}$ $c_{c} \text{ the smalle}$	ht concrete is r of: half the b er simplification	used, 1.3, par spacing	else use	use 1.0	listace	$\psi_{s,c}$ = $\lambda_{c}$ = $c_{c}$ =	1 1.0 3.50 0 2.500		
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover: [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term:	$\lambda_{_{_{\mathbf{C}}}}$ = if lightwieg $c_{_{_{\mathbf{C}}}}$ the smalle $k_{tr_{_{_{\mathbf{C}}}}}$ = 0 in (p	tht concrete is r of: half the b er simplification i, (c_c + ktr_c	used, 1.3, par spacing	else use	use 1.0	listace	$\psi_{s,c} = \frac{\lambda_{c}}{\lambda_{c}} = \frac{\lambda_{c}}{c_{c}} = \frac{\kappa_{s,c}}{c_{c'}} = \frac{\kappa_{c,c}}{\kappa_{c,c}} = \frac{\kappa_{c,c}}{\kappa_{c,c}}$	1 1.0 3.50 0 2.500 0.55	În	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover: [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (tensile): [ACI 12.2.2]	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ = MIN( 2.5 $R_c$ = Mmax / 1 $L_{at\_c}$ = (3 / 40) •	ht concrete is  r of: half the b  er simplification  i, (c_c + ktr_c  Mallow  (Fy / \(\text{V(F'c)}\) * (	e used, 1.3, par spacing on) c) / db_c)	else use	use 1.0 1.0 ncrete edge c		$\psi_{s,c} = \frac{\lambda_{s,c}}{\lambda_{s,c}} = \frac{\lambda_{s,c}}{c_{s,c}} = \frac{c_{s,c}}{c_{s,c}} = \frac{c_{s,c}}$	1 1.0 3.50 0 2.500 0.55	in in	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Specing/cover: [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (tensile):	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ ' = MIN( 2.5	ht concrete is  r of: half the b  er simplification  i, (c_c + ktr_c  Mallow  (Fy / \(\text{V(F'c)}\) * (	e used, 1.3, par spacing on) c) / db_c)	else use	use 1.0 1.0 ncrete edge c		$\psi_{s,c} = \frac{\lambda_{c}}{\lambda_{c}} = \frac{\lambda_{c}}{c_{c}} = \frac{\kappa_{s,c}}{c_{c'}} = \frac{\kappa_{c,c}}{\kappa_{c,c}} = \frac{\kappa_{c,c}}{\kappa_{c,c}}$	1 1.0 3.50 0 2.500 0.55	În	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover: [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (tensile): [ACI 12.2.2] Minimum length:	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ = MIN( 2.5 $R_c$ = Mmax / 1 $L_{at\_c}$ = (3 / 40) •	ht concrete is r of: half the b er simplification i, (c_c + ktr_c Mallow (Fy / \(\text{(F'c)}\) * (i)	s used, 1.3, par spacing par spacing pon) c) / db_c)	else use	use 1.0 1.0 ncrete edge c		$\psi_{s,c} = \frac{\lambda_{s,c}}{\lambda_{s,c}} = \frac{\lambda_{s,c}}{c_{s,c}} = \frac{c_{s,c}}{c_{s,c}} = \frac{c_{s,c}}$	1 1.0 3.50 0 2.500 0.55	in in	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover: [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (tensile): [ACI 12.2.2] Minimum length: [ACI 12.2.1]	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.02 * dl	ht concrete is  r of: half the b  er simplification  i, (c_c + ktr_c  Mallow  (Fy / √(F'c)) * (c  s  d_min, Ldt'_c  b_c * Fy * R_c	s used, 1.3, par spacing par spacing pon)  c) / db_c)  wtwe_c * ws  c / √(F'c)	else use	use 1.0 1.0 ncrete edge c		ψ <sub>e_c</sub> =  λ <sub>c</sub> =  C <sub>_c</sub> =  k <sub>y_c</sub> =  C <sub>_c</sub> ' =  C <sub>_c</sub> ' =  L <sub>d'_c</sub> =  L <sub>d'_c</sub> =  L <sub>d'_c</sub> =  L <sub>d'_c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47	in in in in	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover: [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (lensile): [ACI 12.2.2] Minimum length: [ACI 12.2.1] Development length: Development (comp.): [ACI 12.3.2]	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ ' = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.02 * di $L_{dc\_c}$ = 0.0003 *	ht concrete is r of: half the b er simplification i, (c_c + ktr_c  Mallow  (Fy / √(F'c)) * (c  s  f_min, Ldt'_c  c_c * Fy * R_c  db_c * Fy * F	s used, 1.3, par spacing par spacing pon)  c) / db_c)  wtwe_c * ws  c / √(F'c)	else use	use 1.0 1.0 ncrete edge c		ψ <sub>e_c</sub> =  λ <sub>c</sub> =  C <sub>_c</sub> =  k <sub>ψ_c</sub> =  C <sub>_c</sub> ' =  C <sub>_c</sub> ' =  L <sub>d'_c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47 9.93	in in in in in in in in	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover: [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (tensile): [ACI 12.2.2] Minimum length: [ACI 12.2.1] Development length: Development (comp.):	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.02 * dl	ht concrete is r of: half the b er simplification i, (c_c + ktr_c  Mallow  (Fy / √(F'c)) * (c  s  f_min, Ldt'_c  c_c * Fy * R_c  db_c * Fy * F	s used, 1.3, par spacing par spacing pon)  c) / db_c)  wtwe_c * ws  c / √(F'c)	else use	use 1.0 1.0 ncrete edge c		ψ <sub>e_c</sub> =  λ <sub>c</sub> =  C <sub>_c</sub> =  k <sub>y_c</sub> =  C <sub>_c</sub> ' =  C <sub>_c</sub> ' =  L <sub>d'_c</sub> =  L <sub>d'_c</sub> =  L <sub>dc'_c</sub> =  L <sub>dc'_c</sub> =  L <sub>dc''_c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47	in in in in	
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover. [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (lensile): [ACI 12.2.2] Minimum length: [ACI 12.2.1] Development length: Development (comp.): [ACI 12.3.2]	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ ' = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.02 * di $L_{dc\_c}$ = 0.0003 *	ht concrete is  r of: half the b  er simplification  i, (c_c + ktr_c  Mallow  (Fy / \(\frac{1}{2}\)(F'c)) * (0  s  d_min, Ldt'_c  c_c * Fy * R_1  db_c * Fy * B  Ldc'_c, Ldc''  Ldc'_c, Ldc''	s used, 1.3, par spacing par spacing pon)  c) / db_c)  wtwe_c * ws  c / √(F'c)	else use	use 1.0 1.0 ncrete edge c		ψ <sub>e_c</sub> =  λ <sub>c</sub> =  C <sub>_c</sub> =  k <sub>ψ_c</sub> =  C <sub>_c</sub> ' =  C <sub>_c</sub> ' =  L <sub>d'_c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47 9.93	in in in in in in in in	
[ACI 12.2.4]  Light weight concrete:  [ACI 12.2.4]  Spacing/cover:  [ACI 12.2.4]  Transverse bars:  [ACI 12.2.3]  Max term:  [ACI 12.2.3]  Excess reinforcement:  [ACI 12.2.5]  Development (tensile):  [ACI 12.2.2]  Minimum length:  [ACI 12.2.1]  Development length:  Development (comp.):  [ACI 12.3.2]	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ ' = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d_\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.02 * dl $L_{dc\_c}$ = 0.0003 * $L_{dc\_c}$ = MAX( 8,	ht concrete is  r of: half the b  er simplification  i, (c_c + ktr_c  Mallow  (Fy / \(\frac{1}{2}\)(F'c)) * (0  s  d_min, Ldt'_c  c_c * Fy * R_1  db_c * Fy * B  Ldc'_c, Ldc''  Ldc'_c, Ldc''	s used, 1.3, par spacing par spacing pon)  c) / db_c)  wtwe_c * ws  c / √(F'c)	else use	use 1.0 1.0 ncrete edge c		ψ <sub>e_c</sub> =  λ <sub>c</sub> =  C <sub>_c</sub> =  k <sub>y_c</sub> =  C <sub>_c</sub> ' =  C <sub>_c</sub> ' =  L <sub>d'_c</sub> =  L <sub>d'_c</sub> =  L <sub>dc'_c</sub> =  L <sub>dc'_c</sub> =  L <sub>dc''_c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47 9.93 10.47	in	ОК
[ACI 12.2.4]  Light weight concrete:  [ACI 12.2.4]  Spacing/cover:  [ACI 12.2.4]  Transverse bars:  [ACI 12.2.3]  Max term:  [ACI 12.2.3]  Excess reinforcement:  [ACI 12.2.5]  Development (tensile):  [ACI 12.2.2]  Minimum length:  [ACI 12.2.1]  Development length:  Development (comp.):  [ACI 12.3.2]	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ ' = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d_\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.02 * dl $L_{dc\_c}$ = 0.0003 * $L_{dc\_c}$ = MAX( 8,	ht concrete is r of: half the b er simplification i, (c_c + ktr_c  Mallow  (Fy / √(F'c)) * (c  s f_min, Ldt'_c c_c * Fy * R_c  db_c * Fy * R_c  Ldc'_c, Ldc'' E - cc	s used, 1.3, par spacing pon)  c) / db_c)  wtwe_c • ws  c / √(F'c)  R_c _c)	else use or the con	use 1.0  1.0  norete edge o	)_C	ψ <sub>e,c</sub> =  λ <sub>c</sub> =  C <sub>c</sub> =  k <sub>y,c</sub> =  C <sub>c</sub> ' =  R <sub>c</sub> =  L <sub>d',c</sub> =  L <sub>d,min</sub> =  L <sub>dc',c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47 9.93 10.47 69.0	in i	ОК
[ACI 12.2.4] Light weight concrete: [ACI 12.2.4] Spacing/cover. [ACI 12.2.4] Transverse bars: [ACI 12.2.3] Max term: [ACI 12.2.3] Excess reinforcement: [ACI 12.2.5] Development (lensile): [ACI 12.2.2] Minimum length: [ACI 12.2.1] Development (comp.): [ACI 12.3.2] Development (comp.):	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ ' = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d_\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.02 * dl $L_{dc\_c}$ = 0.0003 * $L_{dc\_c}$ = MAX( 8,	ht concrete is r of: half the b er simplification i, (c_c + ktr_c  Mallow  (Fy / \(\frac{1}{2}\)(F'c)) * (0  s f_min, Ldt'_c c_c * Fy * R_1  db_c * Fy * R_2  Ldc'_c, Ldc'' E - cc  Check	s used, 1.3, par spacing pon)  c) / db_c)  wtwe_c • ws  c / √(F'c)  R_c _c)	else use or the con s_c * \lambda_c * \lam	use 1.0  1.0  norete edge o	)_c >=	ψ <sub>e,c</sub> =  λ <sub>c</sub> =  C <sub>c</sub> =  k <sub>y,c</sub> =  C <sub>c</sub> ' =  R <sub>c</sub> =  L <sub>d',c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47 9.93 10.47 69.0 20.4	in i	
[ACI 12.2.4]  Light weight concrete:  [ACI 12.2.4]  Spacing/cover.  [ACI 12.2.4]  Transverse bars:  [ACI 12.2.3]  Max term:  [ACI 12.2.3]  Excess reinforcement:  [ACI 12.2.5]  Development (tensile):  [ACI 12.2.2]  Minimum length:  [ACI 12.2.1]  Development length:  Development (comp.):  [ACI 12.3.2]  Development length:  Length available in pier:	$\lambda_{\_c}$ = if lightwieg $c_{\_c}$ the smalle $k_{tr\_c}$ = 0 in (p $c_{\_c}$ = MIN( 2.5 $R_c$ = Mmax / 1 $L_{dt\_c}$ = (3 / 40) * $L_{d_\_min}$ = 12 inche $L_{dt\_c}$ = MAX( Lo $L_{dc\_c}$ = 0.0003 * $L_{dc\_c}$ = MAX( 8, $L_{vc}$ = D - T + 1	ht concrete is r of: half the b er simplification i, (c_c + ktr_c  Mallow  (Fy / \(\frac{1}{2}\)(F'c)) * (0  s f_min, Ldt'_c c_c * Fy * R_1  db_c * Fy * R_2  Ldc'_c, Ldc'' E - cc  Check	s used, 1.3, par spacing pon)  c) / db_c)  wtwe_c • ws  c / √(F'c)  R_c _c)	else use or the con s_c * \lambda_c * \lam	use 1.0  1.0  norete edge o	)_c >=	ψ <sub>e,c</sub> =  λ <sub>c</sub> =  C <sub>c</sub> =  k <sub>y,c</sub> =  C <sub>c</sub> ' =  R <sub>c</sub> =  L <sub>d',c</sub> =  L <sub>d,min</sub> =  L <sub>dc',c</sub> =  L <sub>dc,c</sub> =	1 1.0 3.50 0 2.500 0.55 20.41 12.0 20.41 10.47 9.93 10.47 69.0 20.4 10.5	in i	

Vertical Rebar Hook Ending:											
Bar size & clear cover: [ACI 12.5.3]	$\alpha_{h}$ if the bar si	ze <= 11 ar	nd side cc >=	2.5", use	0.7, €	lse use	1.0	ψ <sub>th</sub> =	0.7		
Epoxy coating: [ACI 12.5.2]	$\beta_h$ if epoxy-co	ated bars a	re used, use	1.2, else	use 1.	0		$\psi_{\mathrm{e_h}} =$	1.0		
Light weight concrete: [ACI 12.5.2]	λ <sub>h</sub> if lightwiegl	nt concrete	is used, 1.3, 6	else use	1.0			λ <sub>_h</sub> =	1.0		
Development (hook): [ACI 12.5.2]	L <sub>dh</sub> ' = 0.02 * ψt	_h * ψe_h *	λ_h * Fy / √(l	-'c) * db_	c			L <sub>dh</sub> ' =	13.3	in	
Minimum length: [ACI 12.5.1]	L <sub>dh_min</sub> the larger of	of; 8 * db or	6 in					L <sub>dh_min</sub> =	8.0	in	
Development length:	L <sub>dh</sub> = MAX( Ld	h_min, Ldh'	)					L <sub>ah</sub> =	13.3	in	
		Check	L <sub>vo</sub> =	15.0	in		>=	L <sub>ah</sub> =	13.3	in	OK
Hook tail length:	L <sub>h tail</sub> 12 * db be	ond the ber	nd radius			-4-11		L <sub>h tail</sub> =	16.0	in	
Length available in pad:	L <sub>h pad</sub> = (W - W -	To community in						L <sub>h.pad</sub> =	33	in	
	Cardinate Cardinates	Check	L <sub>h_pad</sub> =	33.0	in	EN .2	>=	L <sub>dh_tail</sub> =	16.0	in	OK
Pier Ties:	HE IIA-A-C-C-A					- 12 11 12 1					
Minimum size: [ACI 7.10.5.1]	s_t_min =IF(s_c <=	10, 3, 4)						S_t_min =	3		
z factor:	z = 0.5 if the s	eismic zone	is less than 2	2, else 1.	0			z =	0.5		
Tie parameters:	s <sub>_t</sub> = 4 m <sub>_t</sub> = 7					$d_{b_{\underline{t}}} = A_{b_{\underline{t}}} =$	0.5	in in <sup>2</sup>			
Allowable tie spacing:											
per vertical rebar [ACI 7.10.5.2] & [ACI 21.3.3.2]	$B_{s_t_max1} = 8 / z * dt$	_c						B <sub>s_t_max1</sub> =	16	in	
per tie size [ACI 7.10.5.2] & [ACI 21.3.3.2]	B <sub>s_t_max2</sub> = 24 / z * 0	lb_t						B <sub>s_t_max2</sub> =	24	in	
per pier diameter [ACI 7.10.5.2] & [ACI 21.3.3.2]	B <sub>s_t_max3</sub> = di / (4 * 2	t²)						B <sub>s_t_max3</sub> =	78	in	
per seismic zone [ACI 7.10.5.2] & [ACI 21.3.3.2]	B <sub>s_t_max4</sub> = 12" in ac	tive seismic	zones, else	18"				B <sub>s_t_max4</sub> =	18	in	
	B <sub>s t max</sub> = MIN( Bs	t_max1, B	s_t_max2, Bs	_t_max3	Bs_t	_max4)		B <sub>s_t_max</sub> =	16	in	
	m t min = (D - T +	E) / Bs_t_m	ax + 2					m <sub>t min</sub> =	6.5		
		Check	m_t =	7.0	1		>=	m_t_min =	6.5	NOT THE	ОК
Anchor Steel:											
A/S parameters:	P <sub>as</sub> = 109881					L <sub>as</sub> =	80	in			
	d <sub>as</sub> = 1.25	in				E <sub>as</sub> =	71.50	in			
Development available:	L <sub>das</sub> per Anchor	Bolts (see atta	ached)					L <sub>das</sub> =	43.50	in	
Required development:	L <sub>das_min</sub> per Anchor	Boits (see atta	ached)					L <sub>das_min</sub> =	20.41	in	
		Check	L <sub>das</sub> =	43.50	in	Viete	>=	L <sub>das_min</sub> =	20.41	in	OK
To bottom rebar grid:	E <sub>as_max</sub> =D + E - c	c - 2 * db_p		.11.				E <sub>as_max</sub> =	84.744	in	
		Check	E <sub>as</sub> =	71.50	in		<=	E <sub>as_max</sub> =	84.74	in	OK
To top rebar grid:	rebar @ = D + E -	T + cc						rebar @	75.00	in	
		Check	75 + 6 in	>=	To be	E <sub>as</sub> =	71.50	in or	<=	75 in	OK
Min. cage dia:	d <sub>o_min</sub> per ancstee	el.xls (see atta	ched)		-1			d <sub>o_min</sub> =	36.20	in	
		Check	d <sub>o</sub> =	72.00	in		>=	d <sub>o min</sub> =	36.20	in	OK

#### Pad Reactions:



Required moment: $\varphi t = 0.9$ [ACI 9.3.2.1]	M <sub>n</sub> = Mmaxp	/φt	M <sub>n</sub> =	3625.56	ft*kips	
Max moment:	M <sub>maxp</sub> =Max(Mm	ax2_1,Mmax2_2)	M <sub>maxp</sub> =	3263.00	ft*kips	
Option 2:	M <sub>max2_2</sub> =M <sub>max2_2</sub>	(Max. Moment calculated from MDsolids for Option 2)	M <sub>max2_2</sub> =	1927.00	ft*kips	
Option 1:	$M_{\text{max2}\_1} = M_{\text{max2}\_1}$	(Max. Moment calculated from MDsolids for Option 1)	M <sub>max2_1</sub> =	3263.00	ft*kips	
MDSolids Design Result						
		This linearly increasing load is a	pplied from e=14	31ft to W=41	t	
(Linearly Increasing)	q <sub>2_2R</sub> =q <sub>a</sub> * W		q <sub>2_2R</sub> =	149.53	klf	
Distributed Soil Pressure:	q <sub>2_2L</sub> =0		q <sub>2_2L</sub> =	0.00	klf	
(Distributed)		Applied over the beam star	ting at 0' and en	ding at W=41f		
Weight of Overburden:	w <sub>2_1</sub> =0.9 * (W <sub>0</sub>	<sub>5</sub> + W <sub>s</sub> ) / W	w <sub>2_1</sub> =	31.27	klf	
Compression:	P <sub>2,2</sub> =C		P <sub>2,2</sub> =	785.13	kips	
Uplift:	$P_{2_1} = U$		P <sub>2_1</sub> =	700.8	kips	
IDSolids Load Input (Option 1 & Option	on 2)					
ocation of Right Support:	S <sub>R2_2</sub> =S <sub>L1_2</sub> + w	1	S <sub>R2_2</sub> =	35.00	ft	
Location of Left Support:	S <sub>L2_2</sub> =(W - wl)		S <sub>L2_2</sub> =	6.00	ft	
Total Beam Length:	B <sub>L2_2</sub> =W		B <sub>L2_2</sub> =	41.0	ft	
DSolids Geometry Input (Option 2)						
ocation of Right Support:	S <sub>R2_1</sub> =W-O		S <sub>R2_1</sub> =	33.06	ft	
Location of Left Support:	S <sub>L2_1</sub> =0		S <sub>L2_1</sub> =	7.943	ft	
Total Beam Length:	B <sub>L2_1</sub> =W		B <sub>L2_1</sub> =	41	ft	
DSolids Geometry Input (Option 1)						

Pad Reinforcement:	. 2										
	$\beta = IF(F'c \le$	4000, 0.85, IF(F'c	>= 8000, 0.65	, 0.85 - (F'	c - 400	0.05))		β =	0.85		
Effective width:	$W_e = w' * 0.8$							W <sub>e</sub> =	31.614	ft	
	$A_{st_p}' = Mn / (0$	).9 * Fy * dc)						A <sub>st_p</sub> ' =	55.810	in <sup>2</sup>	
	$a_p = Ast_p'$	* Fy / (β * F'c *	We)					a <sub>p</sub> =	2.60	in	
Required steel:	$A_{st_p_st} = Mn / (F$	y * (dc - ap / 2)	) * (W / We	)				A <sub>st_p_st</sub> =	71.578	in <sup>2</sup>	
Shrinkage;	$\rho_{sh} = IF(Fy)$	<b>&gt;=</b> 60000, 0.001	18, 0.002)					$\rho_{\sf sh}$ =	0.0018	127	
	$A_{st_p_sh} = \rho sh * V$	N*T/2						A <sub>st_p_sh</sub> =	7.970	in <sup>2</sup>	
	$A_{st_p} = MAX(A$	Ast_p_st, Ast_p	_sh)					A <sub>st_p</sub> =	71.578	in <sup>2</sup>	
Rebar:	s_p = 9	Equally spaced	d, top and			$d_{b_p} =$	1.128	in			
	m_p = 72	bottom, both di	rections.			A <sub>b_p</sub> =	- 1	in <sup>2</sup>			
	$A_{s_p} = Ab_p$	'm_p				10 <del>10</del> 0.		A <sub>s.p</sub> =	72.00	in <sup>2</sup>	
		Check	A <sub>s p</sub> =	72.00	in <sup>2</sup>		>=	A <sub>st p</sub> =	71.58	in <sup>2</sup>	OK
Bar separation:	B <sub>s_p</sub> = (W - 2	* cc - db_p) / (i	m_p - 1) - d	b_p		. 5		B <sub>s_p</sub> =	5.70	in	
		Check	10.87	>=		B <sub>s_p</sub> =	5.70	in	>=	4.5"	ок
Pad Development Length:											
Reinforcement location:	$\psi_{t,p}$ = if the spa	ace under the re	ebar > 12 in	use 1.3,	else	use 1.0		$\psi_{t_{-p}} =$	1		
[ACI 12.2.4]											
Epoxy coating: [ACI 12.2.4]	$\psi_{e_p}$ = if epoxy- bars are	coated bars are used, then if B						$\psi_{\mathbf{e}_{\_P}}$ =	1.0		
Max term: [ACI 12.2.4]	$\psi_t \psi_{e_p} = \text{the prod}$	uct of ψt & ψe,	need not be	taken la	rger tl	han 1.7		$\psi_t \psi_{e_p} =$	1		
Reinforcement size:	$\psi_{s,p}$ = if the bar	r size is 6 or les	s then use	0.8 else	use 1	0		$\psi_{s_p} =$	1		
[ACI 12.2.4]	7 3_0		0, 0,0,, 000	0.0, 0.00				وره ۲			
Light weight concrete:	$\lambda_p = \text{if lightwise}$	eght concrete is	used, 1.3,	else use	1.0			λ_ρ =	1.0		
[ACI 12.2.4]	c = the eme	ller of helf the h	or engelne	or the on	norote	odao di	otoco		2.50	16	
Spacing/cover: [ACI 12.2.4]	C_p = the sma	ller of: half the b	oar spacing	or the co	ncrete	eage a	stace	c_p =	3.56	in	
Transverse bars:	$k_{tr_p} = 0$ in	(per simplification	on)					k <sub>tr_p</sub> =	0	in	
[ACI 12.2.3]											
Max term: [ACI 12.2.3]	$c_p' = MIN(2$	2.5, (c_p + ktr_p	o) / db_p)					c_ <sub>p</sub> ' =	2.500		
Excess reinforcement:	$R_p = Ast_p$	/ As_p						R_p =	0.99		
		-						-			
[ACI 12.2.5]	1 = 12 / 40	) * (Fy / √(F'c))	* wtwe_p *	ψs_p * λ	_p * R	?_p * db_	p / c_p'	L <sub>dp</sub> ' =	31.9	in	
[ACI 12.2.5] Development (tensile): [ACI 12.2.2]	Ld - (3740										
Development (tensile): [ACI 12.2.2] Minimum length:	L <sub>d_min</sub> = 12 incl	nes						$L_{d_{min}} =$	12.0	in	
Development (tensile): [ACI 12.2.2]  Minimum length: [ACI 12.2.1]	L <sub>d_min</sub> = 12 incl										
Development (tensile): [ACI 12.2.2] Minimum length:	L <sub>d_min</sub> = 12 incl	Ld_min, Ldp')						L <sub>d_min</sub> = L <sub>dp</sub> = L <sub>pad</sub> =	12.0 31.9 69.0	in in in	

#### THIS SPREADSHEET IS SET UP FOR A MAXIMUM OF 56 BARS. MAXIMUM FACTORED MOMENT OF A CIRCULAR SECTION

Load	ding		
(negative for	compressi	on)	
Axial load =	700.79	kips	

Found	dation	
Concrete		
Pier diameter =	6.50	ft
Pier area =	4778.4	in^2
Reinforcement		
Clear cover =	3.00	in
Cage diameter =	5.92	ft
Bar size =	8	
Bar diameter =	1.000	in
Bar area =	0.785	in^2
Number of bars =	31	

Material Strengths								
Concrete compressive strength =	4000	psi						
Reinforcement yield strength =	60000	psi						
Modulus of elasticity =	29000	ksi						
Reinforcement yield strain =	0.00207							
Limiting compressive strain =	0.003							

(per ACI 10.3.5 - OK)

Seismic		
Seismic Zone =	1	
Are hooks required?	no	

#### Minimum Area of Steel

Required area of steel = 23.89 Actual area of steel = 24.35 in^2 OK Bar spacing = 6.30 in

#### **Axial Loading**

Load factor = 1.00

Reduction factor = 0.65575 (per ACI 9.3.1 & 2)

Factored axial load = 1068.69 kips

#### **Neutral Axis**

Distance from extreme edge to neutral axis = 3.92

Equivalent compression zone factor = 0.85 (per ACI 10.2.7.3)

Distance from extreme edge to

Equivalent compression zone factor = 3.33 Distance from centroid to neutral axis = 35.08 in

#### Compression Zone

Area of steel in compression zone =

Angle from centroid of pier to intersection of

equivalent compression zone and edge of pier = 23.86 deg Area of concrete in compression = 70.73 in^2

Force in concrete = 0.85 \* fc \* Acc = 240.48 kips

(per ACI 10.3.6.2)

Total reinforcement forces = -1309.17 kips Factored axial load = 1068.69 kips Force in concrete = -240.48 kips

Sum of the forces in concrete = OK 0.00 kips

#### **Maximum Moment**

First moment of the concrete area in compression about the centoid = 2617.33 in^3 Distance between centroid of concrete in compression and centroid of pier = 37.01

Moment of concrete in compression = 8898.91 in-kips Total reinforcement moment = 5287.91 in-kips

Nominal moment strength of column = 14186.81 in-kips

Factored moment strength of column = 9302.96 in-kips 775.25 ft-kips

> Maximum allowable moment of the pier = 775.25 ft-kips

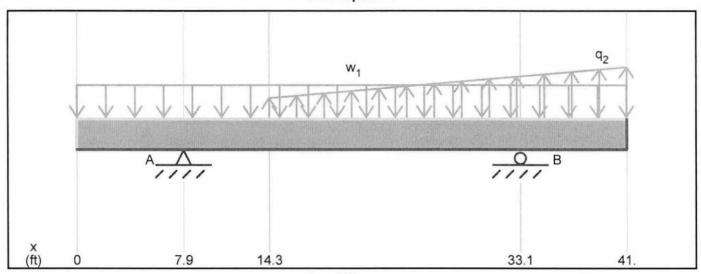
## Individual Bars

Bar	Angle from first bar	Distance to centroid	Distance to neutral axis	Distance to equivalent comp. zone	Strain	Area of steel in compressi on	Axial force	Moment
#	(deg)	(in)	(in)	(in)		(in^2)	(kips)	(in-kips)
1	0.00	0.00	-35.08	-35.67	-0.02684	0.00	-47.12	0.00
2	11.61	7.15	-27.93	-28.52	-0.02137	0.00	-47.12	-336.75
3	23.23	14.00	-21.08	-21.67	-0.01613	0.00	-47.12	-659.72
4	34.84	20.28	-14.80	-15.39	-0.01132	0.00	-47.12	-955.67
5	46.45	25.73	-9.35	-9.94	-0.00715	0.00	-47.12	-1212.50
6	58.06	30.13	-4.95	-5.54	-0.00379	0.00	-47.12	-1419.70
7	69.68	33.29	-1.79	-2.38	-0.00137	0.00	-31.17	-1037.58
8	81.29	35.09	0.01	-0.58	9E-06	0.00	0.21	7.23
9	92.90	35.45	0.38	-0.21	0.00029	0.00	6.55	232.07
10	104.52	34.37	-0.71	-1.30	-0.00054	0.00	-12.41	-426.43
11	116.13	31.87	-3.21	-3.79	-0.00245	0.00	-47.12	-1501.94
12	127.74	28.07	-7.01	-7.59	-0.00536	0.00	-47.12	-1322.89
13	139.35	23.12	-11.96	-12.54	-0.00915	0.00	-47.12	-1089.68
14	150.97	17.23	-17.85	-18.44	-0.01366	0.00	-47.12	-811.86
15	162.58	10.63	-24.45	-25.04	-0.01871	0.00	-47.12	-500.80
16	174.19	3.59	-31.49	-32.08	-0.02409	0.00	-47.12	-169.24
17	185.81	-3.59	-38.67	-39.26	-0.02959	0.00	-47.12	169.24
18	197.42	-10.63	-45.71	-46.29	-0.03497	0.00	-47.12	500.80
19	209.03	-17.23	-52.31	-52.90	-0.04002	0.00	-47.12	811.86
20	220.65	-23.12	-58.20	-58.79	-0.04453	0.00	-47.12	1089.68
21	232.26	-28.07	-63.15	-63.74	-0.04832	0.00	-47.12	1322.89
22	243.87	-31.87	-66.95	-67.54	-0.05122	0.00	-47.12	1501.94
23	255.48	-34.37	-69.45	-70.03	-0.05313	0.00	-47.12	1619.49
24	267.10	-35.45	-70.53	-71.12	-0.05396	0.00	-47.12	1670.75
25	278.71	-35.09	-70.17	-70.76	-0.05368	0.00	-47.12	1653.61
26	290.32	-33.29	-68.37	-68.96	-0.05231	0.00	-47.12	1568.76
27	301.94	-30.13	-65.21	-65.79	-0.04989	0.00	-47.12	1419.70
28	313.55	-25.73	-60.81	-61.40	-0.04652	0.00	-47.12	1212.50
29	325.16	-20.28	-55.36	-55.95	-0.04235	0.00	-47.12	955.67
30	336.77	-14.00	-49.08	-49.67	-0.03755	0.00	-47.12	659.72
31	348.39	-7.15	-42.22	-42.81	-0.03231	0.00	-47.12	336.75

Foundation:	Pier diameter =	6.5		ft	Cover between side of pier and cage =	3.00	in.
	Cage diameter =	6		ft	Cover between top of pier and cage =	3.00	in.
	Rebar size =	9			Compressive strength of concrete =	4000	psi
	Number of bars =	72			Rebar yield strength =	60000	psi
	Clear spacing =	5.70	)	in.			
	Are there hooks?	n					
	Check Compression?	n					
Anchor Steel:	Part number:	109881	v		Actual Bending Moment =	427.70	ft-kips
	Embedment length =	71.5	5	in.	Allowable Bending Moment =	775.25	ft-kips
	Bolt Diameter =	1.25°	•		Excess Reinforcement Ratio =	0.552	5.
Anchor Plate:	Part number:	212009	•				
	Plate width =	22		in.			
Required developm	nent length (compression) =	999.0	00	in.			
I was the commental formation of the problem of the comment of the	elopment length (tension) =	37.0	0	in.			
Required dev	elopment length (tension) =	20.4	1	in.	(reduced)		
Ava	ilable development length =	43.50	00	in.	Management and Control of the		
	5 55	OK					

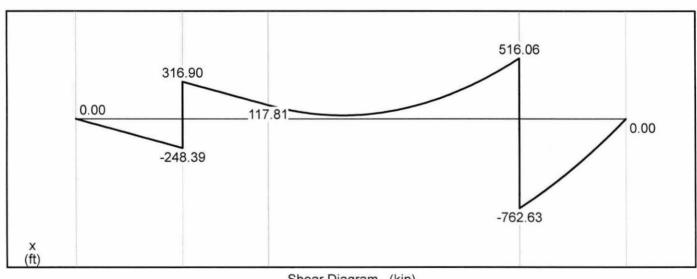
	CHECKE	WIDEDWIL	NIFLA	TE CLEARANCE IN THE PIER	
Foundation:	Pier diameter =	6.5	ft	Cover between side of pier and cage =	3.00 in.
	Cage diameter =	6	ft	Minimum cover between A/S and cage =	3.00 in.
Anchor Steel:	Part number:	109881		Angle of anchor steel in foundation =	3.3 degrees
	Embedment length =	71.5	in.		-
Anchor Plate:	Part number:	212009			
	Largest plate width =	22.00	in.		
	Bolt Diameter =	1.25	in.		
	Minimum cage diameter =	36.20	in.		
	Actual cage diameter =	72	in.		
		OK			

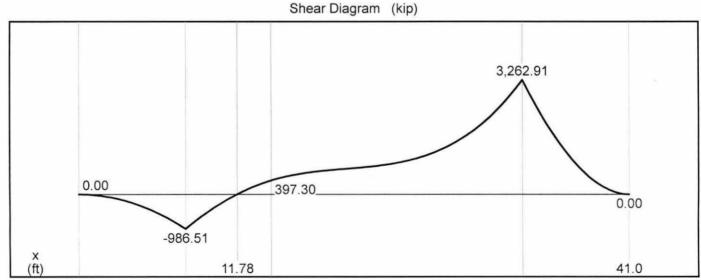
#### LC2- Option 1



w<sub>1</sub> = 31.27 kip/ft (down) q<sub>2</sub> = 0.0 to 149.53 kip/ft (up) Load Diagram

 $A_y = 565.28 \text{ kip (up)}$  $B_y = 1,278.69 \text{ kip (down)}$ 

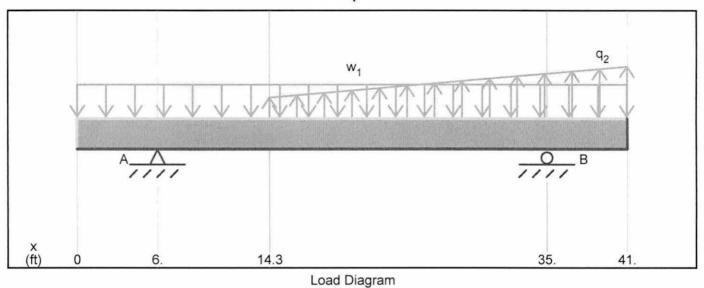




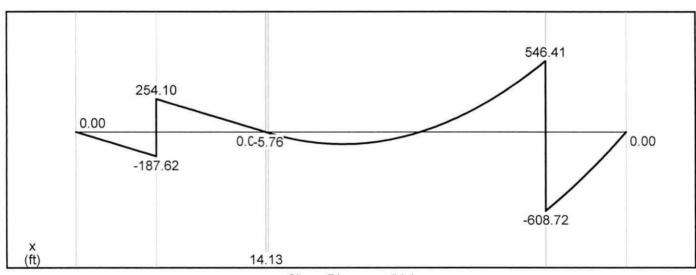
Moment Diagram (kip-ft)

. . . ,

LC2- Option 2



 $w_1 = 31.27 \text{ kip/ft (down)}$  $q_2 = 0.0 \text{ to } 149.53 \text{ kip/ft (up)}$   $A_y = 441.72 \text{ kip (up)}$  $B_y = 1,155.12 \text{ kip (down)}$ 



Shear Diagram (kip)

1,926.99

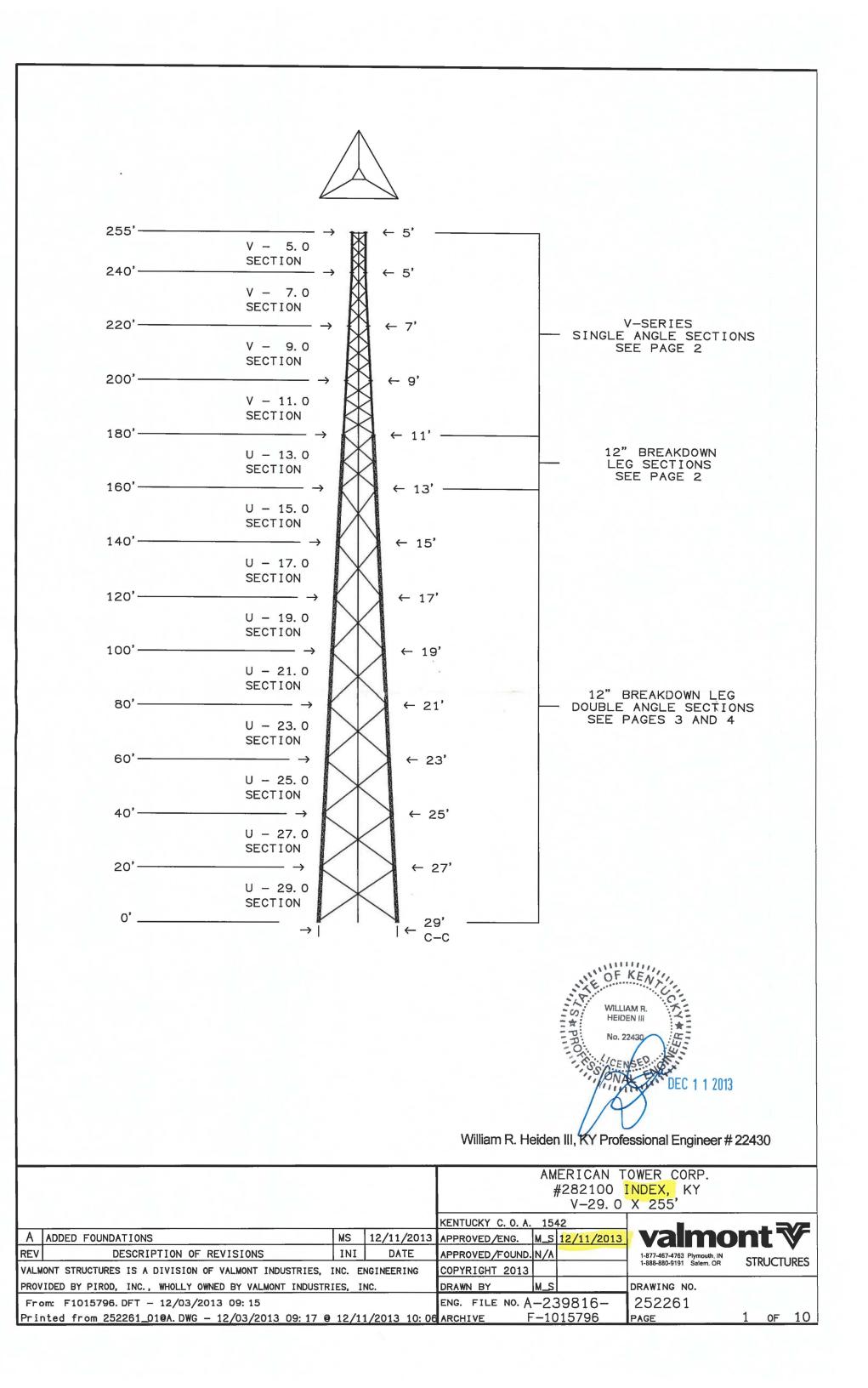
0.00

469.52
468.99

0.00

x
(ft)
8.65
14.13
21.12
29.2
41.0

Moment Diagram (kip-ft)



V-SERIES LEG SECTION DATA 180' - 255' ELEVATION																			
SECTION LEG DIAGONAL BRACE HOP															HOR				
# LENGTH * NOM WALL GRADE CLIMBING NON-CLIMB CONNECT BOLT+ PART NUMBER ** ANGLE CONNECT BOLT CENTER QTY																			
LENGIH	WEIGHT	SIZE	WALL	GRADE	QTY	PART#	QTY	PART#	DIAM	LENGTH	#1	#2	#3	FACE	THICK	DIAM	LENGTH	SPACER	QIY
15'	1013#	4"	0. 237	A572-50	1	228175	2	228176	3/4"	3-1/2"	227077	227077	227077	2*	1/8"	3/4"	2-1/4"	116467	1
20'	1609#	5"	0. 258	A572-50	1	226200	2	226201	3/4"	3-1/2"	226190	226189	231342	2*	3/16"	3/4"	2-1/4"	116467	
20'	1861#	5*	0. 258	A572-50	3	226192			3/4"	3-1/2"	225035	225034	231345	2-1/2"	3/16"	3/4"	2-1/4"	116467	
-11. 0 20' 2390# 6" 0. 280 A572-50 3 229377   1" 4-3/4" 225038 225037 231347 2-1/2" 3/16" 3/4" 2-1/4" 116467																			
	15' 20' 20'	LENGTH WEIGHT 15' 1013# 20' 1609# 20' 1861#	LENGTH # NOM SIZE  15' 1013# 4"  20' 1609# 5"  20' 1861# 5"	**************************************	SECTION           #         NOM WALL         WALL         GRADE           15'         1013#         4"         0.237         A572-50           20'         1609#         5"         0.258         A572-50           20'         1861#         5"         0.258         A572-50	SECTION           #         NOM WALL         WALL         GRADE         CL QTY           15'         1013#         4"         0.237         A572-50         1           20'         1609#         5"         0.258         A572-50         1           20'         1861#         5"         0.258         A572-50         3	SECTION	SECTION   SECT	SECTION   SECT	SECTION   SECTION   LEG   SECTION   SIZE   WALL   GRADE   CLIMBING   NON-CLIMB   CONNECT   CT   CT   CT   CT   CT   CT   CT	SECTION   SECT	SECTION   SECT	SECTION   LEG   LEG   STATE   STATE	SECTION   SECTION   LEG   SECTION   SECTION	SECTION   SECT	SECTION   SECT	SECTION   LEG	SECTION   STATE   SECTION   STATE   SECTION   SECTION   STATE   SECTION   SE	SECTION   LEG

+ AT BOTTOM OF SECTION

• THE WEIGHTS LISTED ARE THEORETICAL. THE ACTUAL WEIGHTS WILL VARY. ALL WEIGHTS SHOULD BE CONFIRMED IN THE FIELD PRIOR TO ERECTION.

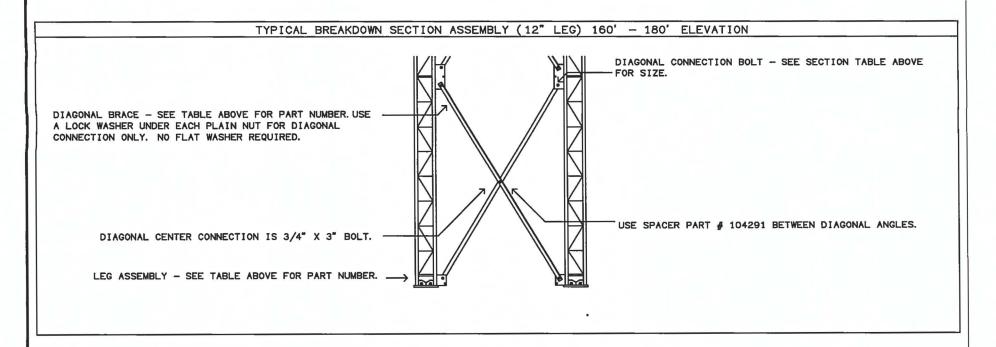
•\* PANELS ARE NUMBERED BEGINNING AT THE TOP OF THE SECTION.

NUMBER.

HORIZONTAL DATA	TYPICAL V-SERIES	SECTION ASSEMBLY 180' - 255' ELEVATION
HORIZ IN HORIZ HT SEC# PART# 255 V- 5. 0 227584	HORIZONTALS AS REQUIRED. SEE TABLE TO LEFT FOR ELEVATION AND PART #.	
	DIAGONAL BRACE - SEE TABLE ABOVE FOR PART NUMBER.	DIAGONAL CONNECTION BOLT - SEE SECTION TABLE ABOVE FOR SIZE.
	DIAGONAL CENTER CONNECTION - 5/8" X 2-1/4" BOLT	DIAGONAL CENTER SPACER - SEE TABLE ABOVE FOR PART NUMBER.

		BREAK	DOWN S	ECTION	DATA (1	2" LEC	3) 160'	- 180	' ELEV	ATION		
SEC	SECTION	LEG	LEG	TOP DIAG	BOT DIAG	DIAGONA	L ANGLE	SECTION	LEG CO	NNECT+	DIAG	CONNECT
#	LENGTH	SIZE	PART#	PART#	PART#	FACE	THICK	WEIGHT	DIAM	LENGTH	DIAM	LENGTH
U-13. 0	20'	1- 3/4"	229588	105575	105577	3"	5/16"	3468#	1"	4-3/4"	1 "	2-1/4"
* THE \	WEIGHTS I	ISTED A	RE THEO	RETICAL.	THE ACTU	AL WEIG	HTS WILL	VARY.	ALL WEI	GHTS SHO	DULD BE	
CON	FIRMED I	N THE FI	ELD PRI	OR TO ERE	ECTION.							
+ USE	1 FLAT W	ASHER UNI	DER EACH	H LOCK WA	ASHER FOR	LEG CO	NNECTION	ONLY.				

LEG ASSEMBLY - SEE TABLE ABOVE FOR PART





LEG CONNECTION - SEE TABLE ABOVE FOR BOLT

William R. Heiden III, KY Professional Engineer # 22430

AMERICAN TOWER CORP.

		#282100 . V-29. 0			
	APPROVED/FOUND.	M_S 12/11/2013 N/A	Valmo  1-877-467-4763 Plymouth, IN 1-888-880-9191 Salem, OR	nt struc	1
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BREAKDOWN SECTION LEG DATA (12" LEG WITH DOUBLE ANGLES) 0' - 160' ELEVATION

		S	SECTION		LE	:G			ONNECT	
	#	MODEL	LENGTH	WEIGHT*	SIZE		PART #	DIAM	LENGTH #	F
L	8	U-15. 0	20'	4545#	2	"	208332	1"	4-3/4" 12	2
	7	U-17. 0	20'	5237#	2- 1/4	22	208334	1"	4-3/4" 12	2
	6	U-19. 0	20'	5916#	2- 1/2	"	208335	1"	4-3/4" 12	2
	5	U-21. 0	20'	6016#	2- 1/2	"	208335	1"	4-3/4" 12	2
	4	U-23. 0	20'	6899#	2- 3/4	"	208337	1"	4-3/4" 12	2
	3	U-25. 0	20'	7007#	2- 3/4	"	208337	1"	4-3/4" 12	2
	2	U-27. 0	20'	8421#	3	"	208336	1-1/4"	5" 12	2
	1	U-29. 0	20'	8211#	3	37	208338			

- \* THE WEIGHTS LISTED ARE THEORETICAL. THE ACTUAL WEIGHTS WILL VARY. ALL WEIGHTS SHOULD BE CONFIRMED IN THE FIELD PRIOR TO ERECTION.
- + QTY IS PER LEG. USE 1 LOCK WASHER AND 1 FLAT WASHER UNDER EACH PLAIN NUT.

В	REAKDOWN	SECTIO	ON DIAG	ONAL DA	ATA (12	LEG	WITH D	OUBLE A	NGLES)	0' -	- 160' E	ELEVATI	ON
	SECTION	DI	AGONAL	PART #	DIAG	ANGLE	DIAG EN	ND BOLT	DIAG CE SPACER		CENTER PLATE	SPACE	R
#	MODEL	UPPER	LOWER	LONG	FACE	THICK	DIAM	LENGTH	DIAM	LENGTH	PART #	PART #	#*
8	U-15. 0	215273	215277	215358	3-1/2"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	5
7	U-17. 0	215281	215285	215362	3-1/2"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	6
6	U-19. 0	215289	215293	215365	3-1/2"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	7
5	U-21. 0	215296	215300	215369	3-1/2"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	. 8
4	U-23. 0	215304	215308	215373	3-1/2"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	. 8
3	U-25. 0	215312	215316	215377	3-1/2"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	8
2	U-27. 0	215321	215325	215381	4"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	. 8
1	U-29. 0	215328	215331	215384	4"	1/4"	7/8"	2-1/2"	5/8"	2-1/4"	211833	104291	. 8
*	QUANTITY	IS PER	PANEL	PER FAC	E. USE	1 LOCK	WASHER	UNDER E	ACH PLA	IN NUT.			



William R. Heiden III, KY Professional Engineer # 22430

AMERICAN TOWER CORP. #282100 INDEX, KY V-29. 0 X 255'

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252261 3 of 10 PAGE

TYPICAL BREAKDOWN SECTION ASSEMBLY (12" LEG WITH DOUBLE ANGLES) O' - 160' ELEVATION DIAGONAL END BOLTS - SEE DIAGONAL TABLE ON PAGE 3 FOR SIZE. NO FLAT WASHER "LONG" DIAGONAL BRACE REQUIRED. (BACK TO BACK ANGLES) - SEE TABLE ON PG. 3 FOR PART #. "UPPER" DIAGONAL BRACES (BACK TO BACK ANGLES) - SEE INTERMEDIATE DIAGONAL BOLTS TABLE ON PG. 3 FOR PART #. WITH SPACER - SEE TABLE ON PG. 3 FOR SIZE, SPACER PART # AND NUMBER OF LOCATIONS PER PANEL ON EACH FACE. USE 1 SPACER PER BOLT. SEE DRAWING # 214823 FOR DETAILS. DIAGONAL CENTER PLATE -SEE DIAGONAL TABLE ON PAGE 3 FOR PART # AND BOLT SIZE. "LOWER" DIAGONAL BRACES (BACK TO BACK ANGLES) - SEE TABLE ON PG. 3 FOR PART #. LEG CONNECTION - SEE TABLE ON PAGE 3 FOR BOLT SIZE. USE 1 LOCK WASHER AND 1 FLAT

## ATTENTION ERECTOR:

- 1. EXTRA CARE MUST BE TAKEN WHEN STANDING BREAKDOWN LEG SECTIONS FROM A FLAT "ASSEMBLY" POSITION ON THE GROUND TO AN UPRIGHT POSITION FOR STACKING. POOR RIGGING AND/OR LIFTING PROCEDURES MAY DAMAGE THE ANGLE BRACES AND/OR BREAKDOWN LEGS. IT IS THE RESPONSIBILTY OF THE TOWER CONTRACTOR TO ENSURE BREAKDOWN LEGS AND ANGLES ARE NOT DAMAGED DURING THE TOWER ASSEMBLY AND ERECTION.
- 2. WHEN LIFTING ("FLYING") SINGLE PANEL TOWER SECTIONS TO PLACE THEM ON PREVIOUSLY ERECTED SECTIONS, A MINIMUM OF TWO (2) FULL SECTIONS (TYPICALLY 40') MUST BE ASSEMBLED TOGETHER TO PROVIDE ADEQUATE STABILITY TO THE TOWER LEGS AND ANGLE BRACES. IT IS THE RESPONSIBILTY OF THE TOWER CONTRACTOR TO ENSURE BREAKDOWN LEGS AND ANGLES ARE NOT DAMAGED DURING THE TOWER ASSEMBLY AND ERECTION.



WASHER UNDER EACH PLAIN NUT

FOR LEG CONNECTION.

William R. Heiden III, KY Professional Engineer # 22430

AMERICAN TOWER CORP.

#282100 INDEX, KY V-29. 0 X 255' KENTUCKY C. O. A. 1542 M\_S 12/11/2013 APPROVED/ENG. APPROVED/FOUND. N/A **STRUCTURES** COPYRIGHT 2013 VALMONT STRUCTURES IS A DIVISION OF VALMONT INDUSTRIES, INC. ENGINEERING PROVIDED BY PIROD, INC., WHOLLY OWNED BY VALMONT INDUSTRIES, INC. DRAWN BY KWD DRAWING NO. From: F1015796.DFT - 12/03/2013 09:15 ENG. FILE NO. A-239816-252261 of 10 Printed from 252261\_0400. DWG - 12/03/2013 09: 17 @ 12/11/2013 10: 0 ARCHIVE F-1015796 PAGE

#### GENERAL NOTES

- 1. TOWER DESIGN CONFORMS TO STANDARD TIA-222-G UTILIZING AN 90 MPH 3-SEC GUST BASIC WIND SPEED WITH A STRUCTURE CLASS OF II, TOPOGRAPHIC CATEGORY OF 3 AND EXPOSURE C CRITERIA WITH NO ICE. TOWER DESIGN CONFORMS TO STANDARD TIA-222-G UTILIZING AN 30 MPH 3-SEC GUST BASIC WIND SPEED WITH A STRUCTURE CLASS OF II, TOPOGRAPHIC CATEGORY OF 3 AND EXPOSURE C CRITERIA WITH .75" RADIAL ICE. CREST HEIGHT OF 240 FEET
- 2. NO TWIST AND SWAY LIMITATIONS SPECIFIED OR USED FOR THIS TOWER.
- 3. MATERIAL: (A) SOLID RODS TO ASTM A572 GRADE 50. (B) ANGLES TO ASTM A36. (C) PIPE TO ASTM A500 GRADE B. (D) STEEL PLATES TO ASTM A36. (E) CONNECTION BOLTS TO ASTM A325 OR ASTM A449 (Fu=120 KSI AND Fy=92 KSI) AND ANCHOR BOLTS TO ASTM F1554 (Fu=150 KSI AND Fy=105 KSI). (F) TOWER LEG PIPE TO BE ASTM A500 GRADE B/C WITH 50KSI MIN. YIELD STRENGTH
- 4. BASE REACTIONS PER TIA-222-G FOR 90 MPH BASIC WIND SPEED WITH NO ICE (REACTIONS INCLUDE TIA-222-G LOAD FACTORS): TOTAL WEIGHT = 107.0 KIPS.

  MAXIMUM COMPRESSION = 782.0 KIPS PER LEG. MOMENT = 18730.0 KIP-FT. MAXIMUM UPLIFT = 698.0 KIPS PER LEG. MAXIMUM SHEAR = 142.0 KIPS TOTAL.
- 5. BASE REACTIONS PER TIA-222-G FOR 30 MPH BASIC WIND SPEED WITH 0.75" RADIAL ICE (REACTIONS INCLUDE TIA-222-G LOAD FACTORS): TOTAL WEIGHT = 331.0 KIPS. MOMENT = 2204.0 KIP-FT. MAXIMUM SHEAR = 16.0 KIPS TOTAL.
- 6. FINISH: ALL BOLTS ARE GALVANIZED IN ACCORDANCE WITH ASTMA153 (HOT DIPPED) OR ASTM B695 CLASS 50 (MECHANICAL). ALL OTHER STRUCTURAL MATERIALS ARE GALVANIZED IN ACCORDANCE WITH ASTM123.
- 7. ANTENNAS: 255'-135 SQ. FT. AREA WITH 3,000# WITH ICE/115 SQ. FT. AREA WITH 2,000# NO ICE AND (18) 1-5/8" LINES. 245'-135 SQ. FT. AREA WITH 3,000# WITH ICE/115 SQ. FT. AREA WITH 2,000# NO ICE AND (18) 1-5/8" LINES. 235'-135 SQ. FT. AREA WITH 3,000# WITH ICE/115 SQ. FT. AREA WITH 2,000# NO ICE AND (18) 1-5/8" LINES. 225'-135 SQ. FT. AREA WITH 3,000# WITH ICE/115 SQ. FT. AREA WITH 2,000# NO ICE AND (18) 1-5/8" LINES.
  - NOTE: (A) ELEVATIONS ARE TO THE BOTTOM OF THE ANTENNAS EXCEPT FOR MICROWAVE DISHES, WHICH ARE TO THE CENTERLINE. (B) ALL TRANSMISSION LINES MUST BE PLACED ON PIROD SUPPLIED LINE BRACKETS.
- 8. REMOVE FOUNDATION TEMPLATE PRIOR TO ERECTING TOWER. INSTALL BASE SECTION WITH MINIMUM OF 2" CLEARANCE ABOVE CONCRETE. SEE BASE SECTION PLACEMENT PAGE FOR MORE INFORMATION. PACK NON-SHRINK STRUCTURAL GROUT UNDER BASE SECTION AFTER LEVELING TOWER.
- 9. MIN. WELDS 5/18" UNLESS OTHERWISE SPECIFIED. ALL WELDING TO CONFORM TO AWS D1.1 SPECIFICATIONS .
- 10. THIS DRAWING DOES NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS. SEQUENCES AND PROCEDURES.
- 11. ALL BOLTS AND NUTS MUST BE IN PLACE BEFORE THE ADJOINING SECTIONS ARE INSTALLED.
- 12. ALL STRUCTURAL BOLTS ARE TO BE TIGHTENED TO A SNUG TIGHT CONDITION AS DEFINED BY AISC SPECIFICATION UNLESS OTHERWISE NOTED.
- 13. ATTENTION TOWER ERECTOR: COAT ALL BOLT ASSEMBLIESTHAT USE PIN LOCK NUTS WITH ZINC RICH COLD GALVANIZING COMPOUND AFTER FINAL TIGHTNENING.
- 14. TIA-222-G GROUNDING FOR TOWER.
- 15. TOWER LIGHTING SUPPLIED BY OTHERS.



William R. Heiden III, KY Professional Engineer # 22430

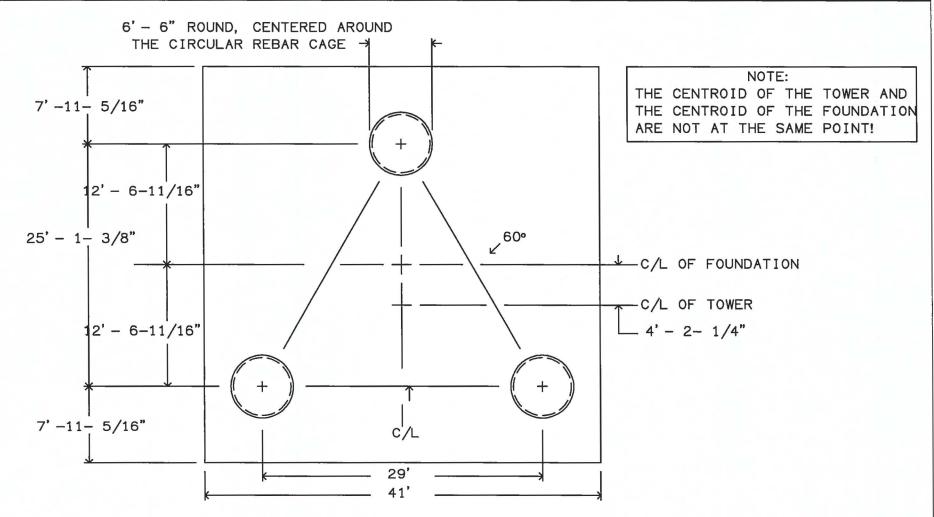
AMERICAN TOWER CORP. #282100 INDEX, KY V-29. 0 X 255' KENTUCKY C. O. A. 1542 M\_S 12/11/2013 APPROVED/ENG. APPROVED/FOUND.N/A **STRUCTURES** VALMONT STRUCTURES IS A DIVISION OF VALMONT INDUSTRIES, INC. ENGINEERING COPYRIGHT 2013 PROVIDED BY PIROD, INC., WHOLLY OWNED BY VALMONT INDUSTRIES, INC. DRAWN\_BY KWD DRAWING NO. From: F1015796. DFT - 12/03/2013 09: 15 ENG. FILE NO. A-239816-252261 Printed from 252261\_05@@. DWG - 12/03/2013 09: 18 @ 12/11/2013 10: 00 5 of 10 ARCHIVE <u>F-1015796</u> PAGE

## FOUNDATION NOTES

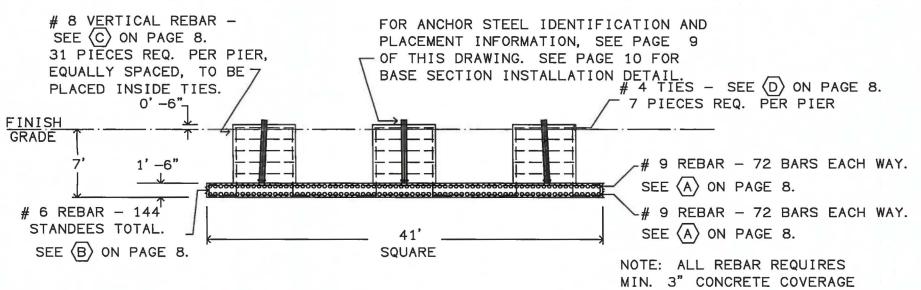
- 1. ULTIMATE SOIL PRESSURE ASSUMED TO BE 5000 PSF. ULTIMATE PASSIVE PRESSURE ASSUMED TO BE 400 LB PCF. THE PURCHASER & OWNER/CONTRACTOR MUST VERIFY THAT THE ACTUAL SITE SOIL PARAMETERS MEET OR EXCEED THE ASSUMED SOIL PARAMETERS PER THIS NOTE AND/OR SHOULD OBTAIN A SOIL REPORT TO DETERMINE THE SOIL CONDITIONS AT THE SITE. FOUNDATION DESIGN MODIFICATIONS MAY BE REQUIRED IN THE EVENT THE ASSUMED SOIL PARAMETERS ARE NOT APPLICABLE FOR THE ACTUAL SUBSURFACE CONDITIONS ENCOUNTERED.
- 2. CONCRETE TO BE 4000 PSI 28 DAYS. REINFORCING BAR TO CONFORM TO ASTM A615 GRADE 60 SPECIFICATIONS. CONCRETE INSTALLATION TO CONFORM TO ACI-318 (2008) BUILDING REQUIREMENTS FOR REINFORCED CONCRETE. ALL CONCRETE TO BE PLACED AGAINST UNDISTURBED EARTH FREE OF WATER AND ALL FOREIGN OBJECTS AND MATERIALS. A MINIMUM OF THREE INCHES OF CONCRETE SHALL COVER ALL REINFORCEMENT. WELDING OF REBAR NOT PERMITTED.
- 3. A COLD JOINT IS PERMISSIBLE UPON CONSULTATION WITH PIROD. ALL COLD JOINTS SHALL BE COATED WITH BONDING AGENTS PRIOR TO SECOND POUR.
- 4. ALL FILL SHOULD BE PLACED IN LOOSE LEVEL LIFTS OFNO MORE THAN 12" THICK. FILL MATERIALS SHOULD BE CLEAN AND FREE OF ORGANIC AND FROZEN MATERIALS OR ANY OTHER DELETERIOUS MATERIALS. COMPACT FILL TO 97% OF STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698.
- 5. BENDING, STRAIGHTENING OR REALIGNING (HOT OR COLD) OF THE ANCHOR BOLTS BY ANY METHOD IS PROHIBITED.
- 6. CROWN TOP OF FOUNDATION FOR PROPER DRAINAGE.
- 7. IN THE ABSENCE OF A GEOTECHNICAL REPORT, THE FOLLOWING PRESUMPTIVE SOIL PARAMETERS WERE USED: AN ULTIMATE BEARING PRESSURE OF 5000 PSF, A COHESION OF 1000 PSF, A SOIL UNIT WEIGHT OF 110 PCF, AN ANGLE OF INTERNAL FRICTION OF O DEGREES AND NO GROUNDWATER ENCOUNTERED. THESE SOIL PARAMETERS ARE IN COMPLIANCE WITH THE REQUIREMENTS OF ANSI/TIA-222-G-2005 AND CAN BE FOUND IN ANNEX F OF THIS STANDARD.



		AMERICAN TOWER CORP. #282100 INDEX, KY V-29.0 X 255'								
				KENTUCKY C. O. A.	15	42			_ =	
Α	ADDED FOUNDATIONS	MS	12/11/2013	APPROVED/ENG.	M_S	12/11/2013	valmo	n	t٦	
REV	DESCRIPTION OF REVISIONS	INI	DATE	APPROVED/FOUND.	M_S	12/11/2013	1-877-467-4763 Plymouth, IN			*
VALM	ONT STRUCTURES IS A DIVISION OF VALMONT INDUSTRIES,	INC. E	NGINEERING	COPYRIGHT 2013			1-888-880-9191 Salem, OR	311	RUCTI	JKE2
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NOTE: THE FOUNDATIONS DEPICTED ON THIS DRAWING WERE DESIGNED PER ASSUMED SOIL PARAMETERS.
ALTHOUGH, IT IS OUR EXPECTATION THAT THE SOIL WILL EXHIBIT SUFFICIENT STRENGTH TO COMPLY WITH THE ASSUMED STRENGTHS, IT IS POSSIBLE THAT THE SOIL MAY NOT EXHIBIT THE REQUIRED STRENGTHS. THEREFORE, IT IS HIGHLY RECOMMENDED THAT THE ASSUMED PROPERTIES BE CONFIRMED BY A GEOTECHNICAL ENGINEER VIA A SOIL REPORT OR AN ON-SITE INSPECTION DURING INSTALLATION.



# TOWER FOUNDATION

115.5 CUBIC YARDS CONCRETE REQUIRED

FOR INSTALLATION SPECIFICATIONS AND
ADDITIONAL INFORMATION, SEE PAGE 6

OF THIS DRAWING.



		AMERICAN TOWER CORP. #282100 INDEX, KY V-29.0 X 255'							
				KENTUCKY C. O. A.	15	42			
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# 9 REBAR - 288 PIECES REQ. TOTAL APPROX WT = 137.7# EACH, 39658# TOTAL

REBAR SUPPORTS MAY CONSIST OF ANY ACCEPTABLE MEANS OF SECURELY SUPPORTING THE TOP REINFORCEMENT GRID ABOVE THE BOTTOM REINFORCEMENT GRID WHILE MAIN—TAINING A SEPARATION OF 1' (OUTSIDE REBAR TO OUTSIDE REBAR).

(B)<sub>7-1/2"</sub> | | 1'

# 6 REBAR - 144 PIECES REQUIRED TOTAL TYPE 26 STANDEE PLACED BETWEEN REBAR GRIDS ON NOMINAL 4' SPACING THROUGHOUT APPROX UNBENT LENGTH = 4' - 2 - 5/8" APPROX WT = 6.3# EACH, 907# TOTAL

6' → C

# 4 REBAR - 21 PIECES REQUIRED TOTAL APPROX UNBENT LENGTH = 20' - 4- 3/4" APPROX WT = 13.6# EACH, 286# TOTAL

LAP DIMENSION: 1'-6-1/2"
PLACE CIRCULAR TIES SO THAT LAPS ON
ADJACENT TIES ARE 180 DEGREES APART.
PLACE ONE TIE AT TOP OF PAD AND TWO
TIES AT TOP OF PIER REBAR. EQUALLY
SPACE REMAINING TIES ALONG PIER.

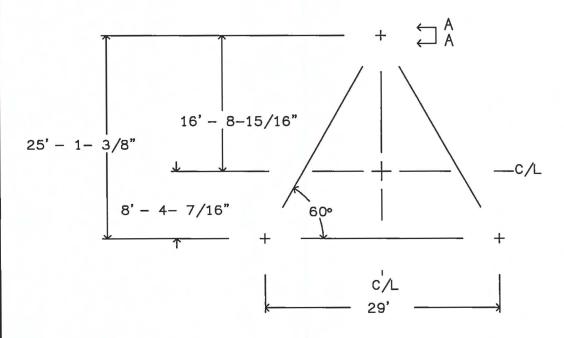
NOTE: THE FOUNDATIONS DEPICTED ON THIS DRAWING WERE DESIGNED PER ASSUMED SOIL PARAMETERS.
ALTHOUGH, IT IS OUR EXPECTATION THAT THE SOIL WILL EXHIBIT SUFFICIENT STRENGTH TO COMPLY WITH THE ASSUMED STRENGTHS, IT IS POSSIBLE THAT THE SOIL MAY NOT EXHIBIT THE REQUIRED STRENGTHS. THEREFORE, IT IS HIGHLY RECOMMENDED THAT THE ASSUMED PROPERTIES BE CONFIRMED BY A GEOTECHNICAL ENGINEER VIA A SOIL REPORT OR AN ON-SITE INSPECTION DURING INSTALLATION.

## REBAR DETAIL

TOTAL APPROX REBAR WEIGHT = 42990# REINFORCING BAR TO CONFORM TO ASTM A615 GRADE 60 SPECIFICATIONS.



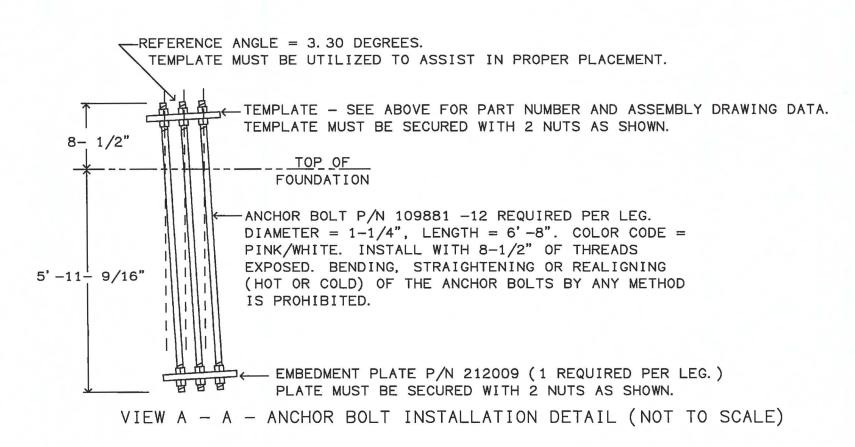
		AMERICAN TOWER CORP. #282100 INDEX, KY V-29.0 X 255'								
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TOWER ANCHOR STEEL PLACEMENT - TOP VIEW

TEMPLATE ASSEMBLY P/N 216165 INCLUDES CORNER PLATE P/N 211904, IS REQUIRED FOR INSTALLATION AND MUST BE PLACED AS SHOWN. SEE DRAWING # 211875 FOR TEMPLATE ASSEMBLY DETAILS. SEE PAGE 7 FOR TOWER C/L LOCATION RELATIVE TO THE FOUNDATION LAYOUT. TEMPLATE PLACEMENT +/- 3". EACH LEG MUST BE CENTERED IN PIER WITHIN +/-10% OF PIER DIAMETER. TEMPLATE MUST BE LEVEL +/- 1 DEGREE. INSTALL TEMPLATE WITH SUFFICIENT SPACE BENEATH (2" MINIMUM) TO PERMIT FINISHING OF CONCRETE AND TO FACILITATE TEMPLATE REMOVAL PRIOR TO TOWER ERECTION.

SEE PAGE 10 FOR BASE SECTION INSTALLATION DETAIL.



# ATTENTION CONTRACTOR INSTALLING THE ANCHOR BOLTS!

1-1/4" DIAMETER ANCHOR BOLTS FOR TAPERED TOWER.

VERIFY THE PART NUMBERS AND SIZES FOR ALL COMPONENTS ON THIS PAGE AND PAGE 10.

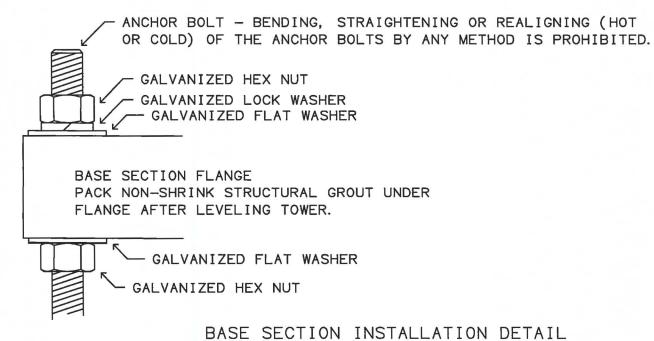
IF THERE ARE ANY DISCREPANCIES, PLEASE NOTIFY PIROD, INC. PRIOR TO INSTALLATION!!



William R. Heiden III, KY Professional Engineer # 22430

AMERICAN TOWER CORP.

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WILLIAM R.
HEIDEN III
No. 22430
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EXHIBIT D
COMPETING UTILITIES, CORPORATIONS, OR PERSONS LIST

Navigation R

PSC Home

# KY Public Service Commission

# Master Utility Search

 Search for the utility of interest by using any single or combination of criteria.

Utility ID

Utility Name

Address/City/Contact Utility Type

**Status** 

▼ Active ▼

Search

 Enter Partial names to return the closest match for Utility Name and Address/City/Contact entries.

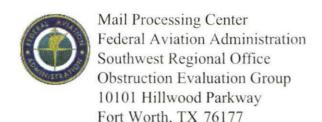
	Utility ID	Utility Name	Utility Type	Class	City	State
View	4107900	365 Wireless, LLC	Cellular	D	Atlanta	GA
View	4109300	Access Point, Inc.	Cellular	D	Cary	NC
View	4108300	Air Voice Wireless, LLC	Cellular	D	Bloomfield Hill	MI
View	44451184	Alltel Communications, LLC	Cellular	А	Basking Ridge	NJ
View	4107800	American Broadband and Telecommunications Company	Cellular	D	Toledo	он
View	4108650	AmeriMex Communications Corp.	Cellular	В	Roswell	GA
View	4105100	AmeriVision Communications, Inc. d/b/a Affinity 4	Cellular	D	Norfolk	VA
View	4107400	Bandwidth.com, Inc.	Cellular	В	Raleigh	NC
View	4108600	BCN Telecom, Inc.	Cellular	D	Morristown	NJ
View	4108750	Blue Jay Wireless, LLC	Cellular	D	Addison	TX
View	4202300	Bluegrass Wireless, LLC	Cellular	Α	Elizabethtown	KY
View	4107600	Boomerang Wireless, LLC	Cellular	D	Hiawatha	IA
View	4105600	Budget PrePay, Inc. dba Budget Mobile	Cellular	А	Bossier City	LA
View	4105500	BullsEye Telecom, Inc.	Cellular	D	Southfield	MI
View	4110050	CampusTVs, Inc.	Cellular	С	Weston	MA
View	4100700	Cellco Partnership dba Verizon Wireless	Cellular	А	Basking Ridge	NJ
View	4106600	Cintex Wireless, LLC	Cellular	D	Rockville	MD
View	4101900	Consumer Cellular,	Cellular	А	Portland	OR

		Incorporated				
View	4104900	Credit Union Wireless, LLC	Cellular	D	Salem	OR
View	4106400	Credo Mobile, Inc.	Cellular	Α	San Francisco	CA
View	4108850	Cricket Wireless, LLC	Cellular	D	Alpharetta	GΑ
View	4001900	CTC Communications Corp. d/b/a EarthLink Business I	Cellular	D	Marlborough	MA
View	10640	Cumberland Cellular Partnership	Cellular	Α	Elizabethtown	KY
View	4109250	Defense Mobile Corporation	Cellular	D	Westport	CT
View	4101000	East Kentucky Network, LLC dba Appalachian Wireless	Cellular	Α	Ivel	KY
View	4002300	Easy Telephone Service Company dba Easy Wireless	Cellular	D	Ocala	FL
View	4109500	Enhanced Communications Group, LLC	Cellular	D	Bartlesville	ОК
View	4109050	EOS Mobile Holdings, LLC	Cellular	D	Southlake	TX
View	4105900	Flash Wireless, LLC	Cellular	D	Concord	NC
View	4107100	Flatel Wireless, Inc dba Zing PCS	Cellular	D	Royal Palm Bch	FL
View	4104800	France Telecom Corporate Solutions L.L.C.	Cellular	D	Oak Hill	VA
View	4109350	Global Connection Inc. of America	Cellular	D	Norcross	GA
View	4102200	Globalstar USA, LLC	Cellular	В	Covington	LA
View	4109600	Google North America Inc.	Cellular	С	Mountain View	CA
View	33350363	Granite Telecommunications, LLC	Cellular	D	Quincy	MA
View	4106000	GreatCall, Inc. d/b/a Jitterbug	Cellular	Α	San Diego	CA
View	10630	GTE Wireless of the Midwest dba Verizon Wireless	Cellular	Α	A Basking Ridge	
View	4103100	i-Wireless, LLC	Cellular	Α	Newport	KY
View	4109800	IM Telecom, LLC d/b/a Infiniti Mobile	Cellular	С	Tulsa	ОК
View	22215360	KDDI America, Inc.	Cellular	С	New York	NY
View	10872	Kentucky RSA #1 Partnership	Cellular	Α	Basking Ridge	NJ
View	10680	Kentucky RSA #3 Cellular General	Cellular	Α	Elizabethtown	KY
View	10681	Kentucky RSA #4 Cellular General	Cellular		Elizabethtown	
View	4109750	Konatel, Inc. dba telecom.mobi			Johnstown	PA
View	4107300	Lycamobile USA, Inc.	Cellular	D	Newark	NJ
View	4108100	MCC Telephony of the South, LLC	Cellular		Mediacom Park	NY
View	4108800	MetroPCS Michigan, LLC	Cellular	Α	Bellevue	WA
View	4109650	Mitel Cloud Services, Inc.	Cellular	С	Mesa	ΑZ
View	4109400	NetZero Wireless, Inc.	Cellular	D	Woodland Hills	CA

	1		1		1	1
View	4202400	New Cingular Wireless PCS, LLC dba AT&T Mobility, PCS	Cellular	Α	San Antonio	TX
View	10900	New Par dba Verizon Wireless	Cellular	Α	Basking Ridge	СИ
View	4000800	Nextel West Corporation	Cellular	Α	Overland Park	KS
View	4104500	Nexus Communications, Inc.	Cellular	D	Columbus	ОН
View	4001300	NPCR, Inc. dba Nextel Partners	Cellular	А	Overland Park	KS
View	4001800	OnStar, LLC	Cellular	Α	Detroit	MI
View	4109450	Pix Wireless, LLC	Cellular	D	Boca Raton	FL
View	4109850	PLATINUMTEL COMMUNICATIONS, LLC d/b/a Care Wireless	Cellular	С	Justice	IL
View	33351182	PNG Telecommunications, Inc. dba PowerNet Global Communications	Cellular	D	Cincinnati	ОН
View	4202100	Powertel/Memphis, Inc. dba T- Mobile	Cellular	A	Bellevue	WA
View	4107700	Puretalk Holdings, LLC	Cellular	Α	Covington	GA
View	4106700	Q Link Wireless, LLC	Cellular	Α	Dania	FL
View	4108700	Ready Wireless, LLC	Cellular	С	Hiawatha	IA
View	4106200	Rural Cellular Corporation	Cellular	VICTOR:	Basking Ridge	NJ
View	4108550	Sage Telecom Communications, LLC	Cellular	D	Dallas	TX
View	4109150	SelecTel, Inc. d/b/a SelecTel Wireless	Cellular	D	Freemont	NE
View	4110000	Senior Tech, LLC d/b/a Snapfon	Cellular	С	Chattanooga	TN
View	4106300	SI Wireless, LLC	Cellular	Α	Carbondale	IL
View	4109100	Solavei, LLC	Cellular	С	Bellevue	WA
View	4200100	Sprint Spectrum, L.P.	Cellular	Α	Atlanta	GA
View	4200500	SprintCom, Inc.	Cellular	А	Atlanta	GA
View	4109550	Stream Communications, LLC	Cellular	С	Dallas	TX
View	4202200	T-Mobile Central, LLC dba T- Mobile	Cellular	Α	Bellevue	WA
View	4002500	TAG Mobile, LLC	Cellular	D	Carrollton	TX
View	4109700	Telecom Management, Inc. dba Pioneer Telephone	Cellular	С	South Portland	ME
View	4107200	Telefonica USA, Inc.	Cellular	D	Miami	FL
View	4108900	Telrite Corporation dba Life Wireless	Cellular	D	Covington	GA
View	4108450	Tempo Telecom, LLC	Cellular	D	Kansas City	МО
View	4109950	The People's Operator USA, LLC	Cellular	С	New York	NY
View	4109000	Ting, Inc.	Cellular	В	Toronto	ON
View	4103900	Total Call Mobile, Inc.	Cellular	А	Gardena	CA
View	4103300	Touchtone Communications,	Cellular	D	Whippany	NJ

		Inc.				
View	4104200	TracFone Wireless, Inc.	Cellular	D	Miami	FL
View	4002000	Truphone, Inc.	Cellular	D	Durham	NC
View	4105700	Virgin Mobile USA, L.P.	Cellular	Α	Atlanta	GA
View	4104100	WDT Wireless Telecommunications, Inc.	Cellular	D	Dallas	TX
View	4200600	West Virginia PCS Alliance, L.C.	Cellular	Α	Waynesboro	VA
View	4106500	WiMacTel, Inc.	Cellular	D	Omaha	NE
View	4110100	Windward Wireless LLC	Cellular	С	Suwanee	GA
View	4109900	Wireless Telecom Cooperative, Inc. dba theWirelessFreeway	Cellular	С	Louisville	KY

# EXHIBIT E FAA



Issued Date: 01/28/2016

JOHN MONDAY (DC) AT&T MOBILITY 3300 E. RENNER ROAD, B3132 RICHARDSON, TX 75082

### \*\* DETERMINATION OF NO HAZARD TO AIR NAVIGATION \*\*

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure: Antenna Tower Index Location: West Liberty, KY Latitude: 37-53-33.99N NAD 83

Longitude: 83-17-14.13W

Heights: 1029 feet site elevation (SE)

275 feet above ground level (AGL) 1304 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

As a condition to this Determination, the structure is marked/lighted in accordance with FAA Advisory circular 70/7460-1 L, Obstruction Marking and Lighting, a med-dual system - Chapters 4,8(M-Dual),&12.

It is required that FAA Form 7460-2, Notice of Actual Construction or Alteration, be e-filed any time the project is abandoned or:

	At least 10 days prior to start of construction (7460-2, Part 1)
X	Within 5 days after the construction reaches its greatest height (7460-2, Part 2)

This determination expires on 07/28/2017 unless:

- (a) the construction is started (not necessarily completed) and FAA Form 7460-2, Notice of Actual Construction or Alteration, is received by this office.
- (b) extended, revised, or terminated by the issuing office.
- (c) the construction is subject to the licensing authority of the Federal Communications Commission (FCC) and an application for a construction permit has been filed, as required by the FCC, within 6 months of the date of this determination. In such case, the determination expires on the date prescribed by the FCC for completion of construction, or the date the FCC denies the application.

NOTE: REQUEST FOR EXTENSION OF THE EFFECTIVE PERIOD OF THIS DETERMINATION MUST BE E-FILED AT LEAST 15 DAYS PRIOR TO THE EXPIRATION DATE. AFTER RE-EVALUATION OF CURRENT OPERATIONS IN THE AREA OF THE STRUCTURE TO DETERMINE THAT NO SIGNIFICANT AERONAUTICAL CHANGES HAVE OCCURRED, YOUR DETERMINATION MAY BE ELIGIBLE FOR ONE EXTENSION OF THE EFFECTIVE PERIOD.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

Any failure or malfunction that lasts more than thirty (30) minutes and affects a top light or flashing obstruction light, regardless of its position, should be reported immediately to (877) 487-6867 so a Notice to Airmen (NOTAM) can be issued. As soon as the normal operation is restored, notify the same number.

A copy of this determination will be forwarded to the Federal Communications Commission (FCC) because the structure is subject to their licensing authority.

If we can be of further assistance, please contact our office at (816) 329-2523. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2016-ASO-473-OE.

Signature Control No: 277290686-279100514 (DNE)

Steve Phillips Specialist

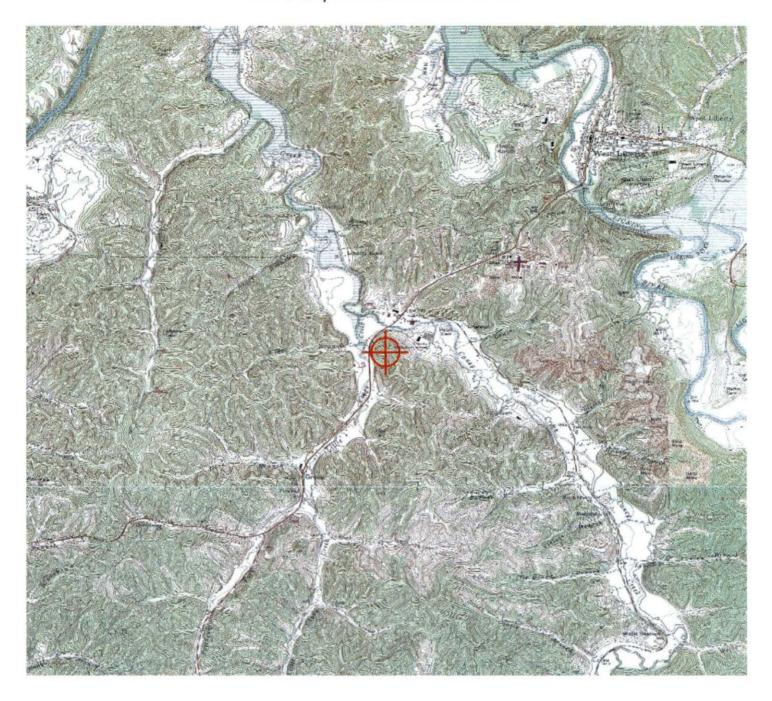
Attachment(s) Frequency Data Map(s)

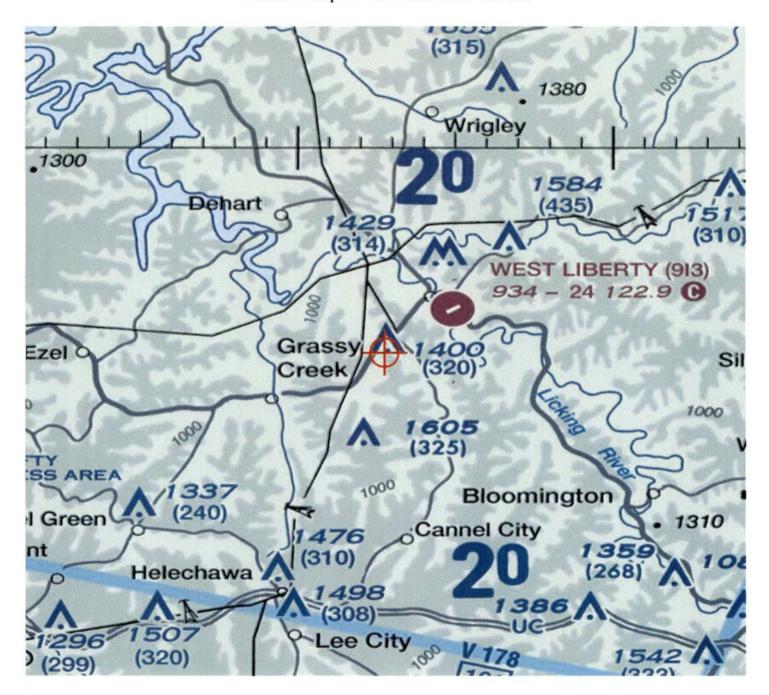
cc: FCC

# Frequency Data for ASN 2016-ASO-473-OE

LOW FREQUENCY	HIGH FREQUENCY	FREQUENCY UNIT	ERP	ERP UNIT
100 av	Access 18	in the same of the	St. 144 Fig. 6	
698	806	MHz	1000	W
806	824	MHz	500	W
824	849	MHz	500	W
851	866	MHz	500	W
869	894	MHz	500	W
896	901	MHz	500	W
901	902	MHz	7	W
930	931	MHz	3500	W
931	932	MHz	3500	W
932	932.5	MHz	17	dBW
935	940	MHz	1000	W
940	941	MHz	3500	W
1850	1910	MHz	1640	W
1930	1990	MHz	1640	W
2305	2310	MHz	2000	W
2345	2360	MHz	2000	W

# 10PO Map for ASN 2016-ASO-473-OE





# EXHIBIT F KENTUCKY AIRPORT ZONING COMMISSION



#### KENTUCKY AIRPORT ZONING COMMISSION

STEVEN BESHEAR Governor

90 Airport Road, Bldg 400 Frankfort, KY 40601 www.transportation.ky.gov/aviation 502 564-4480

October 16, 2015

APPROVAL OF APPLICATION EXTENSION

APPLICANT: A T & T MOBILITY LLC AT&T MOBILITY **601 WEST CHESTNUT STREET** LOUISVILLE, KY 40203

SUBJECT: AS-088-913-2014-038

STRUCTURE: LOCATION:

Antenna Tower West Liberty, KY

COORDINATES: 37° 53' 33.99" N / 83° 17' 14.13" W

HEIGHT:

265' AGL/1294' AMSL

The Kentucky Airport Zoning Commission has approved your application for a permit to construct 265' AGL/ 1294' AMSL Antenna Tower near West Liberty, KY 37° 53' 33.99" N / 83° 17' 14.13" W.

This permit is valid for a period of 18 Month(s) from its date of issuance. If construction is not completed within said 18-Month period, this permit shall lapse and be void, and no work shall be performed without the issuance of a new permit.

Medium Dual Obstruction Lighting is required in accordance with 602 KAR 50:100.

John Houlihan Administrator





#### KENTUCKY AIRPORT ZONING COMMISSION

STEVEN BESHEAR Governor 90 Airport Road, Bldg 400 Frankfort, KY 40601 www.transportation.ky.gov/aviation 502 564-4480

#### CONSTRUCTION/ALTERATION STATUS REPORT

October 16, 2015

AERONAUTICIAL STUDY NUMBER: AS-088-913-2014-038

A T & T MOBILITY LLC AT&T MOBILITY 601 WEST CHESTNUT STREET LOUISVILLE, KY 40203

This concerns the permit extension which was issued to you by the Kentucky Airport Zoning Commission on October 16, 2015. This permit is valid for a period of 18 Month(s) from its date of issuance. If construction is not completed within the said 18-Month period, this permit shall lapse and be void, and no work shall be performed without the issuance of a new permit. When appropriate, please indicate the status of the project in the place below and return this letter to John Houlihan, Administrator, Kentucky Airport Zoning Commission, 90 Airport Road, Bldg 400, Frankfort, KY, 40601. 502 564-4480.

STRUCTURE: Antenna Tower West Liberty, KY LOCATION: COORDINATES: 37° 53' 33.99" N / 83° 17' 14.13" W HEIGHT: 265' AGL /1294' AMSL CONSTRUCTION/ALTERATION STATUS 1. The project ( ) is abandoned. ( ) is not abandoned. 2. Construction status is as follows: Structure reached its greatest height of \_\_\_\_\_\_ ft. AGL ft. AMSL on \_\_\_\_\_ (date). Date construction was completed. Type of obstruction marking/painting. Type of obstruction lighting. As built coordinates. Miscellaneous Information. DATE

SIGNATURE/TITLE

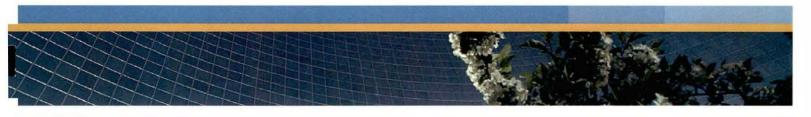


# EXHIBIT G GEOTECHNICAL REPORT



#### ENVIRONMENTAL CORPORATION OF AMERICA

ENVIRONMENTAL | GEOTECHNICAL | WETLANDS | ECOLOGY | CULTURAL RESOURCES



## Geotechnical Investigation

AT&T Site (Index)

2140 Highway 460 W West Liberty, Kentucky Morgan County

ECA Project No. P1247



#### SUBMITTED TO:

WesTower Communications 10400 Linn Station Road, Suite 225 Louisville, KY 40223

#### PREPARED BY:

Environmental Corporation of America 1375 Union Hill Industrial Court, Suite A Alpharetta, GA 30004



#### **ENVIRONMENTAL CORPORATION OF AMERICA**

ENVIRONMENTAL | GEOTECHNICAL | WETLANDS | ECOLOGY | CULTURAL RESOURCES

March 4, 2014

WesTower Communications 10400 Linn Station Road, Suite 225 Louisville, KY 40223

Attention:

Mr. John Boud

Subject:

**Report of Geotechnical Investigation** 

AT&T Site INDEX 2140 Highway 460 W

West Liberty, Morgan County, Kentucky

ECA Project No. P1247

Dear Mr. Boud:

Environmental Corporation of America (ECA) is pleased to submit this report of our investigation for the proposed project. Our services were provided as authorized via purchase order dated October 1, 2013.

This report presents a review of the information provided to us, a description of the site and subsurface conditions, and our recommendations. The appendices contain a Boring Location Plan and a Boring Log.

#### Purpose and Scope of Work

The purpose of this exploration was to obtain specific subsurface data at the site and to provide geotechnical-related design parameters and construction recommendations for the proposed tower.

Our scope of work included the following:

- Due to very steep terrain, our ATV drill rig could not access the proposed tower and level up to facilitate drilling. Therefore, two hand auger borings were drilled to a depth of 5.7 and 5.9 feet below the ground surface (bgs). Figure 1 shows the approximate boring locations.
- The depth to groundwater, if any, was measured in the borings after drilling was completed.

The soil samples were visually classified and a boring log was prepared. The soil
conditions were evaluated by a registered professional engineer and this geotechnical
report was prepared with our recommendations.

No physical testing of soil samples has been conducted to calculate site specific bearing capacities or settlements. We have recommended design parameters and settlements based on an examination of the soil samples, and our experience with similar soil conditions and structures.

#### **Project Information**

We were provided with an undated survey of the Property by BTM Engineering. The Property is located in a wooded area.

We understand that plans call for the construction of a 255-foot self-supporting lattice tower on the site. We assume that the equipment building/cabinets will be pre-fabricated structures supported on a turned-down slab foundation.

#### **Site Conditions**

The fieldwork was conducted on February 25, 2014. Information obtained from the borings was used to help us evaluate the subsurface conditions and to assist in formulating our recommendations.

#### **Subsurface Conditions**

The subsurface conditions were explored with two borings drilled approximately as shown on Figure 1. Several rock outcroppings were noted at the project site. The ground surface at the tower center slopes about 20 percent.

The boring encountered sandy silt and gravel overlying apparent bedrock at approximately 5.9 feet. The soil classified as ML soil type based on the Unified Soil Classification System (USCS). Auger refusal was encountered in boring B-1 at 5.9 feet and in boring B-1A at 5.7 feet. It is possible that the material at 5.9 feet represents a boulder; however, based on our observations, solid rock is very close to the surface. In order to drill deeper, coring would be needed. Also, significant clearing and leveling of the tower center would be needed.

Groundwater was not present in the borings at the time of completion.

#### Recommendations

<u>Tower Foundations</u>: The subsurface conditions are suitable for support of the tower using a mat foundation. Due to the shallow depth to bedrock, a caisson foundation would not be feasible.

For a <u>mat</u> foundation design, we recommend the foundation base be supported on the apparent rock surface. If bearing on weathered rock, a net allowable bearing pressure of 8 ksf may be used. Other soil parameters that may be needed are as follows:

Cohesion

1500 psf

Angle of internal friction

 $0^{\circ}$ 

Unit weight of soil

115 pcf

Total and differential settlement should be less than 1-inch and ½-inch, respectively. Due to the shallow rock, it may be necessary to excavate some depth of the rock to accommodate a belowground foundation pad, or raise the ground surface and the tower foundation to provide sufficient concrete mass and overturning resistance, and/or use rock anchors.

Groundwater should not be encountered in a mat foundation excavation.

<u>Building Foundations:</u> The proposed equipment cabinet(s) can be supported on a spread footing foundation. A maximum allowable net bearing pressure of 2.0 kips per ft<sup>2</sup> should be used to design the building/cabinet foundation. Total and differential settlements should be less than 1/2-inch and 1/4-inch, respectively.

<u>Foundation Excavations</u>: To avoid softening of the shallow soils exposed at the foundation bearing level, excavations should not be left open for extended periods, prior to placing reinforcing steel and concrete. If rain or freezing weather is expected, excavations should not be completed. Leaving the excavations at least 1 ft above final grade should protect the bearing soils from deterioration.

If the excavation must remain open overnight or if rainfall becomes imminent while the bearing soils are exposed, we recommend that a 2 to 4-inch thick "mud-mat" of "lean" (2000 psi) concrete be placed on the bearing soils before the placement of reinforcing steel. If the bearing soils are softened by surface water intrusion or exposure, the softened soils must be removed from the foundation excavation bottom immediately prior to placement of concrete.

<u>Fill Placement</u>: The amount of fill required for this project depends on the planned final grades, but we expect it to be minimal. Any required fill should be placed in maximum 8-inch thick lifts. The soil moisture content should be close to the optimum moisture content. The soil should be compacted to at least 98% of the maximum dry density, as determined by the standard Proctor method (ASTM D-698).

In areas supporting floor slabs or pavements, the upper 18 inches of fill should be compacted to 100% of the standard Proctor density. As no laboratory testing has been conducted, we do not know the capability of the surficial soil to support pavements. However, we suggest that the upper soils be replaced by granular fill in areas of heavy traffic to improve the subgrade support capabilities and moisture sensitivity.

Field density tests should be conducted at routine intervals, as the fill is being placed, to verify that adequate compaction is achieved.

Prior to placing any new fill, any soft or loose near surface soils should be removed and the area proofrolled with a heavy vehicle to confirm that any unsuitable soil conditions have been discovered.

#### **Basis for Recommendations**

The subsurface conditions encountered at the boring location are shown on the Boring Log in Appendix B. This Boring Log represents our interpretation of the subsurface conditions based on the field logs and visual examination of field samples by an engineer. The lines designating the interface between various strata on the Boring Log represent the approximate interface locations. In addition, the transition between strata may be gradual. The water level shown on the Boring Log, if any, represents the condition only at the time of our exploration.

The recommendations contained herein are based in part on project information provided to us and only apply to the specific project and site discussed in this report. If the project information section in this report contains incorrect information or if additional information is available, please let us know so that we may review the validity of our recommendations.

Regardless of the thoroughness of a geotechnical investigation, there is always a possibility that conditions between borings will be different from those at specific boring locations and that conditions will not be as anticipated by the designers or contractors. In addition, the construction process may itself alter soil conditions. Therefore, experienced geotechnical personnel should observe and document the construction procedures used and the conditions encountered. Unanticipated conditions and inadequate procedures should be reported to the design team along with timely recommendations to solve the problems created. ECA is best qualified to provide this service based on our familiarity with the project, the subsurface conditions, and the intent of the recommendations and design.

We wish to remind you that we will store the soil samples for 30 days. The samples will then be discarded unless you request otherwise.

We will be happy to discuss our recommendations with you and look forward to providing the additional studies or services necessary to complete this project. We appreciate the opportunity to be of service. Please call us with any questions at (770) 667-2040.

Sincerely,

Environmental Corporation of America

Kelby Williams, EIT Project Engineer

Appendix A Boring Location Plan

Appendix B Boring Log

J. Richard Rhudy, P.E.

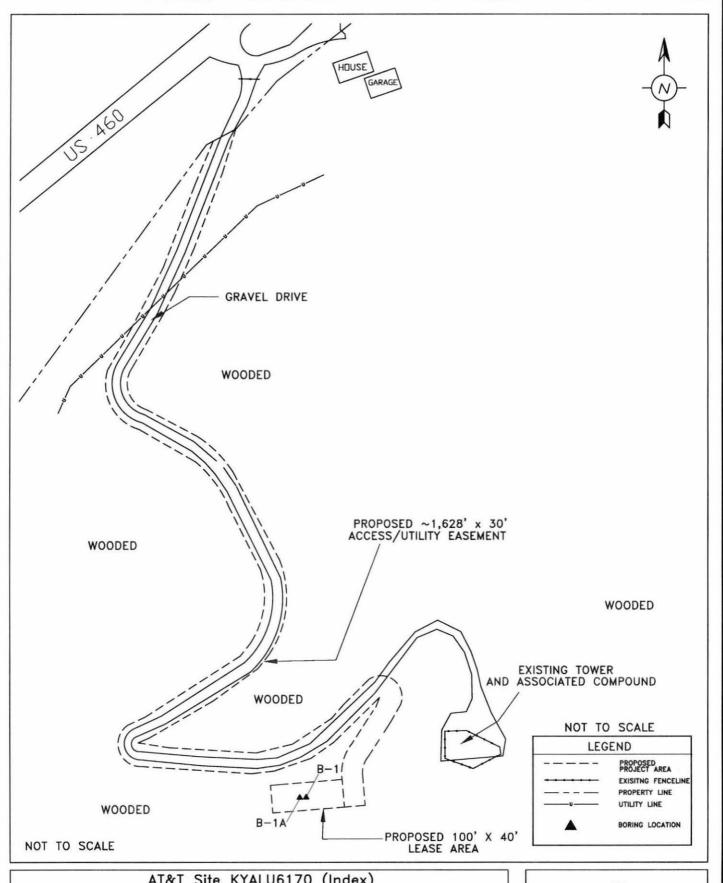
Principal Engineer KY Reg. #27450

RHUDY

27450

### APPENDIX A

**Boring Location Plan** 





2140 US Highway 460W West Liberty, Morgan County, Kentucky

Figure 1: Boring Location Plan

SOURCE: 9/18/13 ECA Site Visit and Site

Survey

DRAWN BY: JLD/KLW DATE: 3/3/2014

FILE NAME: F:\%\P1247.dwg



ECA Project # P1247

### APPENDIX B

Boring Log

Project: AT&T Site (Index)

City, State West Liberty, Kentucky

Client: Westower

ECA Job No: P1247

Log of Boring: B-1/B-1A

Drill Date: February 25, 2014

Field Rep: Tyler

			SUBSURFACE PROFILE	SAMPLE								
Elevation (ft)	Depth	Symbol	Description	Blow Counts (per ft)	SPT Values (blows/ft) 10 20 30 40	Remarks	Water depth					
0	0-		Ground Surface									
-5.9	5-	] : :	Very dense tan sandy SILT (ML) with gravel									
			Boring Terminated			Auger refusal at 5.9 feet  Boring B-1A Offset 5 feet west  Auger refusal at 5.7 feet						
	10-											
	15-											
	20-											
	25			_								
	30-											
	35-											
	40-											

Drilled By: Tri-State Drilling

Depth to Water: N/A

Borehole Size: 3" OD

Total Depth: 5.9 ft

Drill Method: Hand Auger

Sheet: 1 of 1

Environmental Corp. of America 1375 Union Hill Indus. Ct., Ste A Alpharetta, GA 30004 (770) 667-2040



## EXHIBIT H DIRECTIONS TO WCF SITE

#### Driving Directions to Proposed Tower Site:

- 1. Beginning at the offices of the Morgan County Clerk, located at 450 Prestonsburg Street in West Liberty, KY, head northwest on KY-7.
- 2. Turn left onto US-460 W / Main Street and travel for 2.5 miles.
- 3. The site is on the left at 1999 Highway 460 West.
- 4. site coordinates are
  - a. 37 deg 53 min 33.996 sec N
  - b. 83 deg 17 min 14.131 sec W



Prepared by: Aaron L. Roof Pike Legal Group PLLC 1578 Highway 44 East, Suite 6 PO Box 369 Shepherdsville, KY 40165-0369

Telephone: 502-955-4400 or 800-516-4293

## EXHIBIT I COPY OF REAL ESTATE AGREEMENT

LODGED FOR RECORD MORGAN COUNTY CLERK

JAN 24 2014

RANDY WILLIAMS, CLERK

#### MEMORANDUM OF LEASE

Prepared by:

Kit Nickel

PBM Wireless

13714 Smokey Ridge Overlook

Carmel, IN 46033

#### Return to:

New Cingular Wireless PCS, LLC Attn: Network Real Estate Administration 575 Morosgo Drive NE, Suite 13-F West Tower, Atlanta, GA 30324

Re:

Cell Site # KYALU6170; Cell Site Name: INDEX

Fixed Asset # 12568763 State: KENTUCKY County: MORGAN

#### MEMORANDUM OF LEASE

This Memorandum of Lease is entered into on this 15th day of October , 2013, by and between SARAH GEORGE FANNIN, UNMARRIED, ROBIN FANNIN, UNMARRIED, ERMA FANNIN, UNMARRIED, FARRELL FANNIN, UNMARRIED AND KELLY KRISTEN FANNIN KOENIG AND CHRIS KOENIG, HUSBAND AND WIFE, having a mailing address of 2140 HWY 460 W, WEST LIBERTY, KY 41472 (hereinafter referred to as "Landlord") and New Cingular Wireless PCS, LLC, a Delaware limited liability company, having a mailing address of 575 Morosgo Drive NE, Suite 13-F West Tower, Atlanta, Ga 30324 (hereinafter referred to as "Tenant").

Landlord and Tenant entered into a certain Option and Lease Agreement ("Agreement") on the 15 Hz day of 6 to 6 cr , 20 13, for the purpose of installing, operating and maintaining a communications facility and other improvements. All of the foregoing is set forth in the Agreement.

- The initial lease term will be five (5) years commencing on the effective date of written notification
  by Tenant to Landlord of Tenant's exercise of its option, with four (4) successive five (5) year
  options to renew.
- The portion of the land being leased to Tenant and associated easements are described in Exhibit 1 annexed hereto.
- 4. This Memorandum of Lease is not intended to amend or modify, and shall not be deemed or construed as amending or modifying, any of the terms, conditions or provisions of the Agreement, all of which are hereby ratified and affirmed. In the event of a conflict between the provisions of this Memorandum of Lease and the provisions of the Agreement, the provisions of the Agreement shall control. The Agreement shall be binding upon and inure to the benefit of the parties and their respective heirs, successors, and assigns, subject to the provisions of the Agreement.

IN WITNESS WHEREOF, the parties have executed this Memorandum of Lease as of the day and year first above written.

#### "LANDLORD"

SARAH GEORGE FANNIN, UNMARRIED, ROBIN FANNIN, UNMARRIED, ERMA FANNIN, UNMARRIED, FARRELL FANNIN, UNMARRIED AND KELLY KRISTEN FANNIN KOENIG AND CHRIS KOENIG, HUSBAND AND WIFE

By: Sarah George Fannin
Its: Owner
Date: 9-28-13

By: Qul J.

Print Name: Robin Fannin
Its: Owner
Date: 9-28-13

By: Amul James
Print Name: Farrell Fannin
Its: Owner
Date: 1-28-13

By: Lama James
Print Name: Erma Fannin
Its: Owner
Date: 9-28-13

[SIGNATURES CONTINUE ON NEXT PAGE]

[ACKNOWLEDGMENTS APPEAR ON THE NEXT PAGE]

#### LANDLORD ACKNOWLEDGMENT

EARDEORD ACI	NOW ELEDONIEM
COUNTY OF Morgan	
On the 28 day of September, 2013 acknowledged under oath, that he/she is the person/of executed the same in his/her stated capacity as the volumer therein contained.	
	Notary Public: Harry Frish My Commission Expires: 24-19-2016
LANDLORD ACK	NOWLEDGMENT
COUNTY OF Morgan ) ss:	
On the 28 day of September, 2013 acknowledged under oath, that he/she is the person/of executed the same in his/her stated capacity as the votherein contained.	
	Notary Public: Ham Fund My Commission Expires: 4-19-2019
LANDLORD ACK	NOWLEDGMENT
COUNTY OF <u>Greene</u> ) ss:	
mach 11	

On the SOrday of Septembre 2013 before me, personally appeared Kelly Kristen Fannin Koenig and Chris Koenig, who acknowledged under oath, that they are the persons named in the within instrument, and that they executed the same in their stated capacity as the voluntary act and deed of the Landlord for the purposes therein contained.

DIANA ROYSTON

Notary Public - Notary Seal

State of Missouri

Commissioned for Greene County

My Commission Expires: Desarts 11, 2016

Commission Number: 12408220

Notary Public: Aleana Moziston
My Commission Expires: 12-11-2016

TENANT ACKNOWLEDGMENT
On the 1512 day of October, 2013, before me personally appeared Terry R. Kilgore, and acknowledged under oath that he/she is the Area Manager, C&E of AT&T Mobility Corporation, the Manager of New Cingular Wireless PCS, LLC, the Tenant named in the attached instrument, and as such was authorized to execute this instrument on behalf of the Tenant.
Notary Public: THOME PORCOTORU  My Commission Expires: 7 5 5  LANDLORD ACKNOWLEDGMENT  O S OTARL
On the 28 day of September, 2013 before me, personally appeared Sarah George Fannin, who acknowledged under oath, that he/she is the person/officer named in the within instrument, and that he/she
executed the same in his/her stated capacity as the voluntary act and deed of the Landlord for the purposes therein contained.  Notary Public: Ham Fund
My Commission Expires: 4 - 19 - 2814  LANDLORD ACKNOWLEDGMENT
STATE OF Kentucky ) ss:
On the 28 day of September, 2013 before me, personally appeared Robin Fannin, who acknowledged under oath, that he/she is the person/officer named in the within instrument, and that he/she executed the same in his/her stated capacity as the voluntary act and deed of the Landlord for the purposes therein contained.
Notary Public: Harry Frish My Commission Expires: 4-19-2014

#### **EXHIBIT 1**

#### DESCRIPTION OF PREMISES

Page 1 of 2

to the Option and Lease Agreement dated 600 15, 2013, by and between SARAH GEORGE FANNIN, UNMARRIED, ROBIN FANNIN, UNMARRIED, ERMA FANNIN, UNMARRIED, FARRELL FANNIN, UNMARRIED AND KELLY KRISTEN FANNIN KOENIG AND CHRIS KOENIG, HUSBAND AND WIFE, as Landlord, and New Cingular Wireless PCS, LLC, a Delaware limited liability company, as Tenant.

The Property is legally described as follows:

Beginning at the mouth of Little Caney creek; thence up Little Caney creek with its meanders to the line and land of Bill Elam (formerly Kola Noble); thence with the line of Bill Elam to the line of Kola Noble; thence with Kola Noble's line to the line of J.T. Thomas' thence with J.T. Thomas' line to the Lewis Henry line; thence with Lewis Henry's line around to Big Caney Creek just above the ford opposite Isaac Henry's (now Henry's Heirs) line; thence down Big Caney creek with its meanders to the place of beginning, containing 40 acres, more or less, and to contain and include all of the land in the above described boundary with the exception of two lots that have been previously deeded to S.S. Oldfield and wife, of Index, Kentucky.

There is excepted from the foregoing described tract of land a tract of land heretofore conveyed by Stella D. Fannin and others to L. Clifford Long and Aleene F. Long, by deed dated October 19, 1956, and recorded in Deed Book 93, Page 204, Morgan County Court Clerk's records, and reference is hereby made to said deed of conveyance for a more particular description of the portion of land excepted from the above described tract.

#### EXHIBIT 1

#### DESCRIPTION OF PREMISES

Page 2 of 2

to the Option and Lease Agreement dated 6 Chober 15, 2013, by and between SARAH GEORGE FANNIN, UNMARRIED, ROBIN FANNIN, UNMARRIED, ERMA FANNIN, UNMARRIED, FARRELL FANNIN, UNMARRIED AND KELLY KRISTEN FANNIN KOENIG AND CHRIS KOENIG, HUSBAND AND WIFE, as Landlord, and New Cingular Wireless PCS, LLC, a Delaware limited liability company, as Tenant.

The Premises are described and/or depicted as follows:



#### Notes:

- THIS EXHIBIT MAY BE REPLACED BY A LAND SURVEY AND/OR CONSTRUCTION DRAWINGS OF THE PREMISES ONCE RECEIVED BY TENANT.
- ANY SETBACK OF THE PREMISES FROM THE PROPERTY'S BOUNDARIES SHALL BE THE DISTANCE REQUIRED BY THE APPLICABLE GOVERNMENTAL AUTHORITIES.
- 3. WIDTH OF ACCESS ROAD SHALL BE THE WIDTH REQUIRED BY THE COUNTY OF MORGAN

  COUNTY OF MORGAN

4. THE TYPE, NUMBER AND MOUNTING POSITIONS AND LOCATIONS OF ANTENNAS AND TRANSMISSION LINESSENGE ILLUSTRATIVE ONLY. ACTUAL TYPES, NUMBERS AND MOUNTING POSITIONS WITH YOUR FROM WHAT IS SHOWN ABOVE.

aforesaid do hereby certify that the foregoing of lodged for record, whereupon the foægoing and the certificate have been duly reorded in my office.

Given under my hand this the day of 20 4 Randy Villiams Clerk

KY Land Lease

MiscBR 57 page 155

## EXHIBIT J NOTIFICATION LISTING

#### Index - Notice List

Sarah G, Robin and Farrell Fannin 2140 Hwy 460 W West Liberty, KY 41472

Appalachian Wireless East Kentucky Network 101 Technology Trail Ivel, KY 41642

Samuel Long P.O. Box 456 West Liberty, KY 41472

Alex Goodpaster & Hillary Murray c/o Allan Goopaster P.O. Box 503 West Liberty, KY 41472

Alex Goodpaster & Hillary Murray c/o Allan Goodpaster P.O. Box 503 West Liberty, KY 41472

Sarah & Robin Fannin 2140 Hwy 460 W West Liberty, KY 41472

William G Holbrook DVM P.O. Box 66 West Liberty, KY 41472

George Elam, Walter Elam and Sharlene Copas c/o George Elam 3832 Hwy 711 West Liberty, KY 41472

David Stacy 2144 Hwy 460 W West Liberty, KY 41472

Betty Lou Elam and Linda Blackburn 309 Larkwood Dr Lexington, KY 40509 Woodford B. Gevedon & Mary Beth Popplewell 173 Index Rd West Liberty, KY 41472

Fairanna Nickell 173 Index Rd West Liberty, KY 41472

Caney Farms c/o Buford Sherman 12094 Hwy 437 West Liberty, KY 41472

David Earl & Susan May 1042 Liberty Rd West Liberty, KY 41472

K&M Rentals P.O. Box 273 West Liberty, KY 41472

K&M Rentals Tim Keller & John Motley P.O. Box 273 West Liberty, KY 41472

Ky. Mt. Holiness Box 2 Vancleave, KY 41385

Ky. Mt. Holiness c/o Index Community Church 1749 W. Main Street West Liberty, KY 41472

Anthony Frederick 2919 Hwy 1000 West Liberty, KY 41472

Alex Goodpaster & Hillary Murray 437 Henry Clay Blvd Lexington, KY 40502

Kentucky State Right of Way Jackson, KY

## EXHIBIT K COPY OF PROPERTY OWNER NOTIFICATION



1578 Highway 44 East, Suite 6 P.O. Box 369 Shepherdsville, KY 40165-0369 Phone (502) 955-4400 or (800) 516-4293 Fax (502) 543-4410 or (800) 541-4410

# Notice of Proposed Construction of Wireless Communications Facility Site Name: Index

Dear Landowner:

New Cingular Wireless PCS, LLC, a Delaware limited liability company, d/b/a AT&T Mobility has filed an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located at 1999 Highway 460 West, West Liberty, Kentucky 41472 (37°53'33.996" North latitude, 83°17'14.131" West longitude). The proposed facility will include a 255-foot tall antenna tower, plus a 10-foot lightning arrestor and related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

This notice is being sent to you because the Morgan County Property Valuation Administrator's records indicate that you may own property that is within a 500' radius of the proposed tower site or contiguous to the property on which the tower is to be constructed. You have a right to submit testimony to the Kentucky Public Service Commission ("PSC"), either in writing or to request intervention in the PSC's proceedings on the application. You may contact the PSC for additional information concerning this matter at: Kentucky Public Service Commission, Executive Director, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2016-00076 in any correspondence sent in connection with this matter.

We have attached a map showing the site location for the proposed tower. AT&T Mobility's radio frequency engineers assisted in selecting the proposed site for the facility, and they have determined it is the proper location and elevation needed to provide quality service to wireless customers in the area. Please feel free to contact us toll free at (800) 516-4293 if you have any comments or questions about this proposal.

Sincerely, David A. Pike Attorney for AT&T Mobility

enclosure

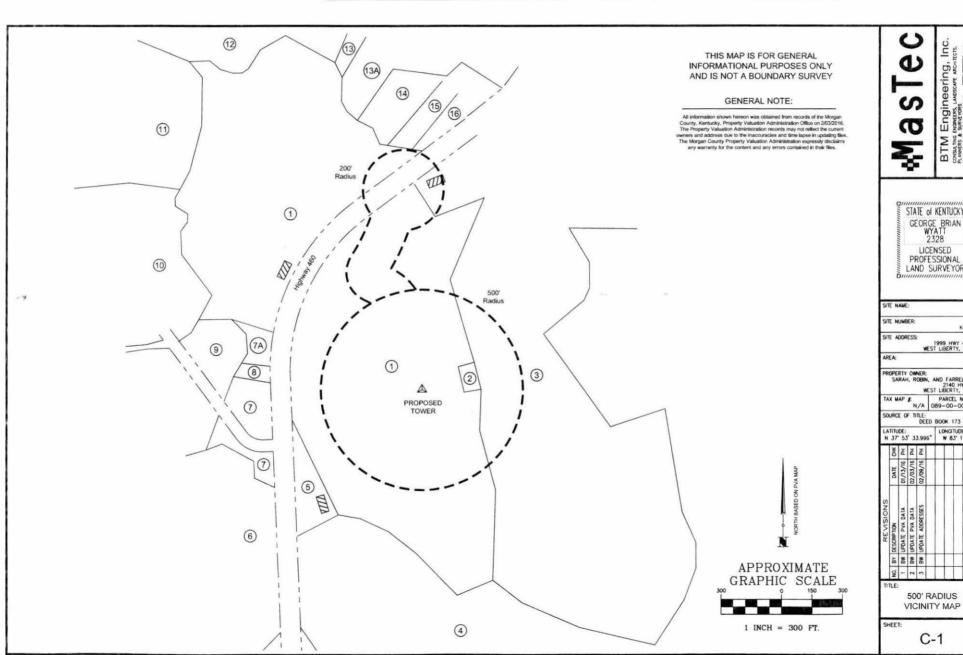
#### Driving Directions to Proposed Tower Site:

- 1. Beginning at the offices of the Morgan County Clerk, located at 450 Prestonsburg Street in West Liberty, KY, head northwest on KY-7.
- 2. Turn left onto US-460 W / Main Street and travel for 2.5 miles.
- 3. The site is on the left at 1999 Highway 460 West.
- 4. site coordinates are
  - a. 37 deg 53 min 33.996 sec N
  - b. 83 deg 17 min 14.131 sec W



Prepared by: Aaron L. Roof Pike Legal Group PLLC 1578 Highway 44 East, Suite 6 PO Box 369 Shepherdsville, KY 40165-0369

Telephone: 502-955-4400 or 800-516-4293



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STATE OF KENTUCKY GEORGE BRIAN WYATT 2328 LICENSED PROFESSIONAL LAND SURVEYOR

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- 1 PARCEL NUMBER: 089-00-00-017.00 Sarah G., Robin & Farrell Fannin 2140 Highway 460 W West Liberty, Kentucky 41472
- 2 PARCEL NUMBER: 089-00-00-017.01 Appalachian Wireless East KY Network 101 Technology Trall Ivel, Kentucky 41642
- PARCEL NUMBER: 089-00-00-025.00
   Samuel Long
   P.O. Box 456
   West Liberty, Kentucky 41472
- PARCEL NUMBER: 089-00-00-021.00
   Alex Goodpaster & Hillary Murray
   Clo Alian Goodpaster
   P.O. Box 503
   West Liberty, Kentucky 41472
- 5 PARCEL NUMBER: 089-00-00-016.01 Sarah & Robin Fannin 2140 Highway 460 W West Liberty, Kentucky 41472
- 6 PARCEL NUMBER: 089-00-00-019.00 William G. Holbrook DVM P.O. Box 66 West Liberty, Kentucky 41472
- PARCEL NUMBER: 089-00-00-016.00 Sharlene Copas & Walter & George Elam c/o George Elam 3832 Highway 711 West Liberty, Kentucky 41472
- (7A) PARCEL NUMBER: 089-00-00-014.00 Sharlene Copas & Walter & George Elam c/o George Elam 3832 Highway 711 West Liberty, Kentucky 41472
- 8 PARCEL NUMBER: 089-00-00-015.00 David Stacy 2144 Highway 460 W West Liberty, Kentucky 41472

- PARCEL NUMBER: 089-00-00-009.00
   Betty Lou Elam & Linda Blackburn
   309 Larkwood Drive
   Lexington, Kentucky 40509
- (1) PARCEL NUMBER: 089-00-00-006.00 Woodford B. Gevedon & Mary Beth Popplewell 173 Index Road West Liberty, Kentucky 41472

and

Fairanna Nickell 173 Index Road West Liberty, Kentucky 41472

- 11 PARCEL NUMBER: 089-00-00-007.00 Caney Farms c/o Buford Sherman 12094 Highway 437 West Liberty, Kentucky 41472
- (12) PARCEL NUMBER: 089-00-00-024.00 David Earl & Susan May 1042 Liberty Road West Liberty, Kentucky 41472
- (13) PARCEL NUMBER: 089-00-00-024.01 K & M Rentals P.O. Box 273 West Liberty, Kentucky 41472
- (13A) PARCEL NUMBER: 089-03-00-002.00 K & M Rentals (Tim Keller & John Motley) P.O. Box 273 West Liberty, Kentucky 41472
- PARCEL NUMBER: 089-03-00-011.00 Mt. Holiness Kentucky Box 2 VanCleave, Kentucky 41385

and

Ky, Mt. Holiness c/o Index Community Church 1749 W. Main St. West Liberty, KY 41472

- 15 PARCEL NUMBER: 089-03-00-012.00 Anthony Frederick 2919 Highway 1000 West Liberty, Kentucky 41472
- (16) PARCEL NUMBER: 089-03-00-013.00 No online PVA data found for this parcel

THIS MAP IS FOR GENERAL INFORMATIONAL PURPOSES ONLY AND IS NOT A BOUNDARY SURVEY

#### GENERAL NOTE:

All information shown hereon was obtained from records of the Morpan Country, Kentucky, Property Valuation Acministration Office on 2012/016. The Property Valuation Administration records may not reflect the current owners and address due to the inaccuracies and time lapse in updating files. The Morpan Country Property Valuation Administration expressly disclaims, any warranty for the content and any errors contraded in their files.

# ΜasTec

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500' RADIUS VICINITY MAP

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## EXHIBIT L COPY OF COUNTY JUDGE/EXECUTIVE NOTICE



1578 Highway 44 East, Suite 6 P.O. Box 369 Shepherdsville, KY 40165-0369 Phone (502) 955-4400 or (800) 516-4293 Fax (502) 543-4410 or (800) 541-4410

#### VIA CERTIFIED MAIL

Hon. Stanley Franklin Morgan County Judge Executive 450 Prestonsburg Street West Liberty, KY 41472

RE:

Notice of Proposal to Construct Wireless Communications Facility

Kentucky Public Service Commission Docket No. 2016-00076

Site Name: Index

Dear Judge Franklin:

New Cingular Wireless PCS, LLC, a Delaware limited liability company, d/b/a AT&T Mobility has filed an application with the Kentucky Public Service Commission ("PSC") to construct a new wireless communications facility on a site located at 1999 Highway 460 West, West Liberty, Kentucky 41472 (37°53'33.996" North latitude, 83°17'14.131" West longitude). The proposed facility will include a 255-foot tall antenna tower, plus a 10-foot lightning arrestor and related ground facilities. This facility is needed to provide improved coverage for wireless communications in the area.

You have a right to submit comments to the PSC or to request intervention in the PSC's proceedings on the application. You may contact the PSC at: Executive Director, Public Service Commission, 211 Sower Boulevard, P.O. Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2016-00076 in any correspondence sent in connection with this matter.

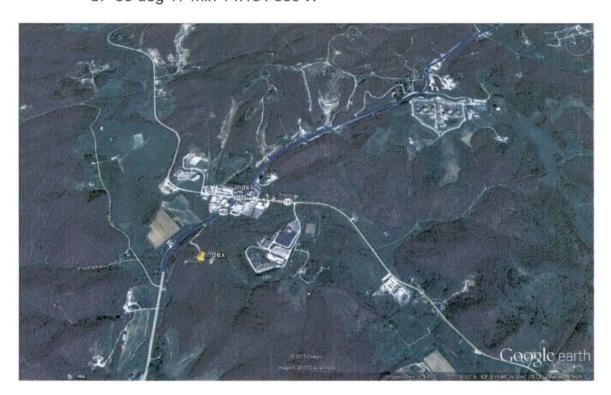
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Sincerely,

David A. Pike Attorney for AT&T Mobility enclosure

#### Driving Directions to Proposed Tower Site:

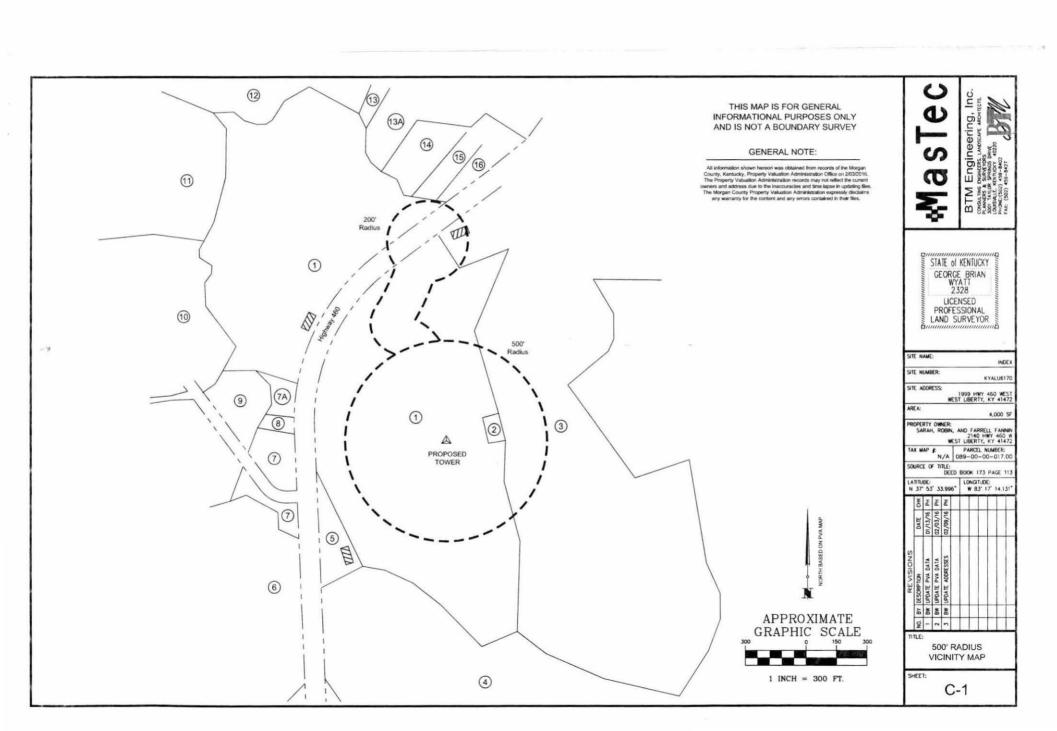
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  - a. 37 deg 53 min 33.996 sec N
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Prepared by:
Aaron L. Roof
Pike Legal Group PLLC
1578 Highway 44 East, Suite 6
PO Box 369
Shopherdsville, KX 40165,0360

Shepherdsville, KY 40165-0369

Telephone: 502-955-4400 or 800-516-4293



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TITLE: 500' RADIUS VICINITY MAP

SHEET:

C-1A

## EXHIBIT M COPY OF POSTED NOTICES

## SITE NAME: INDEX NOTICE SIGNS

The signs are at least (2) feet by four (4) feet in size, of durable material, with the text printed in black letters at least one (1) inch in height against a white background, except for the word "**tower**," which is at least four (4) inches in height.

New Cingular Wireless PCS, LLC d/b/a AT&T Mobility proposes to construct a telecommunications **tower** on this site. If you have questions, please contact Pike Legal Group, PLLC, P.O. Box 369, Shepherdsville, KY 40165. (800) 516-4293, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2016-00076 in your correspondence.

New Cingular Wireless PCS, LLC d/b/a AT&T Mobility proposes to construct a telecommunications **tower** near this site. If you have questions, please contact Pike Legal Group, PLLC, P.O. Box 369, Shepherdsville, KY 40165 (800) 516-4293, or the Executive Director, Public Service Commission, 211 Sower Boulevard, PO Box 615, Frankfort, Kentucky 40602. Please refer to docket number 2016-00076 in your correspondence.



1578 Highway 44 East, Suite 6 P.O. Box 369 Shepherdsville, KY 40165-0369 Phone (502) 955-4400 or (800) 516-4293 Fax (502) 543-4410 or (800) 541-4410

VIA TELEFAX: 606-743-3565

The Licking Valley Courier Attn: Greg Kinner 142 Prestonsburg Street P.O. Box 187 West Liberty, KY 41472

RE: Legal Notice Advertisement

Site Name: Index

Dear Jamie:

Please publish the following legal notice advertisement in the next edition of *The Licking Valley Courier*.

#### NOTICE

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After this advertisement has been published, please forward a tearsheet copy, affidavit of publication, and invoice to Pike Legal Group, PLLC, P.O. Box 369, Shepherdsville, KY 40165. Please call me at (800) 516-4293 if you have any questions. Thank you for your assistance.

Sincerely,

Aaron L. Roof Pike Legal Group, PLLC

## EXHIBIT N COPY OF RADIO FREQUENCY DESIGN SEARCH AREA

