

RECEIVED

AUG 042008

PUBLIC SERVICE COMMISSION

July 30, 2008

Kentucky Public Service Commission P.O. Box 615 211 Sower Blvd. Frankfort, KY 40602-0615

RE: KY-00-0817 WOLF CREEK

2008-261

Dear Public Service Commission;

Please accept the attached application for a Certificate of Public Convenience and Necessity for a cellular communications tower at 520 Sutton Road, Olive Hill, KY 41164.

Please find enclosed, one(1) original and five (5) copies of the entire application. Should you have any questions, please feel free to contact me at (231) 929-4555, ext. 28 or via email at <u>syagle@cellere.us</u>.

Sincerely,

Title and Leasing Specialist Enclosures

 TEL
 231.929.4555

 FAX
 231.929.0099

 WWW.cellere.us

 info@cellere us

 4110
 Copper Ridge Drive, Suite 204, Traverse City, MI 49684

COMMONWEALTH OF KENTUCKY BEFORE THE PUBLIC SERVICE COMMISSION

In the Matter of:

Application of Central States Tower Holdings, LLC for Issuance of a Certificate of Public Convenience and Necessity to Construct a Cell Site (KY-00-0817A WOLF CREEK) in Olive Hill Kentucky

Case No. 2008-00261 RECEIVED

APPLICATION FOR A CERTIFICATE OF PUBLIC CONVENIENCE AND NECESSITY

ALIG 0 4 2008 PUBLIC SERVICE COMMISSION

Cellere, LLC ("Cellere") as agent for Central States Tower Holdings, LLC ("Central States"), pursuant to KRS 278.020 and 278.040, hereby submits this application for a certificate of public convenience and necessity to construct a cell site to be known as the KY-00-0817A WOLF CREEK ("WOLF CREEK") cell site in Olive Hill, Kentucky, namely the county of Carter, Kentucky.

1. As required by 807 KAR 5:001 Sections 8(1) and (3), and 807 KAR 5:063, Cellere states that it is a Michigan limited liability company who is acting as agent for Central States Tower Holdings, LLC, who is a Delaware limited liability company and whose full name and address are: Cellere, LLC, 4110 Copper Ridge Drive, Suite 204, Traverse City, Michigan 49684. Central States Tower Holdings, LLC, whose address is: 323 S. Hale Street, Suite #100, Wheaton, IL 60187.

2. Pursuant to 807 KAR §1(1)(b), a copy of the applicant's applications to and approval from the Federal Aviation Administration and Kentucky Airport Zoning Commission are submitted as Exhibit "A".

3. Pursuant to 807 KAR 5:063 § 1(1)(d), applicant is submitting as Exhibit "B", a geotechnical investigation report, signed and sealed by a professional engineer registered in Kentucky, that includes boring logs and foundation design recommendations; and as Exhibit "E", a map that outlines the finding as to the susceptibility of the area surrounding the proposed site to flood hazard.

4. Pursuant to 807 KAR 5:063 § 1(1)(e), clear directions from the county seat to the proposed site, including highway numbers and street names, if applicable, with the telephone number of the person who prepared the directions are submitted as Exhibit "C".

5. Pursuant to 807 KAR § 1(1)(f), a copy of the lease for the property on which the cell tower is proposed to be located is submitted as Exhibit "D".

6. Pursuant to 807 KAR § 1(1)(g), experienced personnel will manage and operate the WOLF CREEK cell site. The Vice President of Construction for Cellere, LLC., Chuck Norris, is ultimately responsible for all construction of the cell tower. Mr. Norris has over 15 years of experience. Arthur J. Krueger, Licensed Professional Engineer of Wilcox Professional Services, is responsible for the design specifications of the proposed tower (identified in Exhibit "B"). S.M. Naeem Akhter, Licensed Professional Engineer of Glenmartin, is responsible for the foundation design of the proposed tower (identified in Exhibit "B"). Central States Tower Holdings, LLC, is responsible for the operations of the tower, once constructed. Central States operates cellular communications towers in 19 states with the principals having 35+ years of experience.

7. Pursuant to 807 KAR 5:063 § 1(1)(h), a site development plan or survey, signed and sealed by a professional engineer registered in Kentucky, that shows the proposed location of the tower and all easements and existing structures within 500 feet of the proposed site on the property on which the tower will be located, and all easements and existing structures within 200 feet of the access drive, including the intersection with the public street system, is submitted as Exhibit "E"

8. Pursuant to 807 KAR 5:063 § 1(1)(i), a vertical profile sketch of the tower, signed and sealed by a professional engineer registered in Kentucky, indicating the height of the tower and the placement of all antennae is submitted as Exhibit "B".

9. Pursuant to 807 KAR 5:063 § 1(1)(j), the tower and foundation design plans and a description of the standard according to which the tower was designed, signed and sealed by a professional engineer registered in Kentucky, is submitted as Exhibit "B".

10. Pursuant to 807 KAR 5:063 § 1(1((k), a map, drawn to a scale no less than one (1) inch equals 200 feet, that identifies every structure and every owner of real estate within 500 feet of the proposed tower, is submitted as Exhibit "E".

11. Pursuant to 807 KAR 5:063 § 1(1)(I), applicant hereby affirms that every person who owns property within 500 feet of the proposed tower has been: (i) notified by certified mail, return receipt requested, of the proposed construction, (ii) given the commission docket number under which the applications will be processed; and (iii) informed of his or her right to request intervention.

12. Pursuant to KRS 278.665 (2), applicant hereby affirms that every person who, according to the records of the property valuation administrator, owns property contiguous to the property where the proposed cellular tower will be located has been; (i) notified by certified mail, return receipt requested, of the proposed construction; (ii) given the commission docket number under which the application will be processed; and (iii) informed of his or her right to request intervention.

13. Pursuant to 807 KAR 5:063 § 1(1)(m), a list of the property owners who received the notice together with copies of the certified letters sent to listed property owners, is submitted as Exhibit "F".

14. Pursuant to 807 KAR 5:063 § 1(1)(n), applicant hereby affirms that the Office of Carter County Judge Executive has been: (i) notified by certified mail, return receipt requested, of the proposed construction;(ii) given the commission docket number under which the application will be processed; and (iii) informed of its right to request intervention.

15. Pursuant to 807 KAR 5:063 § 1(1)(o), a copy of the notice send to the Carter County Judge Executive is submitted as Exhibit "G".

16. Pursuant to 807 KAR 5:063 § 1(1)(p), applicant hereby affirms that (i) two written notices meeting subsection two (2) of this section have been posted, one in a visible location on the proposed site and one on the nearest public road; and (ii) the notices shall remain posted for at least two weeks after the application has been filed.

17. Pursuant to 807 KAR 5:063 § 1(2)(a), applicant affirms that:

- (a) A written notice, of durable material at least two (2) feet by four (4) feet in size, stating that "Central States Tower Holdings, LLC proposes to construct a telecommunications tower on this site", including the addresses and telephone numbers of the applicant and the Kentucky Public Service Commission, has been posted and shall remain in a visible location on the proposed site until final disposition of the application; and
- (b) A written notice, of durable material at least two (2) feet by four (4) feet in size, stating that "Central States Tower Holdings, LLC, proposes to construct a telecommunications tower near this site", including the addresses and telephone numbers of the applicant and the Kentucky Public Service Commission, has been posted on the public road nearest the site.

A Copy of each sign is attached as Exhibit "H".

18. Pursuant to 807 KAR 5:063 § 1(1)(q), a statement that notice of the location of the proposed construction has been published in a newspaper of general circulation in the county in which the construction is proposed, a copy of which is submitted as Exhibit "1".

19. Pursuant to 807 KAR 5:063 § 1(1)(r), the cell site, which has been selected, is in a relatively undeveloped area in Olive Hill, in Carter County, Kentucky.

20. Pursuant to 807 KAR 5:063 § 1(1)(s), Central States, LLC, has considered the likely effects of the installation on nearby land uses and values and has concluded that there is no more suitable location reasonably available from which adequate service to the area can be provided, and that there is no reasonably available opportunity to co-locate. Central States, LLC, has attempted to co-locate on towers

designed to host multiple wireless service provider's facilities or existing structures, such as a telecommunications tower, or another suitable structure capable of supporting the utility's facilities.

21. Pursuant to 807 KAR 5:063 § 1(1)(t), a map of the area in which the tower is proposed to be located, that is drawn to scale and that clearly depicts the search area in which a site should, pursuant to radio frequency requirements, be located is submitted as Exhibit "J".

22. Pursuant to KRS 100.987 (2((a), a grid map, that is drawn to scale, that shows the location of all existing cellular antenna towers and that indicates the general position of proposed construction sites for new cellular antenna towers is submitted as Exhibit "K".

23. No reasonably available telecommunications tower, or other suitable structure capable of supporting the cellular facilities of Central States, LLC and which would provide adequate service to the area exists.

24. Correspondence and communication with regard to this application should be addressed to:

Benjamin Meredith Cellere, LLC 4110 Copper Ridge Drive, Suite 204 Traverse City, MI 49684 (231) 929-4555 (fax) 929-0099 <u>bmeredith@cellere.us</u>

WHEREFORE, Cellere, LLC , as agent for Central States Tower Holdings, LLC, requests the Commission to enter and order:

1. Granting a certificate of public convenience and necessity to construct the WOLF CREEK cell site;

and

2. Granting all other relief as appropriate.

Respectfully submitted,

Benjamin Meredith Cellere, LLC 4110 Copper Ridge Drive, Suite 204 Traverse City, MI 49684 (231) 929-4555 (fax) 929-0099 <u>bmeredith@cellere.us</u>

Index to Exhibits

- EXH. A FAA Application and Determination; Kentucky Airport Zoning Commission Application and Approval
- EXH. B Geotechnical Report; Survey; Tower Design; Tower Foundation Design
- EXH. C Directions to Site from County Seat
- EXH. D Memorandum of Lease
- EXH. E Site Plan- 500' Radius Map with Flood Plain Information
- EXH. F Affidavit of Notification of Adjacent Property Owners and Owners within 500 feet.
- EXH. G Certified Letter to Judge Executive

EXH. H Public Notice Signs (photos)

- EXH. I Affidavit of Publication of Public Notice
- EXH. J Map of Search Area
- EXH. K Map of Existing and Proposed Towers

EXHIBIT A

FAA Application and Determination And Kentucky Airport Zoning Commission Application and Approval



Federal Aviation Administration Air Traffic Airspace Branch, ASW-520 2601 Meacham Blvd. Fort Worth, TX 76137-0520 Aeronautical Study No. 2008-ASO-1516-OE

Issued Date: 04/23/2008

Brian Meier Central States Tower Holdings, LLC 323 South Hale Street Suite 100 Wheaton, IL 60187

**** DETERMINATION OF NO HAZARD TO AIR NAVIGATION ****

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure:	Tower KY-00-0817 WOLF CREEK
Location:	Olive Hill, KY
Latitude:	38-22-24.45N NAD 83
Longitude:	83-05-58.04W
Heights:	300 feet above ground level (AGL)
	1151 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

As a condition to this Determination, the structure is marked and/or lighted in accordance with FAA Advisory circular 70/7460-1 K Change 2, Obstruction Marking and Lighting, a med-dual system - Chapters 4,8(M-Dual),&12.

It is required that the enclosed FAA Form 7460-2, Notice of Actual Construction or Alteration, be completed and returned to this office any time the project is abandoned or:

_ At least 10 days prior to start of construction (7460-2, Part I)

___X__ Within 5 days after the construction reaches its greatest height (7460-2, Part II)

See attachment for additional condition(s) or information.

While the structure does not constitute a hazard to air navigation, it would be located within or near a military training area and/or route.

This determination expires on 10/23/2009 unless:

- (a) extended, revised or terminated by the issuing office.
- (b) the construction is subject to the licensing authority of the Federal Communications Commission (FCC) and an application for a construction permit has been filed, as required by the FCC, within 6 months of the date of this determination. In such case, the determination expires on the date prescribed by the FCC for completion of construction, or the date the FCC denies the application.

NOTE: REQUEST FOR EXTENSION OF THE EFFECTIVE PERIOD OF THIS DETERMINATION MUST BE POSTMARKED OR DELIVERED TO THIS OFFICE AT LEAST 15 DAYS PRIOR TO THE EXPIRATION DATE.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

A copy of this determination will be forwarded to the Federal Communications Commission if the structure is subject to their licensing authority.

If we can be of further assistance, please contact our office at (817) 838-1994. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2008-ASO-1516-OE.

Signature Control No: 567008-102023748 Linda Steele Technician

Attachment(s) Additional Information

7460-2 Attached

(DNE)

It should be noted that no transmitted frequencies were submitted or approved for this tower at this time.

A separate study is required for the addition of any transmitting frequency(ies) on this antenna tower in the future.

Notice of Proposed Construction or Alteration - Off Airport

lame: CENTR-(000090648-08		Sponsor: Central States	Tower Holdings, LLC
		Details for Case : K	Y-00-0817 WOLF CREE	к
		Show P	roject Summary	
Case Stat	tus			
ASN: 2	008-ASO-1516-	•OE	Date Accepted:	03/19/2008
Status: D	etermined		Date Determined:	04/23/2008
			Letters:	04/23/2008 🛱 DNEFT18
) required within 5 days after reaches its greatest height.		
β	dd Supplement	al Notice (7460-2)		
Construc	tion / Alterat	tion Information	Structure Summ	nary
Notice Of:		Construction	Structure Type:	Antenna Tower
Duration:		Permanent	Structure Name:	KY-00-0817 WOLF CREEK
if	Temporary :	Months: Days:	FCC Number:	
Work Sche	edule - Start:		Prior ASN:	
Work Sche	edule - End:			
State Filin	g:	Not filed with State		
Structure	e Details		Common Freque	ency Bands
Latitude:		38° 22' 24.44" N	Low Freq H	igh Freq Freq Unit ERP ERP Ur
L.ongitude	:	83° 5' 58.04" W	Specific Freque	ncies
Horizonta	Datum:	NAD83	opeane rieque	
Site Eleva	tion (SE):	851 (nearest foot)		
Structure	Height (AGL):	300 (nearest foot)		
Marking/L	ighting:	Dual-red and medium intensity		
	Other :			
Nearest C	ity:	Olive Hill		
Nearest S	tate:	Kentucky		
Descriptio Location:	n of	Wooded lot		
Descriptio Proposal:	n of	Tower only		

Notice of Proposed Construction or Alteration - Off Airport

Project Name: CENTR-000090648-08

Sponsor: Central States Tower Holdings, LLC

Details for Case : KY-00-0817 WOLF CREEK

Show Project Summary

Case Status				
ASN: 2008-ASO-1516-OE		Date Accepted: 03/19/2008		
Status: Accepted		Date Determined:		
		Letters: None		
Construction / Alterat	ion Information	Structure Summary		
Notice Of:	Construction	Structure Type: Antenna Tower		
Duration:	Permanent	Structure Name: KY-00-0817 WOLF CREEK		
if Temporary :	Months: Days:	FCC Number:		
Work Schedule - Start:		Prior ASN:		
Work Schedule - End:				
State Filing:	Not filed with State			
Structure Details		Common Frequency Bands		
Latitude:	38° 22' 24 44" N	Low Freq High Freq Freq Unit ERP ERP U		
Longitude:	83° 5' 58.04" W	Specific Frequencies		
Horizontal Datum:	NAD83	Specific Frequencies		
Site Elevation (SE):	851 (nearest foot)			
Structure Height (AGL):	300 (nearest foot)			
Marking/Lighting:	Dual-red and medium intensity			
Other :				
Nearest City:	Olive Hill			
Nearest State:	Kentucky			
Description of Location:	Wooded lot			
Description of Proposal:	Tower only			



May 12, 2008

APPROVAL OF APPLICATION

APPLICANT: Central States Tower, Inc 323 South Hale Street Suite 100 Wheaton, IL 60187

SUBJECT: AS-022-2KY5-08-054

STRUCTURE:Antenna TowerLOCATION:Olive Hill, KYCOORDINATES:38-22-24.44 N / 83-05-58.04 WHEIGHT:300'AGL/1151'AMSL

The Kentucky Airport Zoning Commission has approved your application for a permit to construct 300'AGL/1151'AMSL Antenna Tower near Olive Hill, KY 38-22-24.44 N / 83-05-58.04 W.

This permit is valid for a period of 18 Month(s) from its date of issuance. If construction is not completed within said 18-Month period, this permit shall lapse and be void, and no work shall be performed without the issuance of a new permit.

A copy of the approved application is enclosed for your files.

Dual obstruction lighting is required in accordance with 602 KAR 50:100.

Jøhn Houlihan, Administrator



a 502-564-4480 fax: 502-564-7953 No.: AS-022-2KY5-08-054

CONSTRUCTION/ALTERATION STATUS REPORT

May 12, 2008

AERONAUTICAL STUDY NUMBER: AS-022-2KY5-08-054

Central States Tower, Inc 323 South Hale Street Suite 100 Wheaton, IL 60187

This concerns the permit which was issued to you by the Kentucky Airport Zoning Commission on May 9, 2008. This permit is valid for a period of 18 Month(s) from its date of issuance. If construction is not completed within the said 18-Month period, this permit shall lapse and be void, and no work shall be performed without the issuance of a new permit. When appropriate, please indicate the status of the project in the place below and return this letter to John Houlihan, Administrator, Kentucky Airport Zoning Commission, 90 Airport Road, Bldg 400, Frankfort, KY 40601.

502-564-4480.

STRUCTURE:Antenna TowerLOCATION:Olive Hill, KYCOORDINATES:38-22-24.44 N / 83-05-58.04 WHEIGHT:300'AGL/1151'AMSL

CONSTRUCTION/ALTERATION STATUS

- 1. The project () is abandoned. () is not abandoned.
- 2. Construction status is as follows: Structure reached its greatest height of ______ft. AGL ft. AMSL on ______ (date).
- Date construction was completed.
- Type of obstruction marking/painting.
- Type of obstruction lighting.

As built coordinates.

Miscellaneous Information:

DATE

SIGNATURE/TITLE

entuckip ^s	TC 56-50E (Rev. 02/
Kentucky Transportation Cabinet, Kentucky Airport Zoning Commission, 200 Mere APPLICATION FOR PERMIT TO CONSTRUCT OR ALTE: INSTRUCTIONS INCLUDED	
 APPLICANT Name, Address, Telephone, Fax, etc. 	38 • 22 • 24 44 *
Central States Tower, Inc.	9. Lalilude;
323 South Hale Street, Suite 100	10 Longilude:
Wheaton, ILL 60187	11. Datum: Ct NAD83 D NAD27 Olher
(630) 221-8500	12. Nearest Kentucky City: Olive Hill County Carter
. Representative of Applicant Name, Address, Telephone, Fax	13. Nearest Kentucky public use or Military airport: Fleming Mason
Cellere	
4110 Copper Ridge Drive, Suite 204	14. Distance from #13 to Structure: +/- 37 miles
Traverse City, MI 49684	SE Star New York to Star Arms
(231) 929-4555	15. Direction from #13 to Structure:
9.1 11111111111111111111111111111111111	16. Site Elevation (AMSL): 851 Feat
Application for: XI New Construction C Alteration C Existing	17. Total Structure Height (AGL):Feet
. Duration: 🖾 Permanent 🗌 Temporary (MonthsDays)	1151. 18. Overall Height (#16 + #17) (AMSL):Feet
. Work Schedule: Start End	 Previous FAA and/or Kentucky Aeronautical Study Number(s): N/A
, Type: 🛛 Antenna Yowar 🗋 Crane 🗌 Building 🗌 Power Line	N/A
. Marking/Painling and/or Lighting Preferred: □ Red Lights and Paint ☑ Dual - Red & Medium Intensity White □ White - Medium Intensity □ Dual - Red & High Intensity White	20. Description of Location: (Atlach USGS 7.5 minute Quadrangle Map or an Airport layout Drawing with the precise site marked and any certified survey) See attached 1-A and Quad Map
White - High Intensity Other	
FAA Aeronautical Study Number	
1. Description of Proposal: TOWER ONLY	
2. Has a "NOTICE OF CONSTRUCTION OR ALTERATION" (FAA Form 7460-1)) been filed with the Federal Avialion Administration?
ERTIFICATION: I hereby certify that all the above statements made by me are t	true, complete and correct to the best of my knowledge and bellef.
Braxton Dougherty, VP Construction	on Dociaherty 3/24/08
rinted Name and Tille Signature	() () Date
ENALTIES: Persons failing to comply with Kentucky Revised Statutes (KRS 18) 50:Sertes) are liable for fines and/or imprisonment as set forth in KRS 183.990(3), further penalties.	3.861 (hrough 183.990) and Kentucky Administrative Regulations (602 KAR Non-compliance with Federal Aviation Administration Regulations may result
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Commission Action:	man, maco Alla Automostrator, Maco
Commission Action:	man, KAZC Administrator, KAZC



March 24, 2008

Administrator Kentucky Airport Zoning commission Department of Aviation 200 Mero Street Frankfort, KY 40622

RE: Form TC 56-50E – Application for New Construction

Hello,

Attached please find Form TC 56-50E for your review and approval for the construction of a new 300' tower located in Olive Hill, Carter County, Kentucky. I have also attached a copy of the FAA Form 7460-1, a quadrangle map and a copy of the 1A previously submitted to the FAA on 3/19/08.

If you have any questions or require any additional information please don't hesitate to contact me.

Thank you,

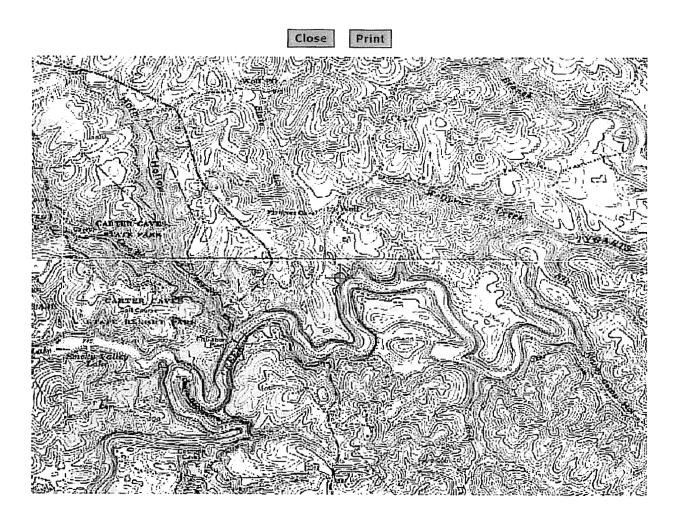
Joann Wendels Joann Wendels Cellere, Agent for Central States Tower, Inc.

> TEL 231.929.4555 FAY 231.929.0099 WWW.cellere.us info@cellere.us 4110 Copper Ridge Drive, Suite 204, Traverse City, MI 49684

Kentuckir

TC 58-50E (Rev. 02/05)

Kentucky Transportation Cabinat, Kentucky Airport Zoning Commission, 200 Mero APPLICATION FOR PERMIT TO CONSTRUCT OR ALTER INSTRUCTIONS INCLUDED	
1. APPLICANT Name, Address, Talephone, Føx, etc.	B Laffluda 38 • 22 · 24 44 •
Central States Tower, Inc.	9. Lafitude:
323 South Hale Street, Suite 100	10. Longitude:
Wheaton, ILL 60187	11. Datum: 🖸 NAD83 🗍 NAD27 🗍 Other
(630) 221-8500	12. Nearest Kenlucky City: Olive Hill County Carter
2. Representative of Applicant - Name, Addross, Telephone, Fax Cellere	13. Nearost Kentucky public use or Military alroad: Fleming Mason
4110 Copper Ridge Drive, Suite 204	14. Distance from #13 to Structure: +/- 37 miles
Traverse City, MI 49684	15. Direction from #13 to Structure:SE
(231) 929-4555	15. Direction from #13 to Structure:
	18. Site Elevation (AMSL): 851 Feel
3. Application for: 🖾 New Construction 🛛 Alteration 🗍 Existing	300 17. Total Structure Height (AGL):Feel
4. Duration: 🖾 Permanent 🗔 Temporary (MonthsOays)	1151 18. Overall Height (#16 + #17) (AMSL):Feet
5. Work Schedule: Slart End	19. Previous FAA and/or Keniucky Aeronaulical Study Number(5):
6. Type: 🛛 Antenna Towar 🗋 Crans 🗌 Building 🗋 Power Line	N/A
7. Marking/Painting and/or Lighting Preferred: Red Lights and Paint Image: Dual - Red & Medium Intensity White White - Medium Intensity Image: Dual - Red & High Intensity White White - High Intensity Image: Other	20. Description of Location: (Atlach USGS 7.5 minute Quadrangle Map or an Atrport layout Drawing with the precise site marked and any certified survey) See attached 1-A and Quad Map
 B. FAA Aeronautical Study Number	
22. Has a "NOTICE OF CONSTRUCTION OR ALTERATION" (FAA Form 7480-1)	been filed with the Federal Aviation Administration?
CERTIFICATION: I hereby certify that all the above statements made by me are t	rue, complete and correct to the best of my knowledge and belief.
Braxton Dougherty, VP Construction	M DOUMDATIN 3/24/08
Printed Name and Tilla Signalure	
PENALTIES: Persons failing to comply with Kentucky Revised Statutes (KRS 18: 050:Serice) are liable for fines and/or imprisonment as set forth in KRS 183.990(3). In further panelities.	3.861 Ihrough 183,990) and Kentucky Administrative Regulations (802 KAR Non-compliance with Federal Aviation Administration Regulations may result
Commission Action:	man, KAZC 🛛 Administrator, KAZC
C Approved	
Disapproved	Date



Howerton Engineering & SURVEYING PLLC

¥.

February 12, 2008

Wilcox Professional Services, LLC One Madison Avenue Cadillac, MI 49601

Subject FAA-1A Certification for KY-00-0817 / WOLF CREEK 520 Sutton Road Olive Hill, Carter County, Kentucky

I, Richard L. Howerton a Kentucky registered professional land surveyor, license No. 3582, hereby certify that the following Latitude and Longitude values for the center of the above-referenced tower are accurate to within +/- 15 feet horizontally; and the following tower site elevation is accurate to within +/- 3 feet vertically.

HORIZONTAL DATUM:	NAD83
LATITUDE: LONGITUDE:	38°22'24.4469" -83°05'58.0371"
KENTUCKY STATE PLANE NORTH ZONE, NAD83,	NORTH: 320140.327 EAST: 1970252.957
VERTICAL DATUM:	NAVD88
GROUND ELEVATION AT BASE OF TOWER: PROPOSED TOWER HEIGHT: PROPOSED HEIGHT OF HIGHEST APPURTENANCE: PROPOSED ELEVATION OF TOP OF TOWER: PROPOSED ELEVATION OF HIGHEST APPURTENANCE:	850.70° 295.00° 300.00° 1,145.7° 1,150.7°

raid How

Richard L. Howerton P.E. P.L.S. KENTUCKY LICENSED SURVEYOR 3582

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	RICHARD L
	S BOWERTON &
	3582
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•	A LAND SUDNEYIB
	CHARGEN INNESS

404A Main Street • Greenup, KY 41144 • (606) 473-5684 • 1-888-473-5684 FAX: • (606) 473-5682 • www.howertoneng.com

533 2nd Street • Portsmouth, OH 45662 • (740) 354-3684 • FAX: (740) 354-3680

Notice of Proposed Construction or Alteration - Off Airport

Project Name: CENTR-000090648-08

Details for Case : KY-00-0817 WOLF CREEK

Sponsor: Central States Tower Holdings, LLC

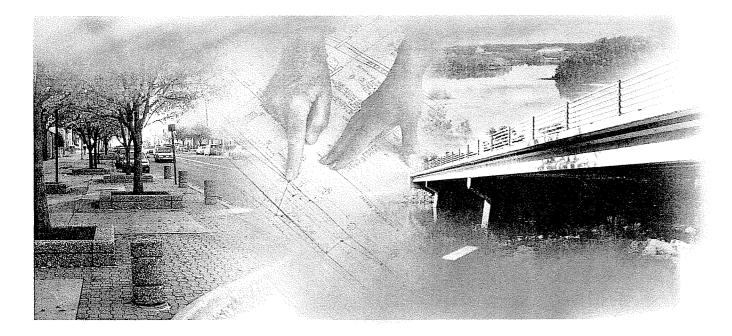
Show Project Summary

Case Status			
ASN: 2008-ASO-1516-OE		Date Accepted:	03/19/2008
Status: Accepted		Date Determined:	
		Letters:	None
Construction / Alterat	ion Information	Structure Summ	ary
Notice Of:	Construction	Structure Type:	Antenna Tower
Duration:	Permanent	Structure Name:	KY-00-0817 WOLF CREEK
if Temporary :	Months: Days:	FCC Number:	
Work Schedule - Start:		Prior ASN:	
Work Schedule - End:			
State Filing:	Not filed with State		
Structure Details		Common Freque	ncy Bands
Latitude:	38° 22' 24.44" N	Low Freq Hig	h Freq Freq Unit ERP ERP Uni
Longitude:	83° 5' 58.04" W	Specific Frequen	ries
Horizontal Datum:	NAD83		
Site Elevation (SE):	851 (nearest foot)		
Structure Height (AGL):	300 (nearest foot)		
Marking/Lighting:	Dual-red and medium intensity		
Other :			
Nearest City:	Olive Hill		
Nearest State:	Kentucky		
Description of Location:	Wooded lat		
Description of Proposali	Tower only		

.

EXHIBIT B

Geotechnical Report; Survey; Tower Design Tower Foundation Design



SOIL BORING AND ROCK CORING INVESTIGATION REPORT

CST SITE NO. KY-00-0817 WOLF CREEK

Olive Hill, Carter County, Kentucky

Prepared for: **CST Holdings, LLC** 323 South Hale Street, Suite 100 Wheaton, Illinois 60187

Prepared by: Wilcox Professional Services, LLC One Madison Avenue Cadillac, MI 49601 Wilcox Project No. 25036.00004.04

Applied Geotechnical Services, Inc.

March 5, 2008



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EXECUTIVE SUMMARY

The proposed center of tower location was approximately 85 feet northwest of an approximately 200-foot high, near vertical rock cliff. Wolf Creek is present at the base of the slope. The driller reported approximately 6 inches of topsoil at the boring locations. Below the topsoil, silty clay was encountered to depths ranging from 4 feet at the location of Boring 2, performed east of the tower center, to 131/2 feet below the existing ground surface at the location of Boring 3 performed west of the tower center. Borings 2 and 3 were terminated at depths of 4 feet and 13¹/₂ feet, respectively upon encountering auger refusal on apparent bedrock. At the location of Boring 1, auger refusal on sandstone bedrock was encountered at a depth of 6 feet. NQ rock coring was then performed from approximate depths of 6 to 16 feet below the existing ground surface. The rock coring revealed sandstone to a depth of 71/2 feet followed by limestone that extended to the explored depth of 16 feet. Our review of the "Carter County Karst Areas" map published by the Kentucky Geological Survey indicates the site is not located within an area described as "Intense Karst". However, the "Generalized Geologic Map for Land-Use Planning: Carter County" published by the Kentucky Geological Survey indicates landslides are relatively common.

The driller reported introducing approximately 750 gallons of water into the borehole during the NQ rock coring operations. No long-term groundwater level readings were obtained. However, based on our review of the site topographic map and the available soil and rock core information, we estimate the prevailing groundwater level may be located below the explored depth of the soil/ rock core borings.

We understand Central States Tower is planning the construction of a 250-foot selfsupporting tower at the site. At the time of our investigation, no information was available to us as to the tower manufacturer or loads. These loads vary considerably depending on the tower characteristics and the number of carriers. Estimated tower loads, based on our experience with similar towers, are presented in Section 1.1 of this report.

In consideration of the variable depth to bedrock and difference in ground surface elevation across the proposed tower compound of approximately 16 feet, we recommend the tower be supported on a drilled pier type foundation system socketed into the medium hard limestone bedrock. For the drilled pier type foundation system, the uplift loads are resisted by the allowable rock socket side shear along the perimeter of the shaft together with the factored weight of the drilled pier concrete (we recommend neglecting the side shear in the overlying silty clays for resisting both uplift and compression loads). We recommend a presumptive allowable rock socket side shear in the sandstone of 1,400 pounds per square foot (psf). We recommend a presumptive allowable rock socket side shear of 3,500 psf in the underlying medium hard limestone bedrock.

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The compression loads are supported by end-bearing together with the above rock socket allowable side shear values to a depth above the drilled pier tip equal to the shaft diameter (we anticipate the compression loads can be entirely supported by end bearing in the soft to medium hard sandstone bedrock). We recommend a maximum net allowable presumptive end-bearing pressure on the medium hard sandstone bedrock of 12 tons per square foot (tsf). We estimate negligible tower foundation settlement on the limestone bedrock.

We anticipate the drilled pier foundation design will be controlled by uplift loads. Based on estimated loads, we anticipate an approximately 4 to 4½-foot diameter shaft, extending to an approximate depth of 8 to 12 feet into the medium hard limestone bedrock (i.e., total depths of approximately 14 to 18 feet) may be required to resist the uplift and compression loads.

The presumptive allowable rock socket values presented above are based on a presumptive uniaxial compressive strength for the sandstone and limestone bedrock of approximately 1,000 pounds per square inch (psi) and 2,500 psi, respectively. Appropriate factors of safety should be used for design.

We recommend the use of a temporary steel casing sealed into the bedrock during drilled pier foundation construction for safety. It may be necessary to twist the casing through the weathered sandstone to obtain a seal in the bedrock. We recommend the casing not be hammered into place to reduce the risk of excess vibrations. A rock core barrel will be required to core through the sandstone and limestone bedrock. It may be necessary to use a temporary liner when coring through the bedrock to provide safe access to the bearing surface for cleaning and testing.

Do not consider this summary separate from the entire text of this report, with all the conclusions and qualifications mentioned herein. Details of our analysis and recommendations are discussed in the following sections and in the appendix of this report.

REPORT PREPARED BY: Applied Geotechnical Services, Inc.

Jefferey T. Anagnostou, P.E., C.P.G. Project Consultant

REPORT REVIEWED BY: Wilcox Professional Services, LLC

Kruegn

Arthur J. Krueger, P.E. Project Manager

1. INTRODUCTION

We have completed the Soil Boring & Rock Coring Investigation for the proposed Central States Tower Site No. KY-00-0817 – Wolf Creek self-supporting lattice tower to be located in Olive Hill, Carter County, Kentucky. Cellere, Inc. retained **Wilcox Professional Services**, **LLC** to perform this investigation. This report presents the results of the soil boring/rock coring investigation and our estimated soil and rock parameters to be used in the design of the tower foundation.

1.1 Project Description

We understand Central States Tower is planning to construct a 250-foot high, selfsupporting lattice type tower at the site. The tower will have three legs on an equilateral triangle. We estimate the tower base width may be approximately 26 feet. At the time this investigation was completed, the tower loads were not yet available. Based on estimated tower loads for a multi-carrier co-locate site, we estimate the tower may impose a compression load per leg of approximately 450 kips, an uplift load per leg of approximately 400 kips, and total shear load in the range of 50 to 60 kips.

We estimate the tower base plate elevation may be slightly above the existing grade.

1.2 Scope of Services

Our scope of services for this project is as follows.

A) Performing one soil boring at the center of the tower to auger refusal on bedrock, followed by NQ rock coring to a depth of 10 feet into the bedrock and performing soil borings extending to auger refusal on bedrock at locations 35 feet up gradient and 35 feet down gradient of the tower center;

- B) Performing appropriate laboratory testing including visual engineering classification, natural moisture content, unconfined compressive strength estimates on representative cohesive samples, performing resistivity, pH, chloride, and sulfate testing of a composite soil sample obtained between depths of 1 to 10 feet; and
- C) Preparing an engineering report providing our recommendations for the tower foundation design and construction. The written report includes recommendations regarding the allowable soil bearing capacity, estimated settlement, and construction considerations related to foundation construction.

The field drilling operations were performed by EnviroProbe Integrated Solutions, Inc. of Nitro, West Virginia with coordination by Wilcox Professional Services, LLC. The laboratory testing and engineering report preparation were performed under the direction and supervision of a registered professional engineer according to generally accepted standards and procedures in the practice of geotechnical engineering. If changes occur in the design, location, or concept of the project, the conclusions and recommendations contained in this report are not valid unless Wilcox Professional Services, LLC reviews the changes. Wilcox Professional Services, LLC will then provide any necessary changes in writing. Our conclusions and recommendations are based on the soil boring/rock coring performed by EnviroProbe Integrated Solutions, Inc. and project information provided by Cellere, Inc.

2. FIELD AND LABORATORY PROGRAM

2.1 Field Program

Cellere, Inc. selected the depth and location of the borings in consultation with Wilcox Professional Services, Inc. As shown on the Schematic Soil/Rock Core Boring Location Plan, a total of three (3) soil/rock core borings were performed for the project. The approximate ground surface elevation at the soil rock core boring locations were estimated based on the ground surface elevation contour lines shown on the Survey Plan prepared by Wilcox Professional Services and are presented in Table 1.

Table 1: Approximate Ground Surface Elevation at Soil/Rock Core Boring Locations		
Soil/Rock Core Boring No. Approximate Ground Surface Elevation		
B-1	850.5 +/-	
B-2	846 +/-	
B-3	854.5 +/-	

A track mounted, GeoProbe® 7720D drill rig was used to perform the soil boring. Standard split-spoon samplers were used to obtain the soil samples by the Standard Penetration Test (SPT) method in general conformance with ASTM Standard D1586. The number of blows required to drive the sampler 12 inches, after an initial seating of 6 inches, with a 140-pound hammer falling 30 inches is termed the Standard Penetration Resistance, N-value. A graphical representation of the N-values is given on the boring logs appended to this report.

A diamond tipped bit in a double tube NQ core barrel was used to core through the sandstone bedrock between approximate depths of 6 to 16 feet below the existing ground surface at the location of Boring 1.

During the field operations, the drill crew maintained a log of the subsurface conditions, including changes in stratigraphy and observed groundwater levels. After completion of the drilling operations, the boreholes were backfilled with drill cuttings and bentonite crumbles.

2.2 Laboratory Testing

The soil and rock samples were placed in sealed containers in the field and brought to the laboratory for testing and classification. A geotechnical engineer classified the samples in general conformance with the Unified Soil Classification System. The cored rock samples were classified by EnviroProbe Integrated Solutions, Inc.

Laboratory testing of the soil samples included estimating the unconfined compressive strength of the split-spoon samples with a calibrated hand penetrometer. With a hand penetrometer, the unconfined compressive strength of a soil sample is estimated by measuring the resistance of the soil sample to the penetration of a small, calibrated spring-loaded cylinder. The penetrometer can measure a maximum unconfined compressive strength of 4½ tons per square foot (tsf).

The cores were logged for core recovery and Rock Quality Designation (RQD) by a EnviroProbe Integrated Solutions, Inc. engineer. The RQD is one the standard measurements of rock competence and is given by the percentage ratio of the total

length of the recovered samples 4 inches or more in length to the total length of the core run. Sometimes, core lengths smaller than 4 inches may be included if they are judged to have been fractured during coring and handling.

We will hold the soil and rock core samples for 60 days from the date of this report. If you would like the samples, please contact us within this time frame.

2.3 Laboratory Soil Box Resistivity Test Results

Estimated earth resistivity values of the subsoil below the proposed development area were obtained by performing laboratory resistivity testing using the Miller Soil Box Resistivity instrument. The testing was performed on selected composite split-spoon samples from Soil/Rock Core Borings B-1 through B-3. The composite samples were prepared by thoroughly mixing prior to placement in the soil box instrument. The following estimated earth resistivity values are presented based on the Miller Soil Box Resistivity test results and may be used with judgment in the design of the lightning protection grounding system:

Table 1. Miller Soil Box Resistivity Results			
Boring Numbers	Sample Numbers	Represented Depth Below Ground Surface (ft)	Resistivity (Ohm-feet)
B-1-B-3	S1 – S4	1 to 10	180

We note measured resistivity value appears to be higher than typically encountered for moist silty clays soils. However, we note the composite soil sample contained occasional weathered rock fragments.

3. SITE AND SUBSURFACE CONDITIONS

3.1 Site Conditions

The subject site is located at +/- 520 Sutton Road Olive Hill, Carter County, Kentucky. Based on our review of the Survey Plan prepared by Wilcox Professional Services and the Central States Tower site Candidate Package, it appears the site is situated within a wooded area southeast of a residential dwelling and appurtenant garage structure. The proposed center of tower location was approximately 85 feet northwest of an approximately 200-foot high, near vertical rock cliff. Wolf Creek is present at the base of the slope. Within the tower compound area, the ground surface sloped downward towards the southeast direction, towards the edge of the cliff.

3.2 Soil and Rock Conditions

The driller reported approximately 6 inches of topsoil at the boring locations. Below the topsoil, silty clay was encountered to depths ranging from 4 feet at the location of Boring 2, performed northeast of the tower center, to 13½ feet below the existing ground surface at the location of Boring 3 performed southwest of the tower center. Borings 2 and 3 were terminated at depths of 4 feet and 13½ feet, respectively upon encountering auger refusal on apparent bedrock. At the location of Boring 1, auger refusal on sandstone bedrock was encountered at a depth of 6 feet. NQ rock coring was then performed from approximate depths of 6 to 16 feet below the existing ground surface. The rock coring revealed sandstone to a depth of 7½ feet followed by limestone that extended to the explored depth of 16 feet. Our review of the "Carter County Karst Areas" map published by the Kentucky Geological Survey indicates the site is not located within an area described as "Intense Karst". However, the "Generalized Geologic Map for Land-Use

Planning: Carter County" published by the Kentucky Geological Survey indicates landslides are relatively common.

The silty clays were stiff to hard with calibrated hand penetrometer unconfined compressive strengths of 1 to in excess of $4\frac{1}{2}$ tsf and natural moisture contents of approximately 15 to 31 percent.

The 10-foot NQ rock core possessed a recovery of 91 percent and an RQD value of 33.5.

The stratification depths shown on the soil boring log represent the soil and rock conditions at the boring location. Variations may occur at locations away from the boring. Additionally, the stratigraphic lines represent the approximate boundary between soil and rock types; the transition may be more gradual than what is shown. The boring log was prepared on the basis of laboratory classification and testing as well as the field logs of the explored soils and bedrock.

The soil/rock core boring logs are presented in the appendix. The soil and rock profile described above is a generalized description of the conditions encountered at the boring location. Please consult the boring logs for more specific information.

3.3 Groundwater Level Observations

The driller looked for indications of groundwater during and after the performance of the soil boring. Groundwater seepage was not encountered during drilling of the borings through the overburden soils. The driller reported introducing approximately 750 gallons of water into the borehole during the rock coring operations. Based on the available information, we estimate the groundwater level may be located below the explored depth of the borings.

Expect the prevailing groundwater level to vary due to changes in precipitation, evaporation, surface run-off, and other factors. The groundwater levels discussed herein and shown on the boring logs represent the conditions at the time of the measurements.

4. RESULTS & RECOMMENDATIONS

4.1 Drilled Pier Foundation Recommendations

In consideration of the variable depth to bedrock and difference in ground surface elevation across the proposed tower compound of approximately 16 feet, we recommend the tower be supported on a drilled pier type foundation system socketed into the medium hard limestone bedrock. For the drilled pier type foundation system, the uplift loads are resisted by the allowable rock socket side shear along the perimeter of the shaft together with the factored weight of the drilled pier concrete (we recommend neglecting the side shear in the overlying silty clays for resisting both uplift and compression loads). We recommend a presumptive allowable rock socket side shear in the sandstone of 1,400 pounds per square foot (psf). We recommend a presumptive allowable rock socket side shear of 3,500 psf in the underlying medium hard limestone bedrock.

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Once the tower loads are known, Wilcox Professional Services, LLC should be notified so we can re-evaluate our design recommendations in the light of the actual loads.

We recommend all foundation construction be performed under the supervision of a qualified geotechnical engineer. The appropriate type and number of field tests and observations should be performed to verify the foundation bearing material is suitable.

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4.2 Engineered Fill Placement

We anticipate several feet of cut and fill will be required to achieve finished grades within the tower compound area. To reduce the risk of a potential slip plane developing between the engineered fill and underlying subgrade soils, we recommend the subgrade surface be properly benched prior to placement of the engineered fill.

Any fill beneath on-grade structures should be an approved, environmentally clean material. The fill should also be free of organic matter, frozen soil, clods, or other harmful material. Spread the fill in level lifts, not exceeding 9 inches in loose thickness, and compact the soil to a minimum of 95 percent of the maximum dry density. Determine the maximum dry density according to ASTM Standard D1557 (Modified Proctor). All engineered fill should be placed at or near the optimum moisture content.

4.3 General Comments

The purpose of this report is to aid in the tower foundation. If changes occur in the design, location, or concept of the project, the recommendations contained in this report are not valid. The changes must be reviewed by **WILCOX PROFESSIONAL SERVICES**, **LLC** with the recommendations of this report modified or affirmed in writing by **WILCOX PROFESSIONAL SERVICES**, **LLC**.

We base the estimated soil and rock parameters presented in this report upon the data from the soil/rock core borings performed at the approximate locations shown on the Schematic Soil Boring/Rock Core Location Plan. This report does not reflect variations that may occur away from the boring location. The nature and extent of any such

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variations may not become clear until the time of construction. If significant variations then become evident, it may be necessary for us to re-evaluate our report recommendations.

We recommend **WILCOX PROFESSIONAL SERVICES**, LLC be given the opportunity to review the final design plans and specifications as they relate to the recommendations presented in this report. The review is necessary to verify that the report conclusions and recommendations have been interpreted according to our intent and are properly incorporated into the design. Further, the review will verify that subsequent changes to the project have not affected our recommendations. Without this review, we cannot be held responsible for misinterpretation of our data, analysis, and/or our recommendations or how these are incorporated in the final design.

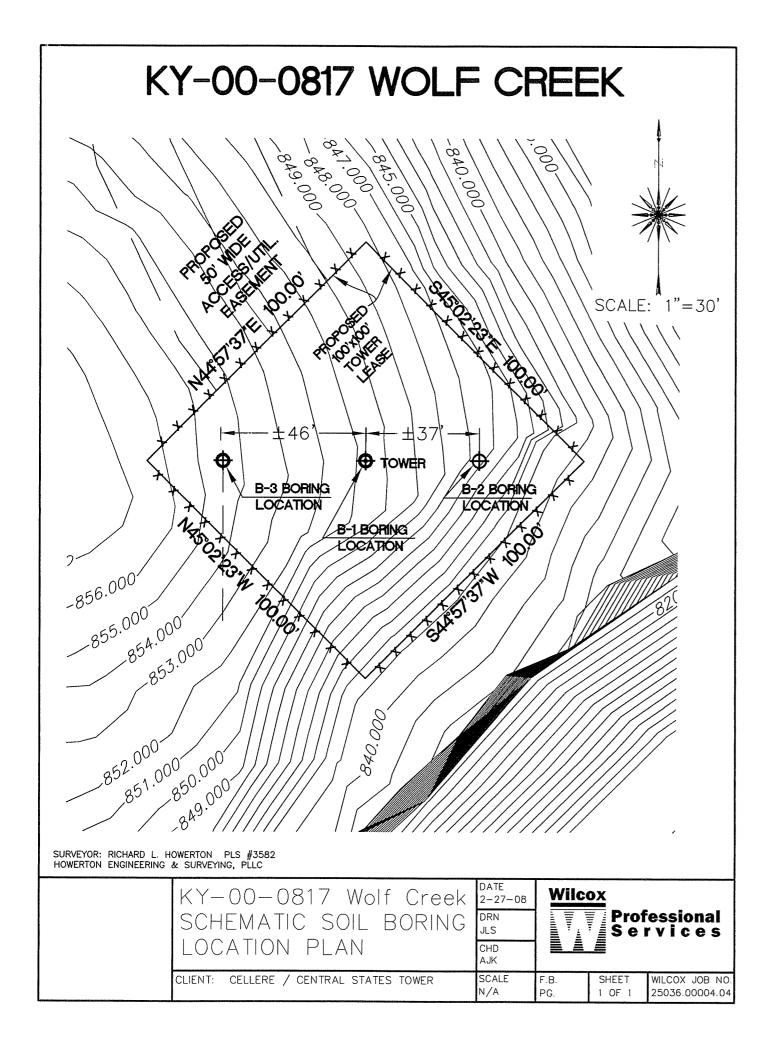
We also recommend a qualified geotechnical engineer supervise all geotechnical related work, including foundation construction, subgrade preparation, and engineered fill placement. The geotechnical engineer should perform the appropriate testing to confirm the geotechnical conditions given in the report are found during construction.

The contract specifications should include the following:

"The contractor will, upon becoming aware of subsurface or latent physical conditions differing from those disclosed by the original soil investigation work, promptly notify the owner verbally to permit verification of the conditions, and in writing, as to the nature of the differing conditions. No claim by the contractor for any conditions differing from those anticipated in the plans and specifications and disclosed by the soil studies will be allowed unless the contractor has so notified the owner, verbally and in writing, as required above, of such differing subsurface conditions."

APPENDIX

- 1. SCHEMATIC SOIL\ROCK CORE LOCATION PLAN
- 2. GENERAL NOTES
- 3. SOIL/ROCK CORE BORING LOGS (B-1 through B-3)
- 4. UNIFIED SOIL CLASSIFICATION SYSTEM



GENERAL NOTES

Drilling & Sampling Symbols

SS –	Split Spoon $(1^{3}/_{8})$ " I.D., 2" O.D., except where	HA –	Hand Auger Boring
	noted	BS –	Bag Sample
ST –	Shelby Tube (3" O.D., except where noted)	RC –	Rock Core with diamond bit,
PA –	Power Auger		NX size, except where noted
PS –	Piston Sample (3" diameter)	RB –	Roller Bit
WB –	Wash Boring	N/A —	Not applicable or available
WS –	Wash Sample		

Standard Penetration Test "N" Value – Blows per foot after an initial 6-inch seating of a 140-pound hammer falling 30 inches on a 2-inch O.D. split spoon, except where noted.

Water Level Measurement Notation

Particle Sizes

First— Completion— HR—	When noted during drilling or sampling process. - After all drilling tools are removed from borehole. Number of hours after completion.		Greater than 6" (152 mm) 3" to 6" (76 to 152 mm) Coarse: ¼ to 3" (19 to 76 mm) Fine: No.4 to ¾" (4.75 to 19 mm) Coarse: No.10 to No.4 (2 to 4.75 mm)
N/R— Dry—	Not recorded. No measurable water level found in borehole.	Silt – Clay –	Medium: No.40 to No.10 (.425 to 2 mm) Fine: No.200 to No.40 (.074 mm to .425mm) Minus No.200 (.005 mm to .074 mm) Less than .005 mm

Water levels indicated on the boring logs are the levels measured in the boring at the time indicated. The accurate determination of groundwater levels may not be possible with short term observations, especially in impervious soils. The level shown may fluctuate throughout the year with variations in precipitation, evaporation, runoff, and other hydrogeologic features.

CLASSIFICATION

<u>Cohesi</u>	onless Soil	<u>C</u>	ohesive Soil
Relative Density "N" Va Very Loose Loose Medium Dense Dense Very Dense Extremely Dense	lue (Blows/ft) 0 to 4 5 to 9 10 to 29 30 to 49 50 to 79 Over 80	$\frac{\text{Unconfined Compressi}}{(\text{tons per ft}^2)}$ Less than 0.25 0.25 to 0.49 0.49 to 0.99 1.00 to 1.99 2.00 to 3.99 Greater than 4.00	<u>Consistency</u> Very Soft Soft Medium Stiff Very Stiff Hard
Soil Co	onstituents	If clay content is suffi	icient so that clay dominates soil

 "Trace"
 Less than 10%

 "Trace to Some"
 10% to 19%

 "Some"
 20% to 34%

 "And"
 35% to 50%

If clay content is sufficient so that clay dominates soil properties, then clay becomes the primary noun with other major soil constituent as modifier, i.e. silty clay. Other minor soil constituents may be added according to estimates of soil constituents present, i.e. silty clay, trace to some sand, trace gravel.

AGS, Inc. 15798 Riverside, Livonia, MI 48154 Tel/Fax: (734) 432-2631

#20121001031944040	Proj	ect: CS	ST Site No. I	KY-00-0817A - Wolf C	Ereek		Арр	lied G	eotechn	ical Ser	vices,	Inc.
			ellere, Inc.				_			ile Road		
				Carter Co., Kentucky						MI 4815		
0)		ct #: 08	-1006		Boring Log #:	B-1		Phone	/Fax: (734) 293	-5077	
Sample No./Type	Recovery (in.)	Depth (ft.)	I Ground S	Description of Mate Surface Elevation = 850	rial).5' +/-	N-Value (t	ontent (%) - c blows/ft) - squ 		Unconf	ined Comp (tsf) - tri	angles	
			Driller F	Reported Approximately 6	" of Topsoil	00 - 8 9	8 1		00	1.50 2.00	3.00	4.50
SS-1 SS-2	18			r - trace sand & gravel - oc ragments - stiff - brown -		10 20 30 40	30 8		20	1 75	2012 - 112 -	
55-2	18	5		r - trace sand & gravel - oc ragments - hard - brown -		5.0		7-9- 50/2	40			
		6 — 7 —	Sandstone ·	- occasional clay filled voi seams - light brown	ids & limestone	60 70			6.0 70			A series of the Alexandron Annual series and the series of
RC-1	109	8 9 10 11 12 13 14 15 16	Lim	estone - medium hard - li	ght gray	80 90 100 11.0 12.0 13.0 14.0						
		10 17 18 19 20		End of Core/Boring @ 1 Q rock coring performed f Recovery = 91% RQD =	rom 6' to 16'.	17 0 18 0 19 0						
Whi At C	Water Level Observations: While Drilling: At Completion: Cave-In At:			Boring Comp	Boring Started: 2/18/08 Boring Completed: 2/19/08 Rig: GeoProbe 7720 Driller: Enviroprobe Integrated					oroved: wn By:		

	Proj	ject: C	ST Site No. 1	KY-00-0817A - Wolf C	reek		Applied G	eotechnical Servio	ces, Inc.			
			ellere, Inc.				37637 Five Mile Road #224					
	Locat	ion: O	live Creek, (Carter Co., Kentucky	Livonia, MI 48154							
		ct #: 08	3-1006		Boring Log #:	B-2	Phone	Phone/Fax: (734) 293-5077				
Sample No./Type	Recovery (in.)	Depth (ft.)		Description of Mate	rial		ntent (%) - circles lows/ft) - squares	Unconfined Compres (tsf) - triang				
		0	ſ	Surface Elevation = 846		0 10 20	30 40 50 60	0.00 0.50 1.00 1.50 2.60 2.50	3.00 3.50 4.n0 4.50			
		1	Driller	Report Approximately 6"	of Topsoil	00 2 8.7		00				
SS-1	18	2-	Silty Clay - ti	race sand & gravel - occas		20 147	15-	20				
SS-2	10	3		stiff to hard - brown (CI	<i>(</i> د	30	50/4"	30	4 54			
		4—	-	End of Boring @ 3.5'		40-		40				
		5— 6—				5.0		50-				
		7-	Note: D	Driller Reported Auger Ref	usal @ 3.5'	60 70		60				
		8-	-			80-		80-				
		9—				90-		9.0				
		10				10 0		100				
		11				110 -		11.0				
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	Water Level Observations: While Drilling: Dry			Boring Sta Boring Compl	rted: 2/18/08 eted: 2/19/08			Approved: JT.	A			
		ion: Di	-	Sound combi	Rig: GeoProbe	e 7720		Drawn By: NJ	А			
	Cave-In			Dr	iller: Enviropro		3 5	~~~~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~				

	Proj	ject: C	ST Site No.	KY-00-0817A - Wolf Creek				Т	Applied	Geotech	nical Se	rvices	, Inc.		
			ellere, Inc.		·····				37637 Five Mile Road #224						
				Carter Co., Kentucky		Farmington Hills, MI 48									
		ct #: 08	-1006	Boring	g Log #:]	B-3			Ph: (248) 615-3000 Fx: (248) 615-3			5-3512			
Sampie No./Typ	Recovery (in.)	Depth (ft.)		Description of Material					tent (%) - circles Unconfined Compressive Sta ws/ft) - squares (tsf) - triangles						
		0 —	Ground	Surface Elevation = 854.5' +/-		0		20 30	40 50	60 8 8	1.50	2.50	4.00		
SS-1	18		Driller	Reported Approximately 6" of Top	soil	0.0	4								
		2		to Medium Sand - trace gravel - oc and organics - very loose - moist -		10				10					
		_				20		24 3		2.0					
SS-2	18	3				30-				30		3 00			
		4				4.0 -				40-					
SS-3	18	5				50		21 4		50-		3 00			
		6				60	n manda i sa			6.0 -		A lease of the second se			
		7—	Silty Clay -	trace sand & gravel - occasional w	eathered	70-		26		70	THE PARTY IN A STATE OF THE ADDRESS	reger			
SS-4	18	8 —	shale s	eams - very stiff to hard - brown (C	CL)	80-	17	9		80-					
		9				90-				90			a deservation and an example and a second and		
SS-5	18	10 —				10 0	en ma program maaamee			100					
55-5	10	11 —				110	18 6	25		110					
		12				120			an in the state of	120			A		
		13 —				13 0 -				13 0					
		 14		End of Boring @ 13.5'		14.0				14.0					
		15 —				15.0				15.0					
		 16	Note: D	riller Reported Auger Refusal @ 1	3.5'	160		AL MARK COMPANY AND A VALUE AND AND A		160-	V quarte a second of the secon				
		17				170		A DEAL AND A		17.0	a dadi dadi u 				
		18				18.0				18.0 -		and the second states			
		19—				19.0 -				190-	new where the substrate of the second		The second secon		
		20				20 0				200	***** / / ****************************		Annual and a		
Water	Level		ations	Raving Stautad. 2	/18/09			-				IT A			
				Boring Started: 2 Boring Completed: 2						Арр	roved:	JIA			
At Co	While Drilling: Dry At Completion: Dry Cave-In At:			Rig: GeoProbe 7720 Driller: Enviroprobe Integrated S						Dra	wn By:	NJA			
					a se anna an seanna a				1	· · · · · · · · · · · · · · · · · · ·					

Unified Soil Classification

Major Divisons			Symbol	Typical Names]	Laboratory Classification	Criteria			
e)	se s			Well graded gravels, gravel- sand mixtures, little or no		·····		$C_u = D_{60}/D_{10} \text{ gr}$				
) siev	of co . 4 si	Grav no f	GW	fines	ve),			$C_{c} = (D_{30})^{2} / (D_{10} \times D_{20})^{2}$	30) between 1 and 3			
> No. 200	Gravels (More than half of coarse fraction is larger than No. 4 sieve)	Clean Gravels (little or no fines)	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	No. 200 sieve), s:	1	ymbols	Not meeting all gradatior	n requirements for GW			
material >	ls (More n is large	Gravels with appreciable amount of fines	GM d u	Silty gravels, gravel-sand-silt mixtures	naller than d as follow:	.GW, GP, SW, SP .GM, GC, SM, SC	Borderline case requiring dual symbols	Atterberg Limits below "A" line or PI less than 4 between 4 and 7 a				
n half of			GC	Clayey gravels, gravel-sand- clay mixtures	(fraction sr re classifie	GW, GP,	e case requ	Atterberg Limits above "A" line with PI greater than 7	borderline cases requiring dual symbols			
More thar	f coarse). 4 sieve)	Clean Sands (little or no fines)	SW	Well graded sands, gravelly sands, little or no fines	n percentage of fines (fraction smaller than N coarse grained soils are classified as follows:	Less than 5%	Borderin	$C_u = D_{60}/D_{10} \text{ gr}$ $C_c = (D_{30})^2 / (D_{10} \times D_{20})^2$				
ed Soils (Sands (More than half of coarse fraction is smaller than No. 4 sieve)	Clean Sa or no	SP	Poorly graded sands, little or no fines	Depending on percentage of fines (fraction smaller than No. coarse grained soils are classified as follows:	Less than 5%. More than 129	12%0	Not meeting all gradation	n requirements for SW			
Coarse Grained Soils (More than half of material > No. 200 sieve)	s (More th i is smalle	Sands with appreciable amount of fines	SM d u	Silty sands, sand-silt mixtures	Depending	5 to	01 0	Atterberg Limits below "A" line or PI less than 4	Liquid Limits plotting between 10 and 30 with PI between 4 and 7 is a			
Соа	Sands fraction	Sand appre amount	SC	Clayey sands, sand-clay mixtures				Atterberg Limits above "A" line with PI greater than 7	borderline case requiring dual symbols (CL-ML)			
200 sieve)			ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity				PLASTICITY CHA	RT			
Fine Grained Soils (more than half of material < No. 2		and Clays Limit < 50)	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, and lean clays		60 50		PI = 0.73(LL-20)				
lalf of n			OL	Organic silts and silty clays of low plasticity	x (PI)	40			CH ''A'' Line			
more than h			MH	Inorganic silts, micaceous or diamaceous fine sandy or silty soils, elastic silts	sticity	30			OH and MH			
od Soils (and Clays Limit > 50)	СН	Inorganic clays of high plasticity, fat clays		10		CL				
ne Graine			ОН	Organic clays of medium to high plasticity, organic silts		0	1	ML and OL 0 20 30 40 50 60	70 80 90 100			
Fi	Highly C	Organic Soils	Pt	Peat and other highly organic soils		-		Liquid Limit (LI				

LATITUDE & LONGITUDE LATITUDE AND LONGITUDE OF SITE ARE BASED ON N4083 LATITUDE: 38'22'24 4469" LONGITUDE. -83'05'58.0371" GROUND ELEVATION AT TOWER BASE 850.70'

SITE INFORMATION SITE ADDRESS ±520 SUTTON ROAD OLIVE HILL, KY 41164

PROPERTY OWNER GARY BOND 520 SUTTON ROAD OLIVE HILL, KY 41164

ACCESS/UTILITY EASEMENT DESCRIPTION

30.00' EASEMENT

BEING A CENTER LINE DESCRIPTION OF 268.92' OF A 30.00' WIDE EASEMENT, 15.00' LEFT AND RIGHT OF CENTER LINE. FOR INGRESS AND EGRESS AND THE INSTALLATION AND MAINTENANCE OF UTILITIES OVER/UNDER AND ACROSS SAID EASEMENT. BEING LOCATED ON THE PROPERTY OF GARY BOND (D.B. 166 PG. 134), 502 SUTTON ROAD, OLIVE HILL KY

UNLESS OTHERWISE INDICATED, ANY (IPS) IRON PIN SET REFERRED TO HEREIN, IS A "" DIAMETER REBAR 18-30 INCHES LONG WITH A PLASTIC CAP STAMPED "HOWERTON 2512".

BEGINNING AT A P.K. NAIL SET IN THE CENTER LINE OF SUTTON ROAD AND THE CENTER LINE OF A GRAVEL DRIVEWAY AT 502 SUTTON ROAD.

THENCE, WITH THE CENTER OF SAID GRAVEL DRIVEWAY N 71-02-29 E AND 49.31 TO A POINT NOT MONUMENTED

THENCE, N 64-18-31 E AND 56.06' TO A POINT NOT MONUMENTED. THENCE, N 65-18-47 E AND 38.27' TO A POINT NOT MONUMENTED. THENCE, N 75-37-07 E AND 41.98' TO A POINT NOT MONUMENTED. THENCE, S 84-27-05 F AND 42.16' TO A POINT NOT MONUMENTED. THENCE, S 58-18-08 E AND 41.14' TO A POINT NOT MONUMENTED. BEING THE END OF SAID 30.00' FASEMENT SAID POINT BEARS \$ 01-14-50 F AND 57.13' FROM A UTILITY POLE NUMBER 1137-4-02 AND BEING THE BEGINNING POINT FOR A 50.00'

WIDE FASEMENT THE ABOVE-DESCRIBED PARCEL IS SUBJECT, HOWEVER, TO THE RIGHTS OF WAY AND PUBLIC UTILITY EASEMENTS, IF ANY, THAT MAY BE ON LEGAL RECORDING COVERING SAID PREMISES

50.00' EASEMENT

BEING A CENTER LINE DESCRIPTION OF 271.74' OF A 50.00' WIDE EASEMENT,

25.00' LEFT AND RIGHT OF CENTER LINE. FOR INGRESS AND EGRESS AND THE INSTALLATION AND MAINTENANCE OF UTILITIES OVER/UNDER AND ACROSS SAID EASEMENT. BEING LOCATED ON THE PROPERTY OF GARY BOND (D.B. 166 PG. 134). 502 SUTTON ROAD, OLIVE HILL, KY

BEGINNING AT A POINT AT THE END OF A 30.00' WIDE EASEMENT SAID POINT BEING THE BEGINNING OF A 50.00' WIDE EASEMENT. SAID POINT IS LOCATED S 01-14-50 E AND 57.13' FROM AN EXISTING UTILITY POLE # 1137-4-02.

THENCE, N 85-42-16 E AND 51.90' TO A POINT NOT MONUMENTED.

THENCE, S 63-19-11 E AND 58.15' TO A POINT NOT MONUMENTED.

THENCE, S 32-04-02 E AND 28.08' TO A POINT NOT MONUMENTED.

THENCE, S 11-48-58 E AND 60.33' TO A POINT NOT MONUMENTED.

THENCE, S 25-28-18 E AND 58.91' TO A POINT NOT MONUMENTED.

THENCE, S 36-50-44 E AND 14.37' TO A POINT NOT MONUMENTED. BEING THE END OF SAID 50.00' EASEMENT. SAID EASEMENT TERMINATES ALONG THE NORTHWEST LINE OF THE LEASE AREA AS DESCRIBED BY THIS SURVEY. SAID POINT BEARS N

44-57-37-E AND 57.12' FROM AN IPS AT THE NORTHWEST CORNER OF SAID LEASE AREA

THE ABOVE-DESCRIBED PARCEL IS SUBJECT, HOWEVER, TO THE RIGHTS OF WAY AND PUBLIC UTILITY EASEMENTS, IF ANY, THAT MAY BE ON LEGAL RECORDING COVERING SAID PREMISES.

LEASE AREA DESCRIPTION

BEGINNING AT AN IPS AT THE NORTHWEST CORNER OF LEASE AREA. SAID IPS BEARS S 65-52-13-E AND 408.93' FROM A P.K. NAIL SET IN THE INTERSECTION OF SUTTON ROAD AND THE CENTER LINE OF THE ABOVE DESCRIBED 30.00' WIDE EASEMENT

THENCE, N 44-57-37 E AND 100.00' TO AN IPS. PASSING THE CENTER LINE POINT OF A 50.00' WIDE EASEMENT AT A DISTANCE OF 57.12'

THENCE, \$ 45-02-23 E AND 100.00' TO AN IPS. THENCE, S 44-57-37 W AND 100.00' TO AN IPS. THENCE, N 45-02-23 W AND 100.00' TO THE PLACE OF BEGINNING. CONTAINING 0.23 ACRES (10,000 SQ. FT.).

Wilcox





CENTRAL STATES TOWER, INC 323 SOUTH HALE STREET SUITE 100 WHEATON, IL 60187

404-A MAIN STREET GREENUP, KENTUCKY (606) 473-5684 533 2ND STREET PORTSMOUTH, OHIO (740) 354-3684

PARENT PARCEL

SCALE: 1"=500

SCALE. 1"=250' (24x36)

HOWIERTON ENGINEERING & SURVEYING PLLC

DETAIL

GENERAL NOTES

NO PROPOSED MUNICIPAL SEWER OR WATER UTIL THERE WILL BE NO CHANGE IN DRAINAGE PATTERN NO HAZARDOUS MATERIALS WILL BE USED, PROCE TOWER LIGHTING SHALL CONFORM TO FAA STANDA ALL WORK SHALL CONFORM TO FAA & FCC REGU

CERTIFICATION:

A 4 4 000

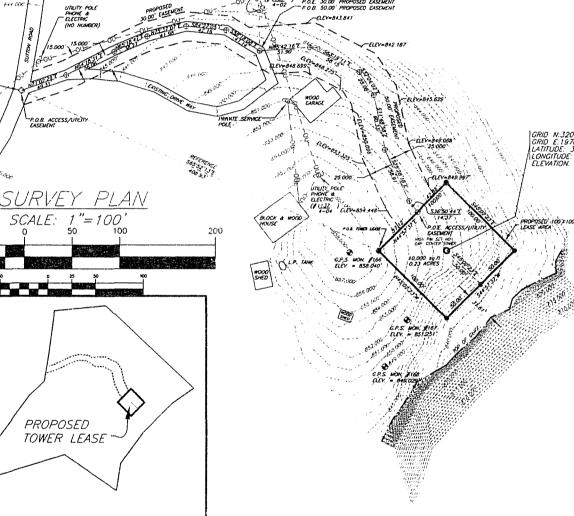
P.K. MAL SET CENTER LINE WTERSECTION ELEV. = 844.837

100

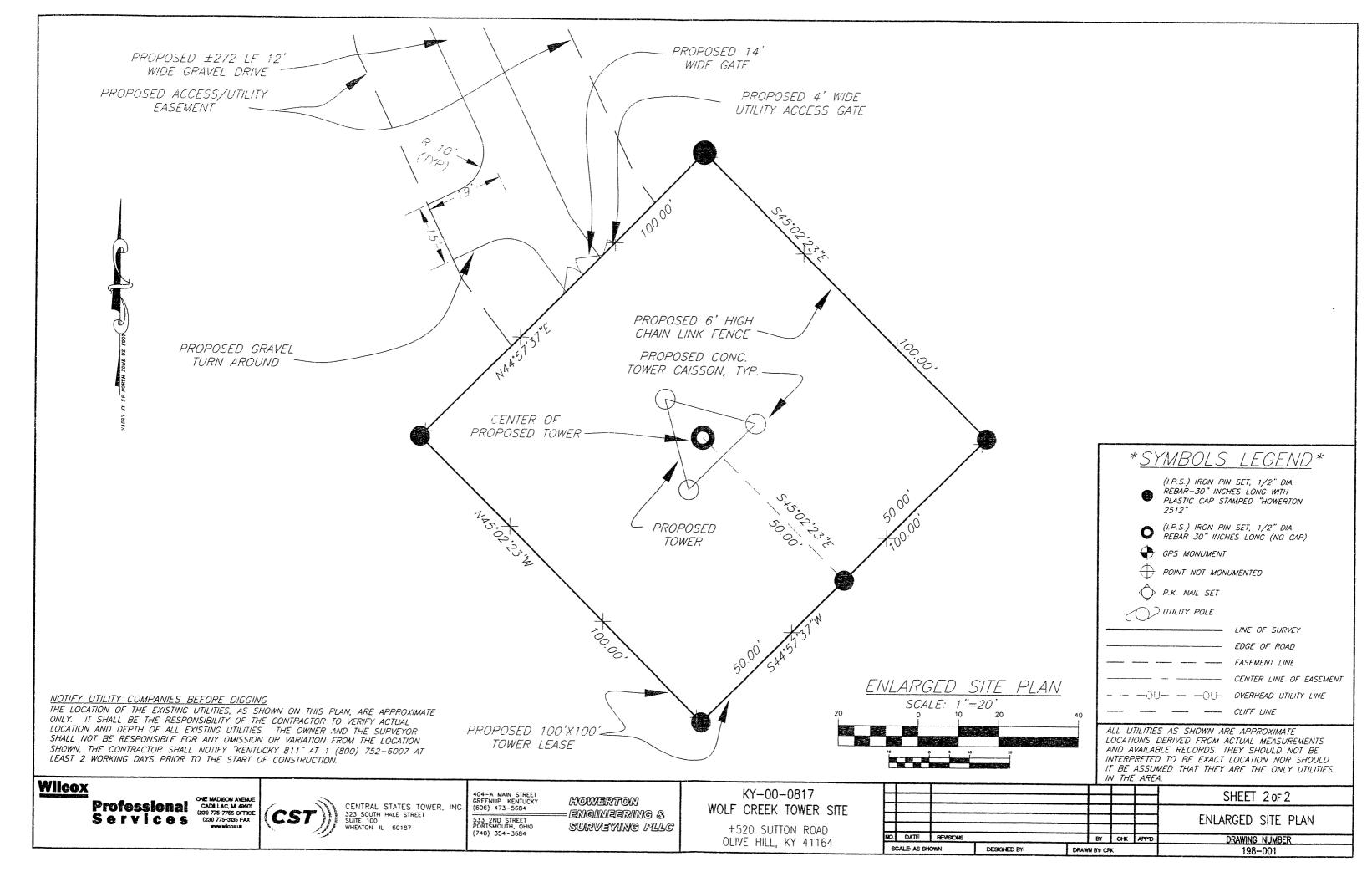
RICHARD L. HOWERTON CERTIFY THAT I HAVE LI THE PROPOSED INSTALLATION, AND EXISTING FEATO

RICHARD L. HOWERTON

KENTUCKY PLS #3582



ATER UTILITIES ARE REQUIRED FOR THIS TE PATTERN DUE TO THE PROPOSED INS ED, PROCESSED OR STORED AT THE SIT TA STANDARDS AS REQUIRED FCC REGULATIONS	TALLATION.	TRUE V	156 m		X			ROPOSED ITILITY EASEMENT
2/13/08 DATE DATE PUT DATE PUT POLE SOLO PROPOSED EXEMPT POLE SOLO PROPOSED EXEMPT	TE CHILAT ICH 4 P D OW E RI 6 3582 LICENSED OPESSION ND SURVEY	AL ST	P.O.B. TOWER	LEASE NO CONTRACTOR	55	STREET	P OF (D TOWER	OSED TOWER LEASE
Converse and the second	49.997	LATITUDE	20140.327 970252.957 - 38122'24.4465 DE:83'05'58 0 N. 850.667'		50 50	WER LEASE		EASE DETAIL $SCALE: 1''=50'$ 100 100
(BLOCK & WOOD)	2 Hi		(100) 1			*	•	(I.P.S.) IRON PIN SET, 1/2" DIA. REBAR-30" INCHES LONG WITH PLASTIC CAP STAMPED "HOWERTON 2512" (I.P.S.) IRON PIN SET, 1/2" DIA. REBAR 30" INCHES LONG (NO CAP) GPS MONUMENT POINT NOT MONUMENTED
				NOT Y SA TA SALA				P.K. NAIL SET UTILITY POLE LINE OF SURVEY EDGE OF ROAD EASEMENT LINE CENTER LINE OF EASEMENT CENTER LINE OF EASEMENT CLIFF LINE
NOTIFY UTILITY COMPANIES BEFORE THE LOCATION OF THE EXISTING UTILITIE. ONLY. IT SHALL BE THE RESPONSIBILIT LOCATION AND DEPTH OF ALL EXISTING SHALL NOT BE RESPONSIBLE FOR ANY SHOWN, THE CONTRACTOR SHALL NOTIFY LEAST 2 WORKING DAYS PRIOR TO THE	S, AS SHOV Y OF THE U UTILITIES OMISSION O ' "KENTUCK	CONTRACTO THE OWNE PR VARIATIO Y 811" AT	R TO VERIH R AND THE N FROM TH 1 (800)	EY ACTUAL SURVEYOR HE LOCATION	E	LOCATH AND A INTERP	ONS L VAILAE RETED ASSUM	S AS SHOWN ARE APPROXIMATE DERIVED FROM ACTUAL MEASUREMENTS BLE RECORDS. THEY SHOULD NOT BE D TO BE EXACT LOCATION NOR SHOULD MED THAT THEY ARE THE ONLY UTILITIES
KY-00-0817								SHEET 1 OF 2
WOLF CREEK TOWER SITE					F			SURVEY PLAN
±520 SUTTON ROAD	NO. DATE	REVISIONS			8	Y СНК	APPD	DRAWING NUMBER
OLIVE HILL, KY 41164	SCALE: AS SH		DESIGNED BY	DRAV				198-001



NOTIFY UTILITY COMPANIES BEFORE DIGGING THE LOCATION OF THE EXISTING UTILITIES, AS SHOWN ON THIS PLAN, ARE APPROXIMATE ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VENIFY ACTUAL LOCATION AND DEPTH OF ALL EXISTING UTILITIES. THE OWNER AND THE SURVEYOR SHALL NOT BE RESPONSIBLE FOR ANY OMISSION OR VARIATION FROM THE LOCATION SHOWN, THE CONTRACTOR SHALL NOTIFY "KENTUCKY 811" AT 1 (800) 752-6007 AT LEAST 2 WORKING DAYS PRIOR TO THE START OF CONSTRUCTION.



APPROVAL

Logencia Market Market Market 1000000000000000000000000000000000000	APPROVAL									
NET SITUATION Description Description 107 SUMME Med Name 117 TITLE SHEET 3-1 SURVEY PLAN 3-2 SURVEY PLAN 3-3 SURVEY PLAN 3-4 SURVEY PLAN 3-5 SURVEY PLAN 3-6 SURVEY PLAN 3-7 SURVEY PLAN <th>ATAT COMPLIANCE</th> <th>Sichuture</th> <th>PHONE NUMBER</th> <th>DATE</th> <th> KY-00-08</th> <th>17A WOLF CF</th> <th>KEEK</th> <th>WV309</th> <th>A CSI V</th> <th>VOLF</th>	ATAT COMPLIANCE	Sichuture	PHONE NUMBER	DATE	KY-00-08	17A WOLF CF	KEEK	WV309	A CSI V	VOLF
Instrument Description Drawling index The sheet The sheet The sheet The sheet Standing index Stand i	ATAT CONSTRUCTION	SIGNATURE	PHONE NUMBER	DATE						
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E-7 COAX COLOR CODING G-1 SITE GRADING PLAN GN-1 GENERAL NOTES NOTES OCONCRETE AND REINFORCING STEEL NOTES SEE PAGE GN-1) APPLICABLE BUILDING CODES AND STANDARDS STEE WORK GENERAL NOTES SEE PAGE GN-1) STEE WORK GENERAL NOTES SEE PAGE GN-1) STEE WORK GENERAL NOTES SEE PAGE GN-1)	E-1 E-2 E-3 E-4 E-5	ELECTRICAL NOTES & DETAIL UTILITY RACK & H-FRAME DET GROUNDING DETAILS GROUNDING DETAILS GROUNDING NOTES & DETAIL	S TAILS S	Switz Privy	s b Warriotk	ang Roos Ironton Rock Camp latwoods Rose Deering Westwood Athland 243 Getaway	Atha	LATITUDE & LONGITUDE BASED ON NAD1983 LATITUDE: 38 ° 22 ' 24.4 LONGITUDE: 83 ° 05 ' 58.0 GROUND ELEV= 850.70' AM PROPOSED RAD CENTER H	45 "N 04 "W ISL	Arthur J. OF Keunger Keunger 25843
NOTES Structure and relation of the structure and	G-1	SITE GRADING PLAN			Oldtown want	Summi Consapeate Bradrick Construction Statement Bradrick Rockdale Catlemburg Meadu Princess 223	AT 10	PROPOSED LEASE AREA: 1 PROPERTY OWNERS:	10,000 SQ. FT.	7/21/08
CONCRETE AND REINFORCING STEEL NOTES APPLICABLE BUILDING CODES AND STANDARDS SITE WORK GENERAL NOTES STRUCTURAL STEEL NOTES GROUNDING NOTES ELECTRICAL INSTALLATION NOTES ELECTRICAL INSTALLATION NOTES ELECTRICAL INSTALLATION NOTES GENERAL NOTES COMPARING REQUIRED E. NOTES STRUCTURAL STALE AND RESIDENT COMPANY OF MEDIANAL DISCOMPTION AND COMPAN		NOTE	S	ater Ha		Rush Shoals Shoals Lavalert	ch Fork La	520 SUTTON ROAD		NOTE: THIS STAMP CERTIFIES THE CIVIL SH ONLY. ARCHITECTURAL, STRUCTURAL, ELECTRI AND/OR MECHANICAL SHEETS ARE NOT CERTIF BY THIS STAMP.
GENERAL NOTES (SEE PAGE GN-1) VICINITY MAP NOT TO SCALE ADDITIONAL PARKING REQUIRED: NONE 752-6007 AT LEAST 2 WORKING DAYS PRIOR TO THE ADDITIONAL PARKING REQUIRED: NONE VICINITY MAP NOT TO SCALE ADDITIONAL PARKING REQUIRED: NONE OF CONSTRUCTION. OF CONSTRUCTION. OF CONSTRUCTION. VICINITY MAP NOT TO SCALE ADDITIONAL PARKING REQUIRED: NONE CENTRAL STATES TOWER, INC. SHEET T-1 VICINITY MAP NOT TO SCALE CENTRAL STATES TOWER, INC. SHEET T-1 VICINITY MAP NOT TO SCALE CENTRAL STATES TOWER, INC. SHEET T-1 VICINITY MAP NOT TO SCALE CENTRAL STATES TOWER, INC. SHEET T-1 VICINITY MAP NOT TO SCALE CENTRAL STATES TOWER, INC. SHEET T-1 VICINITY TO SCALE VV309A / CST WOLF I 07/21/08 REVISED PER NEW AT& TEMPLATE DWGS VICINITY MAP NOT TO SCALE VV309A / CST WOLF I 07/21/08 REVISED PER NEW AT& TEMPLATE DWGS VICINITY MAP NOW WICKLES VV309A / CST WOLF I 07/21/08 REVISED PER NEW AT& TEMPLATE DWGS VICINITY MAP VV309A / CST WOLF I 05/02/08 ISSUED FOR REVIEW III III E SHEET VV309A / CST WOLF VV309A / CST WOLF I 05/02/08 ISSUED FOR REVIEW III AX AIK VV309A / DATE VV309A / CST WOLF I 05/02/08 ISSUED FOR REVIEW III AX AIK VV309A / DATE VV309A / CST WOLF I 05/02/08 ISSUED FOR REVIEW III AX AIK<	APPLICA SITE WO STRUCTI GROUND	ABLE BUILDING CODES AND STA ORK GENERAL NOTES URAL STEEL NOTES DING NOTES	NDARDS (SEE PAGE GN-1 (SEE PAGE GN-1 (SEE PAGE GN-1 (SEE PAGE GN-1 (SEE PAGE E-5)) Gânî) .	ler. Vitiliard Jeriel (2) to the second seco	Boltsfork Burnaugh Boyd Buchanan Bichard Lawrents Wayne		1110-001; BOOK 166, PAGES MAP NO. 062-00-00-012.00. OCCUPANT LOAD: UNOCCUPIED	NOTIFY THE LOC PLAN, AI RESPONS LOCATION AND THE OMISSION	UTILITY COMPANIES BEFORE DIGGING ATION OF THE EXISTING UTILITIES, AS SHOWN ON EE APPROXIMATE ONLY. IT SHALL BE THE IBILITY OF THE CONTRACTOR TO VERIFY ACTUAL I AND DEPTH OF ALL EXISTING UTILITIES. THE O SURVEYOR SHALL NOT BE RESPONSIBLE FOR A O RY VARIATION FROM THE LOCATION SHOWN. THE
Professional Services ONE MADISON AVENUE CADILAC, MI 49601 (231) 775-3135 FAX www.wilcox.us 323 SOUTH HALE STREET WV309A / CST WOLF 1 07/21/08 REVISED PER NEW AT&T TEMPLATE DWGS JLS JLS JLS AJK AJK TITLE SHEET Services VW309A / CST WOLF 1 07/21/08 REVISED PER NEW AT&T TEMPLATE DWGS JLS AJK AJK TITLE SHEET Suffer 100 UVE ATW US AJK AJK MIT SHEET OLIVE HILL KY 41164 DRAWING NUMBER DRAWING NUMBER DRAWING NUMBER)				ADDITIONAL PARKING REQI	UIRED: NONE OF CONS	TRUCTION.
Professional Services ONE MADISON AVENUE Calification venue (231) 775-7755 of FUC (231) 775-3135 FAX www.wildox.us 323 SOUTH HALE STREET WV309A / CST WOLF 1 07/21/08 REMSED PER NEW AT&T TEMPLATE DWGS JLS Aik Aik 1 07/21/08 REMSED PER NEW AT&T TEMPLATE DWGS JLS Aik Aik 0 05/02/08 ISSUED FOR REVIEW JLS Aik Aik 0 DATE REMSIONS BY CHK App'D	Nilcox		11 >	CENTRAL STATE	S TOWER, INC. KY-	00-0817A / WOLF CREEK				SHEET T-1
WHEATON, IL 60187 WHEATON, IL 60187 WHEATON, IL 60187 DLIVE HILL, KY 41164 NO. DATE REVISIONS BY CHK APP'D DRAWING NUMBER	Pr S	OTESSIONAL CADILLAC, MI 49601 (231) 775-7755 OFFICE (231) 775-3135 FAX		SUITE 100		WV309A / CST WOLF				TITLE SHEET
		www.wilcox.us		WHEATON, IL 60187			NO. DATE	REVISIONS	BY CHK APP'D	DRAWING NUMBER





LATITUDE & LONGITUDE LATITUDE AND LONGITUDE OF SITE ARE BASED ON NAD83. LATITUDE: 38°22'24.4469" LONGITUDE: -83"05'58.0371" GROUND ELEVATION AT TOWER BASE: 850.70'

SITE INFORMATION ±520 SUTTON ROAD

PROPERTY OWNER: GARY BOND OLNE HILL, KY 41164 520 SUTTON ROAD OLIVE HILL, KY 41164 GENERAL NOTES

NO PROPOSED MUNICIPAL SEWER OR WATER UTILITIES ARE REQUIRED FOR THIS SITE. THERE WILL BE NO CHANGE IN DRAINAGE PATTERN DUE TO THE PROPOSED INSTALLATION. NO HAZARDOUS MATERIALS WILL BE USED, PROCESSED OR STORED AT THE SITE. TOWER LIGHTING SHALL CONFORM TO FAA STANDARDS AS REQUIRED. ALL WORK SHALL CONFORM TO FAA & FCC REGULATIONS.



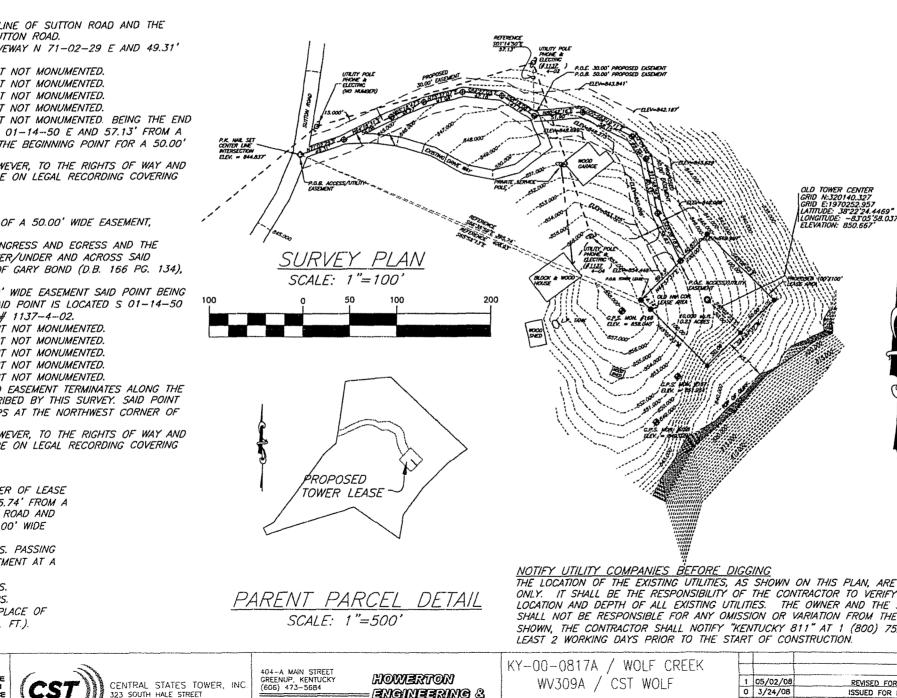
A 3-6-08 MOVED LEASE AREA TO

DESIGNE

NO. DATE REVISIONS

SCALE: AS SHOWN

P.O.R. TOWER



ACCESS/PUBLIC UTILITY EASEMENT DESCRIPTION

<u> 30.00' EASEMENT</u>

BEING A CENTER LINE DESCRIPTION OF 268.92' OF A 30.00' WIDE EASEMENT, 15.00' LEFT AND RIGHT OF CENTER LINE. FOR INGRESS AND EGRESS AND THE INSTALLATION AND MAINTENANCE OF PUBLIC UTILITIES OVER/UNDER AND ACROSS SAID EASEMENT. BEING LOCATED ON THE PROPERTY OF GARY BOND (D.B. 166 PG. 134), 502 SUTTON ROAD, OLIVE HILL, KY.

UNLESS OTHERWISE INDICATED, ANY (IPS) IRON PIN SET REFERRED TO HEREIN, IS A 1/2" DIAMETER REBAR 18-30 INCHES LONG WITH A PLASTIC CAP STAMPED "HOWERTON 2512".

BEGINNING AT A P.K. NAIL SET IN THE CENTER LINE OF SUTTON ROAD AND THE CENTER LINE OF A GRAVEL DRIVEWAY AT 502 SUTTON ROAD. THENCE WITH THE CENTER OF SAID GRAVEL DRIVEWAY N 71-02-29 E AND 49.31

TO A POINT NOT MONIMENTED THENCE, N 64-18-31 E AND 56.06' TO A POINT NOT MONUMENTED. THENCE. N 65-18-47 E AND 38.27' TO A POINT NOT MONUMENTED. THENCE. N 75-37-07 E AND 41.98' TO A POINT NOT MONUMENTED.

THENCE, S 84-27-05 E AND 42.16' TO A POINT NOT MONUMENTED. THENCE, S 58-18-08 E AND 41.14' TO A POINT NOT MONUMENTED. BEING THE END OF SAID 30.00' EASEMENT. SAID POINT BEARS S 01-14-50 E AND 57.13' FROM A UTILITY POLE NUMBER 1137-4-02. AND BEING THE BEGINNING POINT FOR A 50.00' WIDE EASEMENT.

THE ABOVE-DESCRIBED PARCEL IS SUBJECT, HOWEVER, TO THE RIGHTS OF WAY AND PUBLIC UTILITY EASEMENTS, IF ANY, THAT MAY BE ON LEGAL RECORDING COVERING SAID PREMISES.

50.00' EASEMENT

BEING A CENTER LINE DESCRIPTION OF 255.29' OF A 50.00' WIDE EASEMENT,

25.00' LEFT AND RIGHT OF CENTER LINE. FOR INGRESS AND EGRESS AND THE INSTALLATION AND MAINTENANCE OF UTILITIES OVER/UNDER AND ACROSS SAID EASEMENT. BEING LOCATED ON THE PROPERTY OF GARY BOND (D.B. 166 PG. 134), 502 SUTTON ROAD, OLIVE HILL, KY.

BEGINNING AT A POINT AT THE END OF A 30.00' WIDE EASEMENT SAID POINT BEING THE BEGINNING OF A 50.00' WIDE EASEMENT. SAID POINT IS LOCATED S 01-14-50 E AND 57.13' FROM AN EXISTING UTILITY POLE # 1137-4-02.

THENCE, N 85-42-16 E AND 51.90' TO A POINT NOT MONUMENTED.

THENCE, S 63-19-11 E AND 58.15' TO A POINT NOT MONUMENTED.

THENCE, S 32-04-02 E AND 28.08' TO A POINT NOT MONUMENTED.

THENCE, S 11-48-58 E AND 60.33' TO A POINT NOT MONUMENTED.

THENCE, S 25-28-18 E AND 56.83' TO A POINT NOT MONUMENTED.

BEING THE END OF SAID 50.00' EASEMENT. SAID EASEMENT TERMINATES ALONG THE NORTHWEST LINE OF THE LEASE AREA AS DESCRIBED BY THIS SURVEY. SAID POINT BEARS N 44-57-37-E AND 59.13' FROM AN IPS AT THE NORTHWEST CORNER OF SAID I FASE AREA.

THE ABOVE-DESCRIBED PARCEL IS SUBJECT, HOWEVER, TO THE RIGHTS OF WAY AND PUBLIC UTILITY EASEMENTS, IF ANY, THAT MAY BE ON LEGAL RECORDING COVERING SAID PREMISES.

LEASE AREA DESCRIPTION

BEGINNING AT AN IPS AT THE NORTHWEST CORNER OF LEASE ARFA SAID IPS BEARS S 66-35-59-E AND 395.74' FROM A P.K. NAIL SET IN THE INTERSECTION OF SUTTON ROAD AND THE CENTER LINE OF THE ABOVE DESCRIBED 30.00' WIDE EASEMENT.

THENCE, N 44-57-37 E AND 100.00' TO AN IPS. PASSING THE CENTER LINE POINT OF A 50.00' WIDE EASEMENT AT A DISTANCE OF 59.13'.

THENCE, S 45-02-23 E AND 100.00' TO AN IPS. THENCE, S 44-57-37 W AND 100.00' TO AN IPS. THENCE, N 45-02-23 W AND 100.00' TO THE PLACE OF BEGINNING. CONTAINING 0.23 ACRES (10,000 SQ. FT.).

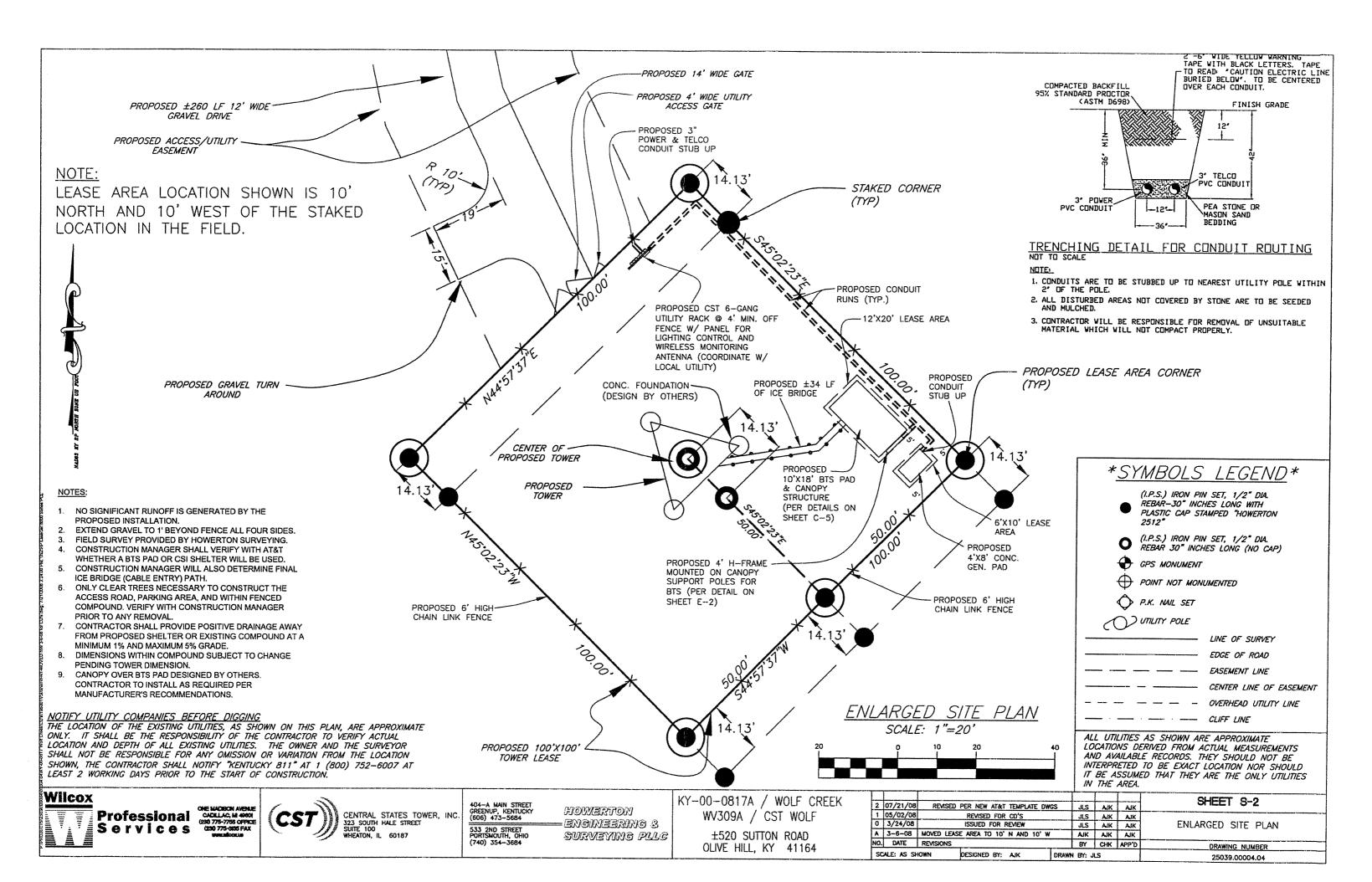


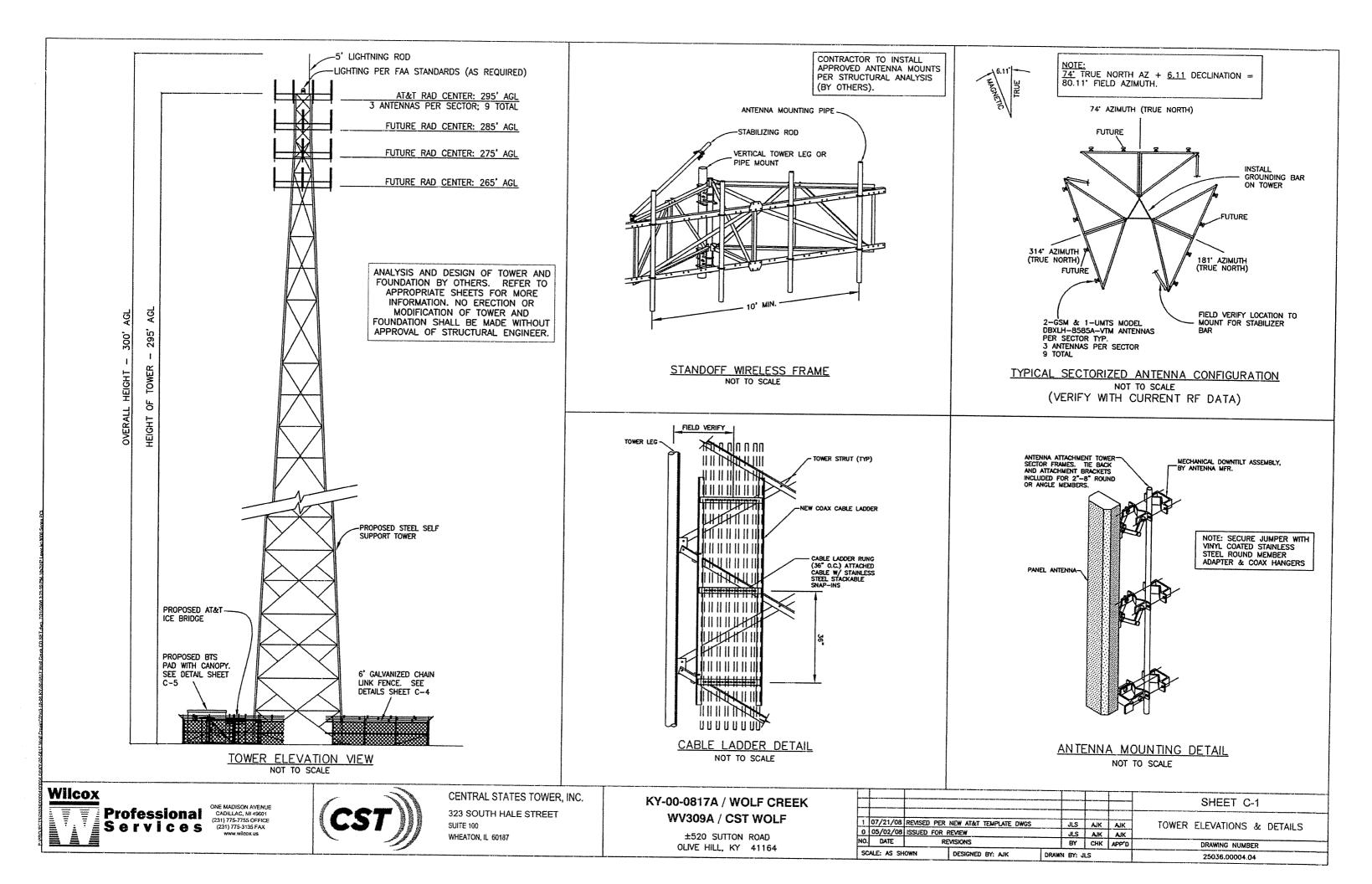


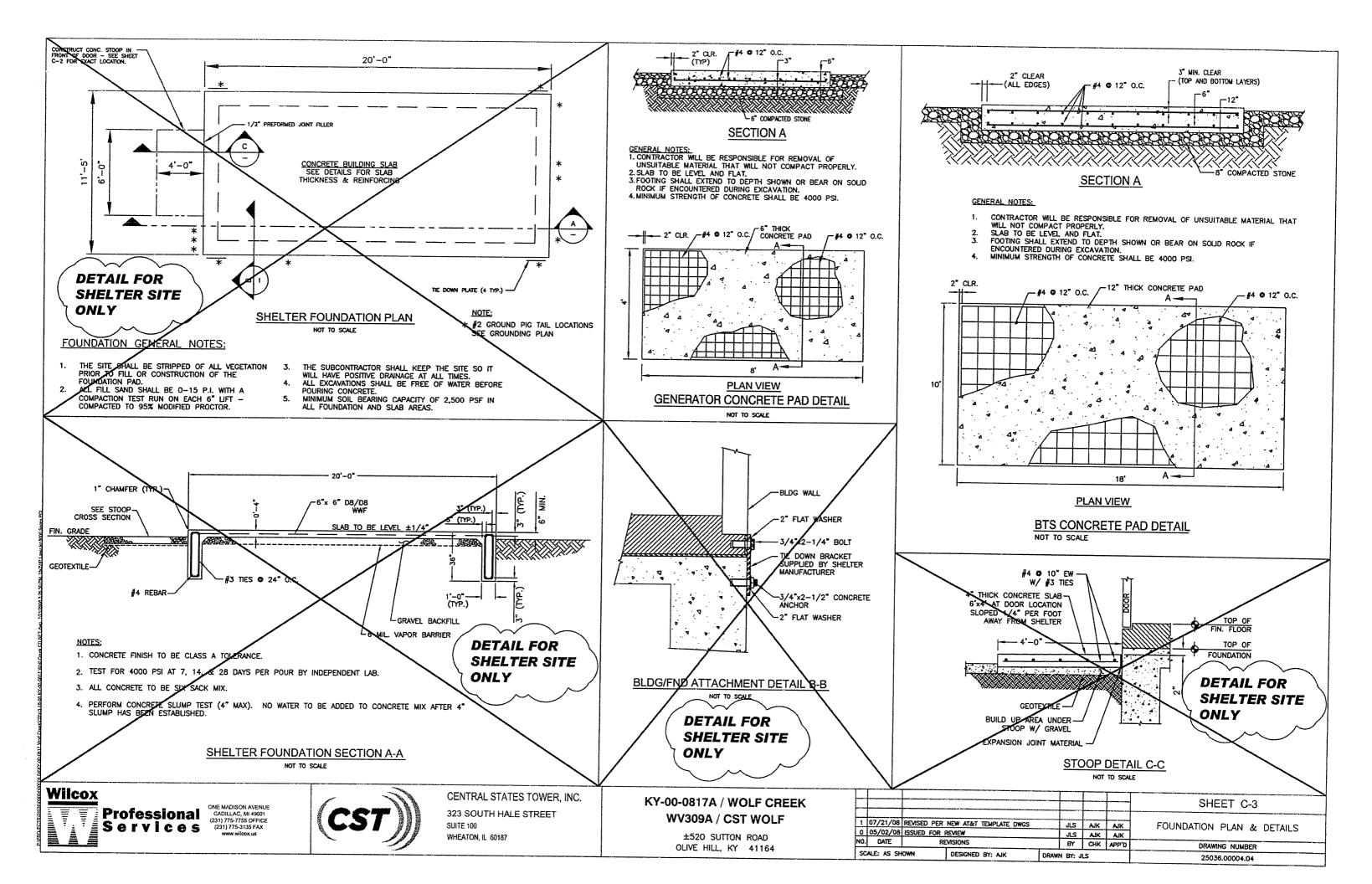
Engineering & 533 2ND STREET PORTSMOUTH, OHIO SURVEYING PLLC (740) 354-3684

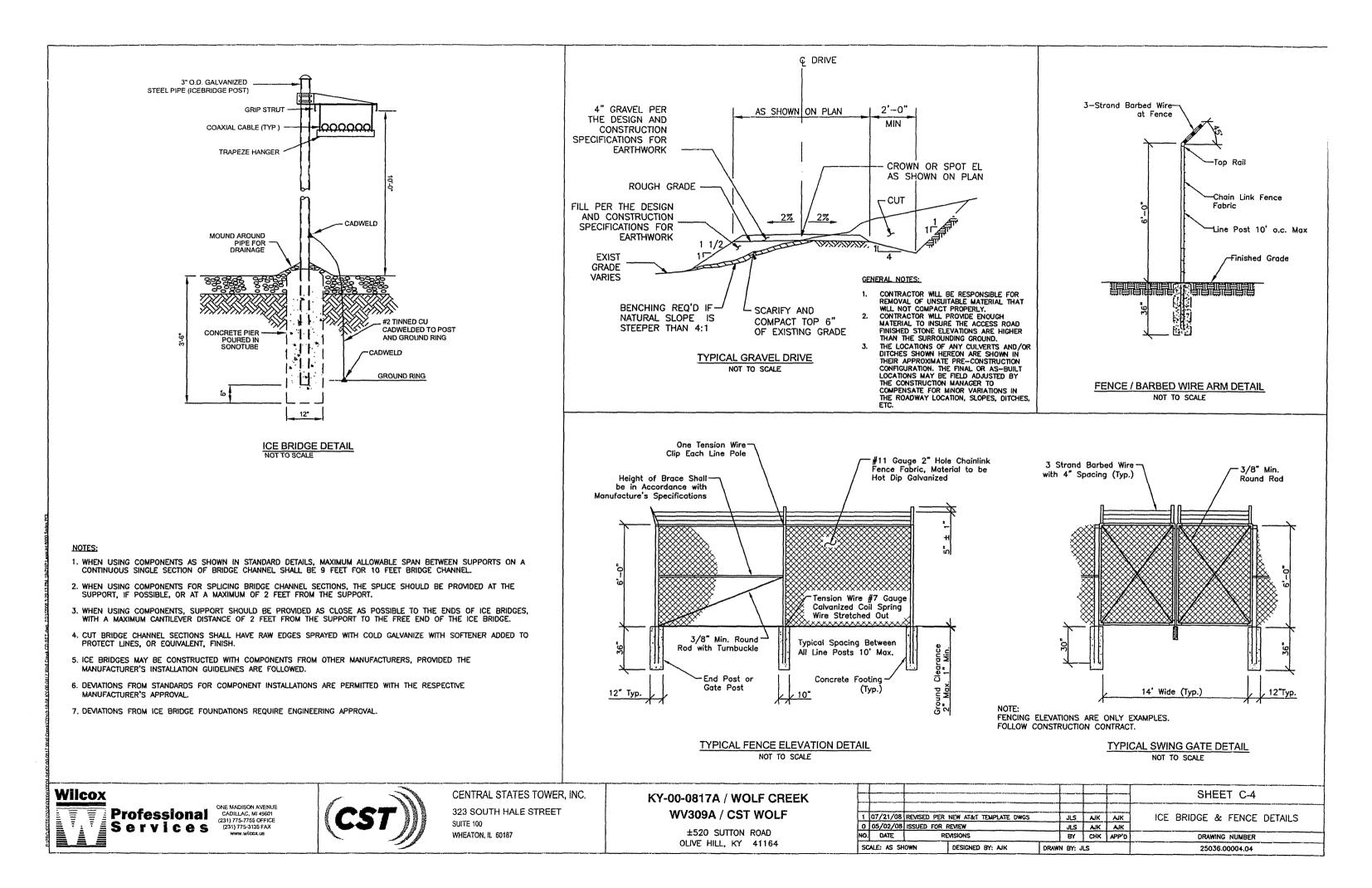
±520 SUTTON ROAD

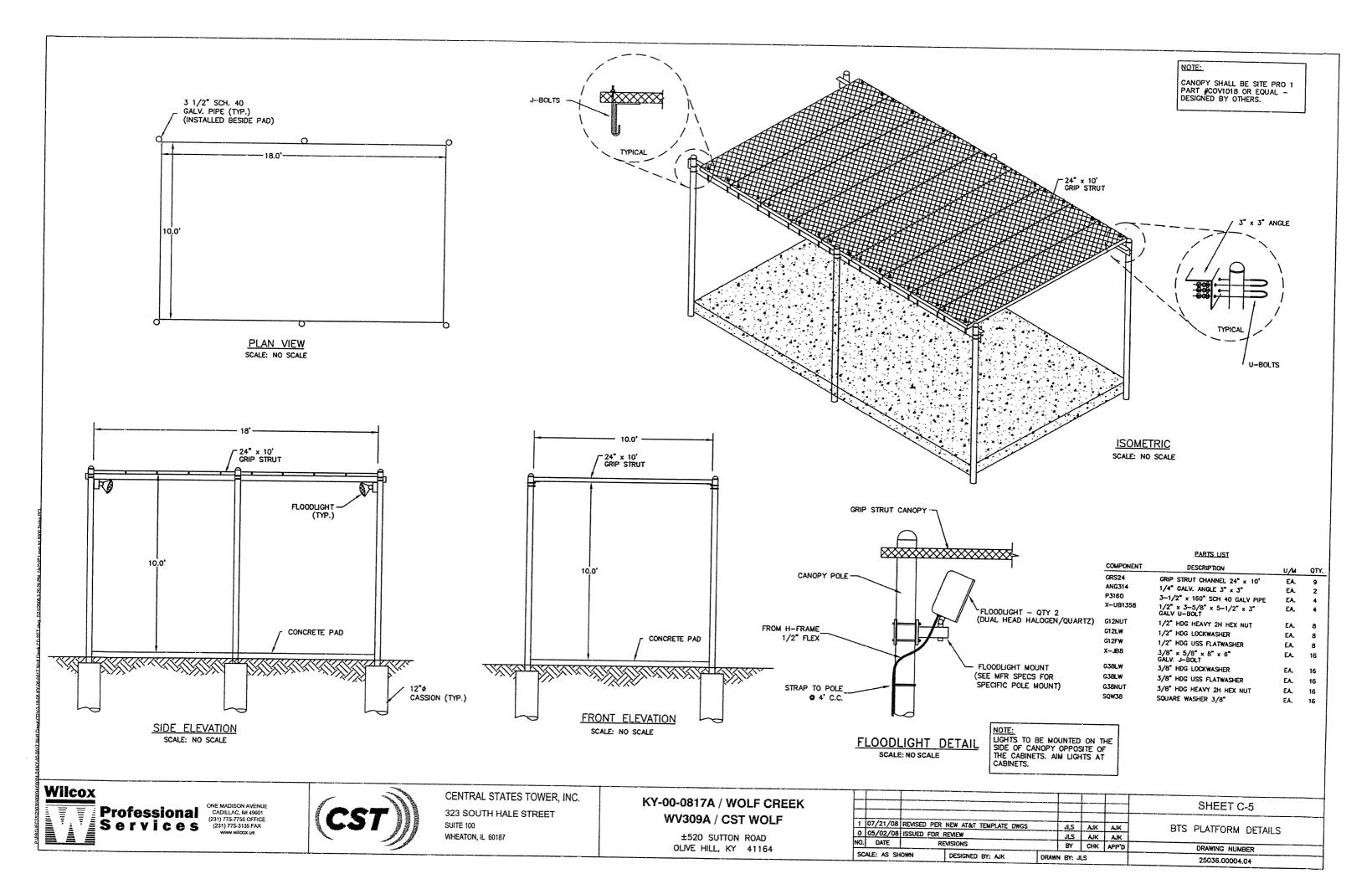
OLIVE HILL, KY 41164

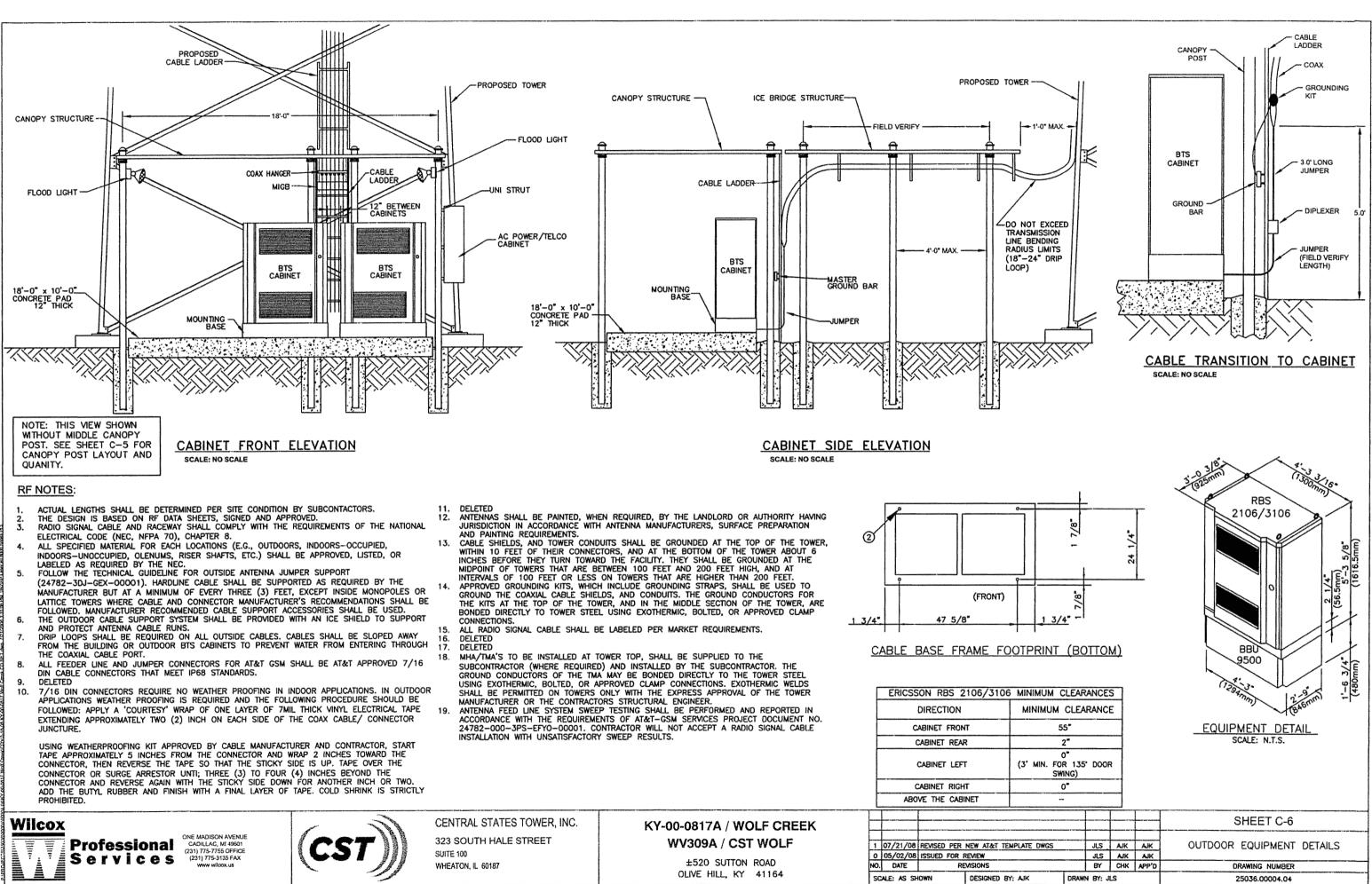












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07/21/08	REVISED PER	NEW AT&T	TEMP
05/02/08	ISSUED FOR F	REVIEW	
DATE	RE	MSIONS	
ALE: AS SH	OWN	DESIGNED	8Y: .

ELECTRICAL INSTALLATION NOTES

1. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE LOCAL CODES.

2. CONDUIT ROUTINGS ARE SCHEMATIC. SUBCONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED.

3. WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC AND TELCORDIA

4. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC AND TELCORDIA.

5. CABLES SHALL NOT BE ROUTED THROUGH LADDER-STYLE CABLE TRAY RUNGS.

6. EACH END OF EVERY POWER, POWER PHASE CONDUCTOR (I.E., HOTS), GROUNDING, AND TI CONDUCTOR AND CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC & OSHA.

7. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS. ALL EQUIPMENT SHALL BE LABELED WITH THEIR VOLTAGE RATING, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING, AND BRANCH CIRCUIT ID NUMBERS (I.E., PANELBOARD AND CIRCUIT ID'S).

8. PANELBOARDS (ID NUMBERS) AND INTERNAL CIRCUIT BREAKERS (CIRCUIT ID NUMBERS) SHALL BE CLEARLY LARFLED WITH ENGRAVED LAMACOID PLASTIC LARELS.

9, ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.

10, POWER, CONTROL, AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE CONDUCTOR (#14 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.

11. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE CONDUCTOR (#6 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN-2 GREEN INSULATION. CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.

12. POWER AND CONTROL WIRING, NOT IN TUBING OR CONDUIT, SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; WITH OUTER JACKET; LISTED OR LABELED FOR THE LOCATION USED, UNLESS OTHERWISE SPECIFIED.

13. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRENUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRENUTS SHALL BE RATED FOR OPERATION AT NO LESS THAN 75°C (90°C IF AVAILABLE).

14. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA. UL, ANSI/IEEE, AND NEC.

15. ELECTRICAL METALLIC TUBING (EMT) OR RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40, OR RIGID PVC SCHEDULE 80 FOR LOCATIONS SUBJECT TO PHYSICAL DAMAGE) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

16. ELECTRICAL METALLIC TUBING (EMT), ELECTRICAL NONMETALLIC TUBING (ENT), OR RIGID NONMETALLIC CONDUIT (RIGID PVC, SCHEDULE 40) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.

17. GALVANIZED STEEL INTERMEDIATE METALLIC CONDUIT (IMC) SHALL BE USED FOR OUTDOOR LOCATIONS ABOVE GRADE.

18. RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80) SHALL BE USED UNDERGROUND; DIRECT BURIED, IN AREAS OF OCCASIONAL LIGHT VEHICLE TRAFFIC OR ENCASED IN REINFORCED CONCRETE IN AREAS OF HEAVY VEHICLE TRAFFIC.

19. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.

20. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SETSCREW FITTINGS ARE NOT ACCEPTABLE.

21. CABINETS, BOXES, AND WIREWAYS SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE, AND NEC.

22. WIREWAYS SHALL BE EPOXY-COATED (GRAY) AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARD; SHALL BE PANDUIT TYPE E (OR EQUAL); AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.







CENTRAL STATES TOWER, INC.

323 SOUTH HALE STREET SUITE 100 WHEATON, IL 60187

ELECTRICAL INSTALLATION NOTES (cont.)

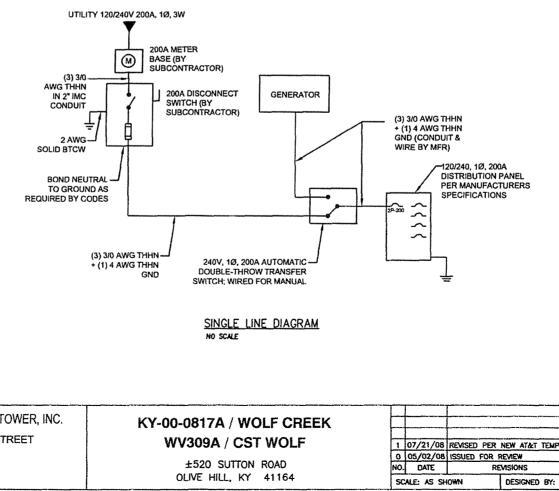
23. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES, AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL, SHALL MEET OR EXCEED UL 50, AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS

24. METAL RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED, OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.

25. NONMETALLIC RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.

26. THE SUBCONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CONTRACTOR BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.

27. THE SUBCONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD AGAINST LIFE AND PROPERTY.



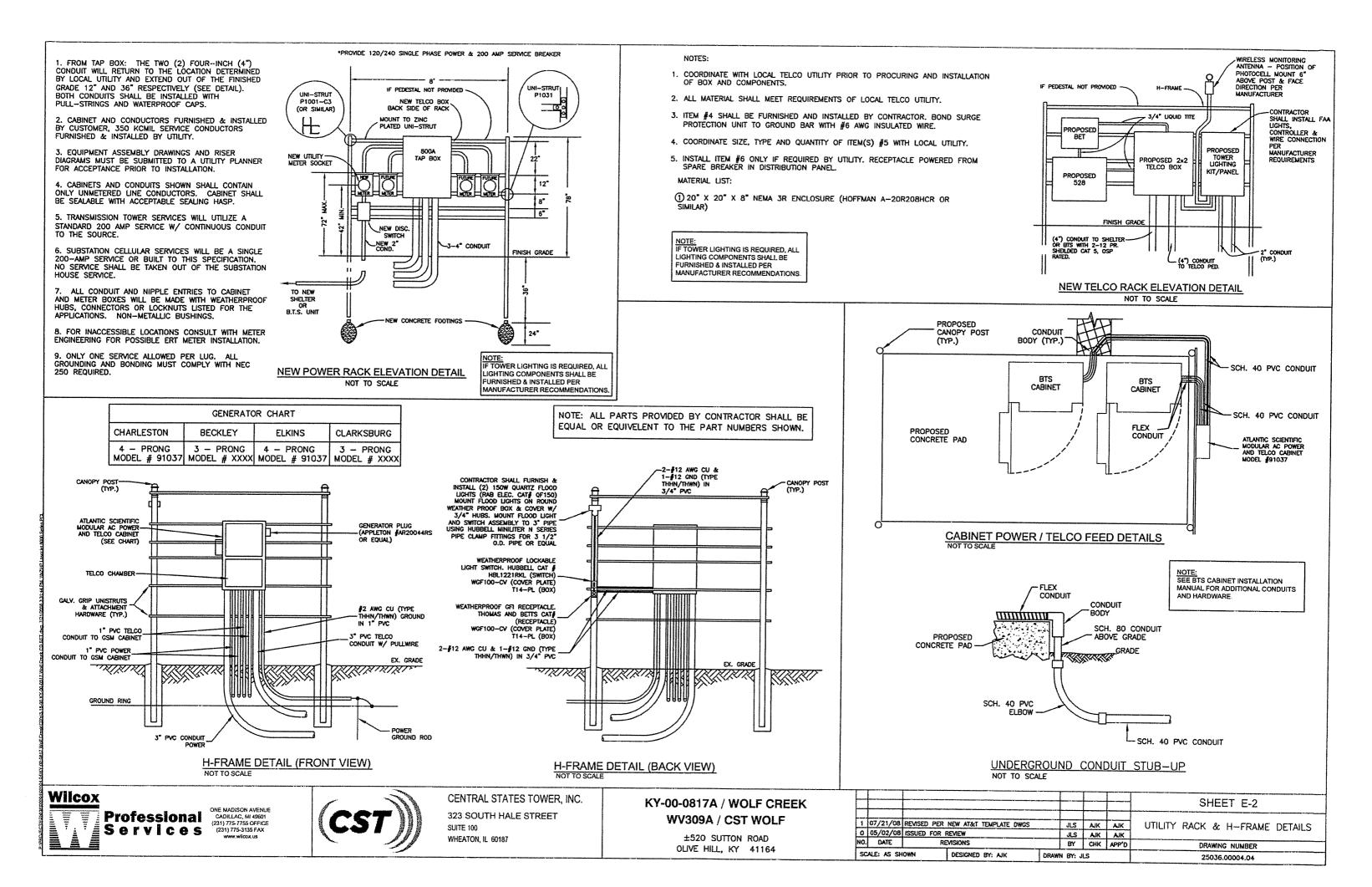
SYMBOLS	ABBREVIATIONS
SOLID GROUND BUS BAR	AGL ABOVE GRADE LEVEL
SOLID NEUTRAL BUS BAR	BTS BASE TRANSCEIVER STATION
SUPPLEMENTAL GROUND CONDUCTOR	(E) EXISTING MIN MINIMUM
2-POLE THERMAL-MAGNETIC CIRCUIT BREAKER	N.T.S. NOT TO SCALE REF REFERENCE
SINGLE-POLE THERMAL-MAGNETIC CIRCUIT BREAKER	RF RADIO FREQUENCY T.B.D. TO BE DETERMINED
CHEMICAL GROUND ROD	T.B.R. TO BE RESOLVED
GROUND ROD	TYP TYPICAL
DISCONNECT SWITCH	REQ REQUIRED
	EGR EQUIPMENT GROUND RING
METER	AWG AMERICAN WIRE GAUGE
CADWELD TYPE CONNECTION	MGB MASTER GROUND BUS
COMPRESSION TYPE CONNECTION	EG EQUIPMENT GROUND
GROUNDING WIRE	BCW BARE COPPER WIRE
	SIAD SMART INTEGRATED ACCESS DEVICE
	GEN GENERATOR
	IGR INTERIOR GROUND RING (HALO)
	RBS RADIO BASE STATION

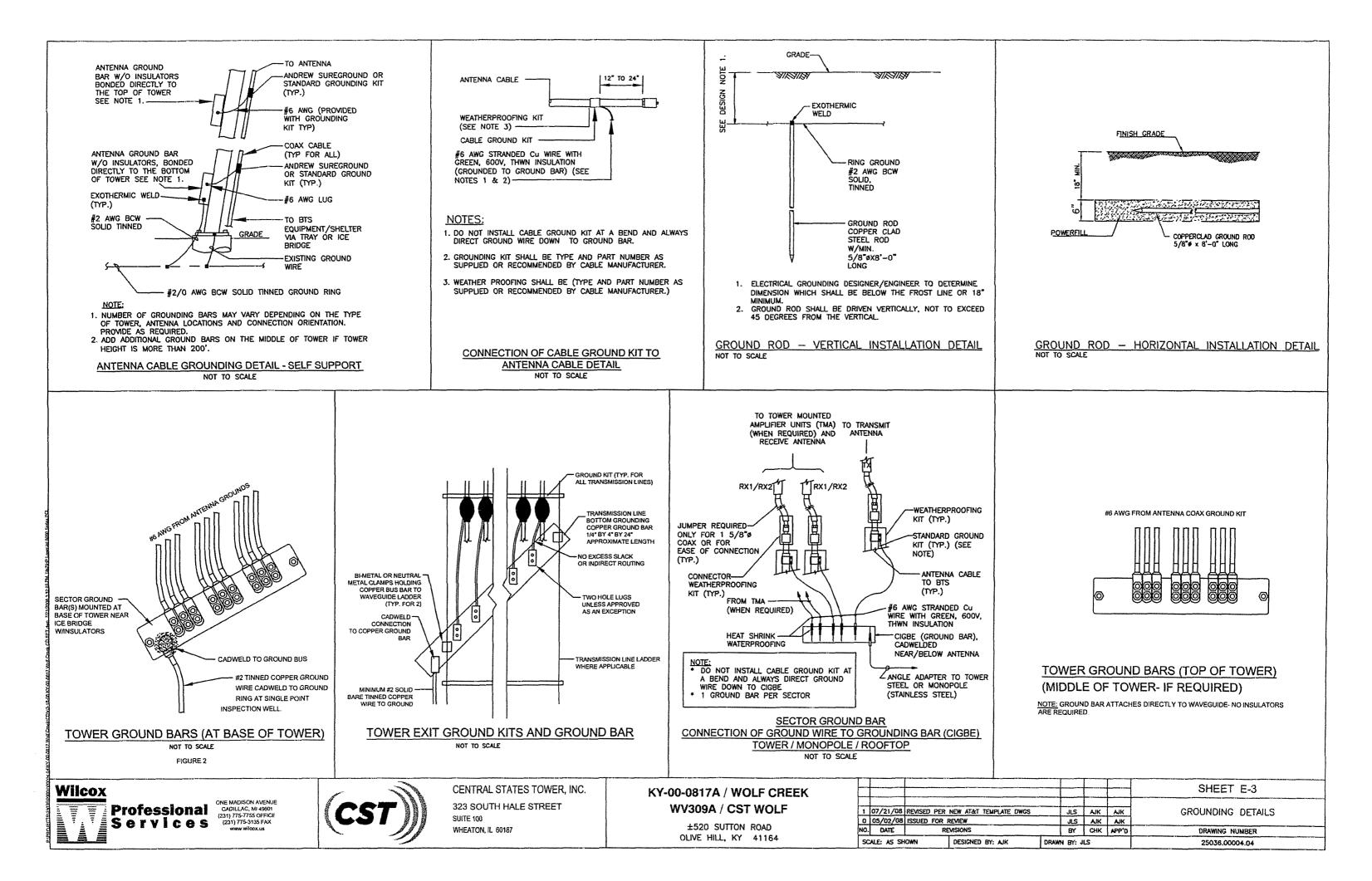
ABBREVIATIONS & SYMBOLS

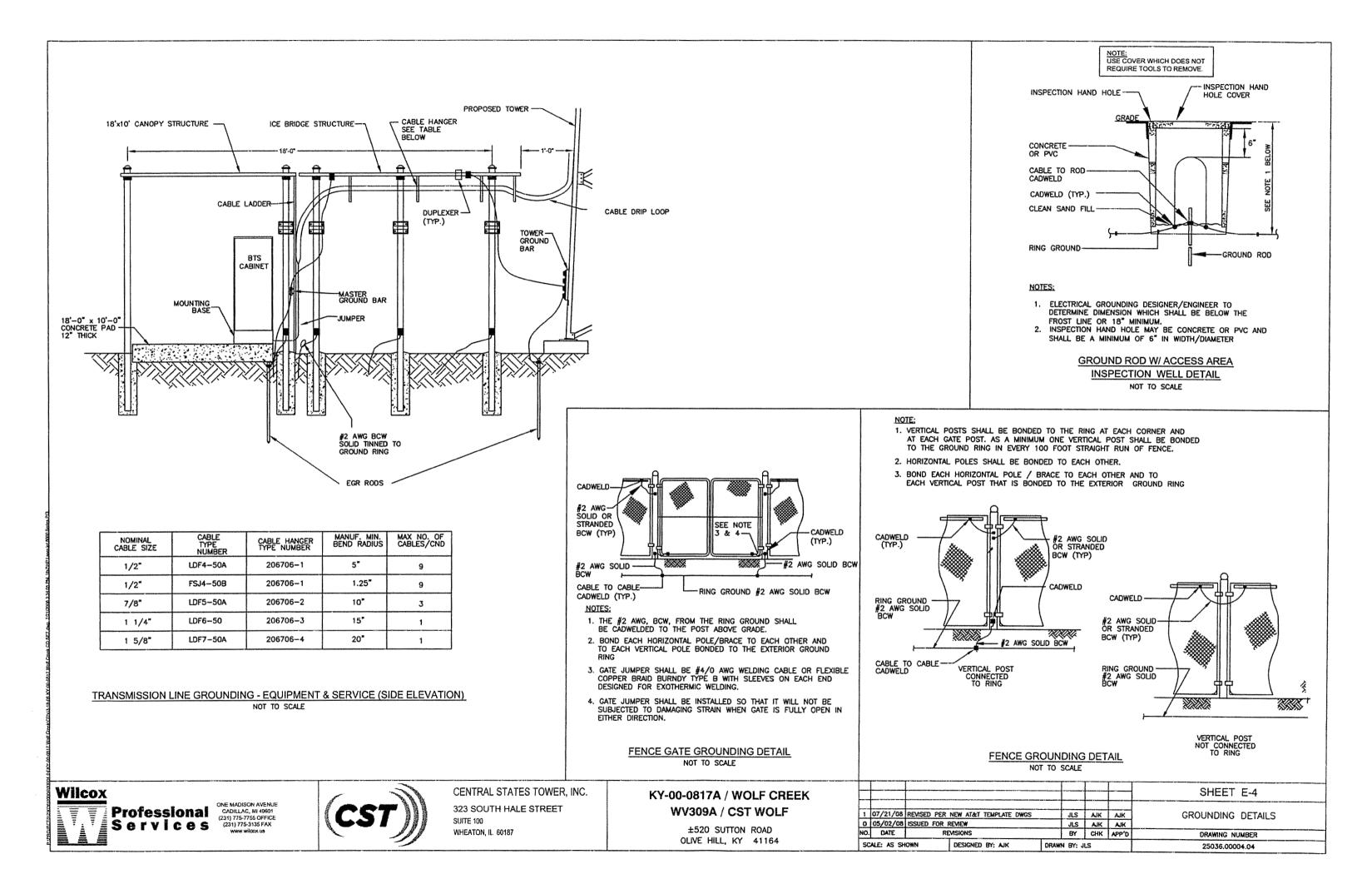
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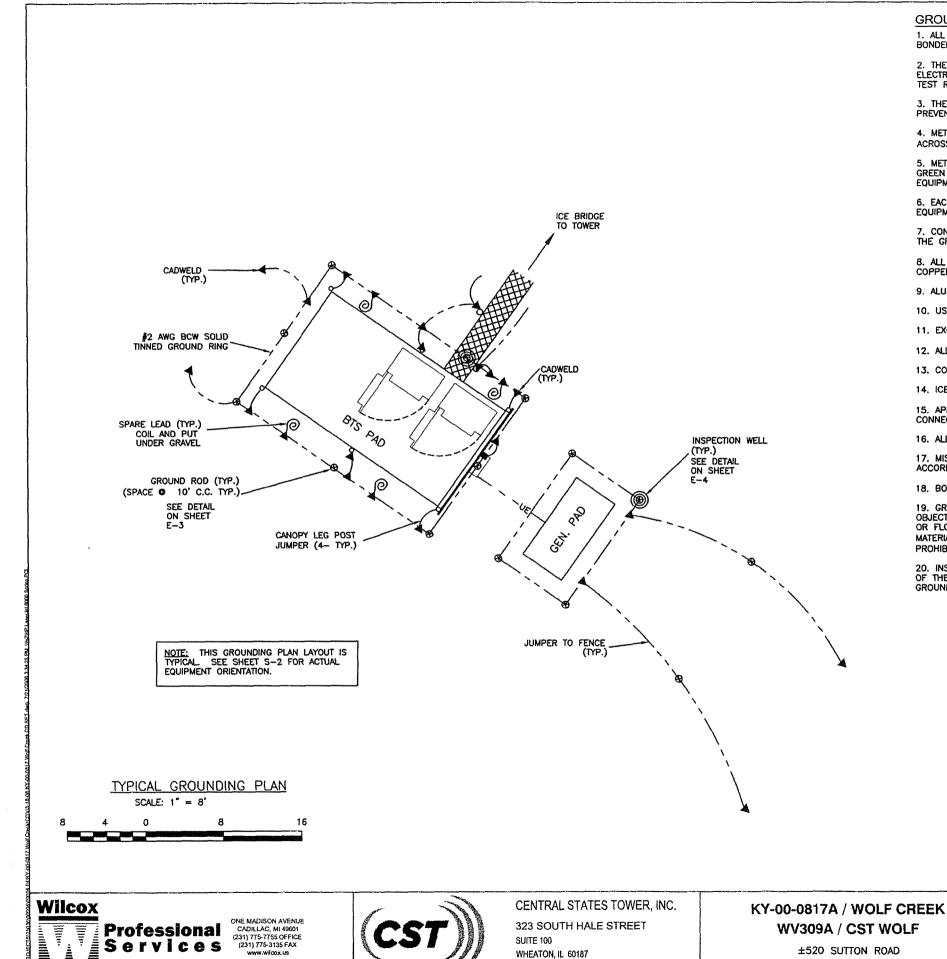
- 1. SUBCONTRACTOR SHALL PROVIDE 200AMP, SINGLE PHASE, 120/240 VAC, 60HZ SERVICE FOR SITE.
- 2. SUBCONTRACTOR SHALL COORDINATE WITH UTILITY COMPANY BEFORE THE START OF CONSTRUCTION. POWER AND TELEPHONE CONDUIT SHALL BE PROVIDED AND INSTALLED PER UTILITY REQUIREMENTS.
- 3. FOR COMPLETE INTERNAL WIRING AND ARRANGEMENT REFER TO DRAWINGS PROVIDED BY PANEL MANUFACTURER.
- 4. SUBCONTRACTOR SHALL PROVIDE ELECTRICAL SERVICE EQUIPMENT WITH FAULT CURRENT RATINGS GREATER THAN THE AVAILABLE FAULT CURRENT FROM THE POWER UTILITY.

					SHEET E-1						
IPLATE DWGS		JLS	AJK	AJK	ELECTRICAL NOTES & DETAILS						
		JLS BY	AJK CHK	AJK APP'D	DRAWING NUMBER						
: AIK	DRAW	N BY: J	us		25036.00004.04						









GROUNDING NOTES

1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER. AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.

2. THE SUBCONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS. THE SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 10 OHMS OR LESS.

3. THE SUBCONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT.

4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 AWG COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.

5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION. SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS FOUIPMENT.

6. EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, 6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS; 2 AWG STRANDED COPPER FOR OUTDOOR BTS.

THE GROUND BUS ARE PERMITTED.

COPPER UNLESS OTHERWISE INDICATED.

9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.

10. USE 45' BENDS MAXIMUM. USE OF 90' BENDS IN THE PROTECTION GROUNDING CONDUCTORS IS PROHIBITED.

11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.

12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR & EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.

13. COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.

14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.

15. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS

16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.

ACCORDANCE WITH THE NEC.

OLIVE HILL, KY 41164

18. BOND ALL METALLIC OBJECTS WITHIN 6 FT OF MAIN GROUND WIRES WITH 1-#2 AWG TIN-PLATED COPPER GROUND CONDUCTOR.

19. GROUND CONDUCTORS USED IN THE FACILITY GROUND AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC PLASTIC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (E.G., NON-METALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.

20. INSTALL GROUND RODS FOR THE NEW GROUND RING. SEPARATION BETWEEN GROUND RODS SHOULD NOT BE LESS THAN THE LENGTH OF THE RODS NOR BE MORE THAN ONE AND ONE-HALF TIMES THE LENGTH OF THE RODS. CONNECT THE NEW RING TO EXISTING GROUND RING IN AT LEAST TWO SEPARATE PLACES.

<u> </u>								SHEET E-5		
			a Maren 1 18a gart dagan gilaktikar na mangan ya ya kanan na ana manan na manangan na manangan na manangan na m							
	1 07/21/08 REVISED PER NEW AT&T TEMPLATE DWGS					AJK	AJK	GROUNDING NOTES & DETAILS		
0	05/02/08	ISSUED FOR F	REVIEW	JL	<u>s</u> [AJK .	AJK			
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7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED. BACK TO BACK CONNECTIONS ON OPPOSITE SIDES OF

8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING, SHALL BE #2 AWG SOLID TINNED

17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN

										850			
ΑΤ&Τ Μ	/ireless RF Site	Build Specifica	tion		NEW TOP				ليحسد	6' T	ſP		
	WV WV309A Cand				CABLE (TYP)								
	-Site Build Status									[
TTVDate:		Final Co.	2C: On Air:					П					
Cingular Ranking: Primary		CD:					DUALBAND TMA W/850 ENERGIZED		DUALI TIMA W ENERG	3AND 1/850		D RAR	
Struct. Height (ft): 300			pe: Build to Suite	(NI		W COAXIAL -		3, LU 7		ζ /	(1 EAC (NEAR	d bar 2H sector) Antennas) Etail	
Lat: 38 22 24.2 Lon: 83 5	57.6 Source: NAD8	3 Structure Own	er: Central State Towers	LEN	GTH GREATER	¥	÷		ľ		365 06		
Mounting Height/ Antenna Spacing Distance	Sector1 (Alpha)	Sector2 (Beta)	Sector3 (Gamma)	COAX	GROUND BAR		÷					ng kit (tyf	ρ
1. Rad Center	295 ft.	295 ft.	295 ft.	1	MOUNTED ON EBRIDE POST		÷ 1			$\rightarrow /$	(BOT SEE	TTOM OF TO	ų
Spacial Diversity Vertical	10 ft.	10 ft.	10 ft.		SEE DETAIL	<u></u>	<u> </u>						
Horizontal	10 ft.	0 ft.	<u>10</u> ft.		T	T	· 7			J			
Antenna/ Cable Specification					L			3' TY	P				
1. Azimuth Orientation	74 deg.	181 deg.	314 deg.							1			
2. # of Ant/ Ant Model GSM	2 DBXLH-8585A-VTM	2 DBXLH-8585A-VTM	2 DBXLH-8585A-VTM				DIPLEXER		DIPLE	XER			
# of Ant/ Ant Model UMTS	1 DBXLH-8585A-VTM	1 DBXLH-8585A-VTM	1 DBXLH-8585A-VTM						िल्ला				
CONFIGURATION												etermined H (+12')	
Manufacturer / Polarization	Andrew / Dual +/-45	Andrew / Dual +/-45	Andrew / Dual +/-45										
Antenna Gain / Fixed Elec. DT	13.3 / 0 87 / 16	13.3 / 0 87 / 16	13.3 / 0 87 / 16		ERICSSON 2106 BTS			5	and the set of the later				
Horiz / Vert 3dB Beamwidth Dimensions L x W x H	48.3" x 12" x 7"	48.3" x 12" x 7"	48.3" x 12" x 7"		RX	1/TX1 RX2		8 5 8		FROM GSM ()UGH 10db (COUPLER		
3. Downtilt: (total)	GSM (2) UMTS (2) deg	GSM (2) UMTS (2) deg	GSM (2) UMTS (2) deg			UMTS 850 NODEB	R/XT R/XT	TX/RX2 TX/RX1 TX/RX2		۲ ۲	9		
Mechanical		0 0 deg	0 0 deg			(DUPLEXED)	850 19	00		Ţ	Ţ		
Remote Electrical	2 2 deg	2 2 deg	2 2 deg				850 19 GSM GS	SM	Ιſ	RX1 LMU	RX2		
4. Main Cable Diameter	AVA7-50 - 1 5/8"	AVA7-50 - 1 5/8"	AVA7-50 - 1 5/8"		FRICS	SON 3106 BTS	ERICSSON	2106 8] L]		
Number Estimated Length / Loss dB	6 315 ft. / 3.213	6 315 ft. / 3.213	6 315 ft. / 3.213			- ANTENNAS				GURATIC	<u>N</u>		
Bending Radius / Weight	15 in / 0.72 lbs/ft	15 in / 0.72 lbs/ft	15 in / 0.72 lbs/ft				NOT TO S						_
5. Top Jmpr Length / Size	6' /1/2"LDF4P	6' / 1/2"LDF4P	6' / 1/2"LDF4P				ANTENNA DEC.	1	ANT	ENNA A			-
6. Bottom Jmpr Length / Size	6' /1/2*SF	6' / <u>1/2"SF</u>	6' / <u>1/2"SF</u>		TAGS SECTOR		ANTENNA ELEC. SERIAL DOWN NUMBER TILT	MECH DOWN TILT	AZIMUTH	RAD CTR 11	P HEIGHT	CABLE CA	a
7. RBS Type	Ericsson 2206	Ericsson 2206	Ericsson 2205		A1 1	DBXLH8585A-VTM (51.4"x15.4"x5")	z	σ	74°	295'	297*	300 AV/	Ň
8. # of TMA/ TMA Type	2 KRY 112 75/1	2 KRY 112 75/1	2 KRY 112 75/1		A2 1 A3 -	08XLH-8585A-VTM (51.4"x15.4"x5")	<i>T</i>	σ -	74*	295'	297	300 AV/	Ŧ.
9. # of GMA/ GMA Type 10. # of Diplexer/Diplexer Type	2 LGP 21903	2 LGP 21903	2 LGP 21903		A4 1	DBXLH-8585A-VTM	- 2	- 0	74	295'	297	AVA	
					81 2	(51.4"x15.4"x5") DEXLH-8585A-VTM (51.4"x15.4"x5")	Z	σ	181*	295'	297*	300 AVA	
Comments/Rev. Notes This is the optimum set for configuration.	We will need two antennas for	GSM1900/GSM850 and One an	tenna for UMTS. Please see		82 2	08XLH-8585A-VTM (51.4*x15.4*x5*)	z	a	181*	295'	297*	300 AV/	
the plumbing diagram Antenna Config_W	V_20 for visual.				B3 -		-	-	-	-	-	- 69/	
Release Signature	Date	RF Engineer	Chris La Tendresse		B4 2	DBXLH-8585A-VTM (51.4"x15.4"x5") DBXLH-8585A-VTM	2	a	181*	295'	297	300 AV/	£
					C1 3 C2 3	(51.4"x15.4"x5") D8XLH8585AVTM	2	σ	314	295'	297*	300 AVA	
4/11/2008 10:30:44 AM			Prepared by: WFI		C2 3	(51.4"x15.4"x5")	Z	- -	314	295'	297'	- AV/	
TATENO DININA NA					C4 3	DBXLH-8585A-VTM (51.4"x15.4"x5")	Z	a	314"	295'	297'	300 AVA	
									<u></u>				_
					<u></u>	N.T.S.				COLO	K COL	DE DET	-
Nilcox		1/ 7	CENTRAL STATES	TOWER, INC.	KY-00-08	17A / WOLF	CREEK	F F	_				•
Professional	ONE MADISON AVENUE CADILLAC, MI 49601 (231) 775-7755 OFFICE	CCT	323 SOUTH HALES	STREET	WV3	09A / CST W	DLF			/08 REVISED			7
Professional Services	(231) 775-3135 FAX www.wilcox.us		SUITE 100 WHEATON, IL 60187		1	20 SUTTON ROA			0 05/02 NO. DAT	E ISSUED	FOR REVI		-
			and the second		OLIV	E HILL, KY 411	64	- F	SCALE: A		DE	ESIGNED BY:	7

NOTES: 1. ALL MATERIALS SHALL BE PROVIDED BY THE CONTRACTOR TO THE SUBCONTRACTOR FOR INSTALLATION.

2. SUBCONTRACTOR SHALL DOCUMENT AS-BUILT CABLE LENGTHS AND PROVIDE ANTENNA SERIAL NUMBERS ON RED-LINED DRAWINGS.

3. ANTENNAS SHALL BE PROCURED AND INSTALLED WITH DOWNTILT BRACKETS AND HEAVY DUTY CLAMPS SUPPLIED BY ANTENNA MANUFACTURER.

4. FOLLOW DETAIL FOR AT&T COAX COLOR CODING.

5. COAX GROUND KITS, COAX WEATHER PROOFING, SNAP-IN HANGER CLAMPS AND HOISTING GRIPS SHALL BE PROVIDED BY THE CONTRACTOR TO THE SUBCONTRACTOR FOR INSTALLATION.

6. RF DATA IS TO BE VERIFIED BY CONTRACTOR WITH AT&T PRIOR TO CONSTRUCTION.

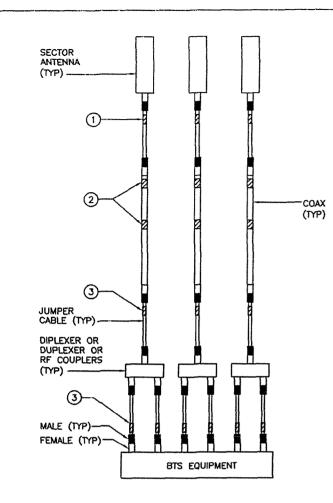
7. ALL JUMPERS FROM TMA AND GROUND KITS TO BE SUPPORTED TO TOWER WITH STAINLESS STEEL BRACKETS OR STRAPS.

CABLE	SCHEDU	JLE				
xaaxaal cable	top Jumper	BOTTOM JUMPER	COLOR CODE	TMA TYPE	DIPLEXER	DC BLOCK
ANDREWS 47-50 1-5/8	(6') 1/2# LDF4P	(6') 1/2# SF	1 GREEN STRIPE	KRY112 75/1	LGP 21903	N
ANDREWS 47-50 1-5/8"	(6') 1/2# LDF4P	(6') 1/26 SF	2 GREEN STRIPES	KRY112 75/1	LGP 21903	N
ANDREWS A7-50 1-5/8"	(6') 1/29 LDF4P	(6') 1/2¢ SF	3 GREEN STRIPES	-	-	N
ANDREWS A7-50 1-5/8	(6') 1/2# LDF4P	(6') 1/2# SF	4 GREEN STRIPES	KRY112 75/1	LGP 21903	N
ANDREWS A7-50 1-5/8	(6') 1/2# LDF4P	(6') 1/2• SF	1 BLUE STRIPE	KRY112 75/1	LGP 21903	N
ANDREWS 47-50 1-5/87	(6') 1/2¢ LDF4P	(6') 1/20 SF	2 BLUE STRIPES	KRY112 75/1	LGP 21903	N
ANDREWS	(6') 1/2# LDF4P	(6') 1/2# SF	3 BLUE STRIPES	-	-	N
ANDREWS 47-50 1-5/8	(6') 1/2# LDF4P	(6') 1/2# SF	4 BLUE STRIPES	KRY112 75/1	LGP 21903	N
ANDREWS A7-50 1-5/8"	(5') 1/2¢ LDF4P	(6') 1/24 SF	1 WHITE STRIPE	KRY112 75/1	LGP 21903	N
ANDREWS 47-50 1-5/8	(6') 1/24 LDF4P	(6') 1/2¢ SF	2 WHITE STRIPES	KRY112 75/1	LGP 21903	N
ANDREWS 47-50 1-5/8	(6') 1/26 LDF4P	(6') 1/2# SF	3 WHITE STRIPES		+	N
ANDREWS A7-50 1-5/8"	(6') 1/20 LDF4P	(6') 1/2 * SF	4 WHITE STRIPES	KRY112 75/1	LGP 21903	N

TAILS	&	NOT	ES				2
						SHEET E-6	
PLATE DWO	s		JŁS	AJK	ajk	RF CONFIG. & ANTENNA SCHEMATIC	cs
			JLS BY	AJK	AJK APP'D	DRAWING NUMBER	
: AJK		DRAW	N BY: J	เร		25036.00004.04	

KIT (TYP)

BAR OF TOWER)



CABLE MARKING LOCATIONS DIAGRAM

ALL RF CABLE SHALL BE MARKED AS PER CABLE MARKING LOCATIONS TABLE BELOW:

	CA	ABLE	MARKING LOCATIONS TABLE
NO.	TAPE	TAG	LOCATIONS
1.	x		EACH TOP-JUMPER SHALL BE COLOR CODED WITH (1) SET OF 3" WIDE BANDS.
2.	x		EACH MAIN COAX SHALL BE COLOR CODED WITH (1) SET OF 3" WIDE BANDS NEAR THE TOP-JUMPER CONNECTION AND WITH (1) SET OF 3/4" WIDE COLOR BANDS JUST PRIOR TO ENTERING THE BTS OR TRANSMITTER BUILDING.
3.	x		ALL BOTTOM JUMPERS SHALL BE COLOR CODED WITH (1) SET OF $3/4^{\circ}$ WIDE BANDS ON EACH END OF THE BOTTOM JUMPER.
4.	·	*	ALL BOTTOM JUMPERS SHALL BE COLOR CODED WITH (1) SET OF 3/4" WIDE BANDS ON EACH END OF THE BOTTOM JUMPER.

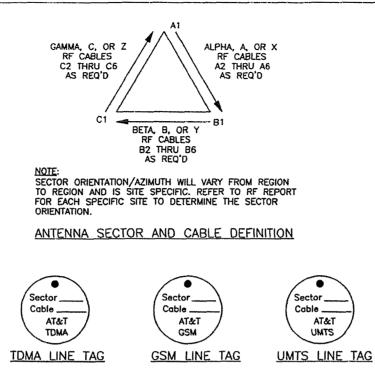
(* - DENOTES TAG OR TAPE.)

ONE MADISON AVENUE CADILLAC, MI 49601 (231) 775-7755 OFFICE

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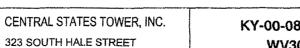
TO PROVIDE ADDITIONAL IDENTIFICATION EACH RF CABLE SHALL BE IDENTIFIED WITH A METAL TAG MADE OF STAINLESS STEEL OR BRASS AND STAMPED WITH THE SECTOR, CABLE NUMBER, AND "AT&T" TO IDENTIFY AT&T MOBILITY CABLES. THE ID MARKING LOCATIONS SHOULD BE AS PER "CABLE MARKING LOCATIONS TABLE". THE TAG SHOULD BE ATTACHED WITH CORROSION PROOF WIRE AROUND THE CABLE. PEFERRED TAG LABELING SHOULD BE AS SHOWN ABOVE "TOMA LINE TAG", "GSM LINE TAG" AND "UMTS LINE TAG".

CABLE MARKING TAGS

			FSA	3 – CABLE	M	ARKING C	OLOR CONVER	TION TABLE
	850 TDMA/GSM	1900 TDMA/GSM	850 TDMA/GSM	1900 TDMA/GSM			850 UMTS (FUTURE)	1900 UMTS
SECTOR ALPHA, A.	CABLE A1	CABLE A2	CABLE A3	CABLE A4			CABLE UMTS1	CABLE UMTS2
OR X	ONE (1) 3/4" GRN	TWO (2) 3/4" GRN	THREE (3) 3/4" GRN	FOUR (4) 3/4" GRN		SECTOR	ONE (1) 1 1/2" GRN	TWO (2) 1 1/2" GRN
SECTOR BETA, B.	CABLE B1	CABLE B2	CABLE B3	CABLE B4		ALPHA, A, OR X	3/4" ORG	3/4" ORG
OR Y	ONE (1) 3/4" BLUE	TWO (2) 3/4" BLUE	THREE (3) 3/4° BLUE	FOUR (4) 3/4" BLUE		SECTOR	ONE (1)	TWO (2)
SECTOR GAMMA, C.	CABLE C1	CABLE C2	CABLE C3	CABLE C4		BETA, B, OR Y	1 1/2" BLUE 3/4" ORG	1 1/2" BLUE 3/4" ORG
OR Z	ONE (1) 3/4" WHT	TWO (2) 3/4" WHT	THREE (3) 3/4" WHT	FOUR (4) 3/4" WHT		SECTOR	ONE (1)	TWO (2)
SECTOR DELTA, D.	CABLE D1	CABLE D2	CABLE 03	CABLE D4		GAMMA, C, OR Z	1 1/2" WHT 3/4" ORG	1 1/2" WHT 3/4" ORG
OR W	ONE (1) 3/4" RED	TWO (2) 3/4" RED	THREE (3) 3/4" RED	FOUR (4) 3/4" RED			ONE (1)	TWO (2)
						SECTOR DELTA, D, OR W	1 1/2" RED 3/4" ORG	1 1/2" RED 3/4" ORG
MICROWA	AVF I YFI	LOW						

SUITE 100

WHEATON, IL 60187



KY-00-0817A / WOLF CREEK WV309A / CST WOLF

±520 SUTTON ROAD OLIVE HILL, KY 41164

1 07/21/08 REVISED PER NEW AT&T TEMP 0 05/02/08 ISSUED FOR REVIEW NO. DATE REVISIONS SCALE: AS SHOWN DESIGNED BY:

NOTES: 1.

- 3.

- 5. INCLUDED

USING COLOR BANDS ON THE CABLES, MARK ALL RF CABLES BY SECTOR AND CABLE NUMBER, AS SHOWN ON "CABLE MARKING COLOR CONVENTION TABLE" (EX. SECTOR ALPHA, CABLE A3 WOULD BE THREE GREEN BANDS)

2. THE STANDARD CABLE MARKING TAPE IS BASED ON THE 5 "NEMA" COLORED TAPES: GREEN, BLUE, WHITE, RED AND ORANGE. UMTS CABLES WILL BE MARKED WITH A MINIMUM OF 3" WIDE AT TOP AND MIDDLE OF TOWER, AND 2" WIDE AT THE BOTTOM. ALL JUMPERS SHALL BE INCLUDED.

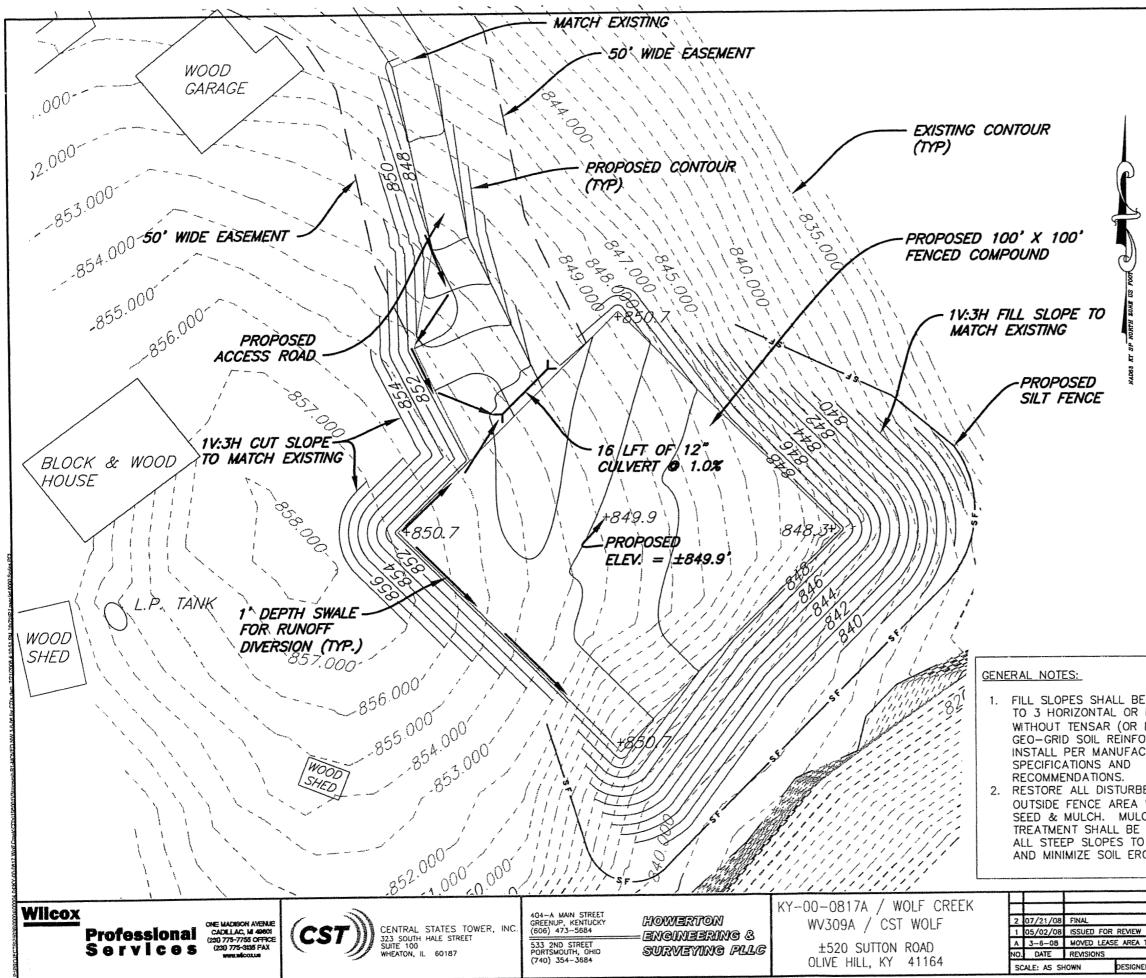
ALL COLOR CODE TAPE SHALL BE 3M-35 AND SHALL BE INSTALLED USING A MINIMUM OF (3) WRAPS OF TAPE AND SHALL BE NEATLY TRIMMED AND SMOOTHED OUT TO AVOID UNWRAPPING.

ALL COLOR CODE TAPE SHALL BE 3" WIDE AT TOP AND MIDDLE OF TOWER, AND 2" WIDE AT THE BOTTOM. ALL JUMPERS SHALL BE

COAX COLOR CODING AND IDENTIFICATION DETAIL FOR OVERLAY SCALE: N.T.S.

850 UMTS	1900
(FUTURE)	UMTS
CABLE	CABLE
UMTS3	UMTS4
THREE (3)	FOUR (4)
1 1/2" GRN	1 1/2" GRN
3/4" ORG	3/4" ORG
THREE (3)	FOUR (4)
1 1/2" BLUE	1 1/2" BLUE
3/4" ORG	3/4" ORG
THREE (3)	FOUR (4)
1 1/2" WHT	1 1/2" WHT
3/4" ORG	3/4" ORG
THREE (3)	FOUR (4)
1 1/2" RED	1 1/2" RED
3/4" ORG	3/4" ORG

					SHEET E-7
APLATE DWGS		JLS	AJK	AJK	COAX COLOR CODING
JI.		JLS	AJK	AJK	
		BY	СНК	APP'D	DRAWING NUMBER
: AJK DRAWN BY: JLS			LS		25036.00004.04



SITE GRADING NOTES:

- 1. SITE HAS BEEN ROUGHLY GRADED AND ALTERED SINCE THE ORIGINAL TOPO CONTOURS WERE MAPPED.
- 2. TEMPORARY GRADING EASEMENTS MAY BE REQUIRED FOR EARTHWORK PERFORMED OUTSIDE OF THE LEASE AREA AND ACCESS/UTILITY EASEMENT.
- 3. GRADE SITE TO DIVERT STORM WATER RUNOFF IN SHALLOW DITCHES OR SWALES FROM THE UPHILL SIDE OF LEASE AREA TO BYPASS AROUND THE SIDES OF THE LEASE AREA.
- 4. INSTALL TEMPORARY AND PERMANENT SOIL EROSION CONTROL DEVICES SUCH AS MULCH BLANKETS AND/OR ROCK CHECK DAMS AS REQUIRED TO AVOID EROSION NEAR THE LEASE AREA AND ALONG ROAD DITCHES.
- 5. REMOVE ALL TOPSOIL, ORGANIC MATERIAL, FROZEN SOIL OR CLODS PRIOR TO PLACING ENGINEERED FILL MATERIAL.
- SCARIFY SURFACE AND PROPERLY BENCH PRIOR TO PLACEMENT OF ENGINEERED FILL.
- 7. ENGINEERED FILL SHALL BE FREE FROM TOPSOIL, ORGANIC MATERIAL, FROZEN SOIL, CLODS OR OTHER HARMFUL MATERIAL.
- SPREAD ENGINEERED FILL IN LEVEL LIFTS OF 9 INCHES OR LESS (LOOSE THICKNESS) AND COMPACT TO 95% MAXIMUM DRY DENSITY PER ASTM D1557 (MODIFIED PROCTOR).
- 9. ALL ENGINEERED FILL SHALL BE PLACED AT OR NEAR OPTIMUM MOISTURE CONTENT.
- 10. A QUALIFIED GEOTECHNICAL ENGINEER SHOULD SUPERVISE ALL GEOTECHNICAL RELATED WORK, INCLUDING FOUNDATION CONSTRUCTION, SUBGRADE PREPARATION, AND ENGINEERED FILL PLACEMENT. THE GEOTECHNICAL ENGINEER SHOULD PERFORM THE APPROPRIATE TESTING TO CONFIRM THE GEOTECHNICAL CONDITIONS GIVEN IN THE SOIL BORING AND ROCK CORING INVESTIGATION REPORT ARE FOUND DURING CONSTRUCTION.
- 11. THE CONTRACTOR SHALL, UPON BECOMING AWARE OF SUBSURFACE OR LATENT PHYSICAL CONDITIONS DIFFERING FROM THOSE DISCLOSED BY THE ORIGINAL SOIL INVESTIGATION WORK, PROMPTLY NOTIFY THE OWNER VERBALLY TO PERMIT VERIFICATION OF THE CONDITIONS, AND IN WRITING, AS TO THE NATURE OF THE DIFFERING CONDITIONS. NO CLAIM BY THE CONTRACTOR FOR ANY CONDITIONS DIFFERING FROM THOSE ANTICIPATED IN THE PLANS AND SPECIFICATIONS AND DISCLOSED BY THE SOIL STUDIES WILL BE ALLOWED UNLESS THE CONTRACTOR HAS SO NOTIFIED THE OWNER, VERBALLY AND IN WRITING, AS REQUIRED ABOVE, OF SUCH DIFFERING SUBSURFACE CONDITIONS.

	1-					ORK QUANTIT PARKING ARE	
	ı E	CU	IT (0	CYD)		FILL (CYD)	
			±7	96		±1046	
E 1 VERTICAL FLATTER			DESLO 1V: 3			SIDESLOPE 1V: 3H	
EQUAL) DRCEMENT.		EXCESS: ±250 CYD FILL					
CTURERS		ASS	SUME	D TOPS	SOIL	THICKNESS:	6"
ED AREAS W/ TOPSOIL, CH BLANKET USED ON			GR	PADIN scale:		<u>PLAN</u> '=30'	
OSION.	30			0	15	30 	6
					ę	SHEET G-1	
	AJK	AJK AJK	ajk ajk		GRA	DING SITE PLAN	
TO 10' N AND 10' W	AJK	AJK	AJK			DRAWING NUMBER	
ED BY: AJK	DRAWN BY:	115				25039.00004.04	

ITE WORK GENERAL NOTES:	CONCRETE AND REINFORCING STEEL	NOTES:	APPLICABLE BUILDING CODES AND	STANINAPHC			
THE SUBCONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE	1. ALL CONCRETE WORK SHALL BE IN ACCOR	DANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184,			BLE NATIONAL STATE AND LOCAL		
TART OF CONSTRUCTION. . ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE NCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES, AND WHERE	2. ALL CONCRETE SHALL HAVE A MINIMUM CO	TION SPECIFICATION FOR CAST-IN-PLACE CONCRETE. DMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS, UNLESS FAK TESTS SHALL BE PERFORMED ON 7, 14 AND 28 DAYS FOR	SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND STANDARDS IN EFFECT ON THE DATE OF CONTRACT AWARD SHALL GOVERN THE DESIGN.				
EQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS RECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE UBCONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. UBCONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL	3. REINFORCING STEEL SHALL CONFORM TO A	ISTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE. ITM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED	International Building Code (IBC 2000)				
ICLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ICLUDE SUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C)	4. THE FOLLOWING MINIMUM CONCRETE COVER	R SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN	National Electrical Code (NEC 2002 part 8	state mendmer	nts) with local		
. ALL SITE WORK SHALL BE AS INDICATED ON THE DRAWINGS AND PROJECT	OTHERWISE ON DRAWINGS: CONCRETE CAST AGAINST EAR	TH	UnderWriter Laboratories Approved Electrical Life Safety Code NFPA — 101	Products			
PECIFICATIONS.	CONCRETE EXPOSED TO EART	H OR WEATHER:	SUBCONTRACTOR'S WORK SHALL COMPLY W	TH THE LATEST	EDITION OF THE FOLLOWING		
. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE HALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.	#6 AND LARGER #5 AND SMALLER & WWF CONCRETE NOT EXPOSED TO		AMERICAN CONCRETE INSTITUTE (ACI) 318,	BUILDING CODE	REQUIREMENTS FOR STRUCTURAL		
ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH TERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED,	CAST AGAINST THE GROUND:		AMERICAN INSTITUTE OF STEEL CONSTRUCTION	IN (AISC), MAN	IUAL OF STEEL CONSTRUCTION, ASD,		
LUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH HE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF CONTRACTOR, OWNER	SLAB AND WALL BEAMS AND COLUMNS		TELECOMMUNICATIONS INDUSTRY ASSOCIATION STRUCTURAL ANTENNA TOWER AND ANTENNA	I (TIA) EIA-222 SUPPORTING S	-G, STRUCTURAL STANDARD FOR		
ND/OR LOCAL UTILITIES.	5. A CHAMFER 3/4" SHALL BE PROVIDED AT WITH ACI 301 SECTION 4.2.4.	ALL EXPOSED EDGES OF CONCRETE, UNO, IN ACCORDANCE	INSTITUTE FOR ELECTRICAL AND ELECTRONIC	S ENGINEERS (I	FEE) 81. GUIDE FOR MEASURING FARTH		
. SUBCONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING ONSTRUCTION.	RECOMMENDED PROCEDURE. THE ANCHOR BOI	EDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN LT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S	RESISTIVITY, GROUND IMPEDANCE, AND EART IEEE 1100 (1999) RECOMMENDED PRACTIC				
. THE SUBCONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE ECHNICAL SPECIFICATION FOR SITE SIGNAGE.	RECOMMENDATION FOR EMBEDMENT DEPTH OR	AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT I DRILLING HOLES IN CONCRETE. EXPANSION BOLTS SHALL BE	IEEE C62.41, RECOMMENDED PRACTICES OF				
. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE TS EQUIPMENT AND TOWER AREAS.	,		CIRCUITS (FOR LOCATION CATEGORY "C3" AN TIA 607 COMMERCIAL BUILDING GROUNDING				
. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. ROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.			TELCORDIA GR-1503 COAXIAL CABLE CONN	IECTIONS			
0. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM RADE PRIOR TO FINISHED SURFACE APPLICATION.	GENERAL NOTES: 1. FOR THE PURPOSE OF CONSTRUCTION DR	WING, THE FOLLOWING DEFINITIONS SHALL APPLY:	FOR ANY CONFLICTS BETWEEN SECTIONS OF METHODS OF CONSTRUCTION, OR OTHER RE SHALL GOVERN. WHERE THERE IS CONFLIC	OUIREMENTS. TH	E MOST RESTRICTIVE REQUIREMENT		
1. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT OVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM	CONTRACTOR - CEL SUBCONTRACTOR - GEN OWNER - CEN	LERE IERAL CONTRACTOR (CONSTRUCTION) TRAL STATE TOWERS INAL EQUIPMENT MANUFACTURE	REQUIREMENT, THE SPECIFIC REQUIREMENT	HALL GOVERN.	ENERAL REQUIREMENT AND A SPECIFIC		
LOPE, AND STABALIZED TO PREVENT EROSION AS SPECIFIED IN THE PROJECT PECIFICATIONS. 2. SUBCONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING	2. PRIOR TO THE SUBMISSION OF BIDS, THE FAMILLARIZE WITH THE EXISTING CONDITIONS AI SHOWN ON THE CONSTRUCTION DRAWINGS, AN	ABBREVIATIONS AND SYMBOLS					
ONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, HALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT	OF CONTRACTOR.		ABBREVIATIONS		SYMBOLS		
CONTROL.	CODES, REGULATIONS, AND ORDINANCES. SUB	D SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC	AGL ABOVE GRADE LEVEL BTS BASE TRANSCEIVER STATION		S/G SOLID GROUND BUS BAR		
	AUTHORITY REGARDING THE RFORMANCE OF TI APLICABLE MUNICIPAL AND UTILITY COMPANY S	THE WORK ALL WORK CARRIED OUT SHALL COMPLY WITH ALL SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES	(E) EXISTING		SVI SOLID NEUTRAL BUS BAR		
	AND APPLICABLE REGULATIONS.	SCALE AND ARE INTENDED TO SHOW OUTLINE ONLY.	MIN MINIMUM N.T.S. NOT TO SCALE		GT₀ 2-POLE THERMAL-MAGNETIC		
STRUCTURAL STEEL NOTES: . ALL STEEL WORK SHALL BE PAINTED IN ACCORDANCE WITH THE PROJECT	5. UNLESS NOTED OTHERWISE, THE WORK SI	HALL INCLUDE FURNISHING MATERIALS, EQUIPMENT,	REF REFERENCE		CIRCUIT BREAKER		
PECIFICATIONS AND IN ACCORDANCE WITH ASTM A36 UNLESS OTHERWISE NOTED.	APPURTENANCES, AND LABOR NECESSARY TO	COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.	RF RADIO FREQUENCY T.B.D. TO BE DETERMINED		SINGLE-POLE THERMAL-MAGNETIC		
2. ALL WELDING SHALL BE PERFORMED USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION".	6. THE SUBCONTRACTOR SHALL INSTALL ALL MANUFACTURER'S RECOMMENDATIONS UNLESS	EQUIPMENT AND MATERIALS IN ACCORDANCE WITH SPECIFICALLY STATED OTHERWISE.	T.B.R. TO BE RESOLVED TYP TYPICAL		CHEMICAL GROUND ROD		
ANNED SURFACES SHALL BE TOUCHED UP.	7. IF THE SPECIFIED EQUIPMENT CANNOT BE SUBCONTRACTOR SHALL PROPOSE AN ALTERNA	INSTALLED AS SHOWN ON THESE DRAWINGS, THE NTVE INSTALLATION FOR APPROVAL BY THE CONTRACTOR.	REQ REQUIRED		GROUND ROD		
5. BOLTED CONNECTIONS SHALL BE ASTM A325 BEARING TYPE $(3/4^{*} \phi)$ CONNECTIONS IND SHALL HAVE MINIMUM OF TWO BOLTS UNLESS NOTED OTHERWISE.	8. SUBCONTRACTOR SHALL DETERMINE ACTUA CABLES AS SHOWN ON THE POWER, GROUNDI	L ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING NG AND TELCO PLAN DRAWING.	EGR EQUIPMENT GROUND RING		METER		
NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE 5/8" DIA. ASTM A 507 BOLTS UNLESS NOTED OTHERWISE.	9. THE SUBCONTRACTOR SHALL PROTECT EXIS	TING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND REPAIRED AT UBCONTRACTOR'S EXPENSE TO THE SATISFACTION	AWG AMERICAN WIRE GAUGE MGB MASTER GROUND BUS				
5. INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH	OF OWNER. 10. SUBCONTRACTOR SHALL LEGALLY AND PRO CABLES AND OTHER ITEMS REMOVED FROM TH	DPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL IE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE	EG EQUIPMENT GROUND BCW BARE COPPER WIRE SIAD SMART INTEGRATED ACCESS DEVICE	BCW BARE COPPER WIRE GROUNDING WIRE			
DR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERNING CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN	RETURNED TO THE OWNER'S DESIGNATED LOCA		GEN GENERATOR				
AANUFACTURER'S MAXIMUM ALLOWABLE LOADS.	11. SUBCONTRACTOR SHALL LEAVE PREMISES 12. CONSTRUCTION SHALL COMPLY WITH SPEC CONSTRUCTION SERVICES FOR CONSTRUCTION	IFICATION 24782-000-3APS-A00Z-00002, "GENERAL	IGR INTERIOR GROUND RING (HALO) RBS RADIO BASE STATION				
IICOX	CENTRAL STATES TOWER, INC.	KY-00-0817A / WOLF CREEK			SHEET GN-1		
	323 SOUTH HALE STREET			LS AIK AIK ILS AIK AIK	GENERAL NOTES		
	WHEATON, IL 60187	±520 SUTTON ROAD		LS FAIK I AIK			

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я	0'500	5205 5+221	8'[l+1	5'8161	C2052	0/5162	18+/Z	17660	L INST	5 1097	9 1987	\$¥125	8,1050	¥1151	7 E1812 (di
290.5 ft	275.5 A	255.8 ft	2.36.2. n	216.5 Å	196.8 ft	177.1 1	157.4 ft	137.8. f .	118.1 A	98.4 f	<u>78.7 n</u>	59.0 A	39.4 ft	n 7.91	0.0 ft
							\triangleleft					MAX. CORNER REACTI DOWN: 637522 Ib UPLIF: -564233 Ib CHEAP: 64743 Ib	Sr SHEAR 11517 Ib	TORQUE 0 Ib-ft 30 mph WIND - 0.7500 in ICE ATAL 89157 Ib 89157 Ib 13948	TORQUE 43 Ib-ft REACTIONS - 90 mph WIND
												U S J H	283856 lb		98

BM-1207		295	BM-1207		275
4)1'x6' Antenna		295	(4)1'x6' Antenna		275
4)1'x6' Antenna		295	(4)1 X6' Antenna		275
(4)1'x6' Anterna		295	(4)1%6' Antenna		275
BM-1207		285	BM-1207		265
(4)1'x6' Antenna		285	(4)1'x6' Antenna	一日, 一日,一日,一日,一日,一日,一日,一日,一日,一百,一百,一百,一百,一百,一百,一百,一百,一百,一百,一百,一百,一百,	265
[4]1'x6' Antenna		285	(4)1'x6' Antenna		265
(4)1'x6' Antenna		285	(4)1'x6' Antenna		265
MARK		SIZE	MARK		SIZE
A P1.5x.145	5		0	1 @ 4.91667	
B L11/2×11/2×1/8	1/2×1/8				
		MATERIAL	MATERIAL STRENGTH	Ŧ	
GRADE	Fγ	Fu	GRADE	۴y	μ
A500-50 50 ksi	5	62 ksi	A36M-50	50 ksi	65 ksi

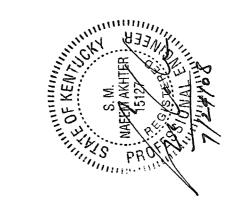
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wer is located in Carter County, Kentucky. wer designed for Exposure C to the TIA-222-G Standard. wer designed for a 90 mb basic wind with 0.75 in ice. Ice is considered to increase in thickness with height. wer is also designed for a 30 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height. effections are based upon a 60 mph wind. If X-braces are based upon a 60 mph wind. If X-braces are center bolted connections. Brace connection bolts meet A325X structural joint specification. If X-braces are center bolted. If X-braces are center bolted. If X-braces are center bolted. If Methanically in mbbers hold giped galvanized after fabrication per ASTM A123. Hardware (Bolts, Nuts. Etc.) galvanized per ASTM SeS Class 50 (Mechanical).

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TIONS AT BASE ОМЕNT 4241 Ib-ft

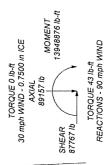
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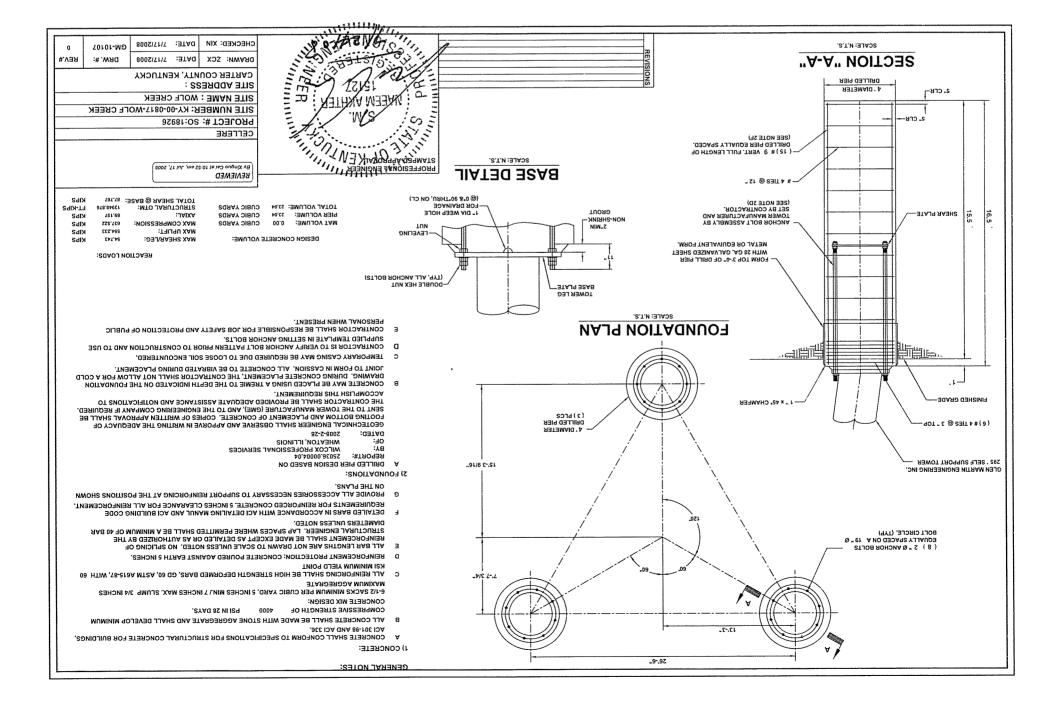


GLENMARTIN	GLENMARTIN Jub Site: Wolf Creek SO: 18926	0: 18926	
13620 Old Hwv 40	90mph-G	3754 model)	
Boonville. Mo 65233 Client Cellere		Drawn by GM	p.ddy
Phone: (660) 882-2734 Code: TiA-222-G	U	Date 07/15/08	Scale NTS
FAX: (660) 882-7200	Path C. Documents and Semigricscott MCHANDOLE	mp188,251235 HS 20mph/G (18754 model) e	Mug No E-1

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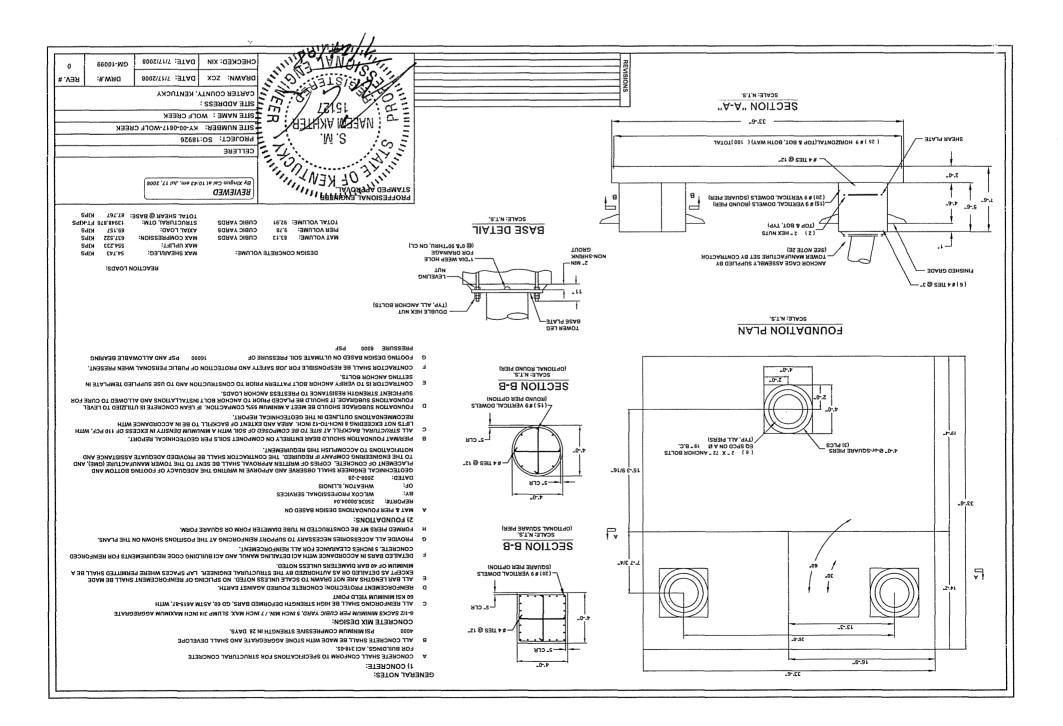
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GLENMARTIN	Job Site: Wolf Creek SO: 18926	Page 1 of 10
GLENMARTIN 13620 ()Id Hwy 40	Project 295' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Client	Designed by GM

SITE NAME: Wolf Creek SITE #: KY-00-0817 SALES ORDER: 18926 **SITE ADDRESS:** Carter County, Kentucky

Purchaser: Cellere Project Contact: Braxton Dougherty 231-929-4555 bdougherty@cellere.us

Contact Address: Attn: Braxton Dougherty Cellere, LLC 4110 Copper Ridge Drive Ste 204 Traverse City MI 49684

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All documents and details prepared in accordance with applicable EIA/TIA-222-G under the direct supervision of a registered professional engineer under the laws of the state of Kentucky , Enclosed calculations are certified and meet all specified purchaser requirements.

CERTIFIED BY: Nacem Akhter

DATE REVIEWED: 7-24-08



GLENMARTIN	Job Site: Wolf Creek SO: 18926	Page 2 of 10
GLENMARTIN 13620 Old Hwy 40	Project 295' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Client	Designed by GM

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 295.52 ft above the ground line. The base of the tower is set at an elevation of 0.00 ft above the ground line. The face width of the tower is 4.00 ft at the top and 26.50 ft at the base. This tower is designed using the TIA-222-G standard. The following design criteria apply: Tower is located in Carter County, Kentucky. Basic wind speed of 90 mph. Structure Class II. Exposure Category C. Topographic Category 1. Crest Height 0.00 ft. Nominal ice thickness of 0.7500 in. lce thickness is considered to increase with height. Ice density of 56 pcf. A wind speed of 30 mph is used in combination with ice. Temperature drop of 50 °F. Deflections calculated using a wind speed of 60 mph. All members stamped for identification in accordance with EIA/TIA-222G.. Lock washers provided for all brace bolted connections. Brace connection bolts meet A325X structural joint specification. All X-braces are center bolted.. Step bolt climb ladder provided on single leg with fall protection cable.. All members hot dipped galvanized after fabrication per ASTM A123. Hardware (Bolts, Nuts, Etc.) galvanized per ASTM B695 Class 50 (Mechanical).. All welded joints and connections certified for integrity and quality per AWS D1:1.. A non-linear (P-delta) analysis was used. Pressures are calculated at each section. Stress ratio used in tower member design is 1.

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Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

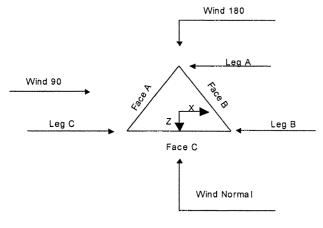
Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification✓ Distribute Leg Loads As Uniform Assume Legs Pinned✓ Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules✓ Originals Use Code Stress Ratios ✓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile ✓ Include Bolts In Member Capacity ✓ Leg Bolts Are At Top Of Section ✓ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC 6D+W Combination✓ Distribute Leg Loads As Uniform Assume Legs Pinned V Distribute Leg Panels As Cylinder Use Clear Spans For KL/r Wind Area V Use Clear Spans For KL/r Wind Area of Appurt Autocalc Torque Arm Areas V Distribute Leg Panels Have Cut Ends Sort Capacity Reports By Component V Se Component✓ Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA Secondary Horizontal Braces Leg Add IBC 6D+W Combination✓ Distribute Leg Loads As Uniform Assume Rigid Index Plate Use Diamond Inner Bracing✓ Treat Feedline Bundles As Cylinder Use Clear Spans For KL/r SR Leg Bolts Are At Top Of Section V Triangulate Diamond Inner Bracing✓ Treat Feedline Torque Use Top Mounted Sockets			
	Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification ↓ Use Code Stress Ratios ↓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile ↓ Include Bolts In Member Capacity ↓ Leg Bolts Are At Top Of Section ↓ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided)	Assume Legs Pinned Use ASCE 10 X-Brace Ly Rules ✓ Assume Rigid Index Plate ✓ Calculate Redundant Bracing Forces ✓ Use Clear Spans For Wind Area Ignore Redundant Members in FEA ✓ Use Clear Spans For KL/r SR Leg Bolts Resist Compression ✓ Use Clear Spans For KL/r SR Leg Bolts Resist Compression ✓ Use Clear Spans For KL/r SR Leg Bolts Resist Compression ✓ Use Clear Spans For KL/r SR Leg Bolts Resist Compression ✓ Project Wind Area of Appurt ✓ Autocalc Torque Arm Areas ✓ SR Members Have Cut Ends Include Shear-Torsion Interaction ✓ SR Members Have Cut Ends Include Shear-Torsion Interaction ✓ Sort Capacity Reports By Component Always Use Sub-Critical Flow	

GLENMARTIN	Job Site: Wolf Creek SO: 18926	Page 3 of 10
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Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Cilent	Designed by GM

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<u>Triangular Tower</u>

		Maximum	TowerL	Jenection	s - Service Wind	
Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist	
	ft	in	Comb.	o	0	
TI	295.52 - 290.52	18.910	47	0.7416	0.0000	
T2	290.52 - 275.52	18.131	47	0.7376	0.0000	
T3	275.52 - 255.84	15.844	47	0.6795	0.0000	
T4	255,84 - 236.16	13.163	47	0.5929	0.0000	
T5	236.16 - 216.48	10.829	47	0.5166	0.0000	
T6	216.48 - 196.8	8.834	47	0.4307	0.0000	
T7	196.8 - 177.12	7.163	47	0.3618	0.0000	
Т8	177.12 - 157.44	5.720	47	0.3157	0.0000	
T9	157.44 - 137.76	4.468	47	0.2687	0.0000	
T10	137.76 - 118.08	3.410	47	0.2212	0.0000	
T11	118.08 - 98.4	2.524	47	0.1876	0.0000	
T12	98.4 - 78.72	1.777	47	0.1536	0.0000	
T13	78.72 - 59.04	1.164	47	0 1 1 9 5	0.0000	
T14	59.04 - 39.36	0.691	47	0.0850	0 0000	
T15	39.36 - 19.68	0 340	47	0.0570	0 0000	
T16	19.68 - 0	0.109	43	0 0286	0 0000	

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ſi		Comb.	in	٥	0	ſŧ
295 00	BM-1207	47	18 829	0.7414	0 0000	93027
285 00	BM-1207	47	17.274	0.7225	0 0000	25319
275.00	BM-1207	47	15 767	0.6770	0 0000	10978
265.00	BM-1207	47	14.363	0.6310	0.0000	12220

GLENMARTIN	Job	Site: Wolf Creek SO: 18926	Page 4 of 10
GLENMARTIN 13620 Old Hwy 40	Project	295' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Client	Cellere	Designed by GM

		Maximum	Tower I	Deflection	s - Design Wind
Section No	Elevation ft	Horz. Deflection in	Gov Load Comb.	Tilt °	Twist o
TI	295.52 - 290.52	68.175	18	2 6741	0 0002
Τ2	290 52 - 275 52	65.366	18	· 2 6597	0 0002
T3	275 52 - 255 84	57.121	18	2.4502	0.0002
T4	255.84 - 236.16	47.458	18	2 1380	0 0002
T5	236.16 - 216.48	39.042	18	1.8630	0 0002
T6	216 48 - 196.8	31.850	18	1 5533	0 0002
T7	196.8 - 177.12	25.823	18	1 3047	0.0001
T8	177 12 - 157 44	20.622	18	1.1383	0 0001
Т9	157.44 - 137.76	16.106	18	0.9690	0 0001
T10	137 76 - 118.08	12 290	18	0.7976	0 0001
T11	118.08 - 98.4	9 098	18	0 6763	0 0001
T12	98.4 - 78.72	6.406	18	0.5538	0.0000
T13	78 72 - 59 04	4.195	18	0 4 3 0 7	0.0000
T14	59.04 - 39.36	2.491	18	0.3065	0 0000
T15	39.36 - 19.68	1 226	18	0 2054	0 0000
T16	19.68 - 0	0.394	18	0 1031	0.0000

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Critical Deflections and Radius of Curvature - Design Wind									
evation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature			
ft		Comb.	in	٥	٥	ft			
95.00	BM-1207	18	67.883	2.6733	0.0002	25166			
285.00	BM-1207	18	62.278	2.6053	0.0002	7021			
275.00	BM-1207	18	56.847	2.4411	0 0002	3054			
265.00	BM-1207	18	51,784	2.2755	0.0002	3399			

Bolt Design Data										
Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load per	Allowable Load	Ratio Load	Allowable Ratio	Criteria
	ft			in	Bolts	Bolt Ib	lb	Allowable		
Τl	295 52	Leg	A325X	0 7500	4	0 08	29820.60	0.000 🖌	1	Bolt Tension
		Diagonal	A325X	0.5000	1	1771 86	7312 50	0 242	1	Member Bearing
T2	290.52	Leg	A325X	0.7500	4	1079 89	29820 60	0.036 🖌	1	Bolt Tension
		Diagonal	A325X	0.5000	1	4924 84	7312 50	0.673	1	Member Bearing
Т3	275 52	Leg	A325X	1.0000	4	8674.69	53014 40	0 164 🖌	1	Bolt Tension
		Diagonal	A325X	0.5000	I	6273 35	8835.73	0.710	1	Bolt Shear
T4	255.84	Leg	A325X	1.0000	4	22077.00	53014.40	0.416	I	Bolt Tension
		Diagonal	A325X	0.5000	1	5952.42	8835.73	0.674	1	Bolt Shear

GLENMARTIN	Job	Site: Wolf Creek SO: 18926	Page 5 of 10
GLENMARTIN 13620 Old Hwy 40	Project	295' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt	Allowable Load Ib	Ratio Load Allowable	Allowable Ratio	Criteria
T5	236 16	Leg	A325X	1 0000	4	<i>lb</i> 33914 80	53014.40	V	1	Bolt Tension
		Diagonal	A325X	0.5000	1	5912 06	8835 73	0 640 🔽	1	Bolt Shear
Т6	216 48	Leg	A325X	1.0000	6	29304.50	53014.40	0 553	1	Bolt Tension
		Diagonal	A325X	0 5000	1	6381 53	8835.73	0 722	1	Bolt Shear
Τ7	196.8	Leg	A325X	1 0000	6	35420 50	53014 40	0 668	1	Bolt Tension
		Diagonal	A325X	0.5000	1	6976 47	8835.73	0 790	1	Bolt Shear
Т8	177 12	Leg	A325X	1 0000	6	41249.70	53014 40	0 778	1	Bolt Tension
		Diagonal	A325X	0.7500	1	7604 15	12339.80	0 616	1	Member Bearing
Т9	157 44	Leg	A325X	1.0000	6	47056.20	53014 40	0 888	1	Bolt Tension
		Diagonal	A325X	0.7500	1	8435 19	12339 80	0 684	1	Member Bearing
T10	137 76	Leg	A325X	1 0000	10	31679 10	53014.40	0.598	1	Bolt Tension
		Diagonal	A325X	0.7500	1	9404.66	16453.10	0 572	1	Member Bearing
ТП	118.08	Leg	A325X	1.0000	10	35075 90	53014.40	0 662	1	Bolt Tension
		Diagonal	A325X	0.7500	1	10441.50	16453 10	0.635	1	Member Bearing
T12	98 4	Leg	A325X	1.0000	10	38500.50	53014.40	0.726	1	Bolt Tension
		Diagonal	A325X	0.7500	1	11208.60	16453.10	0 681	1	Member Bearing
T13	78.72	Leg	A325X	1.0000	10	41918-20	53014.40	0.791	1	Bolt Tension
		Diagonal	A325X	0.7500	2	6269.76	19880.40	0.315	1	Bolt Shear
T14	59.04	Leg	A325X	1.0000	10	45357.20	53014.40	0.856	1	Bolt Tension
		Diagonal	A325X	0.7500	2	6651.12	19880.40	0.335	1	Bolt Shear
T15	39.36	Leg	A325X	1.0000	10	48761.40	53014.40	0.920	1	Bolt Tension
		Diagonal	A325X	0.7500	2	7143.93	19880.40	0.359	1	Bolt Shear
T16	19.68	Leg	A325X	1.0000	10	52180.40	53014.40	0.984	١	Bolt Tension
		Diagonal	A325X	0.7500	2	7977.60	19880.40	0 401	1	Bolt Shear

	Compression Checks
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		Leg Design Data (Compression)									
Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φ <i>P</i> ,	Ratio P"		
	ſŧ		ſŧ	ſì		in²	lb	lЬ	φ <i>P</i> ,		
Tl	295.52 - 290.52	P1 5x 145	5 00	4 92	94.8 K=1 00	0 7995	-3168 53	18657 20	0 170		
T2	290.52 - 275.52	P2x.154	15.00	4.97	75.8 K=1.00	1.0745	-29967 10	31766-40	0.943		
Т3	275.52 - 255 84	P3 5x 226	19.70	4 90	44.0 K=1.00	2 6795	-86789 00	104643.00	0 829 '		
Τ4	255.84 -	P5x 258	19.70	4.90	31.3	4 2999	-138482 00	180083 00	0 769 1		

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GLENMARTIN	Job Site: Wolf Creek SO: 18926	Page 6 of 10
GLENMARTIN 13620 Old Hwy 40	Project 295' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Cilent	Designed by GM

Section No.	Elevation	Size	L	Lu	Kl/r	A	Р"	φP _n	Ratio P"
	ft		ft	ft		in ²	lb	IЬ	φ <i>P</i> _n
	236.16	ann an			K=1 00			**************************************	V
Τ5	236-16 - 216:48	P5x 258	19.70	4.90	31.3 K=1.00	4.2999	-182415 00	180083 00	1.013 ¹
		4.9-3 (1.01 CR) - 88							
T6	216 48 - 196 8	P6x 28	19 70	4 90	26.2 K=1.00	5 5813	-223048 00	238856 00	0 934 '
Τ7	196 8 - 177 12	P8x 322	19.70	4 90	20 0 K=1 00	8.3993	-262624 00	367036 00	0716'
Τ8	177 12 - 157 44	P8x 322	19.70	4 90	20 0 K=1.00	8.3993	-302407 00	367036 00	0 824 '
Τ9	157 44 - 137 76	P8x 322	19 70	6.54	26 7 K=1 00	8.3993	-340120 00	358753 00	0.948
T10	137.76 - 118.08	P10x.365	19.70	6.54	21.4 K=1.00	11 9083	-380056 00	518292.00	0 733 '
TII	118.08 - 98.4	P10x.365	19.70	6.54	21 4 K=1 00	11.9083	-420567 00	518292 00	0 811 '
T12	984 - 7872	P10x 365	19 70	6 54	21 4 K=1 00	11.9083	-461484 00	518292 00	0 890 ¹
T13	78 72 - 59 04	P10x.365	19.70	6 54	21.4 K=1.00	11 9083	-503025 00	518292 00	0 971 '
T14	59.04 - 39 36	P12x.375	19.70	6.54	17.9 K=1.00	14.5790	-545112.00	640815.00	0 851 1
T15	39.36 - 19.68	P12x.375	19.70	6.54	17.9 K=1.00	14.5790	-587980.00	640815.00	0.918
T16	19 68 - 0	P12x.375	19.70	6.54	17.9 K=1.00	14.5790	-630459.00	640815.00	0.984

¹ P_{μ} / ϕP_n controls

							ression		
Section No.	Elevation	Size	L.	L_u	Kl/r	A	Р"	φ <i>P</i> ,	Ratio P _u
	ſt		ft	ſi		in²	Ib	lЬ	φP _n
TI	295.52 - 290.52	L1 1/2x1 1/2x1/8	6.34	3 04	123.3 K=1.00	0.3594	-1771 86	5338 98	0.332
Τ2	290.52 - 275.52	L1 1/2x1 1/2x1/8	6.38	3.03	122.9 K=1.00	0.3594	-4924.84	5377 19	0916
Т3	275.52 - 255 84	L1 3/4x1 3/4x3/16	7.30	3 56	124.3 K=1.00	0.6211	-6273 35	9075.09	0.691 '
T4	255.84 236.16	L1 3/4x1 3/4x3/16	8.56	4.12	143 9 K=1 00	0.6211	-5805 27	6779.15	0 856
T5	236.16 - 216.48	L2x2x3/16	9.92	4 81	146.4 K=1.00	0 7150	-5912 06	7536.18	0 784 '
Т6	216 48 - 196.8	L2 1/2x2 1/2x3/16	11.34	5 48	132.7 K=1.00	0 9020	-6381 53	11563 90	0.552
Τ7	196.8 - 177.12	L2 1/2x2 1/2x3/16	12.81	612	148.5 K=1.00	0.9020	-6976 47	9244.29	0 755 ¹
Т8	177.12 - 157.44	L3x3x3/16	14.31	6.88	138.5 K=1.00	1.0900	-7797 43	12840 00	0 607 '

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GLENMARTIN	Job Site: Wolf Creek SO: 18926	Page 7 of 10
GLENMARTIN 13620 Old Hwy 40	Project 295' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Cilent	Designed by GM

Section No	Elevation	Size	L	Lu	Kl/r	A	Pu	φ <i>P</i> ,	Ratio P"
	ft		ft	ft		in ²	lb	Ib	φP _n
T9	157 44 - 137 76	L3x3x3/16	16.35	7 93	1596 K=100	1 0900	-8783 71	9668 03	0 909 1
T10	137 76 - 118 08	L3x3x1/4	17 83	8.58	173.9 K=1.00	1.4400	-9669 66	10761 70	0 899 '
T11	118 08 - 98.4	L3 1/2x3 1/2x1/4	19.34	9.33	161.4 K=1.00	1.6900	-10709 30	14659.20	0 731 '
T12	98 4 - 78 72	L3 1/2x3 1/2x1/4	20 85	10 10	174.6 K=1.00	1 6900	-11524 50	12527 70	0 920 ¹
T13	78 72 - 59 04	L.4x4x1/4	22 39	10 86	164.0 K=1.00	1.9400	-12539 50	16296 60	0 769 '
T14	59.04 - 39.36	L4x4x1/4	23.93	11.55	174 3 K=1.00	1 9400	-13302 30	14418 00	0 923 '
T15	39.36 - 19.68	L4x4x5/16	25.48	12 33	187 0 K=1 00	2.4000	-14287 90	15502 20	0 922 '
T16	19 68 - 0	L4x4x3/8	27.03	13 11	199.6 K=1.00	2.8600	-15955 20	16216 80	0 984 '

 $^{1}P_{u}$ / ϕP_{n} controls

Top Girt Design Data (Compression)									
Section No.	Elevation	Size	L	Lu	Kl/r	A	Pu	φP _n	Ratio P.,
	ft		ft	ft		in ²	lb	lb	φ <i>P</i> ,
T1	295,52 - 290 52	L1 1/2x1 1/2x1/8	4.00	3.84	155.6 K=1.00	0.3594	-972.47	3351 34	0.290 1

 $^{1}P_{u} / \phi P_{n}$ controls

Tension Checks

	Leg Design Data (Tension)									
Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u	
	ſt		ft	ft		in ²	lb	lb	φP,	
ΤI	295.52 - 290.52	P1.5x.145	5.00	4 92	94.8	0.7995	2694 55	35975.60	0 075 1	
T2	290.52 - 275 52	P2x 154	15.00	4.97	75.8	1.0745	27738 30	48353 90	0 574 '	
тз	275.52 - 255.84	P3.5x.226	19 70	4 90	44 0	2 6795	80964 30	120579 00	0 671 '	
Τ4	255.84 - 236.16	P5x 258	19 70	4.90	31.3	4.2999	129839 00	193494.00	0 671 '	
Т5	236 16 -	P5x 258	19.70	4.90	31.3	4.2999	170687 00	193494 00	0 882 '	

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GLENMARTIN	Job	Site: Wolf Creek SO: 18926	Page 8 of 10
GLENMARTIN 13620 Old Hwy 40	Project	295' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Client	Cellere	Designed by GM

Section No.	Elevation	Size	L	Lu	Kl/r	A	P _u	φ <i>P</i> ,	Ratio P"
	ft		ft	fi		in ²	lb	lb	фP,
n berner afficier of the Sector W	216 48								V
		4.9-3 (1.01 CR) - 88							
Т6	216 48 - 196.8	P6x 28	19.70	4 90	26 2	5 5813	207610 00	251161 00	0 827 '
T7	196 8 - 177 12	P8x 322	19 70	4.90	20 0	8 3993	242774 00	377967 00	0 642 1
Τ8	177.12 - 157 44	P8x.322	19.70	4.90	20.0	8.3993	277659 00	377967 00	0 735 '
Т9	157.44 - 137.76	P8x 322	19.70	6.54	26.7	8 3993	310638 00	377967 00	0 822
T10	137.76 - 118.08	P10x.365	19 70	6 54	214	11 9083	344611 00	535873 00	0 643
T11	118 08 - 98 4	P10x 365	19 70	6 54	214	11 9083	378692 00	535873 00	0 707 •••
T12	98 4 - 78 72	P10x 365	19 70	6 54	214	11 9083	412864 00	535873 00	0 770
T13	78.72 - 59 04	P10x 365	19 70	6.54	21.4	11.9083	447120.00	535873.00	0 834
T14	59.04 - 39.36	P12x.375	19.70	6.54	17.9	14.5790	481254.00	656053.00	0.734
T15	39.36 - 19.68	P12x.375	19.70	6.54	17.9	14.5790	515416.00	656053.00	0.786
T16	19.68 - 0	P12x.375	19.70	6.54	17.9	14.5790	548812.00	656053.00	0.837

 $^{1}P_{\mu}/\phi P_{n}$ controls

		Diaș	gonal [Desig	n Dat	a (Ten	sion)		
Section No.	Elevation	Size	L	L _u	Kl/r	A	Р"	фP _n	Ratio P"
	ft		ft	ft		in ²	Ib	Ib	φ <i>P</i> ,
TI	295.52 - 290.52	L1 1/2x1 1/2x1/8	6.34	3 04	78.5	0.2109	1769 36	10283 20	0.172
Т2	290.52 - 275 52	L1 1/2x1 1/2x1/8	6.38	3 03	78.2	0.2109	4795.16	10283.20	0 466 '
Т3	275 52 - 255 84	L1 3/4x1 3/4x3/16	7.30	3 56	79 5	0.3779	6091 42	18424 10	0 331 1
T4	255.84 - 236 16	L1 3/4x1 3/4x3/16	7 60	3 65	81.6	0.3779	5820 21	18424 10	0316'
Γ5	236.16 - 216.48	L2x2x3/16	9 92	4.81	93.5	0.4484	5645.86	21857 50	0 258 '
Т6	216.48 - 196.8	L2 1/2x2 1/2x3/16	11.34	5.48	84.5	0.5886	6143.06	28694 70	0 214 '
Τ7	196.8 - 177.12	L2 1/2x2 1/2x3/16	12.81	6.12	94.5	0.5886	6784 89	28694.70	0 236 '
Τ8	177.12 - 157.44	L3x3x3/16	14 31	6.88	87.9	0.6945	7604 15	33854 60	0 225 '
Т9	157 44 - 137 76	L3x3x3/16	16 35	7 93	101.3	0.6945	8435 19	33854.60	0 249 1

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GLENMARTIN	Job	Site: Wolf Creek SO: 18926	Page 9 of 10
GLENMARTIN 13620 Old Hwy 40	Project 295	' HS 90mph-G (18754 geometry)	Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Client	Cellere	Designed by GM

Section No.	Elevation	Size	L	Lu	Kl/r	A	P _u	φ <i>P</i> ,	Ratio P"
	ſŧ		ft	fi		in ²	lb	lb	φ <i>P</i> ,,
angen beng menner i ber	an ya an							and and the of the last of the	V
T10	137.76 - 118.08	L3x3x1/4	17.83	8.58	1107	0 9159	9404 66	44652 00	0 211 '
TH	118 08 - 98.4	L3 1/2x3 1/2x1/4	19 34	9 33	102 8	1.1034	10441 50	53792 60	0.194
T12	98.4 - 78 72	L3 1/2x3 1/2x1/4	20.85	10 10	1112	1 1034	11208 60	53792 60	0 208 '
T13	78.72 - 59.04	L4x4x1/4	22.39	10 86	104 3	1 2909	12220 20	62933 20	0 194 '
T14	59 04 - 39 36	L4x4x1/4	23.93	11 55	1109	1.2909	12966.10	62933 20	0.206
T15	39.36 - 19.68	L4x4x5/16	25.48	12.33	119.3	1.5949	13932.00	77752 40	0.179
T16	19.68 - 0	L4x4x3/8	27 03	13.11	127.9	1.8989	15543 80	92571 70	0 168 '

¹ P_{μ} / ϕP_{σ} controls

	Top Girt Design Data (Tension)								
Section No.	Elevation	Size	L	L_u	Kl/r	A	Ри	фP,	Ratio P.,
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	ft		ft	ft		in²	lb	lb	φ <i>P</i> ,
TI	295.52 - 290.52	L1 1/2x1 1/2x1/8	4.00	3.84	99.1	0.2695	923 20	13139 60	0 070

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¹ P_u / ϕP_n controls

			Section Ca	oacity 1	able			
Section No.	Elevation ft	Component Type	Size	Critical Element	P Ib	øP _{allow} Ib	% Capacity	Pass Fail
TI	295 52 - 290 52	Leg	P1.5x 145	1	-3168 53	18657 20	17.0	Pass
		Diagonal	L1 1/2x1 1/2x1/8	7	-1771.86	5338 98	33 2	Pass
		Top Girt	L1 1/2x1 1/2x1/8	5	-972 47	3351.34	29.0	Pass
Τ2	290 52 - 275 52	Leg	P2x 154	15	-29967.10	31766 40	94.3	Pass
		Diagonal	L1 1/2x1 1/2x1/8	16	-4924.84	5377 19	916	Pass
Т3	275.52 - 255.84	Leg	P3.5x.226	34	-86789.00	104643.00	82.9	Pass
		Diagonal	L1 3/4x1 3/4x3/16	37	-6273.35	9075 09	691 71.0 (b)	Pass
Τ4	255.84 - 236.16	Leg	P5x.258	61	-138482.00	180083 00	76 9	Pass
• •		Diagonal	L1 3/4x1 3/4x3/16	64	-5805.27	6779.15	85.6	Pass
Т5	236.16 - 216.48	Leg	P5x.258	89	-182415.00	180083.00	101.3	Pass
		Diagonal	L2x2x3/16	92	-5912.06	753618	78.4	Pass
T6	216.48 - 196.8	Leg	P6x 28	115	-223048 00	238856.00	93 4	Pass
	210.70 190.0	Diagonal	L2 1/2x2 1/2x3/16	118	-6381.53	11563 90	55 2 72 2 (b)	Pass
T7	196 8 - 177 12	Leg	P8x 322	143	-262624 00	367036.00	71.6	Pass

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GLENMARTIN	lob Site: Wolf Creek SC	18926 Page 10 of 10
GLENMARTIN 13620 Old Hwy 40	Project 295' HS 90mph-G (1875	i4 geometry) Date 12:10:49 07/15/08
Boonville, Mo 65233 Phone: (660) 882-2734 FAX: (660) 882-7200	Client Cellere	Designed by GM

Section No.	Elevation ft	Component Type	Size	Critical Element	P Ib	øP _{allow} Ib	% Capacity	Pass Fail
140,	· · · · · · · · · · · · · · · · · · ·	Diagonal	L2 1/2x2 1/2x3/16	145	-6976 47	9244 29	75 5	Pass
							790(b)	1 400
Τ8	177 12 - 157 44	Leg	P8x 322	169	-302407 00	367036 00	82 4	Pass
		Diagonal	L3x3x3/16	172	-7797 43	12840 00	60.7	Pass
		•					616(b)	
Τ9	157.44 - 137.76	Leg	P8x 322	198	-340120 00	358753.00	94.8	Pass
		Diagonal	L3x3x3/16	199	-8783.71	9668.03	90 9	Pass
T10	137 76 - 118 08	Leg	P10x 365	217	-380056 00	518292.00	733	Pass
		Diagonal	L3x3x1/4	220	-9669 66	10761.70	89 9	Pass
T11	118 08 - 98 4	Leg	P10x.365	240	-420567 00	518292.00	811	Pass
		Diagonal	L3 1/2x3 1/2x1/4	243	-10709 30	14659 20	73 1	Pass
T12	984 - 7872	Leg	P10x.365	260	-461484 00	518292 00	89 0	Pass
		Diagonal	L3 1/2x3 1/2x1/4	262	-11524 50	12527 70	92 0	Pass
T13	78 72 - 59 04	Leg	P10x 365	280	-503025 00	518292 00	97	Pass
		Diagonal	L4x4x1/4	283	-12539 50	16296 60	76.9	Pass
T14	59.04 - 39.36	Leg	P12x.375	301	-545112.00	640815 00	85 1	Pass
							85 6 (b)	
		Diagonal	L4x4x1/4	307	-13302 20	14418.00	92 3	Pass
T15	39 36 - 19 68	Leg	P12x.375	322	-587980 00	640815 00	918	Pass
							92.0 (b)	
		Diagonal	L4x4x5/16	325	-14287.90	15502.20	92 2	Pass
T16	19.68 - 0	Leg	P12x 375	343	-630459.00	640815.00	98.4	Pass
							98.4 (b)	
		Diagonal	L4x4x3/8	347	-15955.20	16216.80	98.4	Pass
							Summary	
						Leg (T5)	101.3	Pass
						Diagonal	98.4	Pass
						(T16)		
						Top Girt	29.0	Pass
						(T1)		
						Bolt Checks	98.4	Pass
						RATING =	101.3	Pass

Program Version 5.1.1.4 - 2/24/2008 File:C:/Documents and Settings/Scott H/Desktop/temp/18926/295' HS 90mph-G (18754 model) eri

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SST TOWER CAISSON FOUNDATION DESIGN WORK SHEET:

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			MARTIN		Site: Wolf Cr			
Tower Site Name	WOLF CREEK		d Hwy 40		e: 295' HS 90mp			Apr. c
Project Number (Quote or Sales)	SO:18926		Mo 65233		Int. Cellere		AND DY GM	Scale NTS
Foundation Drawing Number	GM-10107		(C) 652-2734 (C) 852-7200		* TIA-222-G		^{xte:} 07/16/05	E.mg to = -
Site Number	KY-00-0817-WOLF CREEK	F 04 (000	., <u>uoc+/2</u> 00	<u> </u>	a Standard and and a State	All Constant of Party	ananangan konsertasi musa -	weid E
Site Location	CARTER COUNTY, KENTUCKY				MIN OO-	N== -	EACTIONS	TOACC
Client Name	CELLERE						EACTIONS AT	, 640E
Drawing Revision Number	0				DOWN UPLIFT			
Geotech Report Number	25036.00004.04				UPLIFT SHEAR			
Geotech Company Name	WILCOX PROFESSIONAL SERVIC	CES			37548	5474.	U 10	
Geotech Company Location	WHEATON, ILLINOIS				AXIA	W		
Geotech Company Location Geotech Report Date	2/28/2008				283856	-		
Geotech Report Date Concrete Compressive Strength		Knei			20300	- 10		
Consistere Compressive Strength	4	Kpsi		НЕА 1517		-	MOMENT 1944241 lb-ft	
Ultimate Bearing Pressure	25000	osf			TORQUE			
Pier Compression Force (Down)	637.522			30 -	mph WiND - (in ICE	
Pier Tension Force (Uplift)	554.233	•			4,7,14	-		
Pier Shear Force (Pier)	54.743				89:57	7-16	MOMENT	
Tower Axial Force (Down)	89.157					-	13948876 lb-ft	Y
Tower Overturning Moment	13948.876 1	•		HEAL		Ň		
Tower Shear Force (Base)	87.767		67	7767	10	÷		
		,			TORQUE	43 m-ft		
Tower Weight	51813.4			RE	EACTIONS - 9	10 mph	WIND	
Tower Spread (Input)	26.5							
Tower Spread (Print on drawing)	26.5 1		26'-6"/12					
1/3 Distance (Center to Face)	7'-7 3/4" 1	ft 7	7'-8"/12					
2/3 Distance (Center to Leg)	15'-3 9/16" 1	ft 1	15'-4"/12					
1/2 Distance (Half Spread)	13'-3" 1		13'-3"/12					
Pier Diameter	4 1	ft 4	\$					
Dior Hoisht Dolaw Carala	15.5 .5	ц -	E CUAC					
Pier Height Below Grade		ft 1	15'-6"/12					
Total Pier Height	16.5	ft 1	16'-6"/12					
Pier Height Above Grade	11							
Tower Height:	295.5	•••						
rower Height:	295.5	π						

Vertical Rebar Size (Round) Vertical Rebar Quantity (Round) Horizontal Tie Size Fc, Concrete Strength Fy, Rebar Yield Stress	9 # Ret 15 4 12" S 4 ksi 60 ksi		
Anchor Bolt Diameter Anchor Bolt Quantity Anchor Bolt Circle Diameter Anchor Bolt Projectior Anchor Bolt Length	2 in 8 19 in 11 in 72 in	2 XX 11 72	in in in
Base Plate Thickness	in		
DRAWN DATE CHECKED DATE	GD 7/18/2008 XIN 7/18/2008		

PIER DIAMETER	4	WN CHECKIN		200 45425	kina	Soil type	Kc
	4	ft	VERT. LOAD	398.45125	kips	sand	1.0 to 2.0
GROUND WATER	100	ft	UPLIFT	346.395625	kips	silt	1
ARM LENGTH	22 9496615	ft	OTM	8718.0475	ft-kips	clay	1
CONCRETEWT	150	pcf	UPLIFT CALCS:	380	kips		
)	0.008		TOWER SPREAD	26.5	ft	Adh factora	Undrained shear
BGRW	0 00	ft	ULTIMATE END BEARING			1	0.25
PLENGTH	15.50	ft	PER GEOTECH	25000	psf	0.65	0.5
RESULT	ANALYSIS (OK IN UPLIFT				0.5	75 or greater
		OK IN VERTICAL I	DOWN				
	ANALYSIS (DK IN OTM					
		Allowable Skin					
	Soil Depth	Friction (SF-2)	Soil Density	Kc (Down)	Kt (Uplift)	Friction Angle	Cohesion
STRATAS	(ft)	(psf)	(pcf)	Compresison	Tension	δ (degree)	C (psf)
1		0	100	0	0	0	0
2	2 3.5	0	110	0	0	0	0
3	3 1.5	0	110	0	0	0	0
4	1.5	1400	125	0	0	0	0
ŧ		3500	125	0	0	0	0
6		0	0	0	0	0	õ
7		0	0	0	0	0	0
1		0	0	0	0		0
						0	
9		0	0	0	0	0	0
10	0 0	0	0	0	0	0	0
ONT.							
DESIGN UPLIFT	401 42	kips	DESIGN DOWN	613 68	kips		
/ERTICAL BAR	SIZING			dp	34 5	in	
BAR SIZE	9	#		AREA BARSHEAR	0 299629399	in^2	
BAR NUMBER	- 15	-		DESIGN SPACING	8 560839981	in	
		K FOR REBAR SI				in	ACI 11 5 4 3
CHECK SPACE REBAR UP. CHECK				PIER BAR ACI LIMITS	21		ACI 11 5 4 2
		FOR UPLIFT		REBAR UP. CAPACITY	675.2256	kips	
ANCHOR BOLT	DESIGN						
SET INDEX	24			FLANGE THICK	1.5	in	
.EG SIZE (DIA)	P12	in		OTM	12000	ft-lb	OTM=12000
BOLT DIAMETER	2	in		AXIAL WEIGHT	398451.25	lb	AXIALWEIGH
BOLT QUANTITY	8			SHEAR FORCE	34214.375	lb	SHEARFORC
BOLT CIRCLE DIA	19	in	XX	Fc	4000	psi	0112/011 0110
	10			Fy	60000	psi	
OUTIENCTU	70						
BOLT LENGTH	72	in	24	•			
BASE PLATE DIA	26	in	24	BASE PLATE STR	880 7706391	psi	
BASE PLATE DIA BOLT PROJECT	26 11	ìn in	TIA222-G 4.9.61	•			
BASE PLATE DIA	26 11 ANALYSIS (in in DK FOR ANCHOR	TIA222-G 4.9.61 BOLT RATIO	•			
BASE PLATE DIA BOLT PROJECT	26 11 ANALYSIS (ANALYSIS (in IN DK FOR ANCHOR DK FOR PUNCHIN	TIA222-G 4.9.61 BOLT RATIO IG	•			
BASE PLATE DIA BOLT PROJECT	26 11 ANALYSIS (ANALYSIS (in in DK FOR ANCHOR	TIA222-G 4.9.61 BOLT RATIO IG	•			
BASE PLATE DIA BOLT PROJECT RESULT	26 11 ANALYSIS (ANALYSIS (ANALYSIS (in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH	BASE PLATE STR	880 7706391		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING	BASE PLATE STR	880 7706391 5 in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG	BASE PLATE STR 3.13 4 5	880 7706391 i in i in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi psi	TIA222-G 4 9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2	BASE PLATE STR 3.13 4 5 5 7	880 7706391 i in i in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi psi	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT	BASE PLATE STR 3.13 4 5 5 7 2 498225937	880 7706391 in in in in2		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi psi in	TIA222-G 4 9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2	BASE PLATE STR 3.13 4 5 5 7	880 7706391 in in in in2		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi psi in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT	BASE PLATE STR 3.13 4 5 5 7 2 498225937	880 7706391 in in in²2 in²2		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi psi in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416	880 7706391 in in in²2 in²2		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT MELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi psi lb degree	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT	BASE PLATE STR 3.13 4.5 5.7 2 498225937 3 1416 19936 71009	880 7706391 in in in in^2 in^2 psi		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063 45 451.2500	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975	880 7706391 in in in² in²2 jin²2 psi i rad		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT MELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT	BASE PLATE STR 3.13 4.5 5.7 2 498225937 3 1416 19936 71009	880 7706391 in in in² in²2 jin²2 psi i rad		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063 45 451.2500	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2 in^2	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975	880 7706391 in in in^2 in^2 psi rad in		
BASE PLATE DIA BOLT PROJECT VESULT MINTENSTRBLTU LLTENSTRBLT TELDSTRFLANGE DIASTREBOLT XIALBOLT BOLTDEGREE T Ya MINERSINBOLT	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063 45 451.2500 361.0000 0.49665083	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi psi in lb degree in ⁴ 2 in ⁴	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975 9 5	880 7706391 in in in^2 in^2 psi rad in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a MINERSINBOLT	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063 45 451.2500 361.0000 0.49665083	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi psi in lb degree in ⁴ 2 in ⁴	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975 9 5	880 7706391 in in in^2 in^2 psi rad in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.78348889 49806.4063 45 451.2500 361.0000 0.49665083	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2 in^2 in^4 STH DEVELOP	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277	880 7706391 in in in^2 in^2 psi rad in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU VILLTENSTRBLT TELDSTRFLANGE DIASTREBOLT XIALBOLT BOLTDEGREE (t (a MINERSINBOLT MAX ANCHOR E	26 11 ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 4551.2500 361.0000 0.49665083 BOLT LENC	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2 in^2 in^4 STH DEVELOP	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREASTREBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL MENT	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975 9 5 905 83277	880 7706391 in in in² in² in² jn² jn² jn² jn² j psi in² in in in in in² j in² j in² j in² j in² j in² j in in in i in i		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a MINERSINBOLT MAX ANCHOR E	26 11 ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 45 451.2500 361.0000 0.49665083 BOLT LENC 8.69626357	in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2 in^2 in^4 STH DEVELOP	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL MENT DIASHEARIN	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975 9 5 905 83277	880 7706391 in in in^2 psi in in^2 in in in in in in in in in in in in in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a MINERSINBOLT MAX ANCHOR E BHEARPLATETH	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 55000 1.78348889 49806.4063 4551.2500 361.0000 0.49665083 BOLT LENC 8.69626357 0.7	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi in b degree in^2 in^2 in^4 STH DEVELOP in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL MENT DIASHEARIN DIASHEARIN	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975 9 5 905 83277 15 23	880 7706391 in in in^2 psi in in^2 in in in in in in in in in in in in in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU MILTENSTRBLT MILLTENSTRBLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (1 (a MINERSINBOLT MAX ANCHOR E GHEARPLATETH F(DIAMNB)	26 11 ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 49806.4063 49806.4063 361 0000 0.49665083 BOLT LENC 8 69626357 0.7	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in ² 2 in ⁴ TH DEVELOP in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL MENT DIASHEARIN DIASHEARIN	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975 9 5 905 83277 15 23	880 7706391 in in in^2 psi in in^2 in in in in in in in in in in in in in		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a MINERSINBOLT MAX ANCHOR E d SHEARPLATETH C(DIAMNB) (CIDIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 1.7834889 49806.4063 49806.4063 45 451.2500 361.0000 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL MENT DIASHEARIN DIASHEAROT AREAPLATE	BASE PLATE STR 3.13 5 5 2 498225937 3 1416 19936 71009 0 7853975 9 5 905 83277 15 238 7616	880 7706391 in in in² in² in² in² psi in² in² in² in² in² in² in² in in in in in² in²		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU MILTENSTRBLT MILLTENSTRBLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (1 (a MINERSINBOLT MAX ANCHOR E GHEARPLATETH F(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 1.7834889 49806.4063 49806.4063 49806.4063 361.0000 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in ² 2 in ⁴ TH DEVELOP in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL MENT DIASHEARIN DIASHEARIN	BASE PLATE STR 3.13 4 5 5 7 2 498225937 3 1416 19936 71009 0 7853975 9 5 905 83277 15 23	880 7706391 in in in² in² in² in² psi in² in² in² in² in² in² in² in in in in in² in²		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU VILLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a WINERSINBOLT MAX ANCHOR E GHEARPLATETH (DIAMNB) G(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 49806.4063 49806.4063 45 451.2500 361.0000 0.49665083 BOLT LENC 8.69626357 0.7 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0.055	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in? in? psi in? in?? in?? in?? in?? in?? in?? in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (c (a MINERSINBOLT MAX ANCHOR E CHEARPLATETH (DIAMNB) (CIDIAMNB) ST1	26 11 ANALYSIS (ANALYSIS (55000 36300 55000 1.78348889 49806.4063 49806.4063 45 451.2500 361.0000 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0 0 119380 8	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in in in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL MENT DIASHEARIN DIASHEAROT AREAPLATE	BASE PLATE STR 3.13 5 5 2 498225937 3 1416 19936 71009 0 7853975 9 5 905 83277 15 238 7616	880 7706391 in in in? in? psi in? in?? in?? in?? in?? in?? in?? in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU VILLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a WINERSINBOLT MAX ANCHOR E GHEARPLATETH (DIAMNB) G(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 49806.4063 49806.4063 45 451.2500 361.0000 0.49665083 BOLT LENC 8.69626357 0.7 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0 0.055 0 0.055	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in in in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in? in?2 in?2 psi in?2 in?2 in?2 in?2 in?2 in?2 in?2 in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t fa MINERSINBOLT MAX ANCHOR E d SHEARPLATETH S(DIAMNB) S(DIAMNB) S(DIAMNB) S(DIAMNB) S(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 49806.4063 49806.4063 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0 0 119380.8 5.54099376	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in in in in in in in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in? in?2 in?2 psi in?2 in?2 in?2 in?2 in?2 in?2 in?2 in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a MINERSINBOLT MAX ANCHOR E C (DIAMNB) ((DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 49806.4063 49806.4063 6000 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0 0.119380.8 5.54099376 6	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in in in ft	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in? in?2 in?2 psi in?2 in?2 in?2 in?2 in?2 in?2 in?2 in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t fa MINERSINBOLT MAX ANCHOR E d SHEARPLATETH S(DIAMNB) S(DIAMNB) S(DIAMNB) S(DIAMNB) S(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 49806.4063 49806.4063 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0 0 119380.8 5.54099376	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in in in ft	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in? in?2 in?2 psi in?2 in?2 in?2 in?2 in?2 in?2 in?2 in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (c (a MINERSINBOLT MAX ANCHOR E GHEARPLATETH (DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 50000 1.7834889 49806.4063 49806.4063 49806.4063 6000 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0 0.119380.8 5.54099376 6	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in in ft	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in? in?2 in?2 psi in?2 in?2 in?2 in?2 in?2 in?2 in?2 in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT (IELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (t (a MINERSINBOLT MAX ANCHOR E C (DIAMNB) ((DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 55000 1.78348889 49806.4063 49806.4063 49806.4063 361.0000 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0 119380.8 5.54099376 6 0	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL psi in lb degree in^2 in^2 in^4 STH DEVELOP in in in in ft	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in? in? psi in? in?? in?? in?? in?? in?? in?? in?		
BASE PLATE DIA BOLT PROJECT RESULT MINTENSTRBLTU ALLTENSTRBLT VIELDSTRFLANGE DIASTREBOLT AXIALBOLT BOLTDEGREE (c (a MINERSINBOLT MAX ANCHOR E GHEARPLATETH (DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB) G(DIAMNB)	26 11 ANALYSIS (ANALYSIS (ANALYSIS (55000 36300 55000 1.78348889 49806.4063 49806.4063 49806.4063 361.0000 0.49665083 BOLT LENC 8.69626357 0.7 0.05 0 119380.8 5.54099376 6 0	in in DK FOR ANCHOR DK FOR PUNCHIN DK FOR BASE PL. psi psi in lb degree in ² 2 in ⁴ STH DEVELOP in in in in in in in in in in	TIA222-G 4.9 61 BOLT RATIO IG ATE STRENGTH DIA NUTPUNCHING THREADS PER LENG DATA2 AREASTREBOLT AREANOMBOLT STRESSBOLT BOLTRADIUS RADIUSBC MINEBOLTTOTAL DIASHEARIN DIASHEARIN DIASHEAROT AREAPLATE SHPLATETH	BASE PLATE STR 3.13 4.5 5.7 2.498225937 3.1416 19936.71009 0.7853975 9.5 905.83277 15 23 238.7616 0.75	880 7706391 in in in²2 psi rad in rad in ra4 in in²2 psi in²2 in²2 in²2 in in² in in in²2 in²2		

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SST TOWER PIER/MAT FOU	INDATION DESIGN WORK SHEE		n, Jul 18, 2008)
Site Name:	WOLF CREEK		s attrapts-G (13751 motor)
Project #:	SO:18926	The second second	
DRW. #:	GM-10099 KY-00-0817-WOLF CREEK	M4X CORN	ER REACTIONS AT BASE
Site #: Site Location:	CARTER COUNTY, KENTUCKY	DOWN 6	
Client:	CELLERE	UPLIFT -	
Revision:	0	SHEAR. :	547 <i>43 lb</i>
Geotech Report #:	25036.00004.04	AXIAL	
Report By:	WILCOX PROFESSIONAL SERVICES	283856 :	5
Of:	WHEATON, ILLINOIS	~t~	. MOMENT
Report Date:	2/28/2008	SHEAR /	1944241.15-ft
Allowable bearing pressure	8000 psi	11517 10	Y TOTAL THE REAL
Concrete Compressive Strength:	4000 psi	TORQUE 0	1b 4
Sack Mix:		30 mph W/ND - 0.1	• •
Minimum Slump:		AXIAL	
Maximum Slump:		89157 iL	
Ultimate Bearing Pressure	16000 psf		MOMENT 13948876 Ib-#
Vertical. Down:	637.522 kips	SHEAR	X 13840070 (35)
MAX Uplift:	554.233 kips	87767 15	¥
MAX Shear/Leg:	54.743 kips	TORQUE 43	10. 4
Axial Load:	89.157 kips	2510TONS 00	
OTM:	13948.876 ft kip	s	
Total Shear @ Base:	87.767 kips		
Tower weight:	51813.4 lbs		
Fy of Re-bars (ksi)	60 ksi		
Tower Spread (Input)	26,5		
Tower Spread (Dimension sign)	26'-6" ft	26'-6''/12	ft
1/3 Distance:	7'-7 3/4"	7'-8''/12	ft
2/3 Distance:	15'-3 9/16"	15'-4''/12	ft
1/2 Face Distance:	13'-3" ft	13'-3''/12	ft
Pier Diameter (Pad Width):	4'-0'' ft	4	ft
1/2 Pier Diameter (1/2 Pad width)	2'-0''	2	ft
Total PIER HEIGHT:	5'-6" ft	5'-6''/12	ft
Finished Height Above Grade:	1 ft	1	ft
Total Mat Width:	33'-6" ft	33'-6"/12	ft
1/2 Total Mat Width	16'-9"	16'-9''/12	ft
Mat Thickness:	2'-0" ft	2	ft
Tower height	295.5 ft		
Total height	7'-6"	7'-6"/12	ft
Volume of Concrete Pier:			
Total Volume of Concrete:			
Pier height below grade	4'-6"	4'-6''/12	ft
PIER BAR SIZE PIER BAR NUMBER	9 # Re 15	bar	
Size of Horizontal Ties:	4 12" S	nacina	
MAT BAR SIZE	9 # Re		
MAT BAR NUMBER	25		
TOTAL MAT BAR NUMBER	100		
Anchor Bolt Diameter:	2 in	2	inch
Quanity of Anchor Bolts:	8		
Bolt Circle Diameter:	19 in	19	inch
Anchor Bolt Projection:	11 in	11	inch
Anchor bolt length:	72 in	72	inch
Distance Base Plate & Pad:	in		
Thickness Base Plate:	in		
DRAWN	GD		
DATE	7/17/2008		
CHECKED	XIN		
DATE	7/17/2008		
SQUARE BAR SIZE	9		
SQUARE BAR NUMBER	20		
PIERS MOVE UP DIMENSION	2.55 ft		
MAT LARGER HALF DIMENSION	19'-4'' ft	19'-4''/12	ft
MAT SMALLER HALF DIMENSION	14'-2" ft	14'-2''/12	ft

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INPUT DATA AND D					_	
MAT WIDTH	33.5	ft	CONCRETE DENSITY	150	pcf	
MAT THICKNESS	2	ft	BACKFILL DENSITY	110	pcf	
PIER DIAMETER	4	ft	MINSOIL DENSITY	110	pcf	
PIER TOTAL HEIGHT	5.5		AXIAL LOAD	89157	lb	
PIER HEIGHT AGL	1	ft	UPLIFT LOAD	554233	lb	
INPUT WATER TABLE	N/A	ft	BASE SHEAR FORCE	87767	lb	BASE SHEAR
WATER TABLE FOR CALCS	N/A	ft	OTM	13948876	ft-lb	
MAT LENGTH	33.5	ft	CONCRETE STRENGTH	4000	psi	
ρ (PIER)	0.008		ULTIMATE BEAR PRESSURE	16000	psf	SOIL PROPERTIES
ρ (MAT)	0.005		TOWER SPREAD	26.5		
COHESION	500	psf	LOCAL OTM	120000	ft-lb	
COFRICTION	0 45	base footing				
PIER BAR SIZE	9	#	SPACING OK FOR PIER	BAR SIZING	REBAR UP. CAP.	651440.6526 lb
PIER BAR NUMBER	15		SPACING OK FOR SQUA	ARE BAR SIZING	REBAR UP. CHECK	PIER REBAR OK FOR UPLIFT
SQUARE BAR SIZE	9	#	SPACING OK FOR MAT	BAR SIZING		
SQUARE BAR NUMBER	20					
MAT BAR SIZE	9	#	PIER HEIGHT DESIGN O	ĸ		
MAT BAR NUMBER	50					
ECENTRICITY	13.9882424		Qo	99718575	lb	
ECENTRICITY FACTOR	5.58333333		PIER TO CENTER	15.30	ft	
RESULT		TY ANALYSIS OK				
SDIE EDGE CHECK	SIDE EDGE OF		N/A	ft		
BOTTOM EDGE CHECK	MOVE PIERS L		2.55	ft		
DOWN, UPLIFT, AND	OVERTIE		NT CHECKING			
ACTUAL AREA WIDTH	5 52351518		WEIGHT OF SOIL	617866.6725	lb	
ACTUAL AREA WIDTH			WEIGHT OF SOIL		lb	
INVERSE SOIL HEIGHT	2 5965	ft	DESIGN UPLIFT REQUIRED UPLIFT	745606.2544	lb	(WR/2)+(WC/1 25) (WR+WC)/1 5
FOOTING PERIMETER	134	ft		607988.0828	lb IS	0.76 A. CIA 000 C
		ft^3	DESIGN DOWN	2220453.101	lb	0.75 Φs EIA-222-G
INVERSE SOIL WEIGHT	86112.9225		REQUIRED DOWN	2175977.25	lb	
RESULT	UPLIFT ANA					
	DOWN ANA	L1315 UK				
SLIDING FRICTION						
FRICTION CAPACITY	448733.588					
RESULT	ANALYSIS (OK IN HORIZONT	AL MOVEMENT			
PUNCHING SHEAR I	N FOOTING	G				
		-		ONE WAY PUNCHIN	G SHEAR-ASSUMED SQUA	RE BASE FOOTING
PU1	1178791 5	lb				RE BASE FOOTING RE BASE FOOTING WITH COLUMN LOCAT
PU1 d	1178791 5 21		ф			
		lb	ф vc	TWO WAY PUNCHIN		
d	21	lb in		TWO WAY PUNCHIN 0.85	IG SHEAR-ASSUMED SQUA	
d e1	21 156	lb in in	VC	TWO WAY PUNCHIN 0.85 126 4911064	IG SHEAR-ASSUMED SQUA psi	
d e1 vu1	21 156 54.1863868	lb in in psi	vc SH1	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404	IG SHEAR-ASSUMED SQUA psi	
d e1 vu1 PU2	21 156 54.1863868 98072.7	lb in in psi Ib	vc SH1 JF	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627.5	IG SHEAR-ASSUMED SQUA psi	
d e1 vu1 PU2 MU2	21 156 54.1863868 98072.7 1584000	lb in psi lb lb-in	vc SH1 JF R2	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627 5 83 74812876	IG SHEAR-ASSUMED SQUA psi	
d e1 vu1 PU2 MU2 b2	21 156 54.1863868 98072.7 1584000 69	lb in psi lb lb-in in	vc SH1 JF R2 v2	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627 5 83 74812876 0 4	IG SHEAR-ASSUMED SQUA psi psi	
d e1 vu1 PU2 MU2 b2 AREAP	21 156 54.1863868 98072.7 1584000 69 5796 21.551638	lb in psi lb-in in^2 psi	vc SH1 JF R2 v2 AREAF	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627 5 83 74812876 0 4 161604 215.0348809	IG SHEAR-ASSUMED SQUA psi psi in^2	
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d e1 vu1 PU2 MU2 b2 AREAP vu2 RESULT ANCHOR BOLT DES	21 156 54.1863868 98072.7 1584000 69 5796 21.551638 FOUNDATIC	lb in psi lb lb-in in in^2 psi DN DESIGN, ONE	VC SH1 JF R2 V2 AREAF SH2 WAY SHEAR PUNCHING -WAY SHEAR PUNCHING	TWO WAY PUNCHIN 0.85 126 4911054 107.5174404 4705627 5 83 74812876 0 4 161604 215.0348809 OK OK	IG SHEAR-ASSUMED SQUA psi psi in^2	
d e1 vu1 PU2 b2 AREAP vu2 RESULT ANCHOR BOLT DES LEG TYPE & SIZE	21 156 54.1863868 98072.7 1584000 69 5796 21.551638 FOUNDATIC FOUNDATIC	Ib in psi Ib Ib-in in in^2 psi DN DESIGN, ONE DN DESIGN, TWO	VC SH1 JF R2 V2 AREAF SH2 WAY SHEAR PUNCHING -WAY SHEAR PUNCHING SET INDEX	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627 5 83 74812876 0 4 161604 215.0348809 OK OK	IG SHEAR-ASSUMED SQUA psi psi in^2 psi	RE BASE FOOTING WITH COLUMN LOCAT
d e1 vu1 PU2 MU2 b2 AREAP vu2 RESULT ANCHOR BOLT DES LEG TYPE & SIZE LEG TYPE & SIZE	21 156 54.1863868 98072.7 1584000 69 5796 21.551638 FOUNDATIC FOUNDATIC IGN P12 P12	Ib in psi Ib Ib-in in in^2 psi DN DESIGN, ONE DN DESIGN, TWO (FROM TOWER DESIGN) (FROM TOWER DESIGN)	VC SH1 JF R2 V2 AREAF SH2 WAY SHEAR PUNCHING -WAY SHEAR PUNCHING SET INDEX FLANGE THICK	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627 5 83 74812876 0 4 161604 215.0348809 DK OK 24 1.5	in SHEAR-ASSUMED SQUA	RE BASE FOOTING WITH COLUMN LOCAT
d e1 vu1 PU2 MU2 b2 AREAP vu2 RESULT ANCHOR BOLT DES LEG TYPE & SIZE LEG TYPE & SIZE BOLT DIAMETER	21 156 54.1863868 98072.7 1584000 69 5796 21.551638 FOUNDATIC FOUNDATIC FOUNDATIC FOUNDATIC 1GN P12 P12 2	Ib in psi Ib Ib-in in in^2 psi DN DESIGN, ONE DN DESIGN, TWO	VC SH1 JF R2 V2 AREAF SH2 WAY SHEAR PUNCHING WAY SHEAR PUNCHING SET INDEX FLANGE THICK OTM	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627 5 83 74812876 0 4 161604 215.0348809 OK 24 1.5 12000	ig SHEAR-ASSUMED SQUA psi in^2 psi in fi-lb	RE BASE FOOTING WITH COLUMN LOCAT (FROM PIER) OTM=12000
d e1 vu1 PU2 MU2 b2 AREAP vu2 RESULT ANCHOR BOLT DES LEG TYPE & SIZE LEG TYPE & SIZE BOLT DIAMETER BOLT DIAMETER BOLT QUANTITY	21 156 54.1863868 98072.7 1584000 69 5796 21.551638 FOUNDATIC FOUNDATIC IGN P12 P12 2 8	Ib in psi Ib Ib-in in 'in '2 psi DN DESIGN, ONE' DN DESIGN, TWO (FROM TOWER DESIGN) (FROM TOWER DESIGN) (FROM SET INDEX) in	VC SH1 JF R2 V2 AREAF SH2 WAY SHEAR PUNCHING -WAY SHEAR PUNCHING SET INDEX FLANGE THICK OTM AXIALWEIGHT	TWO WAY PUNCHIN 0.85 126 4911064 107.5174404 4705627 5 83 74812876 0.4 161604 215.0348809 OK OK 24 1.5 12000 398451.25	ig SHEAR-ASSUMED SQUA psi psi in^2 psi in ft-lb lb	RE BASE FOOTING WITH COLUMN LOCAT (FROM PIER) OTM=12000 VERTICAL LOAD
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EXHIBIT C

Directions to Site from County Seat



<u>Directions to the Site</u> From the County Seat of Carter County, Kentucky

Wolf Creek Site Carter County, Kentucky

From the Carter County Courthouse in Grayson, Kentucky, begin heading West on US-60/ W. Main Street toward N. Hord Street. Continue to follow US -60 for 9.0 miles. Turn Right onto KY-182 for 3.2 miles. Turn Right onto Sutton Road for 1.5 miles (portions of the road are unpaved). End at 520 Sutton Road, Olive Hill, Kentucky. Site is on the East side of the road.

Sandee L. Yagle, Cellere Date

TEL 231.929.4555 FAX 231.929.0099 WWW.cellere.us info@cellere.us 4110 Copper Ridge Drive, Suite 204, Traverse City, MI 49684

EXHIBIT D

Memorandum of Lease

MEMORANDUM OF LEASE

Return to:

C/O Central States Tower Holdings, LLC 323 S. Hale Street, Suite 100 Wheaton, IL 60187 (630) 221-8500 Main Number Attn: Property Manager Prepared By: Benjamin Meredith Cellere 4110 Copper Ridge Drive Ste. 204 Traverse City, MI 49684 (231) 929-4555

Re: Cell Site #KY-00-0817; Cell Site Name: Wolf Creek

State:KentuckyCounty:Carter

This Memorandum of Lease is entered into on this 26^{h} day of βcc , 2007, by and between <u>Gary Bond</u>, a <u>single man</u>, having a mailing address of <u>520 Sutton Road</u>, <u>Olive Hill, KY 41164</u> (hereinafter referred to as "Landlord") and Central States Tower Holdings, LLC, a Delaware limited liability company, having a mailing address of 323 S. Hale Street, Suite 100, Wheaton, IL 60187 (hereinafter referred to as "**Tenant**").

- 1. Landlord and Tenant entered into a certain Option and Lease Agreement ("Agreement") on the <u>28</u>^h day of <u>hec</u>, <u>2007</u>, for the purpose of installing, operating and maintaining a communications facility and other improvements. All of the foregoing are set forth in the Agreement.
- 2. The initial lease term will be five (5) years ("Initial Term") commencing on the effective date of written notification by Tenant to Landlord of Tenant's exercise of the Option, with five (5) successive five (5) year options to renew.
- 3. A portion of the Property being leased to Tenant contained and described in Exhibit A annexed hereto.
- 4. This Memorandum of Lease is not intended to amend or modify, and shall not be deemed or construed as amending or modifying, any of the terms, conditions or provisions of the Agreement, all of which are hereby ratified and affirmed. In the event of a conflict between the provisions of this Memorandum of Lease and the provisions of the Agreement, the provisions of the Agreement shall control. The Agreement shall be binding upon and inure to the benefit of the parties and their respective heirs, successors, and assigns, subject to the provisions of the Agreement.

IN WITNESS WHEREOF, the parties have caused this Agreement to be effective as of the last date written below.

WITNESSES:	"LANDLORD"
Mille Burtoz	By: Print Name: Gary Bord Its: <u>Owner</u> Date: 12/11/07
Print Name:	By: Print Name:
	Its: Date:
WITNESSES:	"TENANT"
	Central States Tower Holdings, LLC a Delaware limited liability company
Mananne Prant Print Name: MAKIANNE GRAMT	By: Print Name: Brian P. Meier Its: C.O.O Date: $12^{-2} + 2007$
[ACKNOWLEDGMEN	Date: $/2^{-}2^{-}2^{-}2^{-}2^{-}2^{-}2^{-}2^{-}$

LANDLORD ACKNOWLEDGMENT

INDIVIDUAL ACKNOWLEDGMENT

)) ss:

STATE OF Kentucky

COUNTY OF Carter

BE IT REMEMBERED, that on this $\int \frac{1}{12} day$ of $\int u$ 2007 before me, the subscriber, a person authorized to take oaths in the State of <u>Kentucky</u>, personally appeared <u>Gary Bond</u> who, being duly sworn on his/her/their oath, deposed and made proof to my satisfaction that he/she/they is/are the person(s) named in the within instrument; and I, having first made known to him/her/them the contents thereof, he/she/they did acknowledge that he/she/they signed, sealed and delivered the same as his/her/their voluntary act and deed for the purposes therein contained.

Notary Public: J. Stamper Flua My Commission Expires: D4-02-10

PARTNERSHIP (consisting of corporations) ACKNOWLEDGMENT

STATE OF

COUNTY OF

I CERTIFY that on _____, 2007, _____ personally came before me and this/these person(s) acknowledged under oath to my satisfaction, that:

(a) this/these person(s) signed, sealed and delivered the attached document as ______ of _____ a corporation of the State of _____, which is a general partner of the partnership named in this document;

(b) the proper corporate seal of said corporate general partner was affixed; and

) ss:

)

(c) this document was signed and delivered by the corporation as its voluntary act and deed as [a] general partner(s) on behalf of said partnership [by virtue of authority from its Board of Directors].

Notary Public:	
My Commission Expires:	

Site Name: Wolf Creek Site Number: KY-00-817

CORPORATE ACKNOWLEDGMENT

STATE OF	

)) ss:

)

COUNTY OF

I CERTIFY that on _____, ____ personally came before me and acknowledged under oath that he or she:

- (a) is the [title] of [name of corporation] the corporation named in the attached instrument,
- (b) was authorized to execute this instrument on behalf of the corporation and

)) ss:)

(c) executed the instrument as the act of the corporation.

Notary Public:	
My Commission Expires:	

TENANT ACKNOWLEDGMENT

STATE OF ILL	INOIS	
COUNTY OF	DUPACE	

On the <u>2844</u> day of <u>Decembe</u> 2007, before me personally appeared <u>Brian P. Meier, its C.O.O.</u>, and acknowledged under oath that he is duly authorized to sign on behalf of Central States Tower Holding, LLC, the <u>Tenant</u> named in the attached instrument, and as such was authorized to execute this instrument on behalf of the <u>Limited Liability</u> <u>Company</u>.

BARBARA MEINDL

Notary Public: <u>BARBARA</u> ME My Commission Expires: <u>JAN. 22, 2011</u>

~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	OFFICIAL SEAL BARBARA MEINDL	2
٤	NOTARY PUBLIC, STATE OF ILLINOIS My Commission Expires 01/22/2011	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,

Site Name: Wolf Creek Site Number: KY-00-817

#### EXHIBIT A

#### DESCRIPTION OF PROPERTY Page <u>1</u> of <u>1</u>

The Property is described and/or depicted as follows:

A certain tract of Land Situated on Tygarus Creek in Carter County, Kentucky and BEGINNING at a cedar stump on top of a cliff, a corner to J.E. Hignite land; thence; thence with the J.E. Hignite and Ada Bond Line N.47.W with a survey of the J.E. Hignite Line 609 feet to a Hickory S 62 W 271 feet to a stake by the road thence with the road westerly 941 feet to a 14 inch white Oak on the west side of a road leading to Tygarts Creek, South 26 ¼ East passing a Beech at 37 feet in all 46 feet to a stake in the Ada Bond and Charlie Burhett line; thence with the same North 69 East 53 ½ feet to a White Oak; North 73 East 496 feet to win Poplars, marked South 56 East 330 feet to a 12 inch Chestnut Oak on top of the cliff, North 47 East with the meanders of a high cliff to the point beginning, containing the boundary.

There is excluded from the foregoing description and not hereby conveyed a lot 50 ft. by 100 ft. heretofore conveyed to Bob Lee, by deed recorded in Deed Book 141, page 258, Carter County, Kentucky, Deed Records.

Being a part of the same property conveyed to Basil Bond and Ada Bond, his wife, with survivorship, by deed recorded in Deed Book 97, page 418, Carter County, Kentucky, Deed Records. The said Basil Bond is now deceased.



303559 Filed on:1/9/2008 12:29:41 PM Book: OR Number: 209 Pages: 76 - 80 Mike D. Johnston ,Carter County DC: SKANNA BKADLEY

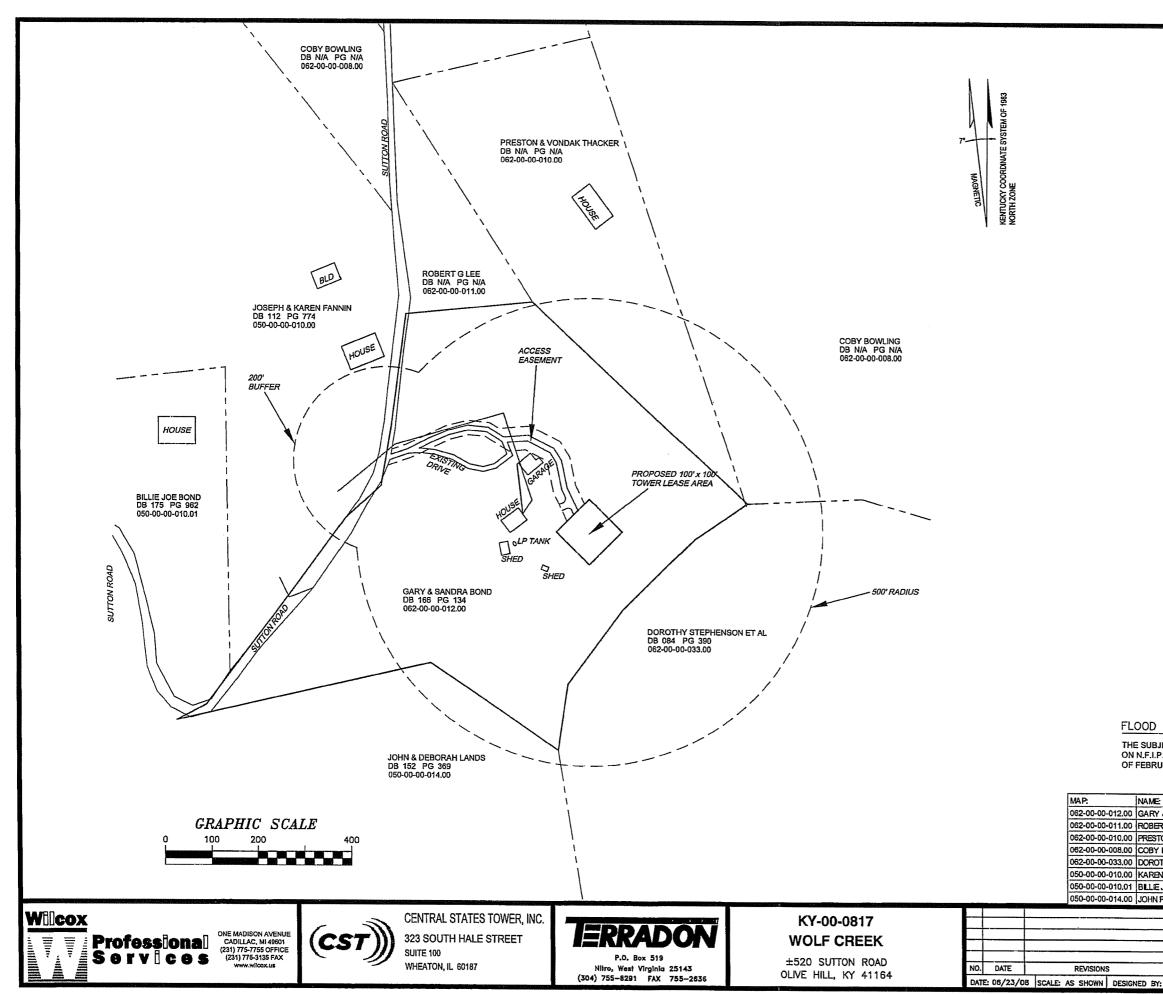
Site Name: Wolf Creek Site Number: KY-00-817

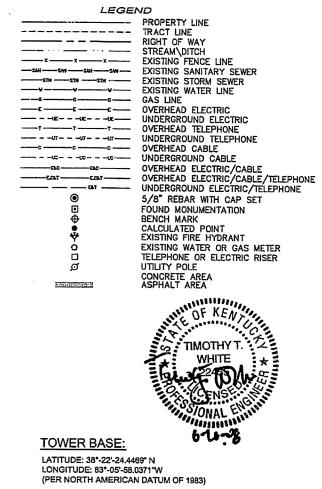
Carter County

OR 209 PG 80

## EXHIBIT E

Site Plan – 500' Radius Map with Flood Plain Information





ELEVATION: 850.7 (PER NORTH AMERICAN VERTICAL DATUM OF 1988)

#### FLOOD PLAIN INFORMATION

THE SUBJECT PROPERTY LIES WITHIN ZONE "C", AN AREA OF MINIMAL FLOODING, AS DESIGNATED ON N.F.I.P. CARTER COUNTY, KY COMMUNITY PANEL #210050 0040 B, BEARING AN EFFECTIVE DATE OF FEBRUARY 15, 1984.

							1
2		ADDRESS:	CITY:	STATE	ZP:	DB:	PG:
Y & SANDRA BOND		520 SUTTON RD	OLIVEHILL	KY	41164	166	134
RT.G. LEE		BOX 124	OLIVEHILL	KY	41164	NVA	N/A
TON & VONDA K THACKER		674 SUTTON RD	OLVEHILL	KY	41164	N/A	N/A
Y BOWLING		716 SUTTON RD	OLVEHILL	KY	41164	N/A	N/A
OTHY L STEPHENSON ET AL C/O	NANCY EDISON	700 SPA RD	LEWISBURG	KY	42256	084	390
IN & JOSEPH DALE FANNIN BON	ID ADA	329 SUTTON RD	OLVEHILL	KY	41164	112	774
E JOE BOND		115 SUTTON RD	OLVEHILL	KY	41164	175	962
IF. & DEBORAH K. LANDS		P.O. BOX 55	CARTER	КY	41128	152	369
		SHEET	R-1				
		500' RADI					
	FLOC	DD PLAIN I		ATIO	N		
BY CHK APP'D		DRAWING	NUMBER				
Y: SFP DRAWN BY: SFP		KY-00-	-0817				
							1.2.2.2

## EXHIBIT F

Affidavit of Notification of Adjacent Property Owners and Owners within 500 feet

### COMMONWEALTH OF KENTUCKY BEFORE THE PUBLIC SERVICE COMMISSION

In the Matter of:

Application of Central States Tower Holdings, LLC for Issuance of a Certificate of Public Convenience and Necessity to Construct a Cell Site (KY-00-0817A WOLF CREEK) in Olive Hill Kentucky Case No. 2008-00261

### Affidavit of Sandee L. Yagle

I, Sandee L. Yagle, being duly sworn, depose and state as follows:

1. My name is Sandee L. Yagle and I am an employee of Cellere, LLC, agent for Central States Tower Holdings, LLC and am submitting this affidavit in conjunction with the above referenced matter.

2. In order to demonstrate compliance with 807 KAR 5:063 § 1(1)(1), Exhibit 1 identifies the names of the residents/ tenants and property owners within 500 feet of the proposed tower who have been: (i) notified by written notice of the proposed construction, sufficient postage prepaid, by United States Certified Mail, return receipt requested; (ii) given the Commission docket number under which the application will be processed; and (iii) informed of the right to request intervention.

3. Attached as Exhibit 2 is a copy of the United States Certified Mail return receipts that demonstrate proof of service of the written notice of the proposed construction upon: (1) Gary and Sandra Bond; (2) Robert G. Lee; (3) Preston and Vonda Thacker; (4) Coby Bowling; (5) Dorothy Stephenson, et. Al., c/o Nancy Edison; (6) Joseph and Karen Finnin; (7) Billie Joe Bond; and (8) John and Deborah Lands. (See Exhibit1)

Further Affiant saith not.

Sandee L. Yagle

State of Michigan ) ) SS: County of Grand Traverse )

Subscribed and Sworn to before me this  $\frac{36^{76}}{200}$  day of July, 2008. My commission expires: _____ 2012 12 Notary Public

DAVID ANTHONY LARSEN Notary Public, State of Michigan County of Grand Traverse My Commission Expires 02-02-2012 Acting in the County of C. Traverse

## Landowner and Adjacent Landowner List

Central States Tower Holdings, LLC Wolf Creek Site Olive Hill, Kentucky

Preston and Vonda Thacker

674 Sutton Road

329 Sutton Road

Olive Hill, KY 41164

Olive Hill, KY 41164

Joseph and Karen Finnin

Gary and Sandra Bond 520 Sutton Road Olive Hill, KY 41164

Coby Bowling 716 Sutton Road Olive Hill, KY 41164

Dorothy Stephenson, et. al. c/o Nancy Edison 700 Spa Road Lewisburg, KY 42256

Billie Joe Bond 115 Sutton Road Olive Hill, KY 41164

991 County Rd. Lane

Olive Hill, KY 41164

Robert G. Lee

John and Deborah Lands 174 Sutton Road Olive Hill, KY 41164

Sandee L. Yagle, Cellere

Date

.

Gary and Sandra Bond 520 Sutton Road Olive Hill, KY 41164

## Public Notice

Cellere, LLC, a Michigan limited liability company as agent for Central States Tower Holdings, LLC is applying to the Public Service Commission of the Commonwealth of Kentucky (the "Commission") for a Certificate of Public Convenience and Necessity to construct a new cellular tower facility to provide cellular telephone service. This facility will include a 300 foot tower to be located at +/- 520 Sutton Road, Olive Hill, Kentucky 44164. A map showing the location is attached.

The Commission invites your comments regarding this proposed construction. Also, the Commission wants you to be aware of your right to intervene in this matter. Your comments and request for intervention should be addressed to:

Executive Director's Office Public Service Commission of Kentucky P.O. Box 615 Frankfort, Kentucky 40602

Please refer to case number 2008-00261 in your correspondence.

Cellere and Central States welcome the opportunity to serve and provide wireless service in your community!

SENDER: COMPLETE THIS SECTION  Complete items 1, 2, and 3. Also complete	COMPLETE THIS SECTION ON DELIVERY A_ Received by (Please Print Clearly) B. Date of Delivery
item 4 if Restricted Delivery is desired.	Gani Bron 17-10-18
Print your name and address on the reverse so that we can return the card to you.	C. Signature)
Attach this card to the back of the mailpiec	e, X hours Agent
or on the front if space permits.	
1. Article Addressed to:	D. Is delivery address different from item 1?  Ves If YES, enter delivery address below: No
Gan and Sandra Band	1
Gary and Sandra Bond 520 Sutton Road	
	3. Service Type
Olive Hill, KY 41164	Registered     Return Receipt for Merchandise
	Insured Mall C.O.D.
	4. Restricted Delivery? (Extra Fee)          □ Yes         □         □         □
2. Article Number (Copy from service label)	

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Robert G. Lee 991 County Road Lane Olive Hill, KY 41164

## **Public Notice**

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and the second second second

Cellere, LLC, a Michigan limited liability company as agent for Central States Tower Holdings, LLC is applying to the Public Service Commission of the Commonwealth of Kentucky (the "Commission") for a Certificate of Public Convenience and Necessity to construct a new cellular tower facility to provide cellular telephone service. This facility will include a 300 foot tower to be located at +/- 520 Sutton Road, Olive Hill, Kentucky 44164. A map showing the location is attached.

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Please refer to <u>case_number 2008-00261</u> in your correspondence.

Cellere and Central States welcome the opportunity to serve and provide wireless service in your community!

<ul> <li>SENDERROOMPLEMENTICS SECTION</li> <li>Complete items 1, 2, and 3. Also complete item 4 if Restricted Delivery is desired.</li> <li>Print your name and address on the reverse so that we can return the card to you.</li> <li>Attach this card to the back of the mallpiece, cr on the front if space permits.</li> <li>Article Addressed to:</li> </ul>	COMPLETE THIS SECTION ON DELIVERY         A.*Received by (Please Print Clearly)         B. Date of Delivery         DOROTHY       LEE         T-12-08         C. Signature         X       Hothy LEE         D. Is delivery address different from item 1?         If YES, enter delivery address below:
Robert G. Lee	3. Service Type
991 County Road Lane Olive Hill, KY 41164	Certified Mail Express Mail     Registered Return Receipt for Merchandise     Insured Mail C.O.D.
2. Article Number (Copy from service label)	7008 0150 0001 5347 7860

Preston and Vonda Thacker 674 Sutton Road Olive Hill, KY 41164

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	SENDER: COMPLETE THIS SECTION	COMPLETE THIS SECTION ON DELIVERY
	<ul> <li>Complete items 1, 2, and 3. Also complete item 4 if Restricted Delivery is desired.</li> <li>Print your name and address on the reverse so that we can return the card to you.</li> <li>Attach this card to the back of the mailpiece, or on the front if space permits.</li> </ul>	A Received by (Please Print Clearly) B. Date of Delivery Voncla K. The Clor 7-19-08 C. Signature Monda K. Arch Agent Agent
	1. Article Addressed to:	D. Is delivery address different from item 1? If YES, enter delivery address below: No
	Preston and Vonda Thacker 674 Sutton Road Olive Hill, KY 41164	3. Service Type Certified Mall Registered Insured Mail C.O.D.
		4. Restricted Delivery? (Extra Fee)
	2. Article Number (Copy from service label)	008 0150 0001 5347 8140
~.	PS Form 3811, July 1999 Domestic Re	tum Receipt 102595-00-M-0952

_____

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Coby Bowling 716 Sutton Road Olive Hill, KY 41164

## **Public Notice**

Cellere, LLC, a Michigan limited liability company as agent for Central States Tower Holdings, LLC is applying to the Public Service Commission of the Commonwealth of Kentucky (the "Commission") for a Certificate of Public Convenience and Necessity to construct a new cellular tower facility to provide cellular telephone service. This facility will include a 300 foot tower to be located at +/- 520 Sutton Road, Olive Hill, Kentucky 44164. A map showing the location is attached.

The Commission invites your comments regarding this proposed construction. Also, the Commission wants you to be aware of your right to intervene in this matter. Your comments and request for intervention should be addressed to:

Executive Director's Office Public Service Commission of Kentucky P.O. Box 615 Frankfort, Kentucky 40602

Please refer to <u>case number 2008-00261</u> in your correspondence.

Cellere and Central States welcome the opportunity to serve and provide wireless service in your community!

SENDER: COMPLETE THIS SECTION	COMPLETE THIS SECTION ON DELIVERY
Complete items 1, 2, and 3. Also complete item 4 if Restricted Delivery is desired.	A./Received by (Please Print Clearly) B. Date of Delivery
Print your name and address on the reverse so that we can return the card to you.	C. Signifiure
<ul> <li>Attach this card to the back of the mailplece, or on the front if space permits.</li> </ul>	X Aug Day Day Day
1. Article Addressed to:	D. is delivery address different from item 1? U Yes If YES, entries delivery address below: U Yes
Coby Bowling 716 Sutton Road	
Olive Hill, KY 41164	3. Service Type         Certified Mail       Express Mail         Registered       Return Receipt for Merchandise         Insured Mail       C.O.D.
	4. Restricted Delivery? (Extra Fee)
2. Article Number (Copy from service label) 7	JO8 0150 0001 5347 7839

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Dorothy L. Stephenson, et al. c/o Nancy Edison 700 Spa Road Lewisburg, KY 42256

## **Public Notice**

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700 Spa_Road       3. Service Type         Lewisburg, KY 42256       Certified Mall       Express Mail         Registered       Registered       Return Receipt for Merchandise         Insured Mail       C.O.D.       1         Kestricted Delivery?       Yes	<ul> <li>SENDER complete items 1, 2, and 3. Also complete items 1, 2, and 3. Also complete item 4 if Restricted Delivery is desired.</li> <li>Print your name and address on the reso that we can return the card to you.</li> <li>Attach this card to the back of the mai or on the front if space permits.</li> <li>1. Article Addressed to:</li> <li>Dorothy Stephenson, et. al c/o Nancy Edison</li> </ul>	A. Received by (Please Print Clearly) NANCHE EIDSON C: Signature X Mancye Cubs D. Is delivery address different from item If YES, enter delivery address below	B. Date of Pelivery	
		Certified Mail E Registered Insured Mail C.O.D.	lpt for Merchandise	- 1 <b>45'</b>
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Joseph and Karen Finnin 329 Sutton Road Olive Hill, KY 41164

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Joseph and Karen Finnin 329 Sutton Road Olive Hill, KY 41164	3. Service Type         Certified Mall       Express Mail         Registered       Return Receipt for Merchandise         Insured Mail       C.O.D.         4. Restricted Delivery? (Extra Fee)       Yes
2. Article Number (Copy from service label)	7008 0150 0001 5347 7846
PS Form 3811, July 1999 Domestic Ret	um Receipt 102595-00-M-0952

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Billie Joe Bond 115 Sutton Road Olive Hill, KY 41164

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### Please refer to case number 2008-00261 in your correspondence.

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Print your name and address on the reverse so that we can return the card to you. Attach this card to the back of the mailpiece, or on the front if space permits. 1. Article Addressed to:   Billie Joe Bond   115 Sutton Road   Olive Hill, KY 41164     3. Service Type   A Certified Mail   Express Mail   Registered   Return Receipt for Merchandise   Insured Mall   C. Signature     X     Attach this card to the back of the mailpiece, or on the front if space permits.     1. Article Addressed to:     Billie Joe Bond   115 Sutton Road   Olive Hill, KY 41164     3. Service Type   A Certified Mail   Express Mail   Registered   Return Receipt for Merchandise   Insured Mall   C. Signature	SENDER: COMPLETE THIS SECTION Complete items 1, 2, and 3. Also complete item 4 if Restricted Delivery is desired.	COMPLETE THIS SECTION ON DELIVERY
1. Article Addressed to:       If YES, enter delivery address below:       In No         Billie Joe Bond       115 Sutton Road	so that we can return the card to you.	X Just Jon Addressee
Billie Joe Bond 115 Sutton Road Olive Hill, KY 41164 3. Service Type Certified Mail Express Mail Registered Return Receipt for Merchandise Insured Mall C.O.D. 4. Restricted Delivery? (Extra Fee) Yes	1. Article Addressed to:	D, is delivery address anotone northern th
Olive Hill, KY 41164       3. Service Type         Service Type       Certified Mail       Express Mail         Registered       Return Receipt for Merchandise         Insured Mall       C.O.D.         4. Restricted Delivery? (Extra Fee)       Yes	Billie Joe Bond	
Insured Mall     C.O.D.       4. Restricted Delivery? (Extra Fes)     Yes		
	•	
2. Article Number (Copy from service label)		4. Restricted Delivery? (Extra Fee)
1008 0120 0001 2341 10CC	2. Article Number (Copy from service label)	7008 0150 0001 5347 7822

John and Deborah Lands 174 Sutton Road Olive Hill, KY 41164

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Executive Director's Office Public Service Commission of Kentucky P.O. Box 615 Frankfort, Kentucky 40602

### Please refer to case number 2008-00261 in your correspondence.

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A. Received by ( <i>Please Print Clearly</i> ) B. Date of Delivery 2. Signature Agent A. Agent A. Agent A. Agent A. Agent A. Addressee D. Is delivery address different from item 1? Yes If YES, enter delivery address below: No
3. Service Type     A Certified Mail     Express Mail     Registered     Return Receipt for Merchandise     Insured Mail     C.O.D.     Yes
7008 0150 0001 5347 7853

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# EXHIBIT G

Certified letter to Judge Executive

July 7, 2008

Via Certified Mail Carter County Judge Executive Charles Wallace 300 West Main Street Room 227 Grayson, KY 41143

### RE: Public Notice – Public Service Commission of Kentucky Case No. 2008-00261

Cellere, LLC, as agent for Central States Tower Holdings, LLC, is applying to the Public Service Commission of Kentucky (the "Commission") for a Certificate of Public Convenience and Necessity to propose construction and operation for a new facility to provide cellular telecommunications service in Carter County. The facility will include a 300 foot tower and an equipment shelter to be located at +/- 520 Sutton Road, Olive Hill, Kentucky 44164. A map showing the location of the proposed new facility is enclosed.

The Commission invites your comments regarding the proposed construction. You also have the right to intervene in this matter.

Your comments and request for intervention should be addressed to:

Executive Director's Office Public Service Commission of Kentucky P.O. Box 615 Frankfort, Kentucky 40602

### Please refer to case number 2008-00261 in your correspondence.

Benjamin Meredith Cellere, LLC	<ul> <li>SENDER: COMPLETE THIS SECTION</li> <li>Complete items 1, 2, and 3. Also complete item 4 if Restricted Delivery is desired.</li> <li>Print your name and address on the reverse so that we can return the card to you.</li> <li>Attach this card to the back of the mailplece.</li> </ul>	COMPLETENTIS SECTION ONIDELIVERY         A. Received by (Please Print Clearly)         B. Date of Dr.         C. Aranies Mailare         C. Signature         X	-0 3 
Enclosure sly	or on the front if space permits.  1. Article Addressed to:	D. Is delivery address different from item 1? Yes If YES, enter delivery address below: No	ressee
	Carter County Judge Execut Charles Wallace 300 W. Main St., Rm. 227	·	111 TETETTETTETTETTETTETT
	Grayson, KY 41143	3. Service Type     2. Service Type     2. Certified Mail	andise
	2. Article Number (Copy from service label)	4. Restricted Delivery? (Extra Fee) 7008 01.50 0001 5347 7808	
	PS Form 3811, July 1999 Domes	tic Return Receipt	-0952

# EXHIBIT H

Public Notice Signs (Photos)





## EXHIBIT I

Affidavit of Publication of Public Notice

## **Morehead News Group**

Newspaper Holdings, Inc.

722 W. First St., Morehead, KY 40351 606-784-4116 or 800-247-6142

Affidavit of Publication

STATE OF KENTUCKY COUNTY OF Carter

I, Betty Kelly, classified clerk, of Morehead News Group, in the aforesaid State and County, hereby certify that the attached advertisement appear d on  $\underline{7-30-08}$  in the  $\underline{01000}$   $\underline{1000}$ .

Betty Kelly, Classified Clerk

Date

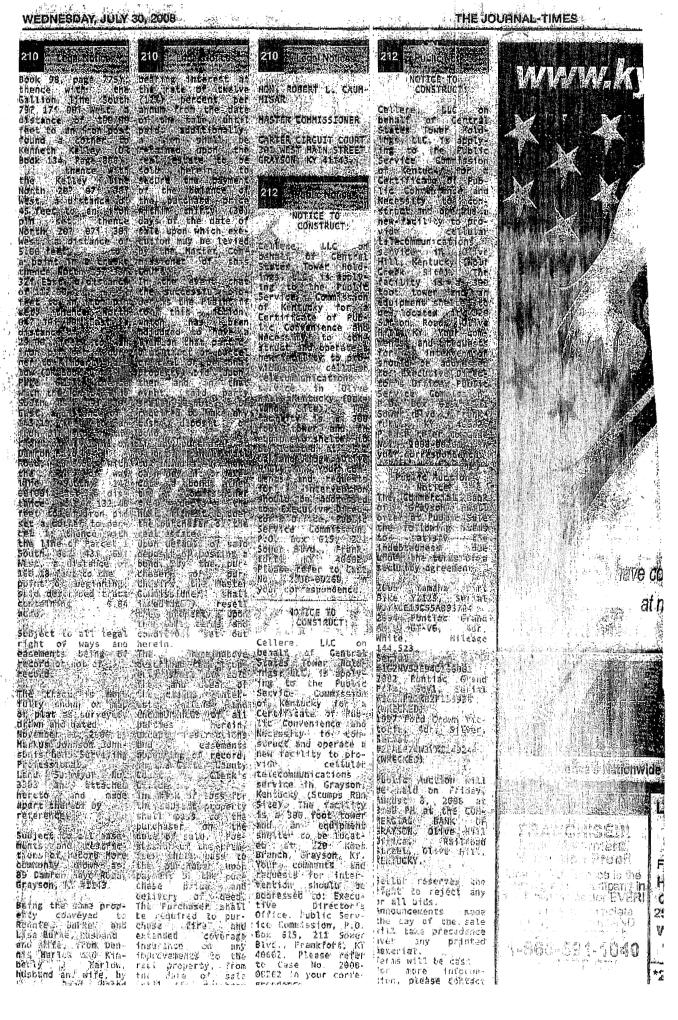
Subscribed and sworn to before me, a Notary Publi:, within and for the State and County aforesaid, by Betty Kelly, on the above date.

Notary Public, State at Large Kentucky

My Commission Expires:

www.themoreheadnews.com The Morehead News Shopping News www.journal-times.com Grayson Journal Engniser Olive (fill Turcs

MOREHEAD NEWS GROUP



# EXHIBIT J

Map of Search Area



